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TRAFFIC IMPACT STUDY

FOR

Steamboat Basecamp

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Project Description

This long-term traffic impact study analyzes the effects that the Pilot Building of the Steamboat Basecamp development will have on traffic operations in the years of 2022 and 2040. The Steamboat Basecamp is a redevelopment of the old Steamboat Pilot Building. Once completed, the new building will include a variety of apartment types and other amenities, such as a fitness center and restaurant. Figure 1 shows a rendering of the proposed development.

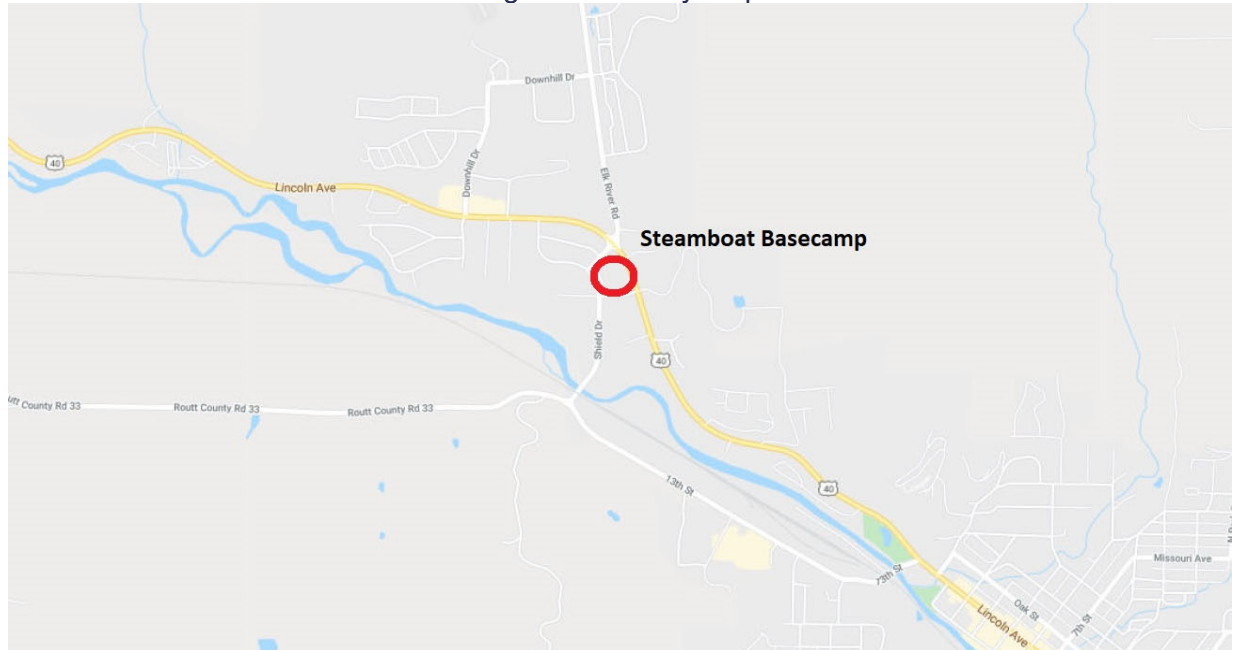
Figure 1: Steamboat Basecamp Rendering



The City of Steamboat Springs has requested that a traffic impact study be prepared for the Steamboat Basecamp. A scope approval form was submitted to the City of Steamboat prior to beginning the study, and approval is pending at this time. It is included in Appendix A, and outlines the key items to be analyzed in this study. The traffic impact study has been prepared in accordance with City of Steamboat Springs requirements, assuming an opening year of 2022.

The Steamboat Basecamp is located on the northeast corner of Elk River Rd & Shield Dr, with site access on each of these roads. A vicinity map is provided in Figure 2.

Figure 2: Vicinity Map



The Steamboat Pilot Building will include several amenities that are expected to generate external trips, shown in Table 1, while others amenities will likely only be used by the residents.

Table 1: Basecamp Amenities Generating External Trips

Amenity	Size
Multifamily Housing	75 Units
Health/Fitness Club*	4,090 SQ FT
Restaurant	3,659 SQ FT

*Class-based fitness center. No Open Gym Hours.

The site plan is shown in Figure 3.

Figure 3: Site Plan



Roadways in the vicinity of the site are described below:

Lincoln Avenue (US 40) is an east/west roadway providing access to Elk River Rd, Curve Ct, and Downhill Dr, as well as serving as the main travel corridor and gateway into Steamboat Springs. Through the study area, US 40 alternates between a two and four lane arterial roadway with intermittent auxiliary lanes at intersections and access points. This segment of roadway is classified as NR-A by the Colorado Department of Transportation (CDOT). The posted speed limit is 40mph through the study area.

Elk River Rd is a two-lane north/south roadway providing direct access to Steamboat Basecamp. Elk Road intersects US 40 in a signalized intersection to the northwest of the development.

Curve CT is a two-lane east/west roadway between US 40 and Shield Dr. Many of the site trips from the south will use Curve Ct to access the site.

Shield Dr is a two-lane north/south roadway between Elk River Rd and Curve Ct, and provides direct access to Steamboat Basecamp.

Downhill Drive is a two lane, north/south roadway that intersects US 40 to the west of the site. While the intersection with US 40 is currently stop-controlled, the City has committed to studying the intersection to determine the most appropriate traffic control (roundabout or signal) and to design and construct an improvement in the next few years. The posted speed limit on Downhill Drive is 25mph.

1. Existing Conditions

1.1. Volumes

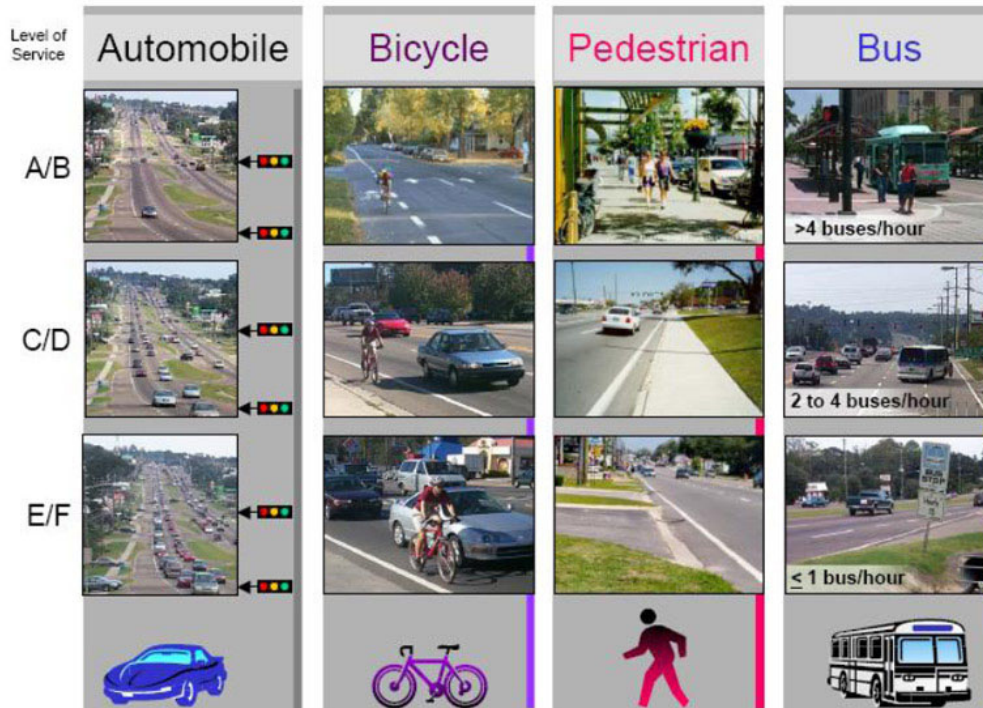
To provide a baseline condition for the traffic study, turning movement counts were taken at the following intersections.

- US 40 & Sunlight Dr/Curve CT
- US 40 & Elk River Rd
- US 40 & Downhill Dr/Riverside Dr
- Curve Plaza & Shield Dr
- Shield Dr & Elk River Rd
- Curve Plaza & Elk River Rd

The counts were collected on Tuesday, March 3, 2020 from 7:00-9:00 AM and 4:00-6:00 PM, and include pedestrian, bicycle, and heavy vehicle data. From these counts, it was determined that the AM peak hour was from 7:45-8:45 AM and the PM peak hour was from 4:45-5:45 PM. The intersection of US 40 & Downhill Dr/Riverside Dr was not included in the traffic models, since the City of Steamboat is planning on constructing intersection improvements in the next several years. The specific nature of the improvements are unknown at this time, making it difficult to accurately model the future scenario at US 40 & Downhill Dr/Riverside Dr. Counts were taken at this intersection in order to be able to assess the percent contribution that the Steamboat Basecamp will have in regards to the intersection improvements. The traffic counts can be found in Appendix B.

1.2. LOS Criteria

Traffic analyses were conducted in accordance with procedures outlined in the Highway Capacity Manual, and included intersection Level-of-Service (LOS). LOS is a measure of the quality of traffic flow and ranges from LOS A (nearly ideal traffic conditions with very little delay for motorists) to LOS F (poor traffic conditions with long motorist delays). LOS C is typically considered a “good” traffic condition. LOS D or better conditions are typically desirable during peak traffic periods; however, LOS E conditions are not uncommon. LOS F, although undesirable, is also not uncommon for side street traffic movements at full movement, unsignalized intersections with high volume arterial roadways. Figure 4 illustrates examples of LOS for various modes of travel.



Source: FDOT Quality/Level of Service Handbook

Figure 4: LOS Conditions

When reporting delay and LOS, the HCM specifies that at a signalized intersection, the average intersection delay be used to derive the LOS. At a stop-controlled intersection, the worst movement is used. Table 2 provides a summary of the Highway Capacity Manual's LOS Criteria. This study area contains both signalized and unsignalized intersections.

Table 2: LOS Criteria

Level of Service (LOS)	Signalized Intersection	Unsignalized Intersection	Traffic Characteristics
	Average Intersection Delay (sec/veh)	Worst Movement (sec/veh)	
A	<= 10	<= 10	Free Flow / Insignificant Delays
B	> 10-20	> 10-15	Stable Flow / Minimal Delays
C	> 20-35	>15-25	Stable Flow / Acceptable Delays
D	> 35-55	>25-35	Nearing Unstable / Tolerable Delays
E	> 55-80	>35-50	Unstable Flow / Significant Delays
F	> 80	> 50	Forced Flow / Excessive Delays

Where an unsignalized intersection operates at LOS E or F, a volume-to-capacity ratio (V/C) has been reported for the worst-case movement. Where V/C exceeds 1.00, traffic demand during peak periods exceeds the capacity for the movement. This condition will cause queues to

grow, potentially filling auxiliary lanes and blocking adjacent traffic lanes until demand decreases.

1.3. Existing Traffic Operations

Existing traffic operations were evaluated using Synchro 10th Edition. The existing traffic models use the March 2020 volumes and the existing roadway geometry. In this scenario, the Steamboat Basecamp has not yet been constructed. The traffic signal splits and cycle lengths were optimized in Synchro. Table 3 shows the existing traffic operations.

Table 3: Existing Delay and LOS

Intersection	AM Peak Hour			PM Peak Hour		
	Movement	Delay (sec/veh)	LOS (V/C)	Movement	Delay (sec/veh)	LOS (V/C)
US 40/Elk River Rd (Signal)	-	30	C	-	49.1	D
US 40/Sunlight Dr/Curve CT	WB	14.7	B	WB	29.6	D
Shield Dr/Elk River Rd	SB	10.8	B	SB	12	B
Curve Plaza/Elk River Rd/Access #1	EB	11.4	B	EB	15.7	C
Shield Dr/Access #2	SB	7.5	A	-	0	A

All of the intersections operate acceptably for the March 2020 conditions. The intersections of US 40 & Elk River Rd and US 40 & Sunlight Dr/Curve CT operate at LOS D in the PM peak, while the remaining intersections operate at LOS C or better. The Synchro result printouts can be found in Appendix C.

2. Short Term Background Conditions

2.1. Background Volumes

The Short-Term Background Conditions analyzes the existing roadway network, with 2022 traffic volume projections. This scenario assumes that the Steamboat Basecamp has not been built. Traffic volumes in Steamboat Springs are highly seasonal. Traffic counts were collected in March, which is one of the lower volume months. In accordance with City of Steamboat requirements, the existing traffic counts were factored up to reflect conditions typical to the month of July. Using the City's ADT conversion table, the March volumes were factored by 1.59 to convert to the traffic volumes typically experienced in July. The ADT conversion table has been included in the Appendix.

The background growth rate was taken from the CDOT count station #101838 at MP 130.57. The projected 20-year factor is 1.16, yielding an annual growth of 0.75%. The existing counts, after being seasonally adjusted, were then inflated by the 0.75% annual growth in order to generate the 2022 volumes. These volumes can be found in the Appendix.

2.2. Short Term Background Traffic Operations

Traffic operations were evaluated using Synchro 10th Edition. Table 4 shows the traffic operations.

Table 4: Short Term Background Delay and LOS

Intersection	AM Peak Hour			PM Peak Hour		
	Movement	Delay (sec/veh)	LOS (V/C)	Movement	Delay (sec/veh)	LOS (V/C)
US 40/Elk River Rd (Signal)	-	51.5	D	-	74.8	E
US 40/Sunlight Dr/Curve CT	NBL	28.6	D	EB	628	F (2.17)
Shield Dr/Elk River Rd	SB	12.9	B	SB	15.4	C
Curve Plaza/Elk River Rd/Access #1	EB	14.6	B	EB	38.3	E (0.76)
Shield Dr/Access #2	SB	7.6	A	-	0	A

Delays have increased at most intersections due to the volume growth. The intersection of US 40 & Sunlight Dr/Curve Ct is expected to operate at LOS F in the PM. This is due to the side street left turn movements having difficulty turning because of the high thru movements along US 40. The West Steamboat Springs US Highway 40 Access Study, conducted by Stolfus and Associates, Inc. states that the intersection of US 40 & Sunlight Dr/Curve CT is to be made right-in, right-out if traffic operations deteriorate. As the projected 2022 volumes are far too low to warrant a signal, movement restrictions at the intersection will likely be the solution should traffic conditions become unacceptable or unsafe.

3. Short Term Total Conditions

3.1. Trip Generation

The ITE Trip Generation Manual 10th Edition was used to calculate the number of trips generated by the Steamboat Basecamp upon opening year. Only land uses expected to generate external trips were included in the calculations. Amenities intended solely for the residents, such as the hot tub and lobby space, will not attract external visitors, and were not part of the trip generation calculations. Table 5 shows the trip generation calculations for the Steamboat Basecamp.

Table 5: ITE Trip Generation Calculations

Multifamily														
ITE Code	Units	ITE Land Use	Weekday Rate	Weekday Trips	AM Peak Rate	AM Peak Entering %	AM Peak Exiting %	PM Peak Rate	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
221	75	Multifamily Housing (Mid-Rise) (General Urban/Suburban)	5.43	407	0.19	26%	74%	0.45	72%	28%	4	10	24	9
Fitness Center														
ITE Code	Sq. Ft (1000 Ft)	ITE Land Use	Weekday Rate	Weekday Trips	AM Number of People	AM Peak Entering %	AM Peak Exiting %	PM Number of People	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
492	4.09	Health/Fitness Club	-	-	16	51%	49%	16	57%	43%	8	8	9	7
Market														
ITE Code	Sq. Ft (1000 Ft)	ITE Land Use	Weekday Rate	Weekday Trips	AM Peak Rate	AM Peak Entering %	AM Peak Exiting %	PM Peak Rate	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
851	1.00	Convenience Market	763.71	382	62.54	50%	50%	20.84	51%	49%	16	16	5	5
936	1.00	Coffee/Donut Shop without Drive-Through	754.55	377	101.14	51%	49%	36.31	50%	50%	26	25	9	9
-	0.5	Kitchen/Storage Space	Not Expected to Generate Trips											
-	1.16	Lounge/Lobby Space	Not Expected to Generate Trips											

Phase 1 Total				
Weekday Trips	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
1342	53	59	48	30

The fitness center is unlikely to follow the rates prescribed by the Trip Generation Manual. The fitness center is class-based and has a limit on the number of people using it at any given time, making it unlikely to generate trips at the rate of a typical fitness center. From the information provided about the Steamboat Basecamp amenities, it was assumed that 8 people would enter and exit the fitness center each hour.

The restaurant was broken down into several land uses when calculating the generated trips. The land uses expected to generate external trips were modeled as a 1,000 SF café and a 1,000 SF market. The areas of the restaurant not expected to generate trips are 500 SF of kitchen/storage space and 1,159 SF of lounge/lobby space. The restaurant is expected to serve both residents of the Basecamp, as well as external customers. For the trip generation calculations, it was assumed that 50% of the customers were external trips. The entering and exiting trips shown in the trip generation table for the market and café reflect the 50% of trips that are external to the site.

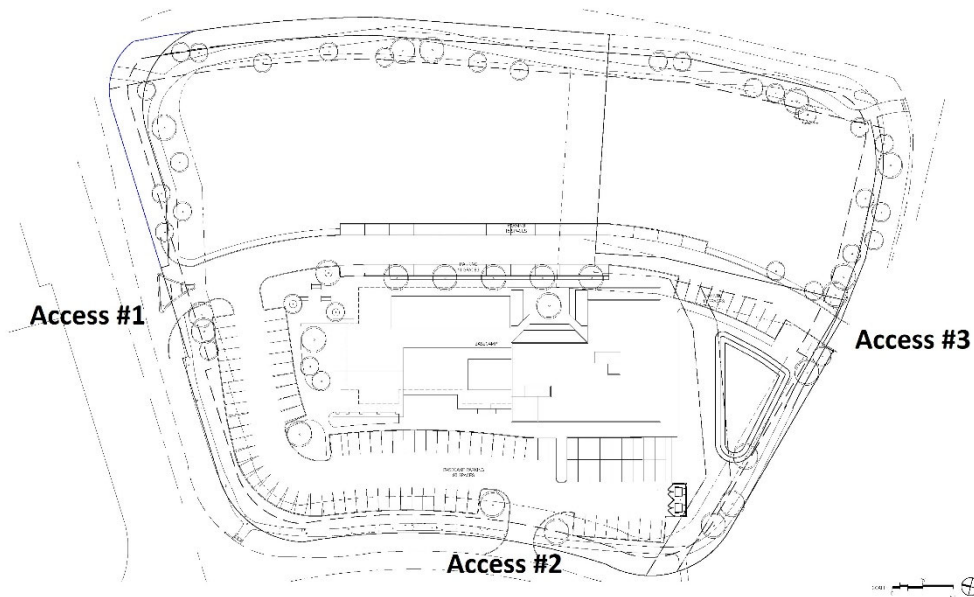
For housing in a general urban/suburban it is assumed that 7% of people will enter/exit the apartments via some form of multimodal transportation, while 8% will do so in the PM. The number of vehicle trips in Table 5 have already been reduced by these percentages, so no further alteration to the vehicle trip numbers was necessary.

3.2. Site Access and Circulation Evaluation

Upon completion of the Pilot Building, there will be three access points to the Steamboat Basecamp.

Figure 5 shows the location of the three access points to the Basecamp.

Figure 5: Steamboat Basecamp Access Points



The trips generated by Phase 1 will use the access point which results in the shortest trip. Once out of the Basecamp parking lot, the proportion of trips from the east was determined by the existing turning movements. The distribution of trips to the north and west was determined by traffic counts taken for a study conducted for the West End Plaza, just west of Downhill Dr. The West End Plaza is a good indicator of the Basecamp's trip distribution as they are in similar locations relative to the center of Steamboat. In addition, it was estimated that 5% of the generated vehicles would go to/from the shopping center on Curve Plaza, just west of the Steamboat Basecamp. Another 5% were estimated to travel south on Shield Dr. Figure 6 shows the estimated trip distribution.

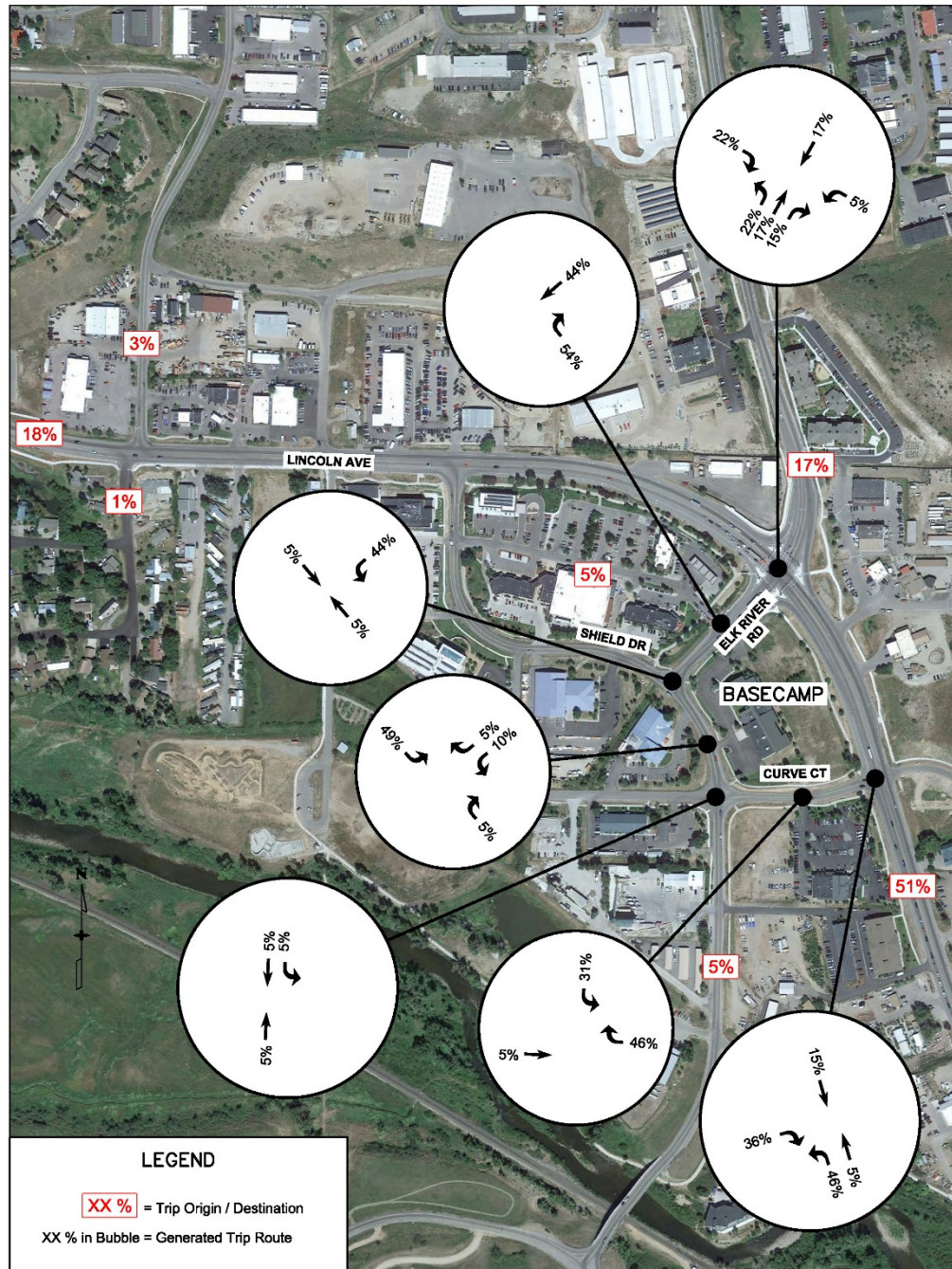


Figure 6: Trip Distribution

3.3. Auxiliary Lanes

Each turning movement on US 40 was assessed to see if SHAC auxiliary lane requirements are met. Since this portion of US 40 is classified as a Non-Rural Regional Highway (NR-A), a volume greater than 10 vehicles per hour (VPH) warrants a left turn deceleration lane, and a volume greater than 25 VPH warrants a right turn deceleration lane. A right turning movement of 50 VPH from the side street warrants an acceleration lane. Table 6 shows the warranted auxiliary lanes.

Table 6: Warranted Auxiliary Lanes

Intersection	Movement	2022 Total Conditions Volume
US 40 & Downhill Dr/Riverside Dr	WBR	162 VPH
US 40 & Sunlight Dr/Curve CT	SBL	28 VPH

As noted previously, the US 40 & Downhill Dr/Riverside Dr intersection is the subject of current study by the City of Steamboat Springs. With respect to the US 40 & Sunlight Dr/Curve Ct intersection, the Steamboat Basecamp project does not contribute any traffic volume to southbound left turn movement.

3.4. Short Term Conditions Traffic Operations

Traffic operations were evaluated for the Short-Term Total Conditions scenario using Synchro 10th Edition. This scenario assumes that Phase 1 of the Steamboat Basecamp has been completed. The roadway geometry remains the same as in the previous scenarios. Table 7 shows the delay and LOS for the Total Conditions scenario. The Synchro printouts can be found in Appendix C.

Table 7: Short Term Total Conditions Delay and LOS

Intersection	AM Peak Hour			PM Peak Hour		
	Movement	Delay (sec/veh)	LOS (V/C)	Movement	Delay (sec/veh)	LOS (V/C)
US 40/Elk River Rd (Signal)	-	51.7	D	-	75.0	E
US 40/Sunlight Dr/Curve CT	NBL	34.3	D	EB	1039.7	F (3.04)
Shield Dr/Elk River Rd	SB	13.7	B	SB	16.5	C
Curve Plaza/Elk River Rd/Access #1	EB	16.4	C	EB	47.7	E (0.82)
	WB	8.9	A	WB	9.6	A
Shield Dr/Access #2	WB	12.3	B	WB	12	B
Curve Ct/Access #3	SB	10.7	B	SB	11	B

The intersection of Curve Plaza & Elk River Rd was modeled with a RIRO access to the Steamboat Basecamp, and a full movement access for the shopping plaza. The RIRO access provides better traffic operations at the intersection than if both accesses were full movement. As shown in the table, the eastbound movements are the cause of poor LOS at the intersection while the westbound movements from the basecamp are expected to operate at LOS A during both time periods. The entering and exiting movements to the Basecamp are expected to experience little to no delay, with the significant delay coming from the left turns exiting the shopping plaza. Traffic operations at the intersection of US 40 & Elk River Rd are expected to be LOS E in the PM due to the westbound thru movement being over capacity. As in the Background scenario, the eastbound movement from Curve CT onto US 40 has significant delays, and is far over capacity.

4. Long Term Background Conditions

4.1 Traffic Volumes

The Long-Term Background Conditions analyzes the existing roadway network, with 2040 traffic volume projections. This scenario assumes that the Steamboat Basecamp has not occurred.

Traffic volumes in Steamboat Springs are highly seasonal. Traffic counts were collected in March which is one of the lower volume months. In accordance with City of Steamboat requirements, the existing traffic counts were factored up to reflect conditions typical to the month of July. Using the City's ADT conversion table, the March volumes were factored by 1.59 to convert to the traffic volumes typically experienced in July. The ADT conversion table has been included in the Appendix.

The background growth rate was taken from the CDOT count station #101838 at MP 130.57. The projected 20-year factor is 1.16, yielding an annual growth of 0.75%. The existing counts, after being seasonally adjusted, were then inflated by the 0.75% annual growth in order to generate the 2040 volumes. These volumes can be found in the Appendix.

4.2 Long-Term Background Traffic Operations

Traffic operations were evaluated using Synchro 10. Table 8 shows the delay and LOS for the study intersections.

Table 8: Long-Term Background Delay and LOS

Intersection	AM Peak Hour			PM Peak Hour		
	Movement	Delay (sec/veh)	LOS (V/C)	Movement	Delay (sec/veh)	LOS (V/C)
US 40/Elk River Rd (Signal)	-	59.3	E	-	135.2	F
US 40/Sunlight Dr/Curve CT	NBL	58.4	F (0.81)	WB	534.1	F (1.25)
Shield Dr/Elk River Rd	SB	14.2	B	SB	18.0	C
Curve Plaza/Elk River Rd/Access #1	EB	16.0	C	EB	66.1	F (0.94)
Shield Dr/Access #2	SB	7.7	A	-	0	A

When compared to the year 2022 background conditions, the delays have increased due to the background volume growth. The intersection of US 40 & Sunlight Dr/Curve CT is expected to operate at LOS F, with both the left turns onto and off of Curve CT failing. The signalized intersection of US 40 & Elk River Rd is expected to operate at LOS E in the AM and LOS F in the PM. With the increased volumes along Elk River Rd, the intersection of Curve Plaza & Elk River Rd operates at LOS F in the PM, with the eastbound approach nearing capacity.

5. Long-Term Total Conditions

5.1 Trip Generation

The Long-Term Total Conditions scenario analyzes the study area assuming that Phase 1 of the Steamboat Basecamp has been completed, and the background traffic volumes have grown to the projected year 2040 levels. The number of trips generated by Phase 1 of the Basecamp remains unchanged from the Short-Term Total Conditions scenario, with the assumed trip generation shown in Table 9.

Table 9: ITE Trip Generation Calculations

Multifamily														
ITE Code	Units	ITE Land Use	Weekday Rate	Weekday Trips	AM Peak Rate	AM Peak Entering %	AM Peak Exiting %	PM Peak Rate	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
221	75	Multifamily Housing (Mid-Rise) (General Urban/Suburban)	5.43	407	0.19	26%	74%	0.45	72%	28%	4	10	24	9
Fitness Center														
ITE Code	Sq. Ft (1000 Ft)	ITE Land Use	Weekday Rate	Weekday Trips	AM Number of People	AM Peak Entering %	AM Peak Exiting %	PM Number of People	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
492	4.09	Health/Fitness Club	-	-	16	51%	49%	16	57%	43%	8	8	9	7
Market														
ITE Code	Sq. Ft (1000 Ft)	ITE Land Use	Weekday Rate	Weekday Trips	AM Peak Rate	AM Peak Entering %	AM Peak Exiting %	PM Peak Rate	PM Peak Entering %	PM Peak Exiting %	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
851	1.00	Convenience Market	763.71	382	62.54	50%	50%	20.84	51%	49%	16	16	5	5
936	1.00	Coffee/Donut Shop without Drive-Through	754.55	377	101.14	51%	49%	36.31	50%	50%	26	25	9	9
-	0.5	Kitchen/Storage Space	Not Expected to Generate Trips											
-	1.16	Lounge/Lobby Space	Not Expected to Generate Trips											

Phase 1 Total				
Weekday Trips	AM Peak Trips Entering	AM Peak Trips Exiting	PM Peak Trips Entering	PM Peak Trips Exiting
1342	53	59	48	30

The trip distribution of the Phase 1 trips will also remain consistent with the Short-Term Total Conditions scenario.

5.2 Auxiliary Lanes

Each turning movement on US 40 was assessed to see if SHAC auxiliary lane requirements are met. Since this portion of US 40 is classified as a Non-Rural Regional Highway (NR-A), a volume greater than 10 vehicles per hour (VPH) warrants a left turn deceleration lane, and a volume greater than 25 VPH warrants a right turn deceleration lane. A right turning movement of 50 VPH from the side street warrants an acceleration lane. Table 6 shows the warranted auxiliary lanes which are not already in place.

Table 10: Warranted Auxiliary Lanes

Intersection	Movement	2040 Total Conditions Volume
US 40 & Downhill Dr/Riverside Dr	WBR	185 VPH
US 40 & Sunlight Dr/Curve CT	NBL	197 VPH
	SBL	32 VPH

As noted previously, the US 40 & Downhill Dr/Riverside Dr intersection is the subject of current study by the City of Steamboat Springs. With respect to the US 40 & Sunlight Dr/Curve CT intersection, the Steamboat Basecamp project does not contribute any traffic volume to southbound left turn movement. The intersections at the Basecamp access #2 and access #3 do not require auxiliary lanes.

5.3 Long-Term Total Conditions Traffic Operations

Traffic operations were evaluated for the Long-Term Total Conditions using Synchro 10. The warranted auxiliary lanes have been included in the Synchro models. Table 11 shows the delay and LOS. The Synchro printouts can be found in the Appendix.

Table 11: Long-Term Total Conditions Delay and LOS

Intersection	AM			PM		
	Movement	Delay (sec)	LOS (v/c)	Movement	Delay (sec)	LOS (v/c)
US 40/Elk River Rd (Signal)	-	59.9	E	-	134.2	F
US 40/Sunlight Dr/Curve CT	NBL	80.7	F (0.93)	EB	1039.7	F (3.04)
US 40/Downhill Dr/Riverside Dr	-	93.8	C	-	34.6	C
Shield Dr/Elk River Rd	SB	15.5	C	SB	19.8	C
Curve Plaza/Elk River Rd/Access #1	EB	19.8	C	EB	117.9	F (1.1)
	WB	9.0	A	WB	9.8	A
Shield Dr/Access #2	WB	13.1	B	WB	12.7	B
Curve Ct/Access #3	SB	11.0	B	SB	11.4	B

As in the Short-Term Conditions, the intersection of Curve Plaza/Access #1 & Elk River Rd was modeled with a RIRO access to the Steamboat Basecamp. A splitter island could be used to make Access #1 a RIRO access, and would provide better traffic operations at the intersection than if both accesses were full movement. The right-in and right-out movements for the Basecamp are expected to experience little to no delay, with the significant delay coming from the left turns exiting the shopping plaza on the other side of Elk River Rd. Since the eastbound approach will be over capacity by the year 2040, alternative designs should be considered. One solution is to make the shopping plaza access a RIRO, however, this will result in out of direction travel. Another possibility is a roundabout, allowing full access to both the Steamboat Basecamp and the shopping plaza.

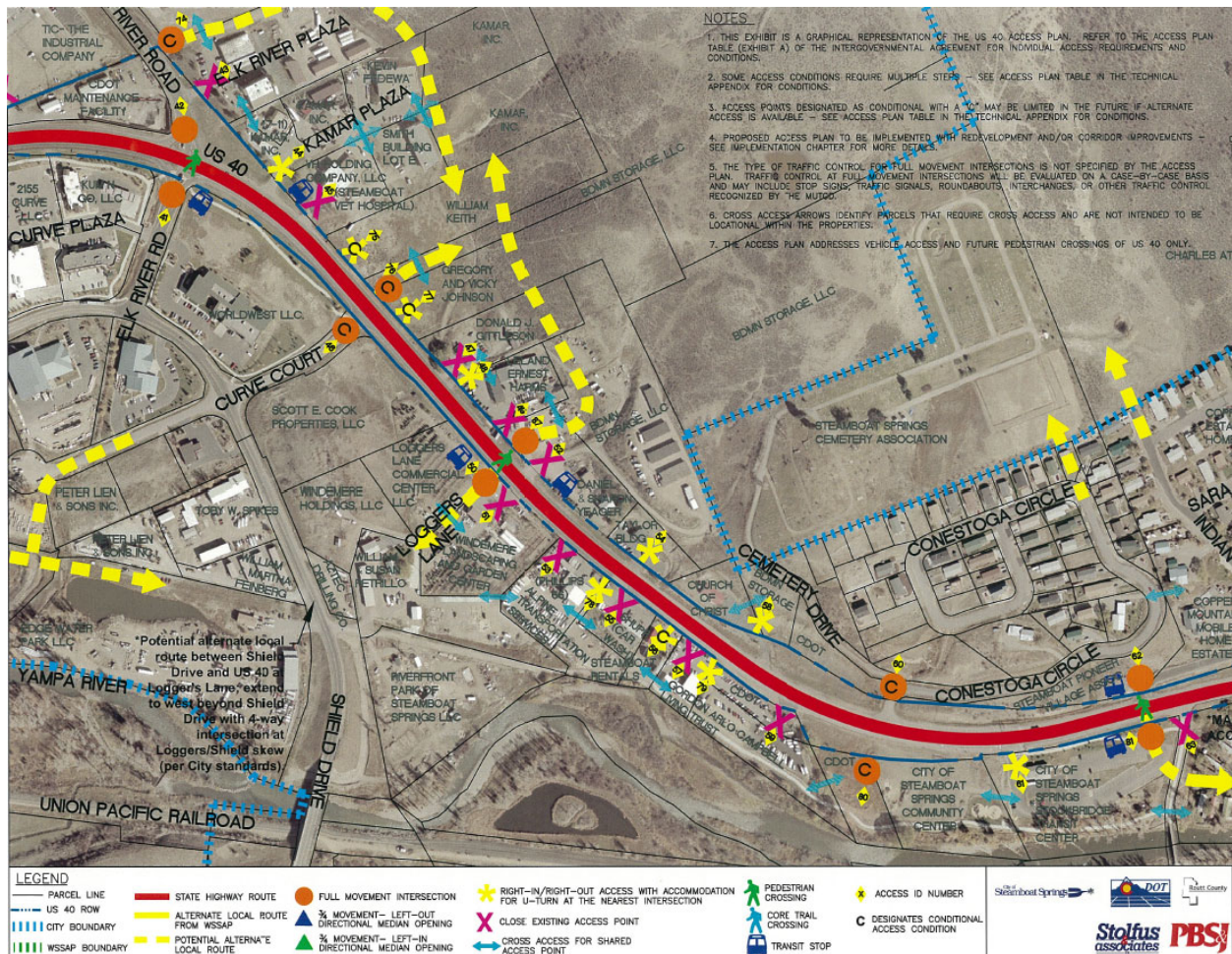
The intersection of US 40 & Elk River Rd fails in the PM, with a comparable delay to the 2040 Baseline Conditions. Much of this problem stems from the westbound direction on US 40 only having one thru lane, putting it over capacity, and is unrelated to the development. Until an additional westbound thru lane is built, it is unlikely that the intersection of US 40 & Elk River Rd will operate effectively during the peak hours of demand. The City of Steamboat has identified capacity issues along US-40 in the "US-40 Highway NEPA Study", and has proposed that US-40 be made a four-lane highway through the western side of town, which includes the study area. The intersection of US 40 & Elk River Rd will operate acceptably if US-40 is a four-lane highway through the intersection.

5.4 US 40 & Sunlight Dr/Curve CT

Traffic operations at US 40 & Sunlight Dr/Curve CT remain problematic for the left turns from Sunlight Dr and Curve Ct to US 40. It should be noted that the trips generated by this project do not contribute to the poor traffic conditions for these movements. The left turns out of Sunlight Dr are expected to experience delays exceeding the acceptable limit with or without the construction of Steamboat Basecamp.

The Synchro results also show the northbound left turn from US 40 onto Curve CT failing in the AM, due to an inability to find sufficient gaps in the southbound thru traffic along US 40. Synchro models a mostly uniform rate of arrival for the southbound movement, resulting in few gaps in traffic for the northbound lefts to make their turn. Since the signal of US 40 & Elk River Rd is only 700 feet upstream, the southbound movement will actually be passing Curve CT in platoons, rather than in a more uniform arrival pattern. The platooning effect will provide larger gaps, allowing a longer opportunity of time for the northbound lefts to turn onto Curve CT. The microsimulation extension of Synchro, SimTraffic, was used to analyze this intersection, as it has the ability to more accurately analyze the platooning effects along US 40. The simulation runs from SimTraffic show the northbound left turn movement having a delay of 37.9 seconds (LOS E). LOS D or better conditions are typically desirable during peak traffic periods; however, LOS E conditions are not uncommon, particularly for unsignalized movements onto and off of side streets.

The West Steamboat Springs US Highway 40 Access Study specifies that the intersection of US 40 & Sunlight Dr/Curve CT may be converted to a RIRO if safety or traffic operational problems occur, or if the intersection of US 40 & Loggers Lane, just to the east, is extended to connect US 40 to Shield Dr. The section of the access control plan in the vicinity of US 40 & Sunlight Dr/Curve CT is shown in Figure 7. Converting the intersection of US 40 & Sunlight Dr/Curve CT to a RIRO would divert all of the northbound lefts to the intersection of US 40 & Elk River Rd, which is expected to already be operating at capacity by year 2040. An alternative long-term strategy may be to make the US 40 & Sunlight Dr/Curve CT intersection a 3/4 movement (left-in, right-in, right-out) in order to minimize impacts to the Elk River Rd intersection.



5.5 Queuing

Since the intersections of US 40 & Elk River Rd and Curve Plaza & Elk River Rd are spaced only 210 feet apart, the queuing between the two intersections was analyzed to ensure that neither intersection would be blocked. The northbound left turn moment at the signal of US 40 & Elk River Rd has a projected 95% queue of 180 feet. This puts the back of the queue only 30 feet away from extending into the intersection of Curve Plaza & Elk River Rd. Signage warning drivers not to block the intersection should be installed at the intersection of Curve Plaza & Elk River Rd if queues become problematic in the future. This will reduce the chances of the northbound queue from US 40 & Elk River Rd blocking drivers turning onto and off of Elk River Rd.

6. Site Contribution

In Colorado, all accesses to the state highway are regulated by the Colorado Department of Transportation (CDOT). Colorado's state highway system constitutes a valuable resource and a major public and private investment. It is the purpose of the SHAC to provide procedures and standards to aid in the management of that investment, to protect the public health, safety, and welfare, to maintain smooth traffic flow, and to protect the functional level of state highways while considering state, regional, and local transportation needs and interests. CDOT requires an access permit to be submitted if the traffic of a facility or operation exceeds 20% of the existing permitted traffic volumes at the access onto a state highway. The year 2022 volumes were used to determine the site contribution. The Steamboat Basecamp is expected to increase the existing traffic volumes by 24% at the access of US 40 & Sunlight Dr/Curve CT, and by 11% at the intersection of US 40 & Elk River Rd. Since the volumes accessing Curve CT are expected to increase by over 20%, an access permit will be required for that intersection.

There are future plans to improve the intersection of US 40 & Downhill Dr/Riverside Dr. Since the development of Steamboat Basecamp will contribute trips to this intersection, the developer will be required to contribute a determined percentage to the cost of intersection improvements. The percent contribution for intersection improvements at US 40 & Downhill Dr/Riverside Dr is determined by the percent of the total traffic volumes entering the intersection that is made up of trips going to or from the Steamboat Basecamp. The higher percentage between the AM and PM peak hours will be used to determine the contribution percentage. From the trip distribution assumptions, there will be 26 site trips entering the intersection in the AM, and 17 trips entering in the PM. The AM site trips account for 1.26% of the total entering trips, meaning that the Steamboat Basecamp is responsible for 1.26% of the intersection improvement costs.

7. Alternate Modes of Transportation

The City of Steamboat Springs has several multimodal options, including bus lines, bike lanes, and bike/walking paths. The Red Line and Blue Line both stop at the Elk River Crossing bus stop, just east of Steamboat Basecamp on Elk River Rd. Each of these bus lines have routes that go into downtown Steamboat Ski Resort. The Red Line and Blue Line stop at Elk River Crossing every 20 minutes from 6:35 AM to 11:45 PM.

The Yampa River Core Trail passes just south of the Steamboat Basecamp, intersecting with Shield Dr, and extending through downtown Steamboat. Cyclists looking to ride from the Steamboat Basecamp into downtown Steamboat will most likely take this trail. Many of the roads in downtown Steamboat have bike lanes making it easy for cyclists to exit the Yampa River Core Trail and use the roadway network to reach their destination.

Findings and Recommendations

The traffic impact study conducted for the Steamboat Basecamp in Steamboat Springs has concluded that the traffic volumes generated by the facility can be accommodated by the surrounding roadway system. The following is a summary of the study's findings:

1. A state highway access permit is required for the access at the intersection of US 40 & Sunlight Dr/Curve CT. In addition, both a southbound and northbound left turn lane should be added. Both of these turn lanes can be added by restriping the two-way left-turn median.
2. The Steamboat Basecamp is not responsible for failing traffic operations at the intersection of US 40 & Elk River Rd. For this intersection to operate well during peak hours in year 2040, and consistent with current long-range plans, a second westbound thru lane will need to be constructed.
3. The Steamboat Basecamp is not responsible for failing traffic operations at the intersection of Sunlight Dr/Curve CT. In order to operate acceptably in the long term, the intersection could be made a $\frac{3}{4}$ movement (left-in, right-in, right-out).
4. The Steamboat Basecamp contributes 1.26% of the traffic volume at the intersection of US 40 & Downhill Dr/Riverside Dr in the AM peak hour.
5. Signage warning drivers not to block the intersection should be installed at the intersection of Access #1 & Elk River Rd if queues become problematic in the future.
6. Access #1 to the Basecamp should be made right-in, right-out to help accommodate increased future traffic volumes. A splitter island and accompanying signage could be used to restrict the movements.

Appendix A

Scope Approval Form

Attachment A
TRAFFIC IMPACT STUDY – SCOPE APPROVAL FORM

Prior to starting a traffic impact study, a Scope Approval Form must be submitted for review and signed by the City Public Works Director. It shall be included in every traffic study submittal as Attachment A. This Scope Approval Form is for City requirements only. Consultants must contact CDOT to determine requirements related to access permits and work in CDOT right-of-way.

Project Information

Project Name:	Steamboat Basecamp (Phase 1)
Project Location:	East of Shield Drive between Elk River Road and Curve Court (Former site of the Steamboat Pilot)
Developer Name/ Contact Number:	Kevin Riegler May Riegler Properties (202) 369-5820
Traffic Engineer Name/ Contact Number:	Max Rusch, PE Stolfus & Associates, Inc. (303) 221-2330; max@stolfusandassociates.com

Study Parameters

Type of Study Required: ☐ Trip Generation Letter ☒ Long-term Traffic Study
 ☐ Short-term Traffic Study ☐ Trip Evaluation Letter

Traffic Counts

☐ Winter Zone ☒ Summer Zone
☒ Counts w/in last 2 years are available
☐ New counts will be collected on _____
☐ Existing counts will be estimated based on:
☒ Future counts will be estimated based on a _____% growth rate. A 1.16 20-year factor (0.75% per year) from OTIS Count Station #101838

Peak Hours Analyzed

☒ AM Peak Hour ☒ PM peak hour ☐ Other _____

Trip Generation Rates

☒ From ITE ☐ Other (cite) _____
☐ No passby or mode split (typical) The suitability of these adjustments will be consistent with the Trip Generation, 10th Edition Supplement and potentially other sources, subject to City and CDOT approval.
☒ Passby or mode split (describe) _____

Trip Distribution – Attach sketch A-1

Study Parameters

List of Study Area Intersections

1.	Lincoln Avenue &	Elk River Road
2.	Lincoln Avenue &	Curve Court
3.	Elk River Road &	Curve Plaza / Site Access #1
4.	Elk River Road &	Shield Drive
5.	Shield Drive &	Site Access #2
6.	Curve Court &	Site Access #3
7.		

Key Analysis items

- ☒ Existing + site traffic at study intersections
☒ Peak Hour LOS at study intersections
☒ % Site contribution to signal at Lincoln Ave & Downhill Drive
☒ Auxiliary lane evaluation at Lincoln Ave & Curve Court, Shield Drive & all Site Accesses
☐ Traffic signal warrants at _____
☐ Four-way stop sign warrants at _____
☒ Queuing Analysis at Lincoln Ave & Elk River Road
☒ Other Evaluate need for CDOT permitting

Approvals

Max Rusch

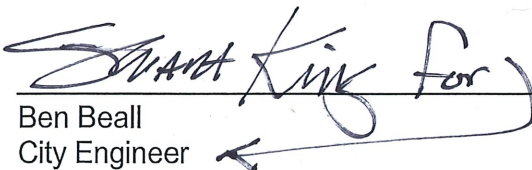
4/12/2021

303-221-2330

Prepared By:

Date

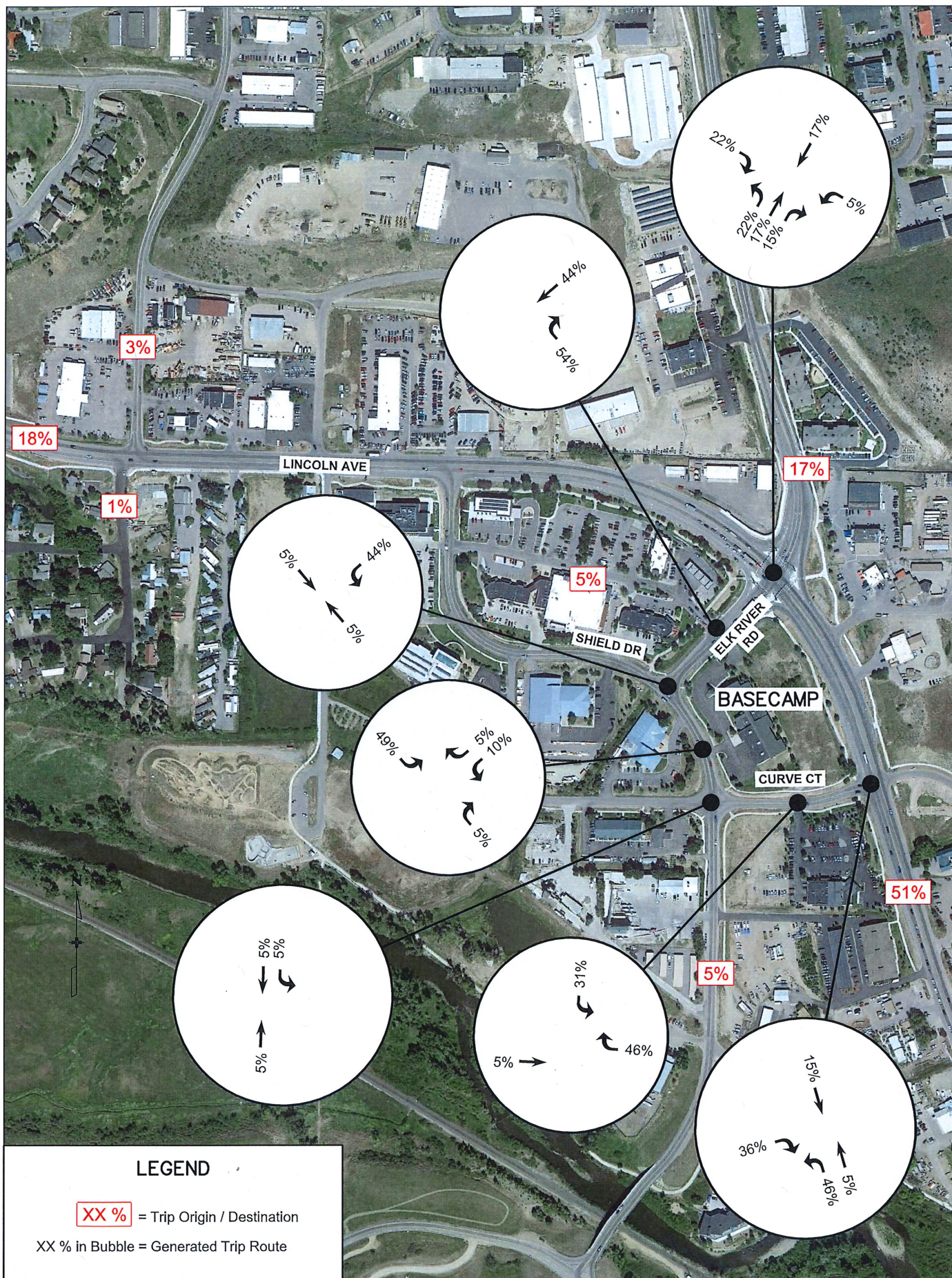
Phone


 Ben Beall
 City Engineer

Date

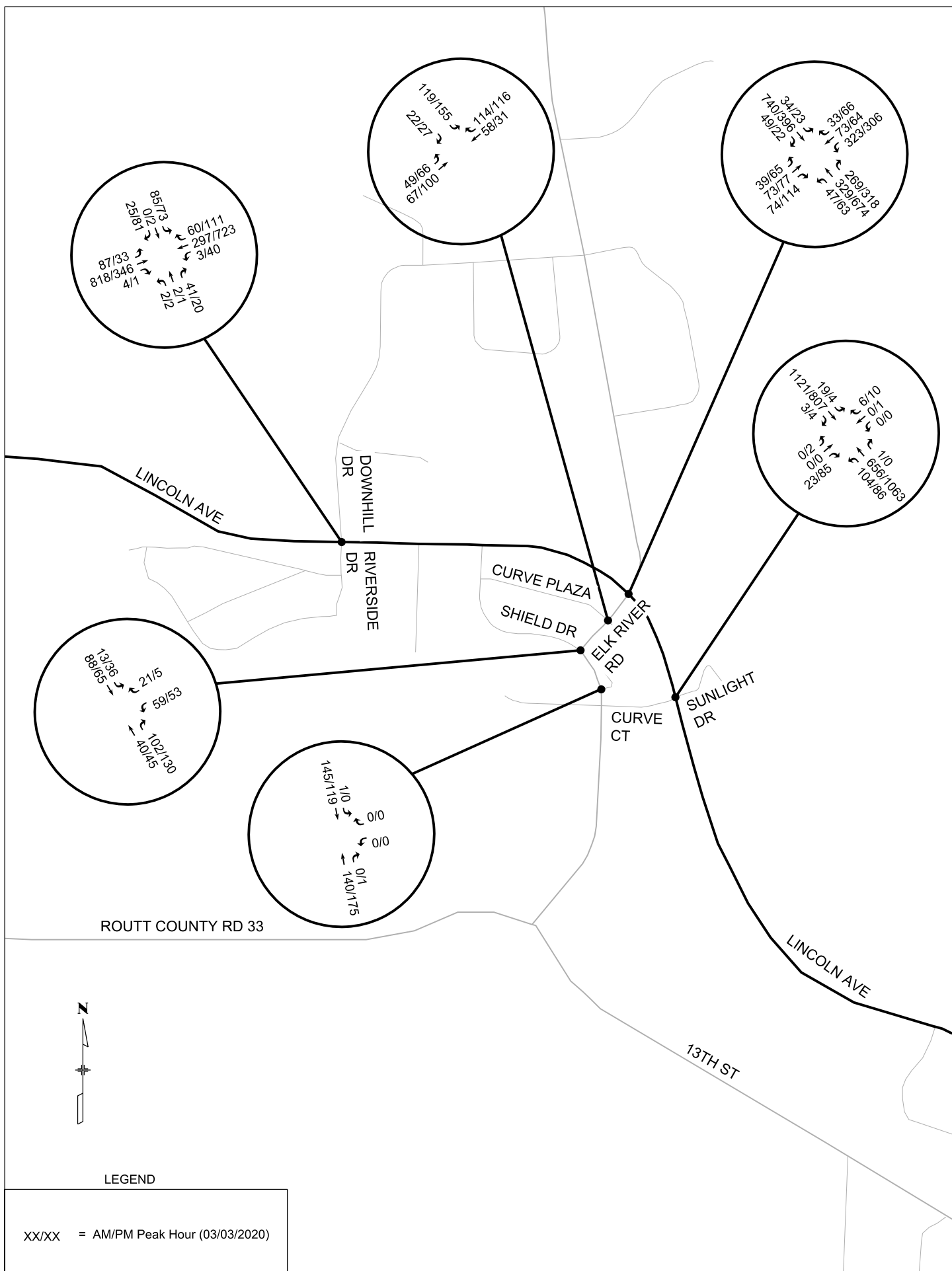
Phone

Please note that the approval of this scope approval form shall not be construed as an approval of the proposed use, but rather a methodology for evaluation of the proposed use. During the city development review process, the proposed use will be reviewed by city staff for compliance with code, standards, and community planning documents.

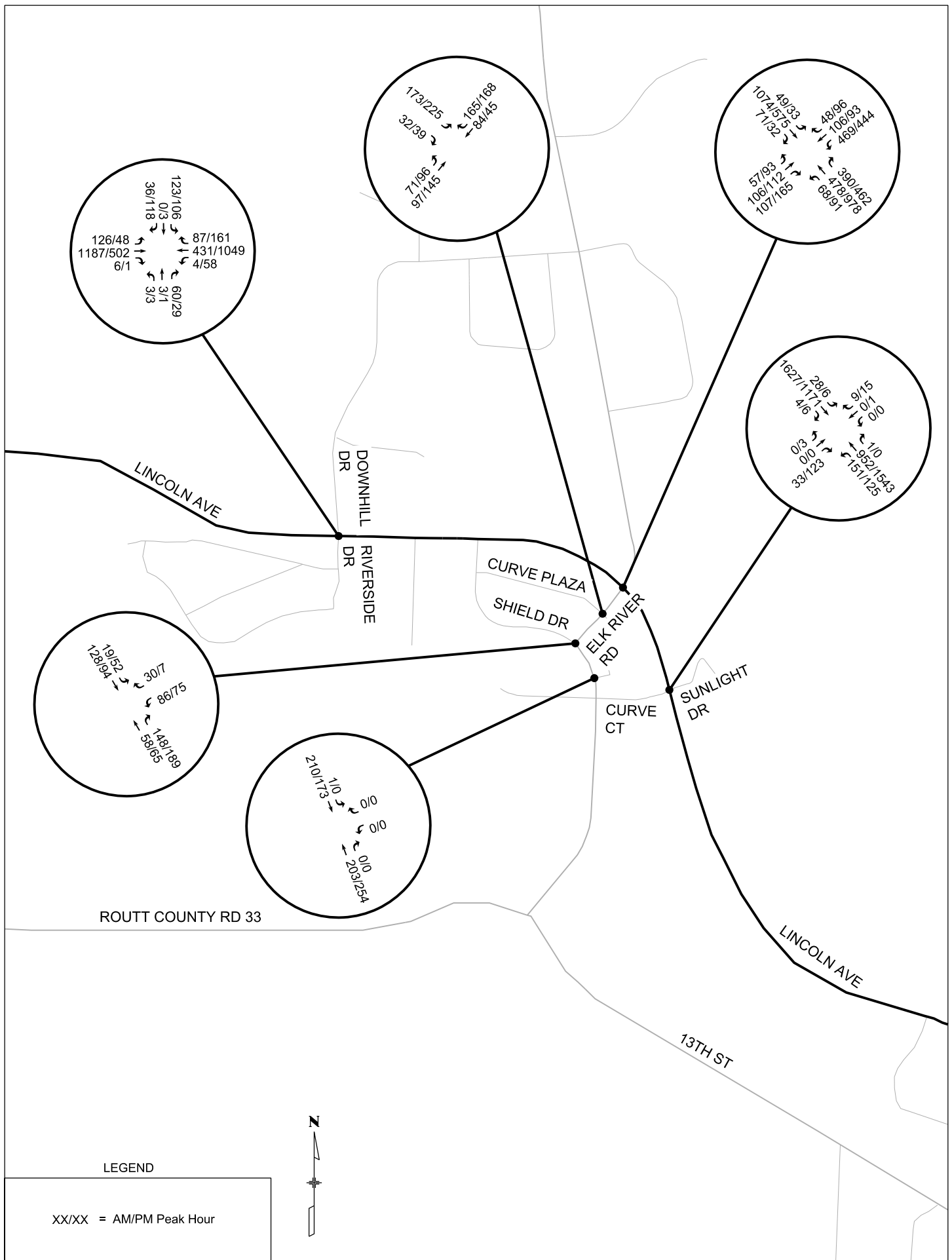


Appendix B

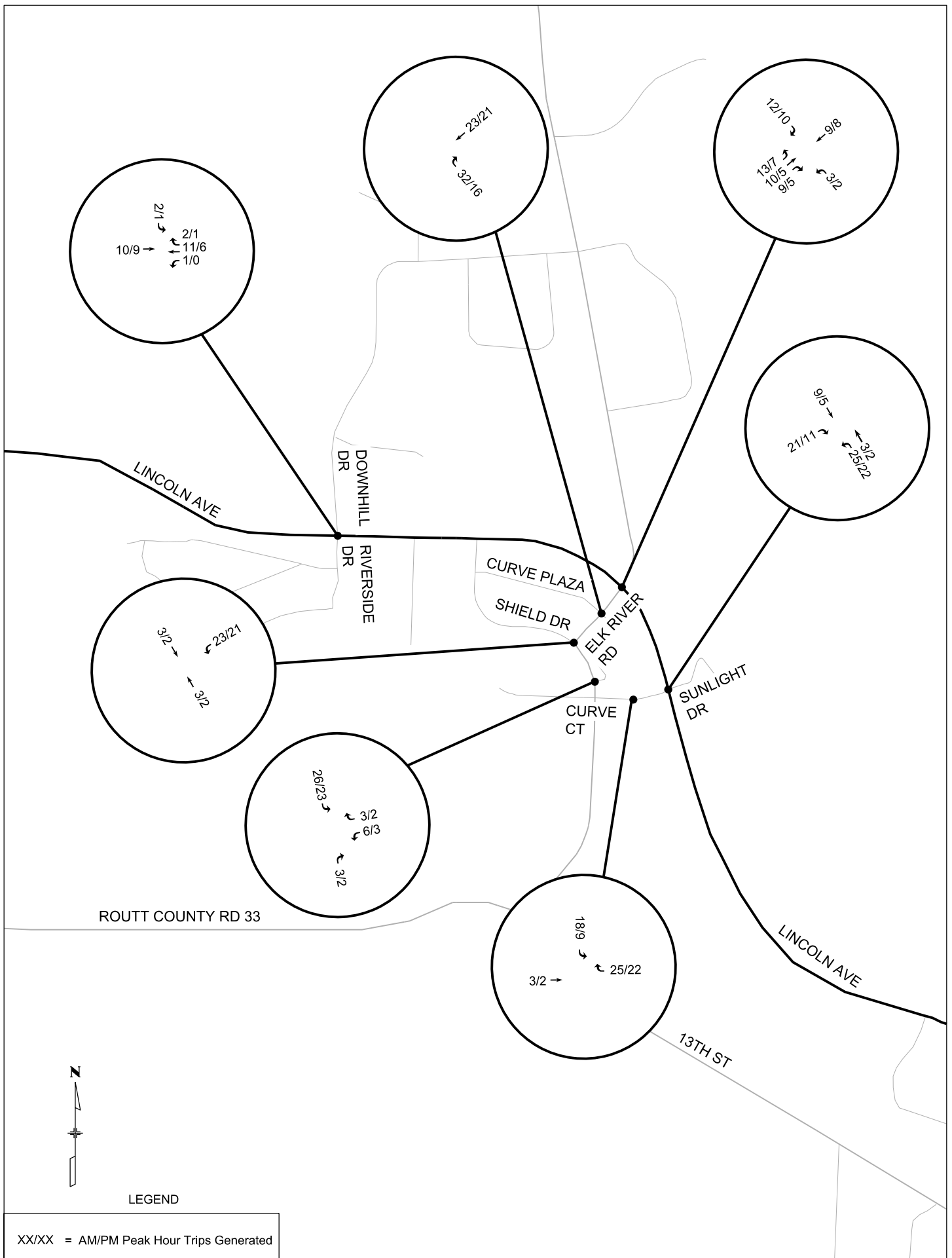
Volume Sheets
Traffic Count Data



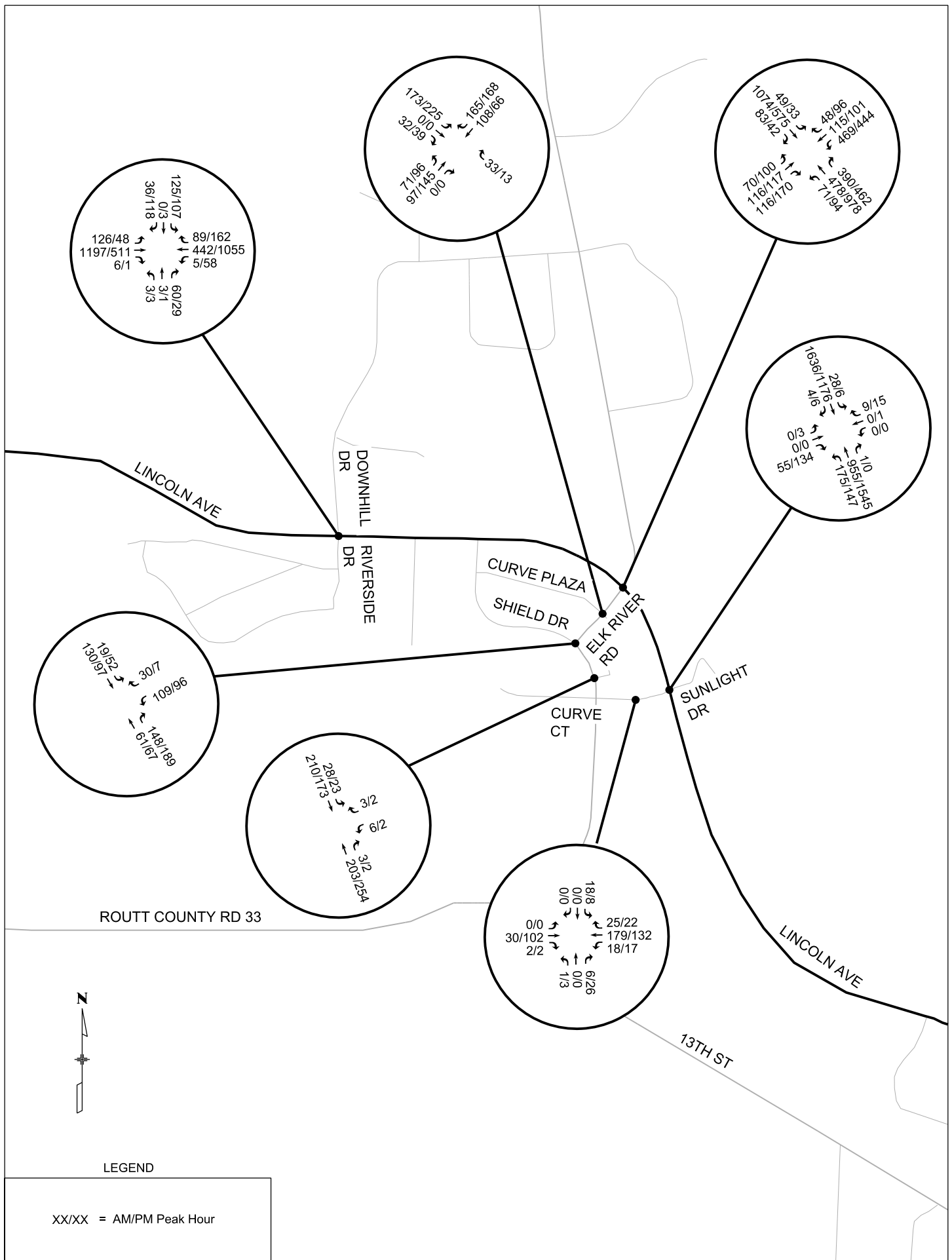
Existing Traffic Volumes



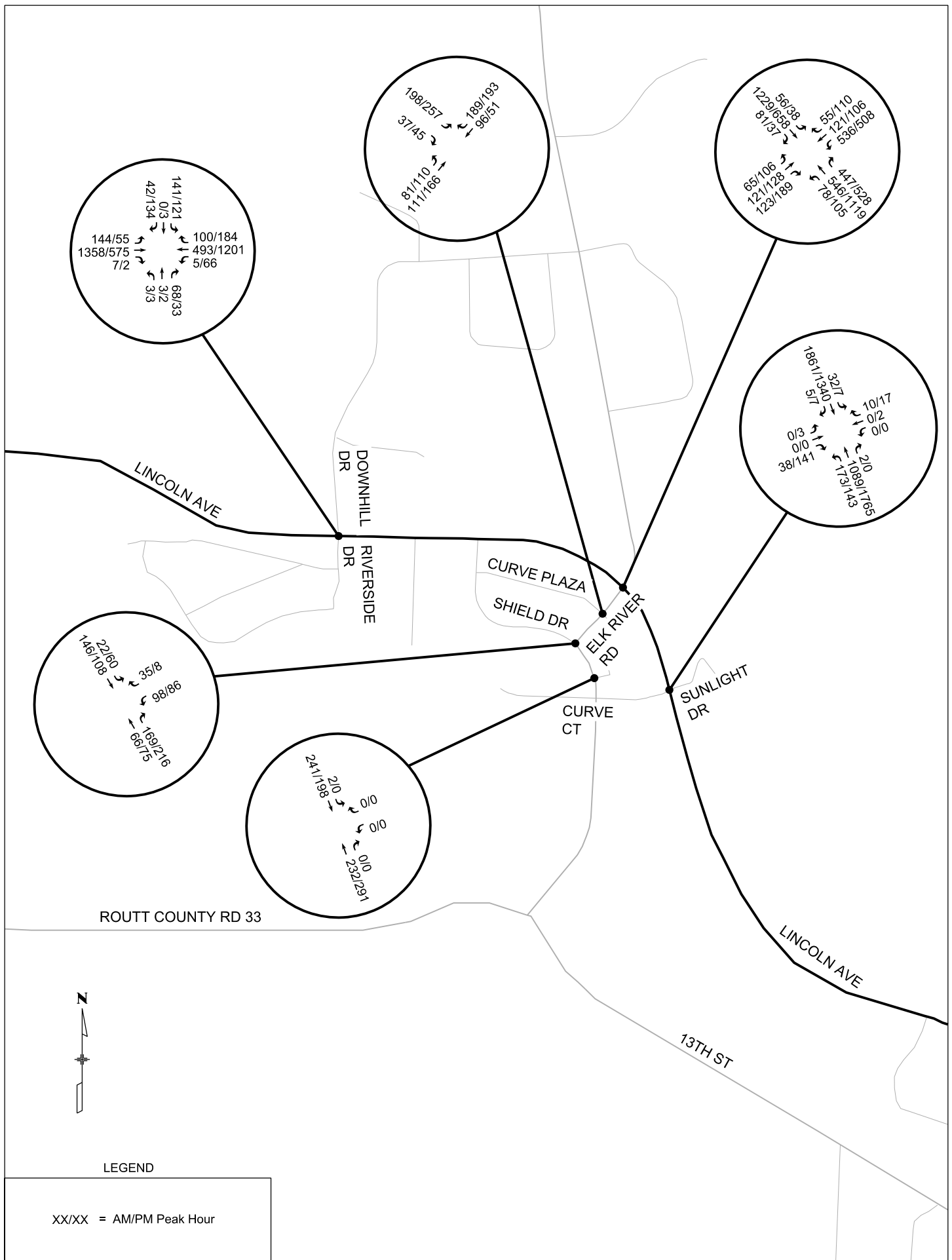
SHORT TERM BACKGROUND CONDITIONS (JULY 2022)



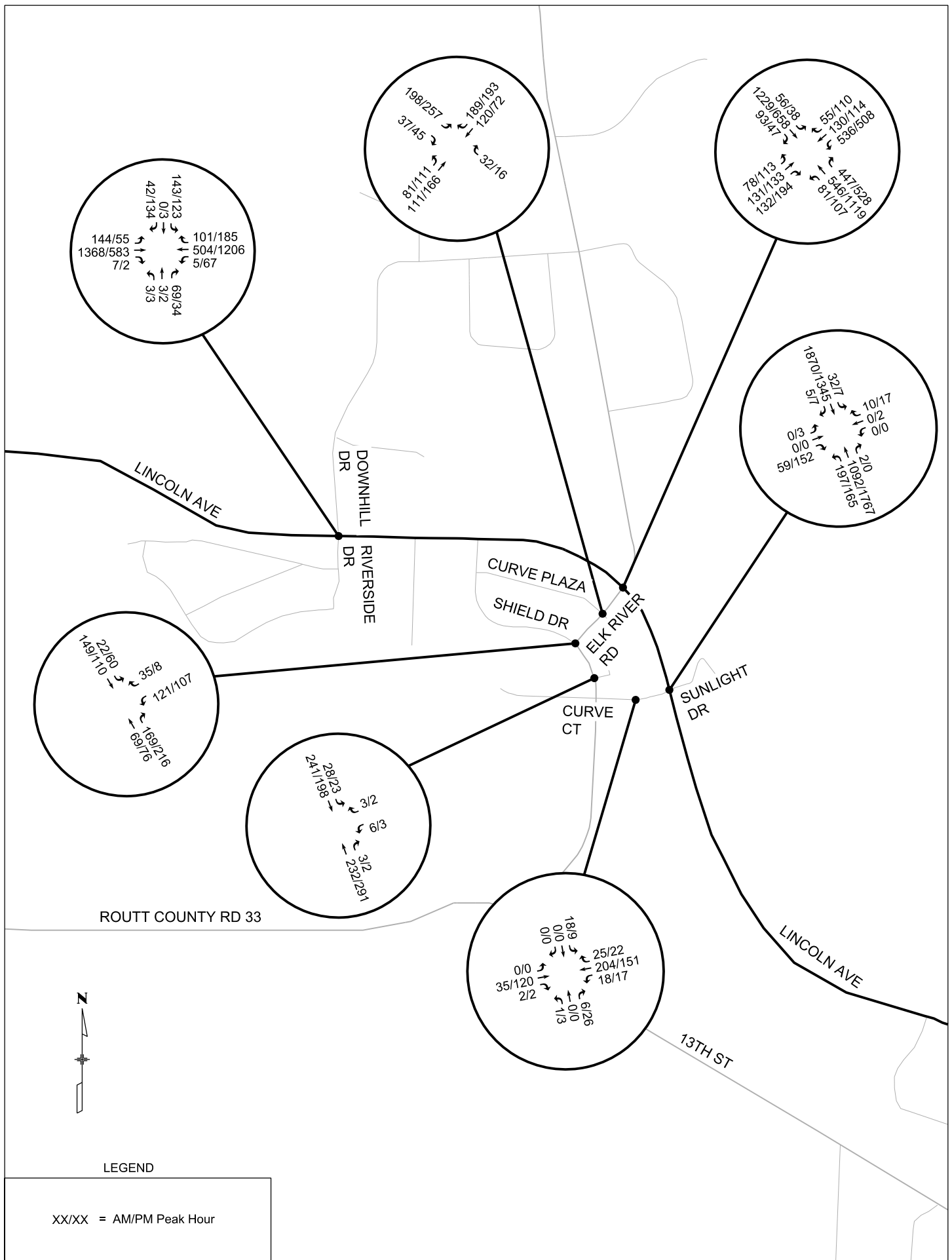
TRIPS GENERATED FROM STEAMBOAT BASECAMP PHASE 1



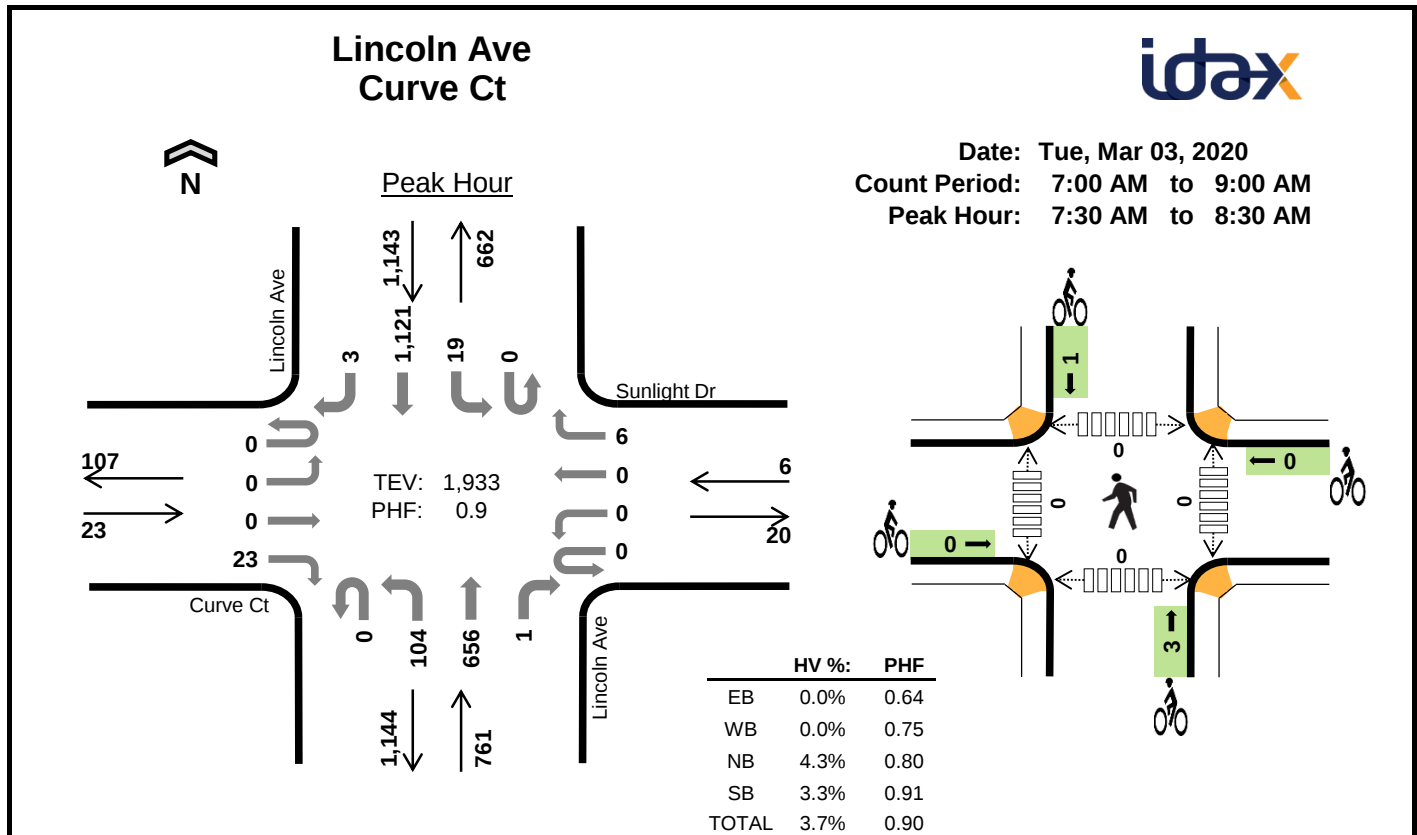
SHORT TERM TOTAL CONDITIONS (JULY 2022)



LONG TERM BACKGROUND CONDITIONS (JULY 2040)



LONG TERM TOTAL CONDITIONS (JULY 2040)

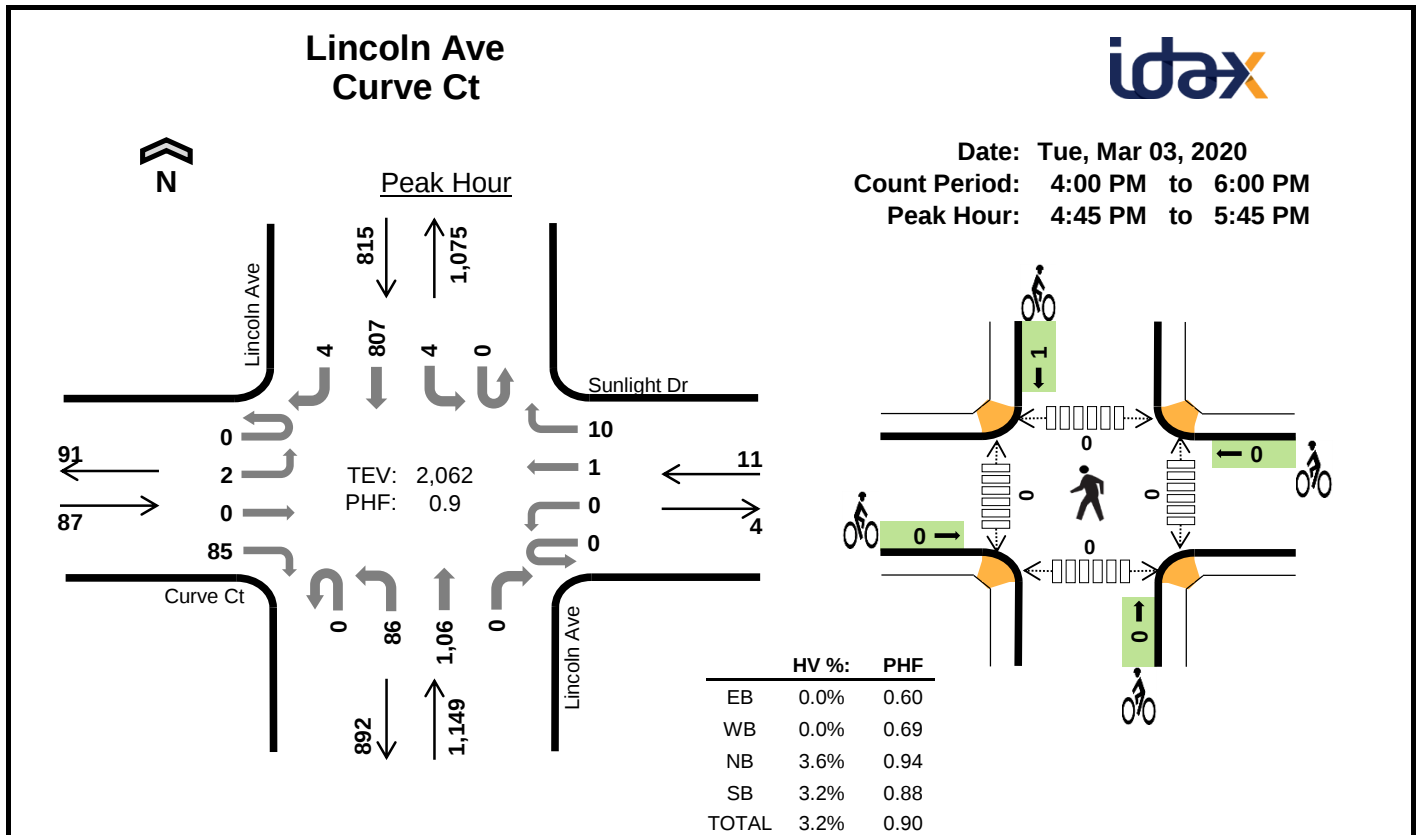


Two-Hour Count Summaries

Interval Start	Curve Ct				Sunlight Dr				Lincoln Ave				Lincoln Ave				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	0	8	0	0	0	0	0	11	59	0	0	2	188	0	268	0
7:15 AM	0	0	0	5	0	0	0	0	0	13	110	0	0	4	188	1	321	0
7:30 AM	0	0	0	5	0	0	0	2	0	16	103	0	0	3	300	0	429	0
7:45 AM	0	0	0	2	0	0	0	0	0	22	180	1	0	7	304	2	518	1,536
8:00 AM	0	0	0	9	0	0	0	2	0	35	204	0	0	5	283	0	538	1,806
8:15 AM	0	0	0	7	0	0	0	2	0	31	169	0	0	4	234	1	448	1,933
8:30 AM	0	0	0	10	0	1	0	1	0	24	125	0	0	1	191	2	355	1,859
8:45 AM	0	1	0	10	0	0	0	0	0	26	127	2	0	4	230	1	401	1,742
Count Total	0	1	0	56	0	1	0	7	0	178	1,077	3	0	30	1,918	7	3,278	0
Peak Hour	0	0	0	23	0	0	0	6	0	104	656	1	0	19	1,121	3	1,933	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	3	0	3	2	8	0	0	0	0	0	0	0	0	0	0
7:15 AM	4	0	6	7	17	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	3	13	16	0	0	0	1	1	0	0	0	0	0
7:45 AM	0	0	1	11	12	0	0	2	0	2	0	0	0	0	0
8:00 AM	0	0	14	8	22	0	0	1	0	1	0	0	0	0	0
8:15 AM	0	0	15	6	21	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	7	10	17	0	0	0	0	0	0	0	0	0	0
8:45 AM	1	0	4	10	15	0	0	0	0	0	0	0	0	0	0
Count Total	8	0	53	67	128	0	0	3	1	4	0	0	0	0	0
Peak Hour	0	0	33	38	71	0	0	3	1	4	0	0	0	0	0



Two-Hour Count Summaries

Interval Start	Curve Ct				Sunlight Dr				Lincoln Ave				Lincoln Ave				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	1	0	32	0	0	1	4	1	42	240	1	0	1	213	2	538	0
4:15 PM	0	0	1	20	0	0	0	2	0	24	237	0	0	2	206	0	492	0
4:30 PM	0	1	0	12	0	0	0	3	0	22	205	1	0	0	166	1	411	0
4:45 PM	0	0	0	15	0	0	0	3	0	32	247	0	0	1	193	1	492	1,933
5:00 PM	0	0	0	36	0	0	0	2	0	25	279	0	0	2	229	1	574	1,969
5:15 PM	0	1	0	14	0	0	1	3	0	15	270	0	0	1	204	1	510	1,987
5:30 PM	0	1	0	20	0	0	0	2	0	14	267	0	0	0	181	1	486	2,062
5:45 PM	0	0	1	11	0	0	1	2	0	14	212	0	0	2	192	0	435	2,005
Count Total	0	4	2	160	0	0	3	21	1	188	1,957	2	0	9	1,584	7	3,938	0
Peak Hour	0	2	0	85	0	0	1	10	0	86	1,063	0	0	4	807	4	2,062	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

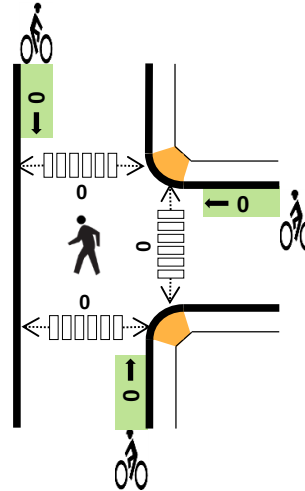
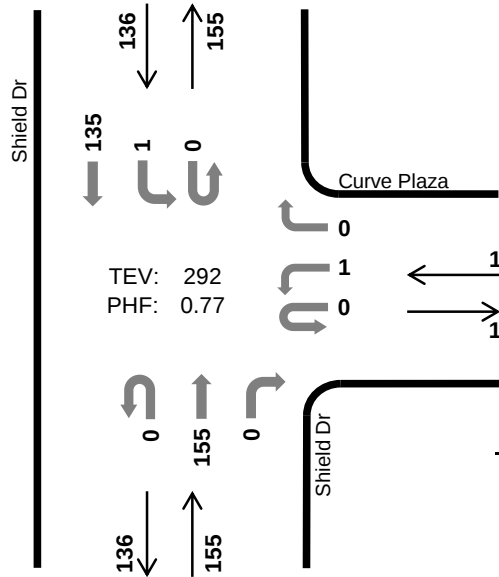
Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	0	14	5	19	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	8	9	17	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	8	6	14	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	13	8	21	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	9	4	13	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	14	8	22	0	0	0	1	1	0	0	0	0	0
5:30 PM	0	0	5	6	11	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	4	5	9	0	0	0	1	1	0	2	0	0	2
Count Total	0	0	75	51	126	0	0	0	2	2	0	2	0	0	2
Peak Hour	0	0	41	26	67	0	0	0	1	1	0	0	0	0	0

Shield Dr Curve Plaza



Peak Hour

Date: Tue, Mar 03, 2020
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:45 AM to 8:45 AM



	HV %:	PHF
EB	-	-
WB	0.0%	0.25
NB	11.0%	0.86
SB	1.5%	0.58
TOTAL	6.5%	0.77

Two-Hour Count Summaries

Interval Start	0				Curve Plaza				Shield Dr				Shield Dr				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	0	0	0	0	0	0	0	0	24	0	0	0	16	0	40	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	21	0	0	0	17	0	38	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	30	0	0	0	35	0	65	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	36	0	0	0	59	0	95	238
8:00 AM	0	0	0	0	0	0	0	0	0	0	37	0	0	1	30	0	68	266
8:15 AM	0	0	0	0	0	0	0	0	0	0	37	0	0	0	21	0	58	286
8:30 AM	0	0	0	0	0	1	0	0	0	0	45	0	0	0	25	0	71	292
8:45 AM	0	0	0	0	0	0	0	0	0	0	33	0	0	0	29	0	62	259
Count Total	0	0	0	0	0	1	0	0	0	0	263	0	0	1	232	0	497	0
Peak Hour	0	0	0	0	0	1	0	0	0	0	155	0	0	1	135	0	292	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

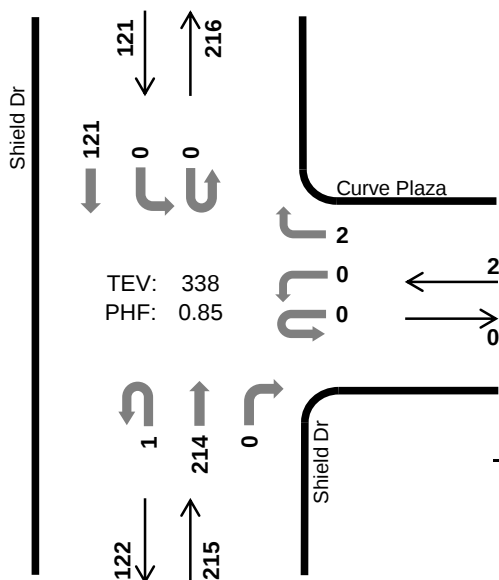
Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	0	0	3	3	6	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	4	2	6	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	6	1	7	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	2	1	3	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	4	1	5	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	4	1	5	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	34	9	43	0	0	0	0	0	0	0	0	0	0
Peak Hr	0	0	17	2	19	0	0	0	0	0	0	0	0	0	0

Shield Dr Curve Plaza

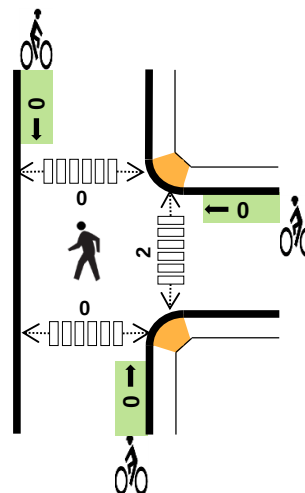


Peak Hour

Date: Tue, Mar 03, 2020
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:00 PM to 5:00 PM



	HV %:	PHF
EB	-	-
WB	0.0%	0.25
NB	4.7%	0.85
SB	5.8%	0.82
TOTAL	5.0%	0.85



Two-Hour Count Summaries

Interval Start	0				Curve Plaza				Shield Dr				Shield Dr				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	0	0	0	0	2	0	0	63	0	0	0	35	0	100	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	52	0	0	0	37	0	89	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	50	0	0	0	21	0	71	0
4:45 PM	0	0	0	0	0	0	0	0	1	0	49	0	0	0	28	0	78	338
5:00 PM	0	0	0	0	0	0	0	0	0	0	56	0	0	0	40	0	96	334
5:15 PM	0	0	0	0	0	0	0	0	0	0	40	0	0	0	30	0	70	315
5:30 PM	0	0	0	0	0	0	0	0	0	0	30	0	0	0	21	0	51	295
5:45 PM	0	0	0	0	0	0	0	0	0	0	24	0	0	0	16	0	40	257
Count Total	0	0	0	0	0	0	0	2	1	0	364	0	0	0	228	0	595	0
Peak Hour	0	0	0	0	0	0	0	2	1	0	214	0	0	0	121	0	338	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

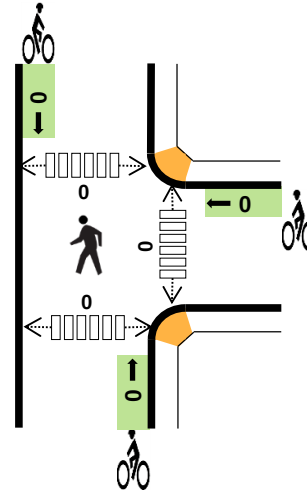
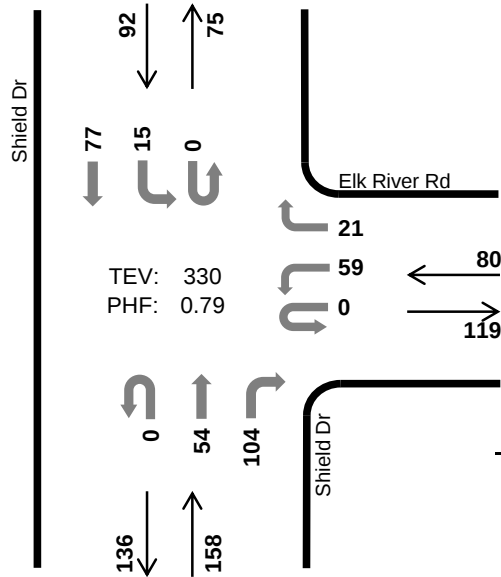
Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	0	3	0	3	0	0	0	0	0	0	1	0	0	1
4:15 PM	0	0	4	5	9	0	0	0	0	0	0	1	0	0	1
4:30 PM	0	0	1	2	3	0	0	0	0	0	1	0	0	0	1
4:45 PM	0	0	2	0	2	0	0	0	0	0	1	1	0	0	2
5:00 PM	0	0	1	1	2	0	0	0	1	1	1	0	0	1	2
5:15 PM	0	0	3	0	3	0	0	0	0	0	2	0	0	2	4
5:30 PM	0	0	1	0	1	0	0	0	0	0	1	0	0	0	1
5:45 PM	0	0	0	0	0	0	0	0	0	0	2	0	0	0	2
Count Total	0	0	15	8	23	0	0	0	1	1	8	3	0	3	14
Peak Hr	0	0	10	7	17	0	0	0	0	0	2	3	0	0	5

Shield Dr Elk River Rd



Peak Hour

Date: Tue, Mar 03, 2020
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:45 AM to 8:45 AM



	HV %:	PHF
EB	-	-
WB	2.5%	0.77
NB	10.8%	0.86
SB	0.0%	0.49
TOTAL	5.8%	0.79

Two-Hour Count Summaries

Interval Start	0				Elk River Rd				Shield Dr				Shield Dr				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	0	0	0	6	0	3	0	0	8	16	0	0	10	0	43	0
7:15 AM	0	0	0	0	0	12	0	4	0	0	5	17	0	3	5	0	46	0
7:30 AM	0	0	0	0	0	15	0	3	0	0	4	26	0	4	20	0	72	0
7:45 AM	0	0	0	0	0	17	0	4	0	0	10	27	0	4	43	0	105	266
8:00 AM	0	0	0	0	0	17	0	9	0	0	11	25	0	2	13	0	77	300
8:15 AM	0	0	0	0	0	10	0	5	0	0	15	24	0	3	12	0	69	323
8:30 AM	0	0	0	0	0	15	0	3	0	0	18	28	0	6	9	0	79	330
8:45 AM	0	0	0	0	1	13	0	4	0	0	10	23	0	4	15	0	70	295
Count Total	0	0	0	0	1	105	0	35	0	0	81	186	0	26	127	0	561	0
Peak Hour	0	0	0	0	0	59	0	21	0	0	54	104	0	15	77	0	330	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

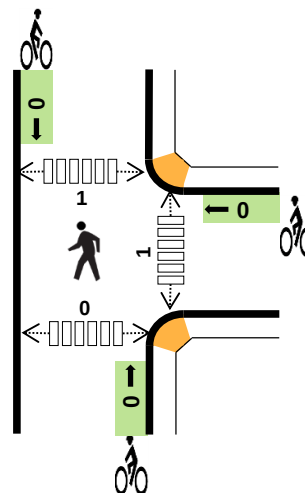
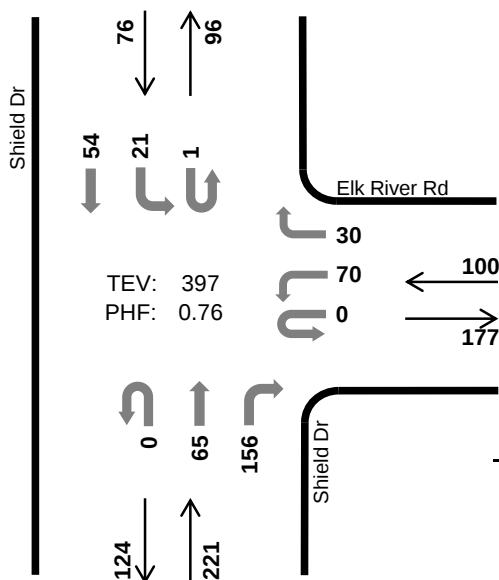
Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	0	3	4	0	7	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	1	4	2	7	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	1	6	0	7	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	6	0	6	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	1	2	0	3	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	5	0	5	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	1	4	0	5	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	4	1	5	0	0	0	0	0	0	0	0	0	0
Count Total	0	7	35	3	45	0	0	0	0	0	0	0	0	0	0
Peak Hr	0	2	17	0	19	0	0	0	0	0	0	0	0	0	0

Shield Dr Elk River Rd



Peak Hour

Date: Tue, Mar 03, 2020
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:00 PM to 5:00 PM



	HV %:	PHF
EB	-	-
WB	3.0%	0.63
NB	4.5%	0.84
SB	5.3%	0.79
TOTAL	4.3%	0.76

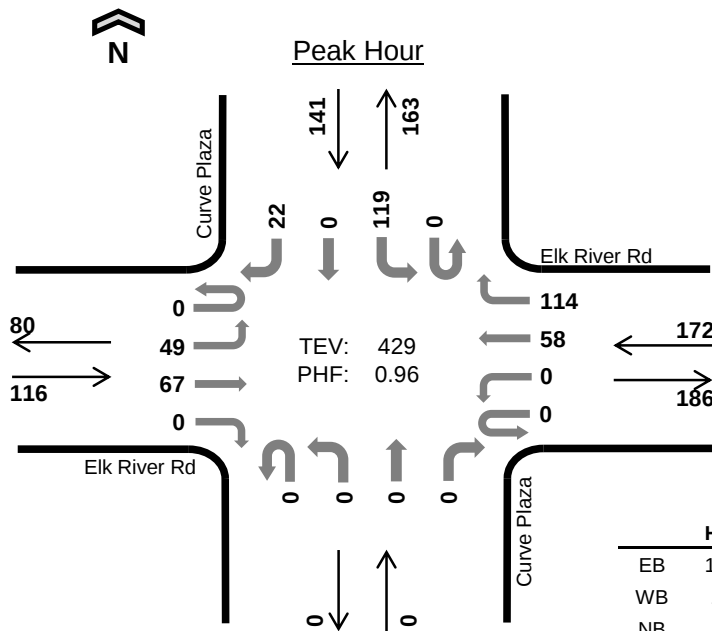
Two-Hour Count Summaries

Interval Start	0				Elk River Rd				Shield Dr				Shield Dr				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	0	0	0	0	19	0	21	0	0	21	45	0	8	16	0	130	0
4:15 PM	0	0	0	0	0	25	0	4	0	0	14	39	1	6	15	0	104	0
4:30 PM	0	0	0	0	0	12	0	3	0	0	13	42	0	3	9	0	82	0
4:45 PM	0	0	0	0	0	14	0	2	0	0	17	30	0	4	14	0	81	397
5:00 PM	0	0	0	0	1	11	0	3	0	0	10	49	0	21	27	0	122	389
5:15 PM	0	0	0	0	0	18	0	0	0	0	10	30	0	6	12	0	76	361
5:30 PM	0	0	0	0	0	9	0	0	0	0	8	21	0	5	12	0	55	334
5:45 PM	0	0	0	0	0	8	0	0	0	0	8	16	0	4	7	0	43	296
Count Total	0	0	0	0	1	116	0	33	0	0	101	272	1	57	112	0	693	0
Peak Hour	0	0	0	0	0	70	0	30	0	0	65	156	1	21	54	0	397	0

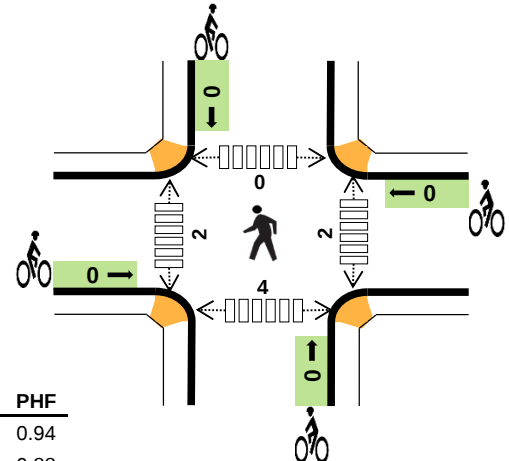
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	2	4	3	9	0	0	0	0	0	0	1	1	0	2
4:30 PM	0	1	1	1	3	0	0	0	0	0	1	0	0	0	1
4:45 PM	0	0	2	0	2	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	1	1	0	2	0	1	0	0	1	1	0	0	0	1
5:15 PM	0	0	3	0	3	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	1	0	1	0	0	0	0	0	0	0	1	0	1
5:45 PM	0	0	0	2	2	0	0	0	0	0	0	0	0	0	0
Count Total	0	4	15	6	25	0	1	0	0	1	2	1	2	0	5
Peak Hr	0	3	10	4	17	0	0	0	0	0	1	1	1	0	3

Curve Plaza Elk River Rd



Date: Tue, Mar 03, 2020
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:30 AM to 8:30 AM



	HV %:	PHF
EB	15.5%	0.94
WB	1.2%	0.88
NB	-	-
SB	2.1%	0.86
TOTAL	5.4%	0.96

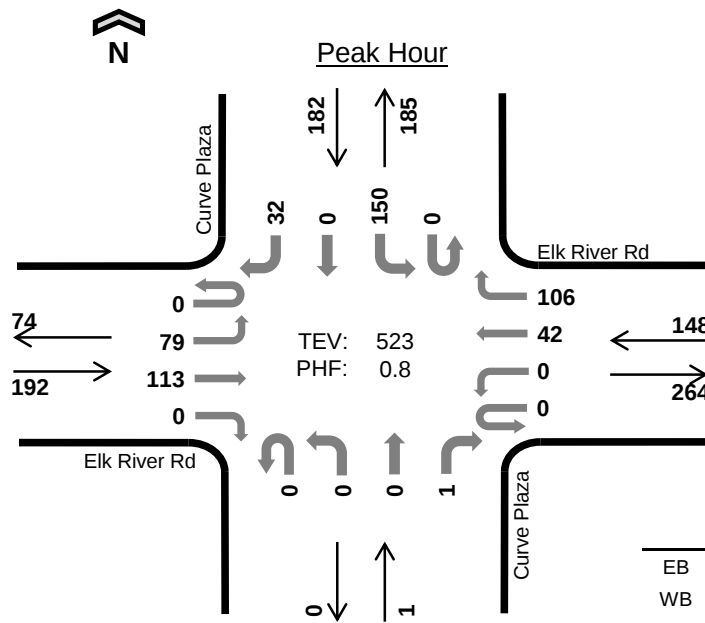
Two-Hour Count Summaries

Interval Start	Elk River Rd Eastbound				Elk River Rd Westbound				Curve Plaza Northbound				Curve Plaza Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	5	11	0	0	0	9	9	0	0	0	0	0	12	0	0	46	0
7:15 AM	0	8	11	0	0	0	13	17	0	0	0	0	0	17	0	3	69	0
7:30 AM	0	13	17	0	0	0	14	29	0	0	0	0	0	25	0	3	101	0
7:45 AM	0	15	16	0	0	0	17	26	0	0	0	0	0	32	0	6	112	328
8:00 AM	0	10	17	0	0	0	19	30	0	0	0	0	0	26	0	8	110	392
8:15 AM	0	11	17	0	0	0	8	29	0	0	0	0	0	36	0	5	106	429
8:30 AM	0	10	24	0	0	0	13	17	0	0	0	0	0	29	0	6	99	427
8:45 AM	0	12	16	0	0	0	13	20	0	0	0	0	0	27	0	6	94	409
Count Total	0	84	129	0	0	0	106	177	0	0	0	0	0	204	0	37	737	0
Peak Hour	0	49	67	0	0	0	58	114	0	0	0	0	0	119	0	22	429	0

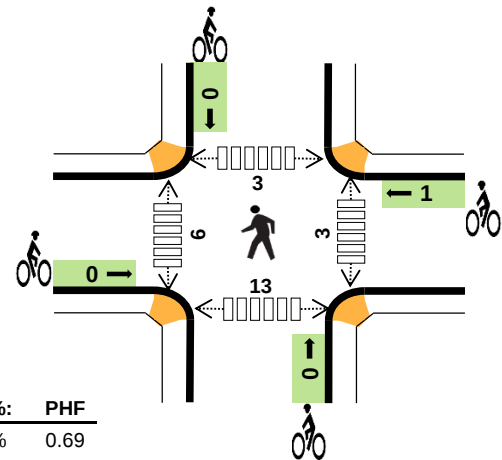
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	4	3	0	1	8	0	0	0	0	0	0	0	0	0	0
7:15 AM	5	1	0	0	6	0	0	0	0	0	0	2	0	0	2
7:30 AM	6	2	0	0	8	0	0	0	0	0	0	1	0	1	2
7:45 AM	5	0	0	0	5	0	0	0	0	0	1	0	0	0	1
8:00 AM	2	0	0	1	3	0	0	0	0	0	1	1	0	1	3
8:15 AM	5	0	0	2	7	0	0	0	0	0	0	0	0	2	2
8:30 AM	3	1	0	2	6	0	0	0	0	0	0	0	0	0	0
8:45 AM	4	0	0	0	4	0	0	0	0	0	0	0	0	0	0
Count Total	34	7	0	6	47	0	0	0	0	0	2	4	0	4	10
Peak Hour	18	2	0	3	23	0	0	0	0	0	2	2	0	4	8

Curve Plaza Elk River Rd



Date: Tue, Mar 03, 2020
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:15 PM to 5:15 PM



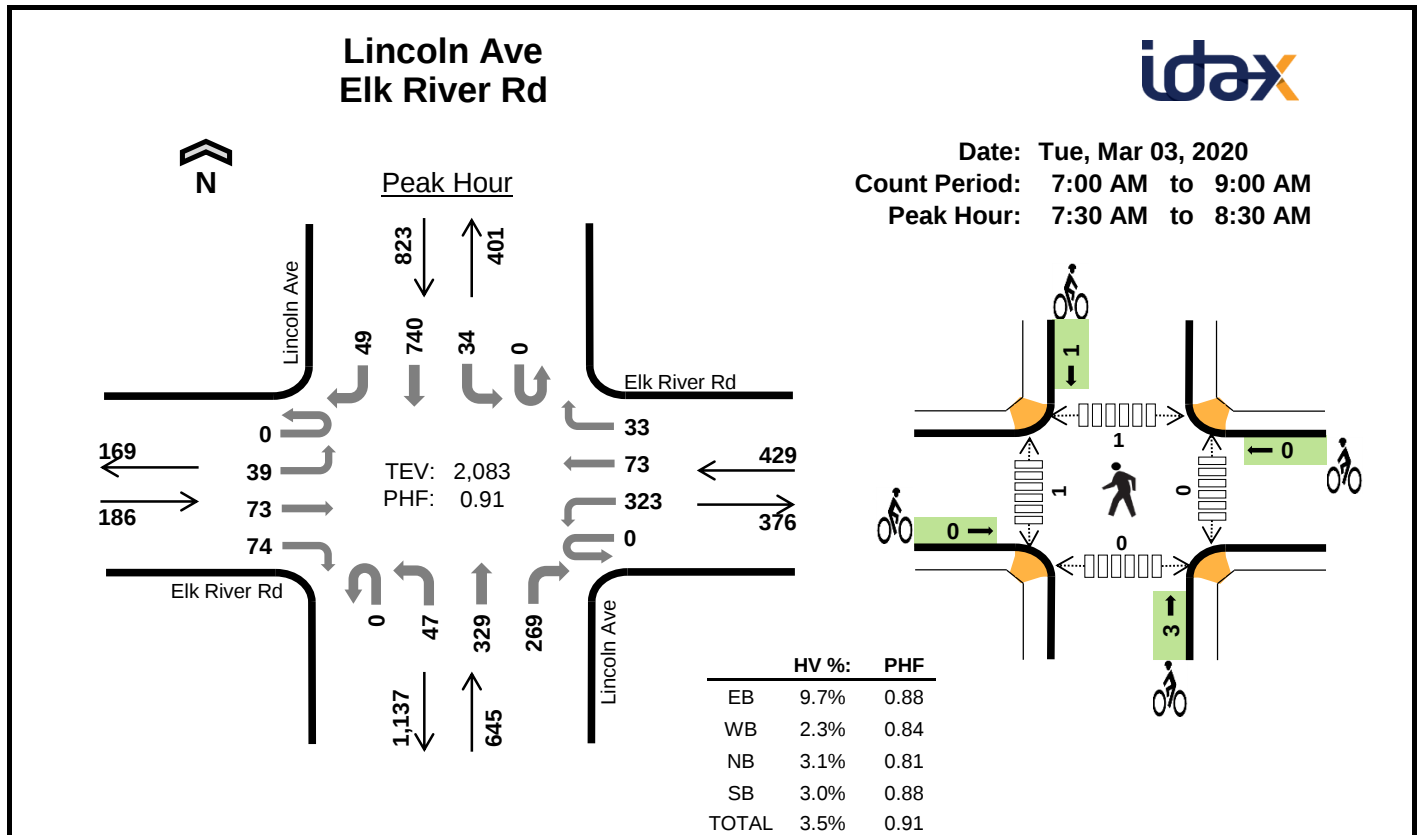
	HV %:	PHF
EB	4.2%	0.69
WB	3.4%	0.86
NB	0.0%	0.25
SB	1.6%	0.91
TOTAL	3.1%	0.80

Two-Hour Count Summaries

Interval Start	Elk River Rd Eastbound				Elk River Rd Westbound				Curve Plaza Northbound				Curve Plaza Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	23	29	1	0	1	35	6	0	0	1	0	0	30	1	7	134	0
4:15 PM	0	20	24	0	0	0	15	25	0	0	0	1	0	38	0	12	135	0
4:30 PM	0	20	25	0	0	0	8	19	0	0	0	0	0	31	0	7	110	0
4:45 PM	0	14	19	0	0	0	9	29	0	0	0	0	0	37	0	7	115	494
5:00 PM	0	25	45	0	0	0	10	33	0	0	0	0	0	44	0	6	163	523
5:15 PM	0	14	23	0	0	0	8	29	0	0	0	0	0	43	0	9	126	514
5:30 PM	0	13	13	0	0	0	4	25	0	0	0	0	0	31	0	5	91	495
5:45 PM	0	8	12	0	0	0	2	27	0	0	0	0	0	41	0	7	97	477
Count Total	0	137	190	1	0	1	91	193	0	0	1	1	0	295	1	60	971	0
Peak Hour	0	79	113	0	0	0	42	106	0	0	0	1	0	150	0	32	523	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	3	0	0	0	3	0	0	0	0	0	1	6	0	7	14
4:15 PM	4	3	0	2	9	0	0	0	0	0	0	0	3	0	3
4:30 PM	1	1	0	0	2	0	0	0	0	0	2	1	0	4	7
4:45 PM	2	0	0	0	2	0	0	0	0	0	1	3	0	3	7
5:00 PM	1	1	0	1	3	0	1	0	0	1	0	2	0	6	8
5:15 PM	3	0	0	0	3	0	0	0	0	0	0	3	0	1	4
5:30 PM	1	0	0	1	2	0	0	0	0	0	0	7	0	5	12
5:45 PM	2	0	0	0	2	0	0	0	0	0	0	2	0	0	2
Count Total	17	5	0	4	26	0	1	0	0	1	4	24	3	26	57
Peak Hour	8	5	0	3	16	0	1	0	0	1	3	6	3	13	25



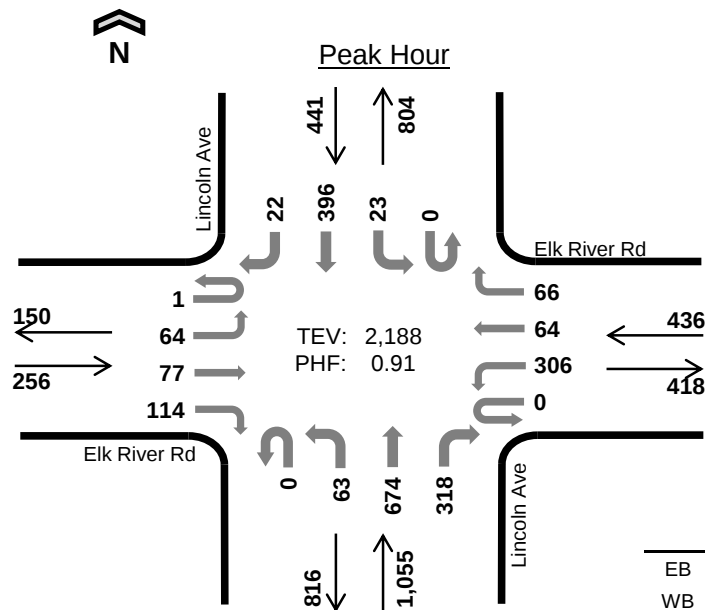
Two-Hour Count Summaries

Interval Start	Elk River Rd Eastbound				Elk River Rd Westbound				Lincoln Ave Northbound				Lincoln Ave Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	8	8	3	0	42	8	5	0	2	35	19	0	6	135	8	279	0
7:15 AM	0	4	11	15	0	41	18	3	0	4	69	36	0	4	143	8	356	0
7:30 AM	0	7	20	13	0	91	19	6	0	5	54	44	0	9	209	16	493	0
7:45 AM	0	10	15	22	0	98	20	9	0	12	93	76	0	10	180	11	556	1,684
8:00 AM	0	11	17	18	0	75	25	7	0	14	104	81	0	10	202	10	574	1,979
8:15 AM	0	11	21	21	0	59	9	11	0	16	78	68	0	5	149	12	460	2,083
8:30 AM	0	10	19	21	0	64	20	6	0	4	66	54	0	4	108	6	382	1,972
8:45 AM	0	4	21	21	0	65	19	4	0	7	64	54	0	5	149	7	420	1,836
Count Total	0	65	132	134	0	535	138	51	0	64	563	432	0	53	1,275	78	3,520	0
Peak Hour	0	39	73	74	0	323	73	33	0	47	329	269	0	34	740	49	2,083	0

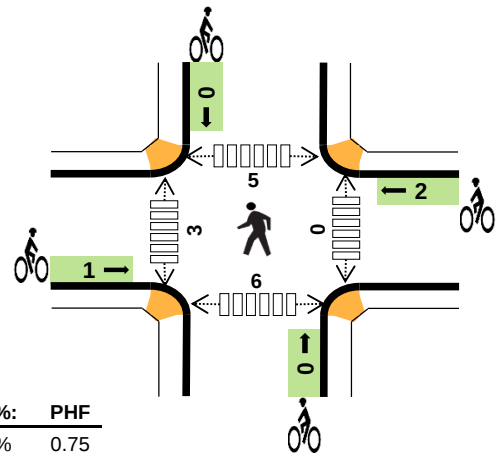
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	4	3	3	2	12	0	0	0	0	0	0	0	0	1	1
7:15 AM	5	2	6	3	16	0	0	0	0	0	1	0	0	1	2
7:30 AM	6	3	3	12	24	0	0	0	1	1	0	0	0	0	0
7:45 AM	4	4	1	6	15	0	0	2	0	2	0	0	0	0	0
8:00 AM	1	0	8	7	16	0	0	1	0	1	0	0	0	0	0
8:15 AM	7	3	8	0	18	0	0	0	0	0	0	1	1	0	2
8:30 AM	5	4	5	6	20	0	0	0	0	0	0	0	0	1	1
8:45 AM	4	2	3	8	17	0	0	0	0	0	0	0	0	0	0
Count Total	36	21	37	44	138	0	0	3	1	4	1	1	1	3	6
Peak Hour	18	10	20	25	73	0	0	3	1	4	0	1	1	0	2

Lincoln Ave Elk River Rd



Date: Tue, Mar 03, 2020
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:45 PM to 5:45 PM



	HV %:	PHF
EB	3.5%	0.75
WB	1.8%	0.83
NB	3.7%	0.94
SB	2.9%	0.95
TOTAL	3.2%	0.91

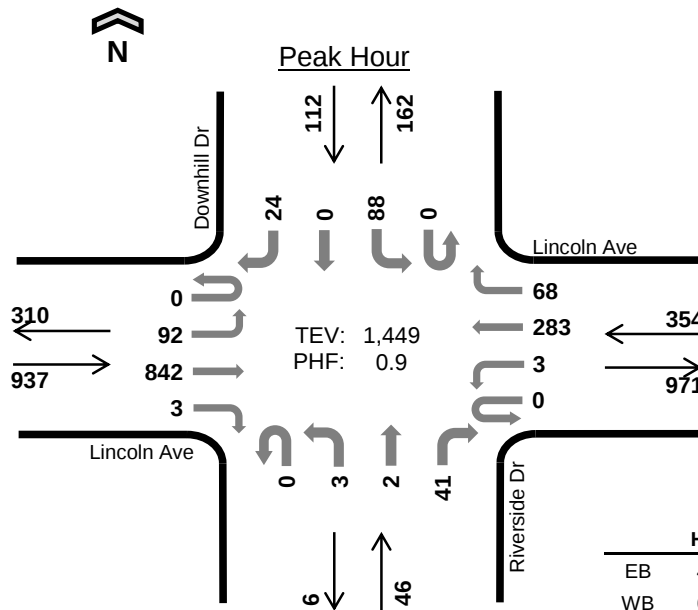
Two-Hour Count Summaries

Interval Start	Elk River Rd Eastbound				Elk River Rd Westbound				Lincoln Ave Northbound				Lincoln Ave Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	1	14	24	21	0	80	19	20	0	16	148	73	0	3	110	6	535	0
4:15 PM	0	12	20	28	0	67	20	13	0	14	149	75	0	7	115	6	526	0
4:30 PM	0	23	20	17	0	59	14	11	0	12	132	56	0	6	85	1	436	0
4:45 PM	1	12	16	28	0	75	15	16	0	17	152	79	0	3	96	7	517	2,014
5:00 PM	0	24	26	35	0	93	22	17	0	17	165	90	0	9	97	6	601	2,080
5:15 PM	0	14	23	32	0	80	16	23	0	13	184	83	0	9	102	5	584	2,138
5:30 PM	0	14	12	19	0	58	11	10	0	16	173	66	0	2	101	4	486	2,188
5:45 PM	0	17	8	27	0	54	11	8	0	11	154	49	0	4	109	6	458	2,129
Count Total	2	130	149	207	0	566	128	118	0	116	1,257	571	0	43	815	41	4,143	0
Peak Hour	1	64	77	114	0	306	64	66	0	63	674	318	0	23	396	22	2,188	0

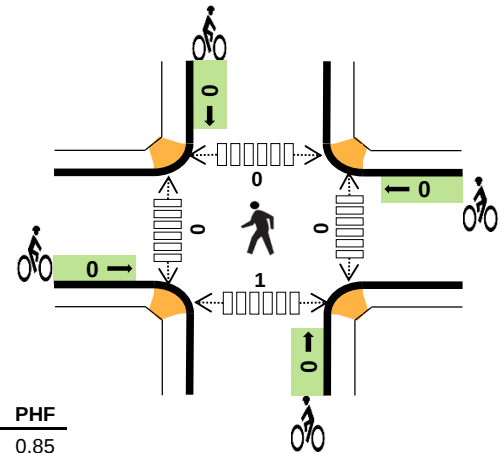
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	3	1	8	4	16	0	0	0	0	0	0	2	0	0	2
4:15 PM	5	3	6	9	23	0	0	0	0	0	1	0	2	0	3
4:30 PM	1	0	5	2	8	0	0	0	0	0	0	0	1	2	3
4:45 PM	3	1	12	4	20	0	0	0	0	0	0	0	1	1	2
5:00 PM	2	3	9	1	15	0	1	0	0	1	0	0	1	1	2
5:15 PM	1	4	12	4	21	1	1	0	0	2	0	1	3	1	5
5:30 PM	3	0	6	4	13	0	0	0	0	0	0	2	0	3	5
5:45 PM	1	2	4	2	9	0	1	0	0	1	0	0	0	0	0
Count Total	19	14	62	30	125	1	3	0	0	4	1	5	8	8	22
Peak Hour	9	8	39	13	69	1	2	0	0	3	0	3	5	6	14

Riverside Dr Lincoln Ave



Date: Tue, Mar 03, 2020
Count Period: 7:00 AM to 9:00 AM
Peak Hour: 7:15 AM to 8:15 AM



	HV %:	PHF
EB	4.1%	0.85
WB	6.8%	0.74
NB	2.2%	0.82
SB	0.9%	0.74
TOTAL	4.4%	0.90

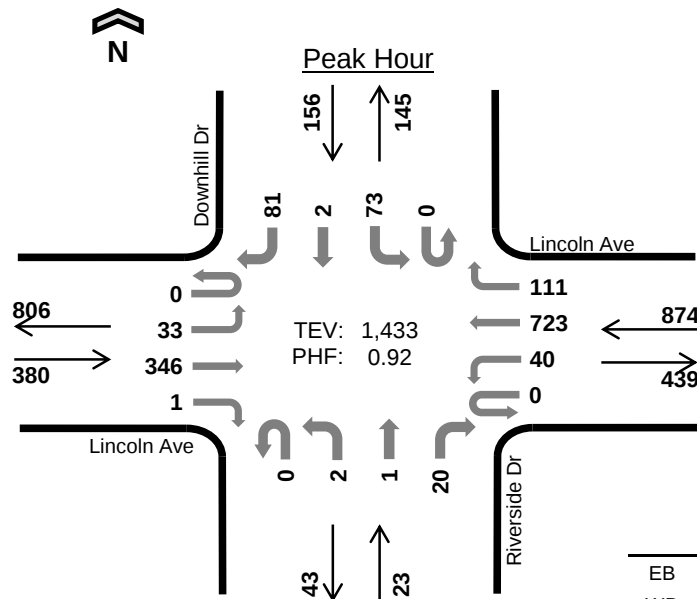
Two-Hour Count Summaries

Interval Start	Lincoln Ave Eastbound				Lincoln Ave Westbound				Riverside Dr Northbound				Downhill Dr Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	12	127	1	0	2	37	2	0	0	0	9	0	20	1	6	217	0
7:15 AM	0	20	159	0	0	0	52	19	0	1	0	7	0	16	0	4	278	0
7:30 AM	0	26	249	1	0	1	54	7	0	0	0	11	0	22	0	5	376	0
7:45 AM	0	30	218	1	0	1	80	21	0	0	1	13	0	24	0	3	392	1,263
8:00 AM	0	16	216	1	0	1	97	21	0	2	1	10	0	26	0	12	403	1,449
8:15 AM	0	15	135	1	0	0	66	11	0	0	0	7	0	13	0	5	253	1,424
8:30 AM	0	11	118	0	0	6	51	18	0	0	1	5	0	15	0	4	229	1,277
8:45 AM	0	19	153	1	0	2	57	10	0	0	1	6	0	21	0	4	274	1,159
Count Total	0	149	1,375	6	0	13	494	109	0	3	4	68	0	157	1	43	2,422	0
Peak Hour	0	92	842	3	0	3	283	68	0	3	2	41	0	88	0	24	1,449	0

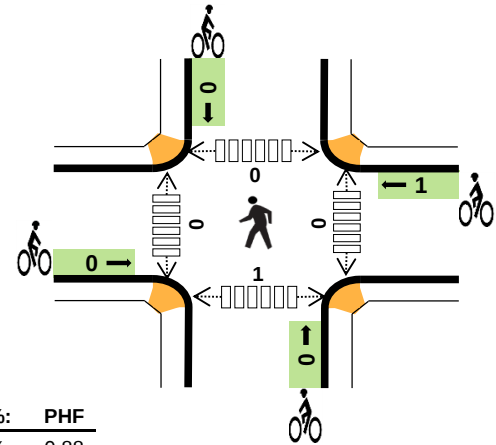
Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	3	5	0	0	8	0	0	0	0	0	0	0	0	0	0
7:15 AM	6	7	0	0	13	0	0	0	0	0	0	0	0	0	0
7:30 AM	15	8	1	1	25	0	0	0	0	0	0	0	0	0	0
7:45 AM	7	4	0	0	11	0	0	0	0	0	0	0	0	0	0
8:00 AM	10	5	0	0	15	0	0	0	0	0	0	0	0	1	1
8:15 AM	3	6	0	0	9	0	0	0	0	0	0	0	0	0	0
8:30 AM	9	6	0	0	15	0	0	0	0	0	0	0	0	0	0
8:45 AM	12	4	0	2	18	0	0	0	0	0	0	0	0	0	0
Count Total	65	45	1	3	114	0	0	0	0	0	0	0	0	1	1
Peak Hour	38	24	1	1	64	0	0	0	0	0	0	0	0	1	1

Riverside Dr Lincoln Ave



Date: Tue, Mar 03, 2020
Count Period: 4:00 PM to 6:00 PM
Peak Hour: 4:45 PM to 5:45 PM



	HV %:	PHF
EB	4.5%	0.88
WB	3.7%	0.91
NB	0.0%	0.72
SB	1.9%	0.91
TOTAL	3.6%	0.92

Two-Hour Count Summaries

Interval Start	Lincoln Ave Eastbound				Lincoln Ave Westbound				Riverside Dr Northbound				Downhill Dr Southbound				15-min Total	Rolling One Hour
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
4:00 PM	0	9	98	2	0	6	143	31	0	0	1	6	0	18	0	14	328	0
4:15 PM	0	9	102	0	0	7	136	30	0	0	0	3	0	23	1	22	333	0
4:30 PM	0	11	67	0	0	10	166	21	0	1	0	8	0	17	0	11	312	0
4:45 PM	0	7	89	1	0	8	149	30	0	0	0	3	0	24	1	15	327	1,300
5:00 PM	0	11	83	0	0	11	204	24	0	0	1	5	0	10	0	33	382	1,354
5:15 PM	0	9	99	0	0	16	193	25	0	1	0	7	0	19	0	21	390	1,411
5:30 PM	0	6	75	0	0	5	177	32	0	1	0	5	0	20	1	12	334	1,433
5:45 PM	0	4	90	0	0	11	136	32	0	0	0	7	0	13	0	13	306	1,412
Count Total	0	66	703	3	0	74	1,304	225	0	3	2	44	0	144	3	141	2,712	0
Peak Hour	0	33	346	1	0	40	723	111	0	2	1	20	0	73	2	81	1,433	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	6	4	0	2	12	0	0	0	0	0	0	0	0	0	0
4:15 PM	13	2	0	1	16	0	0	0	0	0	0	0	0	0	0
4:30 PM	2	4	0	0	6	0	0	0	0	0	0	0	1	1	2
4:45 PM	4	10	0	1	15	0	0	0	0	0	0	0	0	0	0
5:00 PM	3	7	0	1	11	0	0	0	0	0	0	0	0	0	0
5:15 PM	5	9	0	0	14	0	1	0	0	1	0	0	0	1	1
5:30 PM	5	6	0	1	12	0	0	0	0	0	0	0	0	0	0
5:45 PM	1	3	0	0	4	0	0	0	0	0	0	1	0	1	2
Count Total	39	45	0	6	90	0	1	0	0	1	0	1	1	3	5
Peak Hour	17	32	0	3	52	0	1	0	0	1	0	0	0	1	1

Table 1: Steamboat Springs Monthly ADT Data Conversion Table

	Jan	Feb	Mar	Apr	May	Jun	Jul	Aug	Sep	Oct	Nov	Dec
Jan	1	1.02	1.06	0.94	1.1	1.47	1.69	1.56	1.35	1.19	0.99	1.04
Feb	0.98	1	1.04	0.92	1.08	1.44	1.66	1.53	1.32	1.16	0.97	1.02
Mar	0.94	0.96	1	0.89	1.04	1.38	1.59	1.47	1.27	1.12	0.93	0.98
Apr	1.06	1.08	1.12	1	1.17	1.56	1.79	1.66	1.43	1.26	1.05	1.11
May	0.91	0.92	0.96	0.85	1	1.33	1.53	1.42	1.22	1.07	0.89	0.94
Jun	0.68	0.69	0.72	0.64	0.75	1	1.15	1.06	0.92	0.81	0.67	0.71
Jul	0.59	0.6	0.63	0.56	0.65	0.87	1	0.92	0.8	0.7	0.58	0.62
Aug	0.64	0.65	0.68	0.6	0.71	0.94	1.08	1	0.86	0.76	0.63	0.67
Sep	0.74	0.76	0.79	0.7	0.82	1.09	1.26	1.16	1	0.88	0.73	0.77
Oct	0.84	0.86	0.89	0.8	0.93	1.24	1.43	1.32	1.14	1	0.83	0.88
Nov	1.01	1.03	1.07	0.96	1.12	1.49	1.71	1.58	1.36	1.2	1	1.06
Dec	0.96	0.98	1.02	0.9	1.06	1.41	1.62	1.5	1.29	1.14	0.95	1

Appendix C

Synchro Printouts

Existing (Year 2020)

Short Term Background (Year 2022)

Short Term Total (Year 2022)

Long Term Background (Year 2040)

Long Term Total (Year 2040)

Existing AM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	34	740	49	47	329	269	39	73	74	323	73	33
Future Volume (veh/h)	34	740	49	47	329	269	39	73	74	323	73	33
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1752	1752	1752	1870	1870	1870
Adj Flow Rate, veh/h	39	841	0	58	406	0	44	83	0	385	87	0
Peak Hour Factor	0.88	0.88	0.88	0.81	0.81	0.81	0.88	0.88	0.88	0.84	0.84	0.84
Percent Heavy Veh, %	3	3	3	3	3	3	10	10	10	2	2	2
Cap, veh/h	63	1272		79	686		64	418		470	628	
Arrive On Green	0.04	0.36	0.00	0.04	0.37	0.00	0.04	0.24	0.00	0.14	0.34	0.00
Sat Flow, veh/h	1767	3526	1572	1767	1856	1572	1668	1752	1485	3456	1870	1585
Grp Volume(v), veh/h	39	841	0	58	406	0	44	83	0	385	87	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1767	1856	1572	1668	1752	1485	1728	1870	1585
Q Serve(g_s), s	1.8	16.4	0.0	2.7	14.4	0.0	2.1	3.1	0.0	8.9	2.6	0.0
Cycle Q Clear(g_c), s	1.8	16.4	0.0	2.7	14.4	0.0	2.1	3.1	0.0	8.9	2.6	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	63	1272		79	686		64	418		470	628	
V/C Ratio(X)	0.61	0.66		0.73	0.59		0.68	0.20		0.82	0.14	
Avail Cap(c_a), veh/h	119	1272		119	686		102	418		528	628	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	38.9	21.9	0.0	38.6	20.8	0.0	38.8	24.9	0.0	34.4	18.9	0.0
Incr Delay (d2), s/veh	9.3	2.7	0.0	12.3	3.7	0.0	12.0	1.1	0.0	9.0	0.5	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.9	6.7	0.0	1.4	6.4	0.0	1.1	1.3	0.0	4.1	1.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	48.1	24.7	0.0	50.8	24.5	0.0	50.8	26.0	0.0	43.4	19.4	0.0
LnGrp LOS	D	C		D	C		D	C		D	B	
Approach Vol, veh/h		880	A		464	A		127	A		472	A
Approach Delay, s/veh		25.7			27.8			34.6			39.0	
Approach LOS		C			C			C			D	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	8.2	34.0	15.6	24.0	7.4	34.7	7.7	32.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	5.5	29.5	12.5	19.5	5.5	29.5	5.0	27.0				
Max Q Clear Time (g_c+I1), s	4.7	18.4	10.9	5.1	3.8	16.4	4.1	4.6				
Green Ext Time (p_c), s	0.0	4.1	0.3	0.2	0.0	1.9	0.0	0.3				
Intersection Summary												
HCM 6th Ctrl Delay			30.0									
HCM 6th LOS			C									
Notes												
User approved pedestrian interval to be less than phase max green.												
Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Existing AM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 2.9

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	13	88	40	102	59	21
Future Vol, veh/h	13	88	40	102	59	21
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	54	54	91	91	77	77
Heavy Vehicles, %	0	0	13	13	3	3
Mvmt Flow	24	163	44	112	77	27

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	156	0	0 311 100
Stage 1	-	-	- 100 -
Stage 2	-	-	- 211 -
Critical Hdwy	4.1	-	- 6.43 6.23
Critical Hdwy Stg 1	-	-	- 5.43 -
Critical Hdwy Stg 2	-	-	- 5.43 -
Follow-up Hdwy	2.2	-	- 3.527 3.327
Pot Cap-1 Maneuver	1436	-	- 679 953
Stage 1	-	-	- 921 -
Stage 2	-	-	- 822 -
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	1436	-	- 667 953
Mov Cap-2 Maneuver	-	-	- 667 -
Stage 1	-	-	- 904 -
Stage 2	-	-	- 822 -

Approach	EB	WB	SB
HCM Control Delay, s	1	0	10.8
HCM LOS			B

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1436	-	-	-	724
HCM Lane V/C Ratio	0.017	-	-	-	0.144
HCM Control Delay (s)	7.5	0	-	-	10.8
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	0.5

Existing AM
9: US-40 & Curve Ct/Sunlight Dr

Intersection

Int Delay, s/veh 1.4

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕			↕	↕
Traffic Vol, veh/h	0	0	23	0	0	6	104	656	1	19	1121	3
Future Vol, veh/h	0	0	23	0	0	6	104	656	1	19	1121	3
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	64	64	64	75	75	75	80	80	80	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	0	0	36	0	0	8	130	820	1	21	1232	3

Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2359	2355	616	1739	2358	821	1235	0	0	821	0	0
Stage 1	1274	1274	-	1081	1081	-	-	-	-	-	-	-
Stage 2	1085	1081	-	658	1277	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	22	36	438	63	36	378	553	-	-	801	-	-
Stage 1	180	240	-	266	296	-	-	-	-	-	-	-
Stage 2	265	296	-	424	239	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	17	25	438	44	25	378	553	-	-	801	-	-
Mov Cap-2 Maneuver	17	25	-	44	25	-	-	-	-	-	-	-
Stage 1	138	220	-	203	226	-	-	-	-	-	-	-
Stage 2	198	226	-	357	219	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	14		14.7		1.8		0.6	
HCM LOS	B		B					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR
Capacity (veh/h)	553	-	-	438	378	801	-
HCM Lane V/C Ratio	0.235	-	-	0.082	0.021	0.026	-
HCM Control Delay (s)	13.5	-	-	14	14.7	9.6	0.4
HCM Lane LOS	B	-	-	B	B	A	A
HCM 95th %tile Q(veh)	0.9	-	-	0.3	0.1	0.1	-

Existing AM
16: Elk River Rd & Curve Plaza

Intersection

Int Delay, s/veh 4.7

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↑	↗
Traffic Vol, veh/h	119	0	22	0	0	0	49	67	0	0	58	114
Future Vol, veh/h	119	0	22	0	0	0	49	67	0	0	58	114
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	92	92	92	94	94	94	88	88	88
Heavy Vehicles, %	2	2	2	0	0	0	16	16	16	1	1	1
Mvmt Flow	138	0	26	0	0	0	52	71	0	0	66	130

Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	241	241	66	319	371	71	196	0	0	-	-	0
Stage 1	66	66	-	175	175	-	-	-	-	-	-	-
Stage 2	175	175	-	144	196	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.1	6.5	6.2	4.26	-	-	-	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.5	4	3.3	2.344	-	-	-	-	-
Pot Cap-1 Maneuver	713	660	998	638	562	997	1297	-	-	0	-	-
Stage 1	945	840	-	832	758	-	-	-	-	0	-	-
Stage 2	827	754	-	864	742	-	-	-	-	0	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	690	632	998	602	538	997	1297	-	-	-	-	-
Mov Cap-2 Maneuver	690	632	-	602	538	-	-	-	-	-	-	-
Stage 1	905	840	-	797	726	-	-	-	-	-	-	-
Stage 2	792	722	-	842	742	-	-	-	-	-	-	-

Approach	EB		WB		NB		SB	
HCM Control Delay, s	11.4		0		3.3		0	
HCM LOS	B		A					

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR
Capacity (veh/h)	1297	-	-	725	-	-
HCM Lane V/C Ratio	0.04	-	-	0.226	-	-
HCM Control Delay (s)	7.9	0	-	11.4	0	-
HCM Lane LOS	A	A	-	B	A	-
HCM 95th %tile Q(veh)	0.1	-	-	0.9	-	-

Existing AM
20: Shield Dr & Curve Plaza

Intersection

Int Delay, s/veh 0.1

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	140	0	1	145
Future Vol, veh/h	0	0	140	0	1	145
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	95	95	62	62
Heavy Vehicles, %	2	2	14	14	1	1
Mvmt Flow	0	0	147	0	2	234

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	385	147	0
Stage 1	147	-	-
Stage 2	238	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	618	900	-
Stage 1	880	-	-
Stage 2	802	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	617	900	-
Mov Cap-2 Maneuver	617	-	-
Stage 1	880	-	-
Stage 2	800	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0.1
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	1441
HCM Lane V/C Ratio	-	-	-	0.001
HCM Control Delay (s)	-	-	0	7.5
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0

Existing PM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	23	396	22	63	674	318	65	77	114	306	64	66
Future Volume (veh/h)	23	396	22	63	674	318	65	77	114	306	64	66
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1841	1841	1841	1870	1870	1870
Adj Flow Rate, veh/h	24	417	0	67	717	0	87	103	0	369	77	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.75	0.75	0.75	0.83	0.83	0.83
Percent Heavy Veh, %	3	3	3	4	4	4	4	4	4	2	2	2
Cap, veh/h	44	1309		86	728		111	499		393	601	
Arrive On Green	0.02	0.37	0.00	0.05	0.40	0.00	0.06	0.27	0.00	0.11	0.32	0.00
Sat Flow, veh/h	1767	3526	1572	1753	1841	1560	1753	1841	1560	3456	1870	1585
Grp Volume(v), veh/h	24	417	0	67	717	0	87	103	0	369	77	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1841	1560	1753	1841	1560	1728	1870	1585
Q Serve(g_s), s	1.2	7.8	0.0	3.5	35.6	0.0	4.5	4.0	0.0	9.8	2.7	0.0
Cycle Q Clear(g_c), s	1.2	7.8	0.0	3.5	35.6	0.0	4.5	4.0	0.0	9.8	2.7	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	44	1309		86	728		111	499		393	601	
V/C Ratio(X)	0.55	0.32		0.78	0.98		0.79	0.21		0.94	0.13	
Avail Cap(c_a), veh/h	96	1309		177	728		142	499		393	601	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	44.5	20.7	0.0	43.4	27.6	0.0	42.6	26.0	0.0	40.6	22.2	0.0
Incr Delay (d2), s/veh	10.1	0.6	0.0	13.9	29.9	0.0	19.4	0.9	0.0	30.2	0.4	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	0.7	3.1	0.0	1.8	20.3	0.0	2.5	1.8	0.0	5.7	1.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	54.6	21.3	0.0	57.3	57.5	0.0	62.0	26.9	0.0	70.8	22.6	0.0
LnGrp LOS	D	C		E	E		E	C		E	C	
Approach Vol, veh/h		441	A		784	A		190	A		446	A
Approach Delay, s/veh		23.1			57.5			43.0			62.5	
Approach LOS		C			E			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	9.0	38.8	15.0	29.5	6.8	41.0	10.3	34.2				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	9.3	32.2	10.5	25.0	5.0	36.5	7.5	28.0				
Max Q Clear Time (g_c+I1), s	5.5	9.8	11.8	6.0	3.2	37.6	6.5	4.7				
Green Ext Time (p_c), s	0.0	2.5	0.0	0.4	0.0	0.0	0.0	0.3				

Intersection Summary

HCM 6th Ctrl Delay	49.1
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HCM 6th LOS	D
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Notes

Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Existing PM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 2.8

Movement EBL EBT WBT WBR SBL SBR

Lane Configurations 

Traffic Vol, veh/h 36 65 45 130 53 5

Future Vol, veh/h 36 65 45 130 53 5

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Free Free Free Free Stop Stop

RT Channelized - None - None - None

Storage Length - - - - 0 -

Veh in Median Storage, # - 0 0 - 0 -

Grade, % - 0 0 - 0 -

Peak Hour Factor 53 53 74 74 81 81

Heavy Vehicles, % 0 0 4 4 2 2

Mvmt Flow 68 123 61 176 65 6

Major/Minor Major1 Major2 Minor2

Conflicting Flow All 237 0 - 0 408 149

Stage 1 - - - - 149 -

Stage 2 - - - - 259 -

Critical Hdwy 4.1 - - - 6.42 6.22

Critical Hdwy Stg 1 - - - - 5.42 -

Critical Hdwy Stg 2 - - - - 5.42 -

Follow-up Hdwy 2.2 - - - 3.518 3.318

Pot Cap-1 Maneuver 1342 - - - 599 898

Stage 1 - - - - 879 -

Stage 2 - - - - 784 -

Platoon blocked, % - - - -

Mov Cap-1 Maneuver 1342 - - - 567 898

Mov Cap-2 Maneuver - - - - 567 -

Stage 1 - - - - 832 -

Stage 2 - - - - 784 -

Approach EB WB SB

HCM Control Delay, s 2.8 0 12

HCM LOS B

Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1

Capacity (veh/h) 1342 - - - 586

HCM Lane V/C Ratio 0.051 - - - 0.122

HCM Control Delay (s) 7.8 0 - - 12

HCM Lane LOS A A - - B

HCM 95th %tile Q(veh) 0.2 - - - 0.4

Existing PM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	2.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕			↕	↕
Traffic Vol, veh/h	2	0	85	0	1	10	86	1063	0	4	807	4
Future Vol, veh/h	2	0	85	0	1	10	86	1063	0	4	807	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	69	69	69	94	94	94	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	3	0	142	0	1	14	91	1131	0	5	917	5
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	2248	2240	459	1782	2245	1131	922	0	0	1131	0	0
Stage 1	927	927	-	1313	1313	-	-	-	-	-	-	-
Stage 2	1321	1313	-	469	932	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	27	43	554	58	42	250	728	-	-	611	-	-
Stage 1	293	350	-	197	230	-	-	-	-	-	-	-
Stage 2	195	230	-	549	348	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	22	37	554	39	36	250	728	-	-	611	-	-
Mov Cap-2 Maneuver	22	37	-	39	36	-	-	-	-	-	-	-
Stage 1	256	344	-	172	201	-	-	-	-	-	-	-
Stage 2	160	201	-	402	342	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	21.9		29.6		0.8		0.2					
HCM LOS	C		D									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	728	-	-	356	162	611	-	-				
HCM Lane V/C Ratio	0.126	-	-	0.407	0.098	0.007	-	-				
HCM Control Delay (s)	10.7	-	-	21.9	29.6	10.9	0.1	-				
HCM Lane LOS	B	-	-	C	D	B	A	-				
HCM 95th %tile Q(veh)	0.4	-	-	1.9	0.3	0	-	-				

Existing PM
16: Elk River Rd & Curve Plaza

Intersection													
Int Delay, s/veh	6.2												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Vol, veh/h	155	0	27	0	0	0	66	100	0	0	31	116	
Future Vol, veh/h	155	0	27	0	0	0	66	100	0	0	31	116	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	-	-	-	-	-	-	0	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	88	88	88	92	92	92	59	59	59	85	85	85	
Heavy Vehicles, %	1	1	1	0	0	0	4	4	4	1	1	1	
Mvmt Flow	176	0	31	0	0	0	112	169	0	0	36	136	
Major/Minor	Minor2		Minor1		Major1		Major2						
Conflicting Flow All	429	429	36	513	565	169	172	0	0	-	-	0	
Stage 1	36	36	-	393	393	-	-	-	-	-	-	-	
Stage 2	393	393	-	120	172	-	-	-	-	-	-	-	
Critical Hdwy	7.11	6.51	6.21	7.1	6.5	6.2	4.14	-	-	-	-	-	
Critical Hdwy Stg 1	6.11	5.51	-	6.1	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.11	5.51	-	6.1	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.509	4.009	3.309	3.5	4	3.3	2.236	-	-	-	-	-	
Pot Cap-1 Maneuver	538	520	1039	475	437	880	1393	-	-	0	-	-	
Stage 1	982	867	-	636	609	-	-	-	-	0	-	-	
Stage 2	634	608	-	889	760	-	-	-	-	0	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	501	474	1039	429	398	880	1393	-	-	-	-	-	
Mov Cap-2 Maneuver	501	474	-	429	398	-	-	-	-	-	-	-	
Stage 1	895	867	-	579	555	-	-	-	-	-	-	-	
Stage 2	578	554	-	863	760	-	-	-	-	-	-	-	
Approach	EB		WB		NB		SB						
HCM Control Delay, s	15.7		0		3.1		0						
HCM LOS	C		A										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR							
Capacity (veh/h)	1393	-	-	543	-	-	-						
HCM Lane V/C Ratio	0.08	-	-	0.381	-	-	-						
HCM Control Delay (s)	7.8	0	-	15.7	0	-	-						
HCM Lane LOS	A	A	-	C	A	-	-						
HCM 95th %tile Q(veh)	0.3	-	-	1.8	-	-	-						

Existing PM
20: Shield Dr & Curve Plaza

Intersection

Int Delay, s/veh 0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	175	0	0	119
Future Vol, veh/h	0	0	175	0	0	119
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	79	79	74	74
Heavy Vehicles, %	2	2	4	4	1	1
Mvmt Flow	0	0	222	0	0	161

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	383	222	0
Stage 1	222	-	-
Stage 2	161	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	620	818	-
Stage 1	815	-	-
Stage 2	868	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	620	818	-
Mov Cap-2 Maneuver	620	-	-
Stage 1	815	-	-
Stage 2	868	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	1353
HCM Lane V/C Ratio	-	-	-	-
HCM Control Delay (s)	-	-	0	0
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0

6: Elk River Rd & US-40

Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Short Term Background Conditions AM

7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 3.4

Movement

	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	19	128	58	148	86	30
Future Vol, veh/h	19	128	58	148	86	30
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	54	54	91	91	77	77
Heavy Vehicles, %	0	0	13	13	3	3
Mvmt Flow	35	237	64	163	112	39

Major/Minor

	Major1	Major2	Minor2		
Conflicting Flow All	227	0	0	453	146
Stage 1	-	-	-	146	-
Stage 2	-	-	-	307	-
Critical Hdwy	4.1	-	-	6.43	6.23
Critical Hdwy Stg 1	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	5.43	-
Follow-up Hdwy	2.2	-	-	3.527	3.327
Pot Cap-1 Maneuver	1353	-	-	563	898
Stage 1	-	-	-	879	-
Stage 2	-	-	-	744	-
Platoon blocked, %	-	-	-	-	-
Mov Cap-1 Maneuver	1353	-	-	546	898
Mov Cap-2 Maneuver	-	-	-	546	-
Stage 1	-	-	-	853	-
Stage 2	-	-	-	744	-

Approach

	EB	WB	SB
HCM Control Delay, s	1	0	12.9
HCM LOS			B

Minor Lane/Major Mvmt

	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1353	-	-	-	608
HCM Lane V/C Ratio	0.026	-	-	-	0.248
HCM Control Delay (s)	7.7	0	-	-	12.9
HCM Lane LOS	A	A	-	-	B
HCM 95th %tile Q(veh)	0.1	-	-	-	1

Short Term Background Conditions AM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	4.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕			↕	↕
Traffic Vol, veh/h	0	0	33	0	0	9	151	952	1	28	1627	4
Future Vol, veh/h	0	0	33	0	0	9	151	952	1	28	1627	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	64	64	64	75	75	75	80	80	80	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	0	0	52	0	0	12	189	1190	1	31	1788	4
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	3425	3419	894	2525	3423	1191	1792	0	0	1191	0	0
Stage 1	1850	1850	-	1569	1569	-	-	-	-	-	-	-
Stage 2	1575	1569	-	956	1854	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	3	7	288	17	7	231	336	-	-	579	-	-
Stage 1	79	126	-	140	173	-	-	-	-	-	-	-
Stage 2	139	173	-	281	125	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	-	0	288	-	0	231	336	-	-	579	-	-
Mov Cap-2 Maneuver	-	0	-	-	0	-	-	-	-	-	-	-
Stage 1	35	0	-	61	76	-	-	-	-	-	-	-
Stage 2	58	76	-	-	0	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s					3.9		5.7					
HCM LOS	-		-									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	336	-	-	-	-	579	-	-				
HCM Lane V/C Ratio	0.562	-	-	-	-	0.053	-	-				
HCM Control Delay (s)	28.6	-	-	-	-	11.6	5.6	-				
HCM Lane LOS	D	-	-	-	-	B	A	-				
HCM 95th %tile Q(veh)	3.3	-	-	-	-	0.2	-	-				

Short Term Background Conditions AM

16: Elk River Rd & Curve Plaza

Intersection												
Int Delay, s/veh	5.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↑	↗
Traffic Vol, veh/h	173	0	32	0	0	0	71	97	0	0	84	165
Future Vol, veh/h	173	0	32	0	0	0	71	97	0	0	84	165
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	92	92	92	94	94	94	88	88	88
Heavy Vehicles, %	2	2	2	0	0	0	16	16	16	1	1	1
Mvmt Flow	201	0	37	0	0	0	76	103	0	0	95	188
Major/Minor	Minor2		Minor1		Major1			Major2				
Conflicting Flow All	350	350	95	463	538	103	283	0	0	-	-	0
Stage 1	95	95	-	255	255	-	-	-	-	-	-	-
Stage 2	255	255	-	208	283	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.1	6.5	6.2	4.26	-	-	-	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.5	4	3.3	2.344	-	-	-	-	-
Pot Cap-1 Maneuver	605	574	962	513	453	957	1203	-	-	0	-	-
Stage 1	912	816	-	754	700	-	-	-	-	0	-	-
Stage 2	749	696	-	799	681	-	-	-	-	0	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	574	536	962	468	423	957	1203	-	-	-	-	-
Mov Cap-2 Maneuver	574	536	-	468	423	-	-	-	-	-	-	-
Stage 1	851	816	-	703	653	-	-	-	-	-	-	-
Stage 2	699	649	-	768	681	-	-	-	-	-	-	-
Approach	EB		WB		NB			SB				
HCM Control Delay, s	14.6		0		3.5			0				
HCM LOS	B		A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBT	SBR					
Capacity (veh/h)	1203	-	-	613	-	-	-					
HCM Lane V/C Ratio	0.063	-	-	0.389	-	-	-					
HCM Control Delay (s)	8.2	0	-	14.6	0	-	-					
HCM Lane LOS	A	A	-	B	A	-	-					
HCM 95th %tile Q(veh)	0.2	-	-	1.8	-	-	-					

Short Term Background Conditions AM

20: Shield Dr & Curve Plaza

Intersection

Int Delay, s/veh 0

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	203	0	1	210
Future Vol, veh/h	0	0	203	0	1	210
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	95	95	62	62
Heavy Vehicles, %	2	2	14	14	1	1
Mvmt Flow	0	0	214	0	2	339

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	557	214	0
Stage 1	214	-	-
Stage 2	343	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	491	826	-
Stage 1	822	-	-
Stage 2	719	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	490	826	-
Mov Cap-2 Maneuver	490	-	-
Stage 1	822	-	-
Stage 2	718	-	-

Approach	WB	NB	SB
HCM Control Delay, s	0	0	0
HCM LOS	A		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	-	1362
HCM Lane V/C Ratio	-	-	-	0.001
HCM Control Delay (s)	-	-	0	7.6
HCM Lane LOS	-	-	A	A
HCM 95th %tile Q(veh)	-	-	-	0

Short Term Background Traffic PM 6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		 								 		
Traffic Volume (veh/h)	33	575	32	91	978	462	93	112	165	444	93	96
Future Volume (veh/h)	33	575	32	91	978	462	93	112	165	444	93	96
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1841	1841	1841	1870	1870	1870
Adj Flow Rate, veh/h	35	605	0	97	1040	0	124	149	0	535	112	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.75	0.75	0.75	0.83	0.83	0.83
Percent Heavy Veh, %	3	3	3	4	4	4	4	4	4	2	2	2
Cap, veh/h	46	1730		119	979		146	320		493	436	
Arrive On Green	0.03	0.49	0.00	0.07	0.53	0.00	0.08	0.17	0.00	0.14	0.23	0.00
Sat Flow, veh/h	1767	3526	1572	1753	1841	1560	1753	1841	1560	3456	1870	1585
Grp Volume(v), veh/h	35	605	0	97	1040	0	124	149	0	535	112	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1841	1560	1753	1841	1560	1728	1870	1585
Q Serve(g_s), s	2.8	15.2	0.0	7.9	76.5	0.0	10.0	10.5	0.0	20.5	7.0	0.0
Cycle Q Clear(g_c), s	2.8	15.2	0.0	7.9	76.5	0.0	10.0	10.5	0.0	20.5	7.0	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	46	1730		119	979		146	320		493	436	
V/C Ratio(X)	0.76	0.35		0.82	1.06		0.85	0.47		1.09	0.26	
Avail Cap(c_a), veh/h	61	1730		195	979		180	320		493	436	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	69.5	22.5	0.0	66.1	33.6	0.0	65.0	53.4	0.0	61.6	45.0	0.0
Incr Delay (d2), s/veh	30.7	0.6	0.0	12.7	46.6	0.0	25.3	4.8	0.0	65.7	1.4	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.7	6.3	0.0	3.9	44.8	0.0	5.5	5.2	0.0	13.4	3.4	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	100.2	23.1	0.0	78.8	80.3	0.0	90.3	58.2	0.0	127.4	46.4	0.0
LnGrp LOS	F	C		E	F		F	E		F	D	
Approach Vol, veh/h		640	A		1137	A		273	A		647	A
Approach Delay, s/veh		27.3			80.1			72.7			113.3	
Approach LOS		C			F			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.2	75.0	25.0	29.5	8.3	81.0	16.5	38.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	16.0	65.5	20.5	25.0	5.0	76.5	14.8	30.7				
Max Q Clear Time (g_c+I1), s	9.9	17.2	22.5	12.5	4.8	78.5	12.0	9.0				
Green Ext Time (p_c), s	0.1	4.4	0.0	0.5	0.0	0.0	0.1	0.5				
Intersection Summary												
HCM 6th Ctrl Delay			74.8									
HCM 6th LOS			E									

Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Short Term Background Traffic PM

7: Shield Dr & Elk River Rd

Intersection						
Int Delay, s/veh	3.3					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	52	94	65	189	75	7
Future Vol, veh/h	52	94	65	189	75	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	53	53	74	74	81	81
Heavy Vehicles, %	0	0	4	4	2	2
Mvmt Flow	98	177	88	255	93	9
Major/Minor	Major1	Major2		Minor2		
Conflicting Flow All	343	0	-	0	589	216
Stage 1	-	-	-	-	216	-
Stage 2	-	-	-	-	373	-
Critical Hdwy	4.1	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.2	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1227	-	-	-	471	824
Stage 1	-	-	-	-	820	-
Stage 2	-	-	-	-	696	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1227	-	-	-	429	824
Mov Cap-2 Maneuver	-	-	-	-	429	-
Stage 1	-	-	-	-	747	-
Stage 2	-	-	-	-	696	-
Approach	EB	WB		SB		
HCM Control Delay, s	2.9	0		15.4		
HCM LOS	C					
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1227	-	-	-	447	
HCM Lane V/C Ratio	0.08	-	-	-	0.226	
HCM Control Delay (s)	8.2	0	-	-	15.4	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0.3	-	-	-	0.9	

Intersection												
Int Delay, s/veh	40.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	3	0	123	0	1	15	125	1543	0	6	1171	6
Future Vol, veh/h	3	0	123	0	1	15	125	1543	0	6	1171	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	69	69	69	94	94	94	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	5	0	205	0	1	22	133	1641	0	7	1331	7
Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	3264	3252	666	2587	3259	1641	1338	0	0	1641	0	0
Stage 1	1345	1345	-	1907	1907	-	-	-	-	-	-	-
Stage 2	1919	1907	-	680	1352	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	5	9	407	15	9	125	505	-	-	389	-	-
Stage 1	163	222	-	89	118	-	-	-	-	-	-	-
Stage 2	88	118	-	412	220	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	~ 3	6	407	6	6	125	505	-	-	389	-	-
Mov Cap-2 Maneuver	~ 3	6	-	6	6	-	-	-	-	-	-	-
Stage 1	120	206	-	66	87	-	-	-	-	-	-	-
Stage 2	53	87	-	190	205	-	-	-	-	-	-	-
Approach	EB		WB			NB			SB			
HCM Control Delay, s	\$ 628		108.8			1.1			0.6			
HCM LOS	F		F									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	505	-	-	97	56	389	-	-				
HCM Lane V/C Ratio	0.263	-	-	2.165	0.414	0.018	-	-				
HCM Control Delay (s)	14.7	-	-	\$ 628	108.8	14.4	0.5	-				
HCM Lane LOS	B	-	-	F	F	B	A	-				
HCM 95th %tile Q(veh)	1	-	-	18.4	1.5	0.1	-	-				
Notes												
~: Volume exceeds capacity		\$: Delay exceeds 300s			+: Computation Not Defined			*: All major volume in platoon				

Short Term Background Traffic PM
16: Elk River Rd & Curve Plaza

Intersection												
Int Delay, s/veh	13.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↑	↗
Traffic Vol, veh/h	225	0	39	0	0	0	96	145	0	0	45	168
Future Vol, veh/h	225	0	39	0	0	0	96	145	0	0	45	168
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	92	92	92	59	59	59	85	85	85
Heavy Vehicles, %	1	1	1	0	0	0	4	4	4	1	1	1
Mvmt Flow	256	0	44	0	0	0	163	246	0	0	53	198
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	625	625	53	746	823	246	251	0	0	-	-	0
Stage 1	53	53	-	572	572	-	-	-	-	-	-	-
Stage 2	572	572	-	174	251	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	7.1	6.5	6.2	4.14	-	-	-	-	-
Critical Hdwy Stg 1	6.11	5.51	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	3.5	4	3.3	2.236	-	-	-	-	-
Pot Cap-1 Maneuver	399	403	1017	332	311	798	1303	-	-	0	-	-
Stage 1	962	853	-	509	508	-	-	-	-	0	-	-
Stage 2	507	506	-	833	703	-	-	-	-	0	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	355	345	1017	282	266	798	1303	-	-	-	-	-
Mov Cap-2 Maneuver	355	345	-	282	266	-	-	-	-	-	-	-
Stage 1	823	853	-	435	434	-	-	-	-	-	-	-
Stage 2	433	433	-	797	703	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	38.3		0		3.2		0					
HCM LOS	E		A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR						
Capacity (veh/h)	1303	-	-	393	-	-						
HCM Lane V/C Ratio	0.125	-	-	0.763	-	-						
HCM Control Delay (s)	8.2	0	-	38.3	0	-						
HCM Lane LOS	A	A	-	E	A	-						
HCM 95th %tile Q(veh)	0.4	-	-	6.3	-	-						

Short Term Background Traffic PM

20: Shield Dr & Curve Plaza

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	254	0	0	173
Future Vol, veh/h	0	0	254	0	0	173
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	79	79	74	74
Heavy Vehicles, %	2	2	4	4	1	1
Mvmt Flow	0	0	322	0	0	234
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	556	322	0	0	322	0
Stage 1	322	-	-	-	-	-
Stage 2	234	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.11	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.209	-
Pot Cap-1 Maneuver	492	719	-	-	1244	-
Stage 1	735	-	-	-	-	-
Stage 2	805	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	492	719	-	-	1244	-
Mov Cap-2 Maneuver	492	-	-	-	-	-
Stage 1	735	-	-	-	-	-
Stage 2	805	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	0	0		0		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	-	1244	-	
HCM Lane V/C Ratio	-	-	-	-	-	
HCM Control Delay (s)	-	-	0	0	-	
HCM Lane LOS	-	-	A	A	-	
HCM 95th %tile Q(veh)	-	-	-	0	-	

Short Term Build AM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	49	1074	83	71	478	390	70	116	116	469	115	48
Future Volume (veh/h)	49	1074	83	71	478	390	70	116	116	469	115	48
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1752	1752	1752	1870	1870	1870
Adj Flow Rate, veh/h	56	1220	0	88	590	0	80	132	0	558	137	0
Peak Hour Factor	0.88	0.88	0.88	0.81	0.81	0.81	0.88	0.88	0.88	0.84	0.84	0.84
Percent Heavy Veh, %	3	3	3	3	3	3	10	10	10	2	2	2
Cap, veh/h	72	1279		101	704		101	417		589	651	
Arrive On Green	0.04	0.36	0.00	0.06	0.38	0.00	0.06	0.24	0.00	0.17	0.35	0.00
Sat Flow, veh/h	1767	3526	1572	1767	1856	1572	1668	1752	1485	3456	1870	1585
Grp Volume(v), veh/h	56	1220	0	88	590	0	80	132	0	558	137	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1767	1856	1572	1668	1752	1485	1728	1870	1585
Q Serve(g_s), s	3.3	35.4	0.0	5.2	30.4	0.0	5.0	6.5	0.0	16.8	5.4	0.0
Cycle Q Clear(g_c), s	3.3	35.4	0.0	5.2	30.4	0.0	5.0	6.5	0.0	16.8	5.4	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	72	1279		101	704		101	417		589	651	
V/C Ratio(X)	0.78	0.95		0.87	0.84		0.79	0.32		0.95	0.21	
Avail Cap(c_a), veh/h	84	1279		101	704		168	417		589	651	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	49.9	32.6	0.0	49.1	29.6	0.0	48.7	33.0	0.0	43.1	24.1	0.0
Incr Delay (d2), s/veh	32.0	16.3	0.0	51.2	11.4	0.0	13.1	2.0	0.0	24.7	0.7	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	2.1	17.1	0.0	3.7	15.0	0.0	2.4	2.9	0.0	9.0	2.5	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	82.0	48.9	0.0	100.3	41.1	0.0	61.8	34.9	0.0	67.7	24.8	0.0
LnGrp LOS	F	D		F	D		E	C		E	C	
Approach Vol, veh/h		1276	A		678	A		212	A		695	A
Approach Delay, s/veh		50.3			48.8			45.1			59.3	
Approach LOS		D			D			D			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	10.5	42.6	22.4	29.5	8.8	44.3	10.8	41.1				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	6.0	38.1	17.9	25.0	5.0	39.1	10.6	32.3				
Max Q Clear Time (g_c+I1), s	7.2	37.4	18.8	8.5	5.3	32.4	7.0	7.4				
Green Ext Time (p_c), s	0.0	0.5	0.0	0.5	0.0	2.0	0.0	0.6				
Intersection Summary												
HCM 6th Ctrl Delay			51.7									
HCM 6th LOS			D									

Notes

Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Short Term Build AM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 4

Movement EBL EBT WBT WBR SBL SBR

Lane Configurations   

Traffic Vol, veh/h 19 130 61 148 109 30

Future Vol, veh/h 19 130 61 148 109 30

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Free Free Free Free Stop Stop

RT Channelized - None - None - None

Storage Length - - - - 0 -

Veh in Median Storage, # - 0 0 - 0 -

Grade, % - 0 0 - 0 -

Peak Hour Factor 54 54 91 91 77 77

Heavy Vehicles, % 0 0 13 13 3 3

Mvmt Flow 35 241 67 163 142 39

Major/Minor Major1 Major2 Minor2

Conflicting Flow All 230 0 - 0 460 149

Stage 1 - - - - 149 -

Stage 2 - - - - 311 -

Critical Hdwy 4.1 - - - 6.43 6.23

Critical Hdwy Stg 1 - - - - 5.43 -

Critical Hdwy Stg 2 - - - - 5.43 -

Follow-up Hdwy 2.2 - - - 3.527 3.327

Pot Cap-1 Maneuver 1350 - - - 558 895

Stage 1 - - - - 876 -

Stage 2 - - - - 741 -

Platoon blocked, % - - - -

Mov Cap-1 Maneuver 1350 - - - 541 895

Mov Cap-2 Maneuver - - - - 541 -

Stage 1 - - - - 850 -

Stage 2 - - - - 741 -

Approach EB WB SB

HCM Control Delay, s 1 0 13.7

HCM LOS B

Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1

Capacity (veh/h) 1350 - - - 591

HCM Lane V/C Ratio 0.026 - - - 0.305

HCM Control Delay (s) 7.7 0 - - 13.7

HCM Lane LOS A A - - B

HCM 95th %tile Q(veh) 0.1 - - - 1.3

Short Term Build AM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕			↕	↕
Traffic Vol, veh/h	0	0	55	0	0	9	175	955	1	28	1636	4
Future Vol, veh/h	0	0	55	0	0	9	175	955	1	28	1636	4
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	64	64	64	75	75	75	80	80	80	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	0	0	86	0	0	12	219	1194	1	31	1798	4
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	3499	3493	899	2594	3497	1195	1802	0	0	1195	0	0
Stage 1	1860	1860	-	1633	1633	-	-	-	-	-	-	-
Stage 2	1639	1633	-	961	1864	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	3	7	286	15	6	229	333	-	-	577	-	-
Stage 1	77	124	-	129	161	-	-	-	-	-	-	-
Stage 2	128	161	-	279	124	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	1	2	286	5	2	229	333	-	-	577	-	-
Mov Cap-2 Maneuver	1	2	-	5	2	-	-	-	-	-	-	-
Stage 1	26	124	-	44	55	-	-	-	-	-	-	-
Stage 2	42	55	-	195	124	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	22.9		21.6		5.3		0.2					
HCM LOS	C		C									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	333	-	-	286	229	577	-	-				
HCM Lane V/C Ratio	0.657	-	-	0.3	0.052	0.053	-	-				
HCM Control Delay (s)	34.3	-	-	22.9	21.6	11.6	0	-				
HCM Lane LOS	D	-	-	C	C	B	A	-				
HCM 95th %tile Q(veh)	4.4	-	-	1.2	0.2	0.2	-	-				

Short Term Build AM
16: Elk River Rd & Curve Plaza

Intersection												
Int Delay, s/veh	6.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕				↗		↕			↕	↗
Traffic Vol, veh/h	173	0	32	0	0	32	71	97	0	0	108	165
Future Vol, veh/h	173	0	32	0	0	32	71	97	0	0	108	165
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	92	92	92	94	94	94	88	88	88
Heavy Vehicles, %	2	2	2	0	0	0	16	16	16	1	1	1
Mvmt Flow	201	0	37	0	0	35	76	103	0	0	123	188
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	396	378	123	-	-	103	311	0	0	-	-	0
Stage 1	123	123	-	-	-	-	-	-	-	-	-	-
Stage 2	273	255	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.12	6.52	6.22	-	-	6.2	4.26	-	-	-	-	-
Critical Hdwy Stg 1	6.12	5.52	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.12	5.52	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	-	-	3.3	2.344	-	-	-	-	-
Pot Cap-1 Maneuver	564	554	928	0	0	957	1174	-	-	0	-	-
Stage 1	881	794	-	0	0	-	-	-	-	0	-	-
Stage 2	733	696	-	0	0	-	-	-	-	0	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	515	516	928	-	-	957	1174	-	-	-	-	-
Mov Cap-2 Maneuver	515	516	-	-	-	-	-	-	-	-	-	-
Stage 1	820	794	-	-	-	-	-	-	-	-	-	-
Stage 2	658	648	-	-	-	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	16.4		8.9		3.5		0					
HCM LOS	C		A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR						
Capacity (veh/h)	1174	-	-	553	957	-						
HCM Lane V/C Ratio	0.064	-	-	0.431	0.036	-						
HCM Control Delay (s)	8.3	0	-	16.4	8.9	-						
HCM Lane LOS	A	A	-	C	A	-						
HCM 95th %tile Q(veh)	0.2	-	-	2.2	0.1	-						

Short Term Build AM
20: Shield Dr & Curve Plaza

Intersection

Int Delay, s/veh 0.8

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	3	203	3	28	210
Future Vol, veh/h	6	3	203	3	28	210
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	95	95	62	62
Heavy Vehicles, %	2	2	14	14	1	1
Mvmt Flow	7	3	214	3	45	339

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	645	216	0
Stage 1	216	-	-
Stage 2	429	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	437	824	-
Stage 1	820	-	-
Stage 2	657	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	419	824	-
Mov Cap-2 Maneuver	419	-	-
Stage 1	820	-	-
Stage 2	630	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12.3	0	0.9
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	501	1359
HCM Lane V/C Ratio	-	-	0.02	0.033
HCM Control Delay (s)	-	-	12.3	7.7
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0.1	0.1

Short Term Build AM

22: Curve Ct

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	30	2	18	179	25	1	0	6	18	0	0
Future Vol, veh/h	0	30	2	18	179	25	1	0	6	18	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	33	2	20	195	27	1	0	7	20	0	0
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	222	0	0	35	0	0	283	296	34	287	284	209
Stage 1	-	-	-	-	-	-	34	34	-	249	249	-
Stage 2	-	-	-	-	-	-	249	262	-	38	35	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1347	-	-	1576	-	-	669	616	1039	665	625	831
Stage 1	-	-	-	-	-	-	982	867	-	755	701	-
Stage 2	-	-	-	-	-	-	755	691	-	977	866	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1347	-	-	1576	-	-	662	607	1039	653	616	831
Mov Cap-2 Maneuver	-	-	-	-	-	-	662	607	-	653	616	-
Stage 1	-	-	-	-	-	-	982	867	-	755	690	-
Stage 2	-	-	-	-	-	-	744	681	-	971	866	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.6			8.8			10.7		
HCM LOS							A			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	961	1347	-	-	1576	-	-	653				
HCM Lane V/C Ratio	0.008	-	-	-	0.012	-	-	0.03				
HCM Control Delay (s)	8.8	0	-	-	7.3	0	-	10.7				
HCM Lane LOS	A	A	-	-	A	A	-	B				
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1				

Short Term Build PM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	33	575	42	94	978	462	100	117	170	444	101	96
Future Volume (veh/h)	33	575	42	94	978	462	100	117	170	444	101	96
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1841	1841	1841	1870	1870	1870
Adj Flow Rate, veh/h	35	605	0	100	1040	0	133	156	0	535	122	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.75	0.75	0.75	0.83	0.83	0.83
Percent Heavy Veh, %	3	3	3	4	4	4	4	4	4	2	2	2
Cap, veh/h	46	1723		122	979		156	320		493	426	
Arrive On Green	0.03	0.49	0.00	0.07	0.53	0.00	0.09	0.17	0.00	0.14	0.23	0.00
Sat Flow, veh/h	1767	3526	1572	1753	1841	1560	1753	1841	1560	3456	1870	1585
Grp Volume(v), veh/h	35	605	0	100	1040	0	133	156	0	535	122	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1841	1560	1753	1841	1560	1728	1870	1585
Q Serve(g_s), s	2.8	15.2	0.0	8.1	76.5	0.0	10.8	11.0	0.0	20.5	7.7	0.0
Cycle Q Clear(g_c), s	2.8	15.2	0.0	8.1	76.5	0.0	10.8	11.0	0.0	20.5	7.7	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	46	1723		122	979		156	320		493	426	
V/C Ratio(X)	0.76	0.35		0.82	1.06		0.86	0.49		1.09	0.29	
Avail Cap(c_a), veh/h	61	1723		195	979		182	320		493	426	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	69.5	22.7	0.0	66.0	33.6	0.0	64.6	53.6	0.0	61.6	45.9	0.0
Incr Delay (d2), s/veh	30.7	0.6	0.0	13.6	46.6	0.0	27.8	5.2	0.0	65.7	1.7	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.7	6.4	0.0	4.0	44.8	0.0	6.0	5.5	0.0	13.4	3.8	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	100.2	23.2	0.0	79.6	80.3	0.0	92.4	58.8	0.0	127.4	47.5	0.0
LnGrp LOS	F	C		E	F		F	E		F	D	
Approach Vol, veh/h		640	A		1140	A		289	A		657	A
Approach Delay, s/veh		27.5			80.2			74.3			112.5	
Approach LOS		C			F			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.5	74.8	25.0	29.5	8.3	81.0	17.3	37.2				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	16.0	65.5	20.5	25.0	5.0	76.5	14.9	30.6				
Max Q Clear Time (g_c+I1), s	10.1	17.2	22.5	13.0	4.8	78.5	12.8	9.7				
Green Ext Time (p_c), s	0.1	4.4	0.0	0.5	0.0	0.0	0.1	0.5				
Intersection Summary												
HCM 6th Ctrl Delay			75.0									
HCM 6th LOS			E									
Notes												
Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Short Term Build PM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 3.9

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	52	97	67	189	96	7
Future Vol, veh/h	52	97	67	189	96	7
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	53	53	74	74	81	81
Heavy Vehicles, %	0	0	4	4	2	2
Mvmt Flow	98	183	91	255	119	9

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	346	0	0 598 219
Stage 1	-	-	- 219 -
Stage 2	-	-	- 379 -
Critical Hdwy	4.1	-	- 6.42 6.22
Critical Hdwy Stg 1	-	-	- 5.42 -
Critical Hdwy Stg 2	-	-	- 5.42 -
Follow-up Hdwy	2.2	-	- 3.518 3.318
Pot Cap-1 Maneuver	1224	-	- 465 821
Stage 1	-	-	- 817 -
Stage 2	-	-	- 692 -
Platoon blocked, %	-	-	-
Mov Cap-1 Maneuver	1224	-	- 424 821
Mov Cap-2 Maneuver	-	-	- 424 -
Stage 1	-	-	- 744 -
Stage 2	-	-	- 692 -

Approach	EB	WB	SB
HCM Control Delay, s	2.9	0	16.5
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1224	-	-	-	438
HCM Lane V/C Ratio	0.08	-	-	-	0.29
HCM Control Delay (s)	8.2	0	-	-	16.5
HCM Lane LOS	A	A	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	1.2

Short Term Build PM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	71.5											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	3	0	134	0	1	15	147	1545	0	6	1176	6
Future Vol, veh/h	3	0	134	0	1	15	147	1545	0	6	1176	6
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	69	69	69	94	94	94	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	5	0	223	0	1	22	156	1644	0	7	1336	7
Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	3318	3306	668	2638	3313	1644	1343	0	0	1644	0	0
Stage 1	1350	1350	-	1956	1956	-	-	-	-	-	-	-
Stage 2	1968	1956	-	682	1357	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	~ 4	9	405	14	9	124	503	-	-	388	-	-
Stage 1	161	221	-	84	111	-	-	-	-	-	-	-
Stage 2	82	111	-	411	219	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	~ 2	6	405	5	6	124	503	-	-	388	-	-
Mov Cap-2 Maneuver	~ 2	6	-	5	6	-	-	-	-	-	-	-
Stage 1	111	205	-	58	77	-	-	-	-	-	-	-
Stage 2	46	77	-	171	203	-	-	-	-	-	-	-
Approach	EB		WB			NB			SB			
HCM Control Delay, \$	1039.7		108.8			1.3			0.6			
HCM LOS	F		F									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	503	-	-	75	56	388	-	-				
HCM Lane V/C Ratio	0.311	-	-	3.044	0.414	0.018	-	-				
HCM Control Delay (s)	15.4	-	-	\$ 1039.7	108.8	14.4	0.5	-				
HCM Lane LOS	C	-	-	F	F	B	A	-				
HCM 95th %tile Q(veh)	1.3	-	-	22.9	1.5	0.1	-	-				
Notes												
~: Volume exceeds capacity		\$: Delay exceeds 300s			+: Computation Not Defined				*: All major volume in platoon			

Short Term Build PM
16: Elk River Rd & Curve Plaza

Intersection												
Int Delay, s/veh	15.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕				↗		↕			↖	↗
Traffic Vol, veh/h	225	0	39	0	0	16	96	145	0	0	66	168
Future Vol, veh/h	225	0	39	0	0	16	96	145	0	0	66	168
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	0	-	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	88	88	88	92	92	92	59	59	59	85	85	85
Heavy Vehicles, %	1	1	1	0	0	0	4	4	4	1	1	1
Mvmt Flow	256	0	44	0	0	17	163	246	0	0	78	198
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	659	650	78	-	-	246	276	0	0	-	-	0
Stage 1	78	78	-	-	-	-	-	-	-	-	-	-
Stage 2	581	572	-	-	-	-	-	-	-	-	-	-
Critical Hdwy	7.11	6.51	6.21	-	-	6.2	4.14	-	-	-	-	-
Critical Hdwy Stg 1	6.11	5.51	-	-	-	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.11	5.51	-	-	-	-	-	-	-	-	-	-
Follow-up Hdwy	3.509	4.009	3.309	-	-	3.3	2.236	-	-	-	-	-
Pot Cap-1 Maneuver	378	390	985	0	0	798	1275	-	-	0	-	-
Stage 1	933	832	-	0	0	-	-	-	-	0	-	-
Stage 2	501	506	-	0	0	-	-	-	-	0	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	328	332	985	-	-	798	1275	-	-	-	-	-
Mov Cap-2 Maneuver	328	332	-	-	-	-	-	-	-	-	-	-
Stage 1	795	832	-	-	-	-	-	-	-	-	-	-
Stage 2	418	431	-	-	-	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	47.7		9.6		3.3		0					
HCM LOS	E		A									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR						
Capacity (veh/h)	1275	-	-	364	798	-						
HCM Lane V/C Ratio	0.128	-	-	0.824	0.022	-						
HCM Control Delay (s)	8.2	0	-	47.7	9.6	-						
HCM Lane LOS	A	A	-	E	A	-						
HCM 95th %tile Q(veh)	0.4	-	-	7.3	0.1	-						

Short Term Build PM
20: Shield Dr & Curve Plaza

Intersection

Int Delay, s/veh 0.5

Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	2	254	2	23	173
Future Vol, veh/h	3	2	254	2	23	173
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	79	79	74	74
Heavy Vehicles, %	2	2	4	4	1	1
Mvmt Flow	3	2	322	3	31	234

Major/Minor	Minor1	Major1	Major2
Conflicting Flow All	620	324	0
Stage 1	324	-	-
Stage 2	296	-	-
Critical Hdwy	6.42	6.22	-
Critical Hdwy Stg 1	5.42	-	-
Critical Hdwy Stg 2	5.42	-	-
Follow-up Hdwy	3.518	3.318	-
Pot Cap-1 Maneuver	452	717	-
Stage 1	733	-	-
Stage 2	755	-	-
Platoon blocked, %		-	-
Mov Cap-1 Maneuver	439	717	-
Mov Cap-2 Maneuver	439	-	-
Stage 1	733	-	-
Stage 2	733	-	-

Approach	WB	NB	SB
HCM Control Delay, s	12	0	0.9
HCM LOS	B		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT
Capacity (veh/h)	-	-	520	1240
HCM Lane V/C Ratio	-	-	0.01	0.025
HCM Control Delay (s)	-	-	12	8
HCM Lane LOS	-	-	B	A
HCM 95th %tile Q(veh)	-	-	0	0.1

Short Term Build PM

22: Curve Ct

Intersection												
Int Delay, s/veh	1.6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	102	2	17	132	22	3	0	26	9	0	0
Future Vol, veh/h	0	102	2	17	132	22	3	0	26	9	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	0	111	2	18	143	24	3	0	28	10	0	0
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	167	0	0	113	0	0	303	315	112	317	304	155
Stage 1	-	-	-	-	-	-	112	112	-	191	191	-
Stage 2	-	-	-	-	-	-	191	203	-	126	113	-
Critical Hdwy	4.12	-	-	4.12	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.218	-	-	2.218	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1411	-	-	1476	-	-	649	601	941	636	609	891
Stage 1	-	-	-	-	-	-	893	803	-	811	742	-
Stage 2	-	-	-	-	-	-	811	733	-	878	802	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1411	-	-	1476	-	-	643	593	941	611	601	891
Mov Cap-2 Maneuver	-	-	-	-	-	-	643	593	-	611	601	-
Stage 1	-	-	-	-	-	-	893	803	-	811	732	-
Stage 2	-	-	-	-	-	-	800	723	-	852	802	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.7			9.2			11		
HCM LOS							A			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	898	1411	-	-	1476	-	-	611				
HCM Lane V/C Ratio	0.035	-	-	-	0.013	-	-	0.016				
HCM Control Delay (s)	9.2	0	-	-	7.5	0	-	11				
HCM Lane LOS	A	A	-	-	A	A	-	B				
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0				

Long Term Baseline AM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	56	1229	81	78	546	447	65	121	123	536	121	55
Future Volume (veh/h)	56	1229	81	78	546	447	65	121	123	536	121	55
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1752	1752	1752	1870	1870	1870
Adj Flow Rate, veh/h	64	1397	0	96	674	0	74	138	0	638	144	0
Peak Hour Factor	0.88	0.88	0.88	0.81	0.81	0.81	0.88	0.88	0.88	0.84	0.84	0.84
Percent Heavy Veh, %	3	3	3	3	3	3	10	10	10	2	2	2
Cap, veh/h	79	1438		106	785		93	350		650	622	
Arrive On Green	0.04	0.41	0.00	0.06	0.42	0.00	0.06	0.20	0.00	0.19	0.33	0.00
Sat Flow, veh/h	1767	3526	1572	1767	1856	1572	1668	1752	1485	3456	1870	1585
Grp Volume(v), veh/h	64	1397	0	96	674	0	74	138	0	638	144	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1767	1856	1572	1668	1752	1485	1728	1870	1585
Q Serve(g_s), s	4.5	48.6	0.0	6.7	41.1	0.0	5.5	8.6	0.0	23.0	7.0	0.0
Cycle Q Clear(g_c), s	4.5	48.6	0.0	6.7	41.1	0.0	5.5	8.6	0.0	23.0	7.0	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	79	1438		106	785		93	350		650	622	
V/C Ratio(X)	0.81	0.97		0.91	0.86		0.80	0.39		0.98	0.23	
Avail Cap(c_a), veh/h	79	1438		106	785		168	350		650	622	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	59.2	36.3	0.0	58.4	32.7	0.0	58.3	43.4	0.0	50.5	30.2	0.0
Incr Delay (d2), s/veh	44.3	17.8	0.0	58.4	11.7	0.0	14.2	3.3	0.0	30.7	0.9	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.0	23.5	0.0	4.7	20.2	0.0	2.6	4.0	0.0	12.5	3.2	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	103.5	54.1	0.0	116.8	44.4	0.0	72.6	46.7	0.0	81.2	31.1	0.0
LnGrp LOS	F	D		F	D		E	D		F	C	
Approach Vol, veh/h		1461	A		770	A		212	A		782	A
Approach Delay, s/veh		56.2			53.4			55.7			72.0	
Approach LOS		E			D			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.0	55.5	28.0	29.5	10.1	57.4	11.5	46.0				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	7.5	51.0	23.5	25.0	5.6	52.9	12.6	35.9				
Max Q Clear Time (g_c+I1), s	8.7	50.6	25.0	10.6	6.5	43.1	7.5	9.0				
Green Ext Time (p_c), s	0.0	0.3	0.0	0.5	0.0	3.0	0.1	0.7				
Intersection Summary												
HCM 6th Ctrl Delay			59.3									
HCM 6th LOS			E									

Notes

Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.

Long Term Baseline AM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 3.7

Movement EBL EBT WBT WBR SBL SBR

Lane Configurations   

Traffic Vol, veh/h 22 146 66 169 98 35

Future Vol, veh/h 22 146 66 169 98 35

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Free Free Free Free Stop Stop

RT Channelized - None - None - None

Storage Length - - - - 0 -

Veh in Median Storage, # - 0 0 - 0 -

Grade, % - 0 0 - 0 -

Peak Hour Factor 54 54 91 91 77 77

Heavy Vehicles, % 0 0 13 13 3 3

Mvmt Flow 41 270 73 186 127 45

Major/Minor Major1 Major2 Minor2

Conflicting Flow All 259 0 - 0 518 166

Stage 1 - - - - 166 -

Stage 2 - - - - 352 -

Critical Hdwy 4.1 - - - 6.43 6.23

Critical Hdwy Stg 1 - - - - 5.43 -

Critical Hdwy Stg 2 - - - - 5.43 -

Follow-up Hdwy 2.2 - - - 3.527 3.327

Pot Cap-1 Maneuver 1317 - - - 516 876

Stage 1 - - - - 861 -

Stage 2 - - - - 710 -

Platoon blocked, % - - - -

Mov Cap-1 Maneuver 1317 - - - 497 876

Mov Cap-2 Maneuver - - - - 497 -

Stage 1 - - - - 829 -

Stage 2 - - - - 710 -

Approach EB WB SB

HCM Control Delay, s 1 0 14.2

HCM LOS B

Minor Lane/Major Mvmt EBL EBT WBT WBR SBLn1

Capacity (veh/h) 1317 - - - 561

HCM Lane V/C Ratio 0.031 - - - 0.308

HCM Control Delay (s) 7.8 0 - - 14.2

HCM Lane LOS A A - - B

HCM 95th %tile Q(veh) 0.1 - - - 1.3

Long Term Baseline AM
20: Shield Dr & Access #2

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	232	0	2	241
Future Vol, veh/h	0	0	232	0	2	241
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	252	0	2	262
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	518	252	0	0	252	0
Stage 1	252	-	-	-	-	-
Stage 2	266	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	518	787	-	-	1313	-
Stage 1	790	-	-	-	-	-
Stage 2	779	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	517	787	-	-	1313	-
Mov Cap-2 Maneuver	517	-	-	-	-	-
Stage 1	790	-	-	-	-	-
Stage 2	777	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	0	0		0.1		
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	-	1313	-	
HCM Lane V/C Ratio	-	-	-	0.002	-	
HCM Control Delay (s)	-	-	0	7.7	0	
HCM Lane LOS	-	-	A	A	A	
HCM 95th %tile Q(veh)	-	-	-	0	-	

Long Term Baseline AM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↕	↕			↕	↕
Traffic Vol, veh/h	0	0	38	0	0	10	173	1089	2	32	1861	5
Future Vol, veh/h	0	0	38	0	0	10	173	1089	2	32	1861	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	64	64	64	75	75	75	80	80	80	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	0	0	59	0	0	13	216	1361	3	35	2045	5
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	3916	3911	1023	2888	3915	1363	2050	0	0	1364	0	0
Stage 1	2115	2115	-	1795	1795	-	-	-	-	-	-	-
Stage 2	1801	1796	-	1093	2120	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	1	3	237	9	3	183	266	-	-	497	-	-
Stage 1	53	92	-	104	134	-	-	-	-	-	-	-
Stage 2	103	134	-	232	92	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	0	1	237	2	1	183	266	-	-	497	-	-
Mov Cap-2 Maneuver	0	1	-	2	1	-	-	-	-	-	-	-
Stage 1	10	92	-	20	25	-	-	-	-	-	-	-
Stage 2	18	25	-	174	92	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	25.2		26.2		8		0.2					
HCM LOS	D		D									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	266	-	-	237	183	497	-	-				
HCM Lane V/C Ratio	0.813	-	-	0.251	0.073	0.071	-	-				
HCM Control Delay (s)	58.4	-	-	25.2	26.2	12.8	0	-				
HCM Lane LOS	F	-	-	D	D	B	A	-				
HCM 95th %tile Q(veh)	6.4	-	-	1	0.2	0.2	-	-				

Long Term Baseline AM
16: Elk River Rd & Curve Plaza

Intersection

Int Delay, s/veh 6.3

Movement EBL EBR NBL NBT SBT SBR

Lane Configurations       

Traffic Vol, veh/h 198 37 81 111 96 189

Future Vol, veh/h 198 37 81 111 96 189

Conflicting Peds, #/hr 0 0 0 0 0 0

Sign Control Stop Stop Free Free Free Free

RT Channelized - None - None - None

Storage Length 0 - - - - 0

Veh in Median Storage, # 0 - - 0 0 -

Grade, % 0 - - 0 0 -

Peak Hour Factor 86 86 94 94 88 88

Heavy Vehicles, % 2 2 16 16 1 1

Mvmt Flow 230 43 86 118 109 215

Major/Minor Minor2 Major1 Major2

Conflicting Flow All 399 109 324 0 - 0

Stage 1 109 - - - - -

Stage 2 290 - - - - -

Critical Hdwy 6.42 6.22 4.26 - - -

Critical Hdwy Stg 1 5.42 - - - - -

Critical Hdwy Stg 2 5.42 - - - - -

Follow-up Hdwy 3.518 3.318 2.344 - - -

Pot Cap-1 Maneuver 607 945 1161 - - -

Stage 1 916 - - - - -

Stage 2 759 - - - - -

Platoon blocked, % - - -

Mov Cap-1 Maneuver 559 945 1161 - - -

Mov Cap-2 Maneuver 559 - - - - -

Stage 1 844 - - - - -

Stage 2 759 - - - - -

Approach EB NB SB

HCM Control Delay, s 16 3.5 0

HCM LOS C

Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR

Capacity (veh/h) 1161 - 597 - -

HCM Lane V/C Ratio 0.074 - 0.458 - -

HCM Control Delay (s) 8.3 0 16 - -

HCM Lane LOS A A C - -

HCM 95th %tile Q(veh) 0.2 - 2.4 - -

Long Term Baseline PM
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	38	658	37	105	1119	528	106	128	189	508	106	110
Future Volume (veh/h)	38	658	37	105	1119	528	106	128	189	508	106	110
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1841	1841	1841	1870	1870	1870
Adj Flow Rate, veh/h	40	693	0	112	1190	0	141	171	0	612	128	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.75	0.75	0.75	0.83	0.83	0.83
Percent Heavy Veh, %	3	3	3	4	4	4	4	4	4	2	2	2
Cap, veh/h	53	1583		137	915		159	372		433	443	
Arrive On Green	0.03	0.45	0.00	0.08	0.50	0.00	0.09	0.20	0.00	0.13	0.24	0.00
Sat Flow, veh/h	1767	3526	1572	1753	1841	1560	1753	1841	1560	3456	1870	1585
Grp Volume(v), veh/h	40	693	0	112	1190	0	141	171	0	612	128	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1841	1560	1753	1841	1560	1728	1870	1585
Q Serve(g_s), s	2.8	16.7	0.0	7.8	61.5	0.0	9.8	10.1	0.0	15.5	6.9	0.0
Cycle Q Clear(g_c), s	2.8	16.7	0.0	7.8	61.5	0.0	9.8	10.1	0.0	15.5	6.9	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	53	1583		137	915		159	372		433	443	
V/C Ratio(X)	0.75	0.44		0.82	1.30		0.89	0.46		1.41	0.29	
Avail Cap(c_a), veh/h	71	1583		222	915		159	372		433	443	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	59.5	23.4	0.0	56.1	31.1	0.0	55.7	43.4	0.0	54.1	38.7	0.0
Incr Delay (d2), s/veh	25.6	0.9	0.0	11.4	143.4	0.0	41.1	4.1	0.0	199.5	1.6	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	6.9	0.0	3.8	61.8	0.0	6.1	5.0	0.0	18.6	3.3	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	85.1	24.3	0.0	67.5	174.5	0.0	96.7	47.5	0.0	253.6	40.3	0.0
LnGrp LOS	F	C		E	F		F	D		F	D	
Approach Vol, veh/h		733	A		1302	A		312	A		740	A
Approach Delay, s/veh		27.6			165.3			69.7			216.7	
Approach LOS		C			F			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.2	60.1	20.0	29.5	8.2	66.0	15.7	33.8				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	15.7	50.8	15.5	25.0	5.0	61.5	11.2	29.3				
Max Q Clear Time (g_c+I1), s	9.8	18.7	17.5	12.1	4.8	63.5	11.8	8.9				
Green Ext Time (p_c), s	0.1	4.9	0.0	0.6	0.0	0.0	0.0	0.5				
Intersection Summary												
HCM 6th Ctrl Delay			135.2									
HCM 6th LOS			F									
Notes												
Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Long Term Baseline PM
7: Shield Dr & Elk River Rd

Intersection

Int Delay, s/veh 3.7

Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	60	108	75	216	86	8
Future Vol, veh/h	60	108	75	216	86	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	53	53	74	74	81	81
Heavy Vehicles, %	0	0	4	4	2	2
Mvmt Flow	113	204	101	292	106	10

Major/Minor	Major1	Major2	Minor2
Conflicting Flow All	393	0	0 677 247
Stage 1	-	-	- - 247 -
Stage 2	-	-	- - 430 -
Critical Hdwy	4.1	-	- - 6.42 6.22
Critical Hdwy Stg 1	-	-	- - 5.42 -
Critical Hdwy Stg 2	-	-	- - 5.42 -
Follow-up Hdwy	2.2	-	- - 3.518 3.318
Pot Cap-1 Maneuver	1177	-	- - 418 792
Stage 1	-	-	- - 794 -
Stage 2	-	-	- - 656 -
Platoon blocked, %		-	- -
Mov Cap-1 Maneuver	1177	-	- - 373 792
Mov Cap-2 Maneuver	-	-	- - 373 -
Stage 1	-	-	- - 708 -
Stage 2	-	-	- - 656 -

Approach	EB	WB	SB
HCM Control Delay, s	3	0	18
HCM LOS			C

Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1
Capacity (veh/h)	1177	-	-	-	391
HCM Lane V/C Ratio	0.096	-	-	-	0.297
HCM Control Delay (s)	8.4	0	-	-	18
HCM Lane LOS	A	A	-	-	C
HCM 95th %tile Q(veh)	0.3	-	-	-	1.2

Long Term Baseline PM
20: Shield Dr & Access #2

Intersection						
Int Delay, s/veh	0					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	0	0	291	0	0	198
Future Vol, veh/h	0	0	291	0	0	198
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	0	0	316	0	0	215
Major/Minor	Minor1	Major1	Major2			
Conflicting Flow All	531	316	0	0	316	0
Stage 1	316	-	-	-	-	-
Stage 2	215	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	509	724	-	-	1244	-
Stage 1	739	-	-	-	-	-
Stage 2	821	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	509	724	-	-	1244	-
Mov Cap-2 Maneuver	509	-	-	-	-	-
Stage 1	739	-	-	-	-	-
Stage 2	821	-	-	-	-	-
Approach	WB	NB	SB			
HCM Control Delay, s	0	0	0			
HCM LOS	A					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	-	1244	-	
HCM Lane V/C Ratio	-	-	-	-	-	
HCM Control Delay (s)	-	-	0	0	-	
HCM Lane LOS	-	-	A	A	-	
HCM 95th %tile Q(veh)	-	-	-	0	-	

Long Term Baseline PM
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	7.3											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↵	↵			↕↕	↵
Traffic Vol, veh/h	3	0	141	0	2	17	143	1765	0	7	1340	7
Future Vol, veh/h	3	0	141	0	2	17	143	1765	0	7	1340	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	-	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	69	69	69	94	94	94	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	5	0	235	0	3	25	152	1878	0	8	1523	8
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	3735	3721	762	2960	3729	1878	1531	0	0	1878	0	0
Stage 1	1539	1539	-	2182	2182	-	-	-	-	-	-	-
Stage 2	2196	2182	-	778	1547	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	~ 2	5	352	8	5	90	425	-	-	314	-	-
Stage 1	123	179	-	61	85	-	-	-	-	-	-	-
Stage 2	60	85	-	360	177	-	-	-	-	-	-	-
Platoon blocked, %								-	-	-	-	-
Mov Cap-1 Maneuver	0	3	352	2	3	90	425	-	-	314	-	-
Mov Cap-2 Maneuver	0	3	-	2	3	-	-	-	-	-	-	-
Stage 1	79	148	-	39	55	-	-	-	-	-	-	-
Stage 2	27	55	-	99	147	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s	34.5		\$ 534.1		1.4		1.5					
HCM LOS	D		F									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBL	SBT	SBR					
Capacity (veh/h)	425	-	-	352	22	314	-	-				
HCM Lane V/C Ratio	0.358	-	-	0.682	1.252	0.025	-	-				
HCM Control Delay (s)	18.1	-	-	34.5	\$ 534.1	16.8	1.4	-				
HCM Lane LOS	C	-	-	D	F	C	A	-				
HCM 95th %tile Q(veh)	1.6	-	-	4.8	3.6	0.1	-	-				
Notes												
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined				*: All major volume in platoon				

Long Term Baseline PM
16: Elk River Rd & Curve Plaza

Intersection

Int Delay, s/veh 22.1

Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations						
Traffic Vol, veh/h	257	45	110	166	51	193
Future Vol, veh/h	257	45	110	166	51	193
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	0
Veh in Median Storage, #	0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	59	59	85	85
Heavy Vehicles, %	1	1	4	4	1	1
Mvmt Flow	292	51	186	281	60	227

Major/Minor	Minor2	Major1	Major2
Conflicting Flow All	713	60	287
Stage 1	60	-	-
Stage 2	653	-	-
Critical Hdwy	6.41	6.21	4.14
Critical Hdwy Stg 1	5.41	-	-
Critical Hdwy Stg 2	5.41	-	-
Follow-up Hdwy	3.509	3.309	2.236
Pot Cap-1 Maneuver	400	1008	1264
Stage 1	965	-	-
Stage 2	520	-	-
Platoon blocked, %			
Mov Cap-1 Maneuver	330	1008	1264
Mov Cap-2 Maneuver	330	-	-
Stage 1	797	-	-
Stage 2	520	-	-

Approach	EB	NB	SB
HCM Control Delay, s	66.1	3.3	0
HCM LOS	F		

Minor Lane/Major Mvmt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)	1264	-	367	-	-
HCM Lane V/C Ratio	0.148	-	0.935	-	-
HCM Control Delay (s)	8.3	0	66.1	-	-
HCM Lane LOS	A	A	F	-	-
HCM 95th %tile Q(veh)	0.5	-	10	-	-

Long Term AM Build
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	56	1229	93	81	546	447	78	131	132	536	130	55
Future Volume (veh/h)	56	1229	93	81	546	447	78	131	132	536	130	55
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1856	1856	1856	1752	1752	1752	1870	1870	1870
Adj Flow Rate, veh/h	64	1397	0	100	674	0	89	149	0	638	155	0
Peak Hour Factor	0.88	0.88	0.88	0.81	0.81	0.81	0.88	0.88	0.88	0.84	0.84	0.84
Percent Heavy Veh, %	3	3	3	3	3	3	10	10	10	2	2	2
Cap, veh/h	79	1430		110	785		110	350		650	602	
Arrive On Green	0.04	0.41	0.00	0.06	0.42	0.00	0.07	0.20	0.00	0.19	0.32	0.00
Sat Flow, veh/h	1767	3526	1572	1767	1856	1572	1668	1752	1485	3456	1870	1585
Grp Volume(v), veh/h	64	1397	0	100	674	0	89	149	0	638	155	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1767	1856	1572	1668	1752	1485	1728	1870	1585
Q Serve(g_s), s	4.5	48.8	0.0	7.0	41.1	0.0	6.6	9.3	0.0	23.0	7.7	0.0
Cycle Q Clear(g_c), s	4.5	48.8	0.0	7.0	41.1	0.0	6.6	9.3	0.0	23.0	7.7	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	79	1430		110	785		110	350		650	602	
V/C Ratio(X)	0.81	0.98		0.91	0.86		0.81	0.43		0.98	0.26	
Avail Cap(c_a), veh/h	79	1430		110	785		184	350		650	602	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	59.2	36.6	0.0	58.2	32.7	0.0	57.6	43.7	0.0	50.5	31.3	0.0
Incr Delay (d2), s/veh	44.3	18.9	0.0	57.3	11.7	0.0	12.9	3.7	0.0	30.7	1.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	3.0	23.8	0.0	4.9	20.2	0.0	3.1	4.3	0.0	12.5	3.6	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	103.5	55.5	0.0	115.6	44.4	0.0	70.5	47.5	0.0	81.2	32.4	0.0
LnGrp LOS	F	E		F	D		E	D		F	C	
Approach Vol, veh/h		1461	A		774	A		238	A		793	A
Approach Delay, s/veh		57.6			53.6			56.1			71.7	
Approach LOS		E			D			E			E	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	12.3	55.2	28.0	29.5	10.1	57.4	12.8	44.7				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	7.8	50.7	23.5	25.0	5.6	52.9	13.8	34.7				
Max Q Clear Time (g_c+I1), s	9.0	50.8	25.0	11.3	6.5	43.1	8.6	9.7				
Green Ext Time (p_c), s	0.0	0.0	0.0	0.5	0.0	3.0	0.1	0.7				
Intersection Summary												
HCM 6th Ctrl Delay			59.9									
HCM 6th LOS			E									
Notes												
Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Long Term AM Build
7: Shield Dr & Elk River Rd

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	22	149	69	169	121	35
Future Vol, veh/h	22	149	69	169	121	35
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	54	54	91	91	77	77
Heavy Vehicles, %	0	0	13	13	3	3
Mvmt Flow	41	276	76	186	157	45
Major/Minor	Major1	Major2		Minor2		
Conflicting Flow All	262	0	-	0	527	169
Stage 1	-	-	-	-	169	-
Stage 2	-	-	-	-	358	-
Critical Hdwy	4.1	-	-	-	6.43	6.23
Critical Hdwy Stg 1	-	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	2.2	-	-	-	3.527	3.327
Pot Cap-1 Maneuver	1314	-	-	-	510	872
Stage 1	-	-	-	-	858	-
Stage 2	-	-	-	-	705	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1314	-	-	-	491	872
Mov Cap-2 Maneuver	-	-	-	-	491	-
Stage 1	-	-	-	-	826	-
Stage 2	-	-	-	-	705	-
Approach	EB	WB		SB		
HCM Control Delay, s	1	0		15.5		
HCM LOS				C		
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1314	-	-	-	544	
HCM Lane V/C Ratio	0.031	-	-	-	0.372	
HCM Control Delay (s)	7.8	0	-	-	15.5	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0.1	-	-	-	1.7	

Long Term AM Build
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	6.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	0	59	0	0	10	197	1092	2	32	1870	5
Future Vol, veh/h	0	0	59	0	0	10	197	1092	2	32	1870	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	64	64	64	75	75	75	80	80	80	91	91	91
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	0	0	92	0	0	13	246	1365	3	35	2055	5
Major/Minor	Minor2		Minor1			Major1			Major2			
Conflicting Flow All	3990	3985	1028	2957	3989	1367	2060	0	0	1368	0	0
Stage 1	2125	2125	-	1859	1859	-	-	-	-	-	-	-
Stage 2	1865	1860	-	1098	2130	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	1	3	235	8	3	182	264	-	-	496	-	-
Stage 1	52	91	-	95	124	-	-	-	-	-	-	-
Stage 2	95	124	-	230	91	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	0	0	235	1	0	182	264	-	-	496	-	-
Mov Cap-2 Maneuver	0	0	-	1	0	-	-	-	-	-	-	-
Stage 1	4	85	-	6	8	-	-	-	-	-	-	-
Stage 2	6	8	-	130	85	-	-	-	-	-	-	-
Approach	EB		WB			NB			SB			
HCM Control Delay, s	29.9		26.3			12.3			0.2			
HCM LOS	D		D									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	264	-	-	235	182	496	-	-				
HCM Lane V/C Ratio	0.933	-	-	0.392	0.073	0.071	-	-				
HCM Control Delay (s)	80.7	-	-	29.9	26.3	12.8	-	-				
HCM Lane LOS	F	-	-	D	D	B	-	-				
HCM 95th %tile Q(veh)	8.6	-	-	1.8	0.2	0.2	-	-				

Long Term AM Build
16: Elk River Rd & Curve Plaza/Access #1

Intersection													
Int Delay, s/veh	7.5												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations		↕				↗		↕			↕	↗	
Traffic Vol, veh/h	198	0	37	0	0	32	81	111	0	0	120	189	
Future Vol, veh/h	198	0	37	0	0	32	81	111	0	0	120	189	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	0	-	-	-	-	-	0	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	86	86	86	92	92	92	94	94	94	88	88	88	
Heavy Vehicles, %	2	2	2	0	0	0	16	16	16	1	1	1	
Mvmt Flow	230	0	43	0	0	35	86	118	0	0	136	215	
Major/Minor	Minor2		Minor1		Major1		Major2						
Conflicting Flow All	444	426	136	-	-	118	351	0	0	-	-	0	
Stage 1	136	136	-	-	-	-	-	-	-	-	-	-	
Stage 2	308	290	-	-	-	-	-	-	-	-	-	-	
Critical Hdwy	7.12	6.52	6.22	-	-	6.2	4.26	-	-	-	-	-	
Critical Hdwy Stg 1	6.12	5.52	-	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.12	5.52	-	-	-	-	-	-	-	-	-	-	
Follow-up Hdwy	3.518	4.018	3.318	-	-	3.3	2.344	-	-	-	-	-	
Pot Cap-1 Maneuver	524	520	913	0	0	939	1134	-	-	0	-	-	
Stage 1	867	784	-	0	0	-	-	-	-	0	-	-	
Stage 2	702	672	-	0	0	-	-	-	-	0	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	473	478	913	-	-	939	1134	-	-	-	-	-	
Mov Cap-2 Maneuver	473	478	-	-	-	-	-	-	-	-	-	-	
Stage 1	797	784	-	-	-	-	-	-	-	-	-	-	
Stage 2	621	618	-	-	-	-	-	-	-	-	-	-	
Approach	EB		WB		NB		SB						
HCM Control Delay, s	19.8		9		3.6		0						
HCM LOS	C		A										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR							
Capacity (veh/h)	1134	-	-	512	939	-	-						
HCM Lane V/C Ratio	0.076	-	-	0.534	0.037	-	-						
HCM Control Delay (s)	8.4	0	-	19.8	9	-	-						
HCM Lane LOS	A	A	-	C	A	-	-						
HCM 95th %tile Q(veh)	0.2	-	-	3.1	0.1	-	-						

Long Term AM Build
20: Shield Dr & Access #2

Intersection						
Int Delay, s/veh	0.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	6	3	232	3	28	241
Future Vol, veh/h	6	3	232	3	28	241
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	95	95	62	62
Heavy Vehicles, %	2	2	14	14	1	1
Mvmt Flow	7	3	244	3	45	389
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	725	246	0	0	247	0
Stage 1	246	-	-	-	-	-
Stage 2	479	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.11	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.209	-
Pot Cap-1 Maneuver	392	793	-	-	1325	-
Stage 1	795	-	-	-	-	-
Stage 2	623	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	375	793	-	-	1325	-
Mov Cap-2 Maneuver	375	-	-	-	-	-
Stage 1	795	-	-	-	-	-
Stage 2	596	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	13.1	0		0.8		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1	SBL	SBT		
Capacity (veh/h)	-	-	455	1325	-	
HCM Lane V/C Ratio	-	-	0.022	0.034	-	
HCM Control Delay (s)	-	-	13.1	7.8	0	
HCM Lane LOS	-	-	B	A	A	
HCM 95th %tile Q(veh)	-	-	0.1	0.1	-	

Long Term AM Build
22: Car Dealership /Access #3 & Curve Ct

Intersection												
Int Delay, s/veh	1.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Vol, veh/h	0	35	2	18	204	25	1	0	6	18	0	0
Future Vol, veh/h	0	35	2	18	204	25	1	0	6	18	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	4	4	2	2	2	2	2	2
Mvmt Flow	0	38	2	20	222	27	1	0	7	20	0	0
Major/Minor	Major1			Major2			Minor1			Minor2		
Conflicting Flow All	249	0	0	40	0	0	315	328	39	319	316	236
Stage 1	-	-	-	-	-	-	39	39	-	276	276	-
Stage 2	-	-	-	-	-	-	276	289	-	43	40	-
Critical Hdwy	4.1	-	-	4.14	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.2	-	-	2.236	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1328	-	-	1557	-	-	638	591	1033	634	600	803
Stage 1	-	-	-	-	-	-	976	862	-	730	682	-
Stage 2	-	-	-	-	-	-	730	673	-	971	862	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1328	-	-	1557	-	-	631	582	1033	623	591	803
Mov Cap-2 Maneuver	-	-	-	-	-	-	631	582	-	623	591	-
Stage 1	-	-	-	-	-	-	976	862	-	730	672	-
Stage 2	-	-	-	-	-	-	719	663	-	965	862	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	0			0.5			8.8			11		
HCM LOS							A			B		
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	947	1328	-	-	1557	-	-	623				
HCM Lane V/C Ratio	0.008	-	-	-	0.013	-	-	0.031				
HCM Control Delay (s)	8.8	0	-	-	7.3	0	-	11				
HCM Lane LOS	A	A	-	-	A	A	-	B				
HCM 95th %tile Q(veh)	0	0	-	-	0	-	-	0.1				

Long Term PM Build
6: Elk River Rd & US-40

												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (veh/h)	38	658	47	107	1119	528	113	133	194	508	114	110
Future Volume (veh/h)	38	658	47	107	1119	528	113	133	194	508	114	110
Initial Q (Qb), veh	0	0	0	0	0	0	0	0	0	0	0	0
Ped-Bike Adj(A_pbT)	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Parking Bus, Adj	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Work Zone On Approach		No			No			No			No	
Adj Sat Flow, veh/h/ln	1856	1856	1856	1841	1841	1841	1841	1841	1841	1870	1870	1870
Adj Flow Rate, veh/h	40	693	0	114	1190	0	151	177	0	612	137	0
Peak Hour Factor	0.95	0.95	0.95	0.94	0.94	0.94	0.75	0.75	0.75	0.83	0.83	0.83
Percent Heavy Veh, %	3	3	3	4	4	4	4	4	4	2	2	2
Cap, veh/h	53	1578		139	915		176	372		433	424	
Arrive On Green	0.03	0.45	0.00	0.08	0.50	0.00	0.10	0.20	0.00	0.13	0.23	0.00
Sat Flow, veh/h	1767	3526	1572	1753	1841	1560	1753	1841	1560	3456	1870	1585
Grp Volume(v), veh/h	40	693	0	114	1190	0	151	177	0	612	137	0
Grp Sat Flow(s),veh/h/ln	1767	1763	1572	1753	1841	1560	1753	1841	1560	1728	1870	1585
Q Serve(g_s), s	2.8	16.7	0.0	7.9	61.5	0.0	10.5	10.5	0.0	15.5	7.6	0.0
Cycle Q Clear(g_c), s	2.8	16.7	0.0	7.9	61.5	0.0	10.5	10.5	0.0	15.5	7.6	0.0
Prop In Lane	1.00		1.00	1.00		1.00	1.00		1.00	1.00		1.00
Lane Grp Cap(c), veh/h	53	1578		139	915		176	372		433	424	
V/C Ratio(X)	0.75	0.44		0.82	1.30		0.86	0.48		1.41	0.32	
Avail Cap(c_a), veh/h	71	1578		227	915		180	372		433	424	
HCM Platoon Ratio	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Upstream Filter(I)	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	0.00
Uniform Delay (d), s/veh	59.5	23.5	0.0	56.1	31.1	0.0	54.8	43.6	0.0	54.1	39.9	0.0
Incr Delay (d2), s/veh	25.6	0.9	0.0	11.2	143.4	0.0	31.0	4.3	0.0	199.5	2.0	0.0
Initial Q Delay(d3),s/veh	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
%ile BackOfQ(50%),veh/ln	1.6	6.9	0.0	3.9	61.8	0.0	6.1	5.2	0.0	18.6	3.7	0.0
Unsig. Movement Delay, s/veh												
LnGrp Delay(d),s/veh	85.1	24.4	0.0	67.3	174.5	0.0	85.8	47.9	0.0	253.6	41.9	0.0
LnGrp LOS	F	C		E	F		F	D		F	D	
Approach Vol, veh/h		733	A		1304	A		328	A		749	A
Approach Delay, s/veh		27.7			165.1			65.3			214.9	
Approach LOS		C			F			E			F	
Timer - Assigned Phs	1	2	3	4	5	6	7	8				
Phs Duration (G+Y+Rc), s	14.3	59.9	20.0	29.5	8.2	66.0	16.9	32.6				
Change Period (Y+Rc), s	4.5	4.5	4.5	4.5	4.5	4.5	4.5	4.5				
Max Green Setting (Gmax), s	16.0	50.5	15.5	25.0	5.0	61.5	12.7	27.8				
Max Q Clear Time (g_c+I1), s	9.9	18.7	17.5	12.5	4.8	63.5	12.5	9.6				
Green Ext Time (p_c), s	0.1	4.9	0.0	0.6	0.0	0.0	0.0	0.6				
Intersection Summary												
HCM 6th Ctrl Delay			134.2									
HCM 6th LOS			F									
Notes												
Unsignalized Delay for [NBR, EBR, WBR, SBR] is excluded from calculations of the approach delay and intersection delay.												

Long Term PM Build
7: Shield Dr & Elk River Rd

Intersection						
Int Delay, s/veh	4.4					
Movement	EBL	EBT	WBT	WBR	SBL	SBR
Lane Configurations						
Traffic Vol, veh/h	60	110	76	216	107	8
Future Vol, veh/h	60	110	76	216	107	8
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #	-	0	0	-	0	-
Grade, %	-	0	0	-	0	-
Peak Hour Factor	53	53	74	74	81	81
Heavy Vehicles, %	0	0	4	4	2	2
Mvmt Flow	113	208	103	292	132	10
Major/Minor	Major1	Major2		Minor2		
Conflicting Flow All	395	0	-	0	683	249
Stage 1	-	-	-	-	249	-
Stage 2	-	-	-	-	434	-
Critical Hdwy	4.1	-	-	-	6.42	6.22
Critical Hdwy Stg 1	-	-	-	-	5.42	-
Critical Hdwy Stg 2	-	-	-	-	5.42	-
Follow-up Hdwy	2.2	-	-	-	3.518	3.318
Pot Cap-1 Maneuver	1175	-	-	-	415	790
Stage 1	-	-	-	-	792	-
Stage 2	-	-	-	-	653	-
Platoon blocked, %		-	-	-		
Mov Cap-1 Maneuver	1175	-	-	-	370	790
Mov Cap-2 Maneuver	-	-	-	-	370	-
Stage 1	-	-	-	-	706	-
Stage 2	-	-	-	-	653	-
Approach	EB	WB		SB		
HCM Control Delay, s	3	0		19.8		
HCM LOS	C					
Minor Lane/Major Mvmt	EBL	EBT	WBT	WBR	SBLn1	
Capacity (veh/h)	1175	-	-	-	384	
HCM Lane V/C Ratio	0.096	-	-	-	0.37	
HCM Control Delay (s)	8.4	0	-	-	19.8	
HCM Lane LOS	A	A	-	-	C	
HCM 95th %tile Q(veh)	0.3	-	-	-	1.7	

Long Term PM Build
9: US-40 & Curve Ct/Sunlight Dr

Intersection												
Int Delay, s/veh	6.9											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕		↗	↗		↗	↕	↗
Traffic Vol, veh/h	3	0	152	0	2	17	165	1767	0	7	1345	7
Future Vol, veh/h	3	0	152	0	2	17	165	1767	0	7	1345	7
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	50	-	-	50	-	0
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	60	60	60	69	69	69	94	94	94	88	88	88
Heavy Vehicles, %	0	0	0	0	0	0	4	4	4	3	3	3
Mvmt Flow	5	0	253	0	3	25	176	1880	0	8	1528	8
Major/Minor	Minor2		Minor1		Major1		Major2					
Conflicting Flow All	3790	3776	764	3012	3784	1880	1536	0	0	1880	0	0
Stage 1	1544	1544	-	2232	2232	-	-	-	-	-	-	-
Stage 2	2246	2232	-	780	1552	-	-	-	-	-	-	-
Critical Hdwy	7.3	6.5	6.9	7.3	6.5	6.2	4.16	-	-	4.145	-	-
Critical Hdwy Stg 1	6.5	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.5	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	2.238	-	-	2.2285	-	-
Pot Cap-1 Maneuver	~ 2	4	351	7	4	90	423	-	-	314	-	-
Stage 1	122	178	-	57	80	-	-	-	-	-	-	-
Stage 2	56	80	-	359	176	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	-	2	351	1	~ 2	90	423	-	-	314	-	-
Mov Cap-2 Maneuver	-	2	-	1	~ 2	-	-	-	-	-	-	-
Stage 1	71	174	-	33	47	-	-	-	-	-	-	-
Stage 2	22	47	-	97	172	-	-	-	-	-	-	-
Approach	EB		WB		NB		SB					
HCM Control Delay, s			\$ 840.1		1.7		0.1					
HCM LOS	-		F									
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	WBLn1	SBL	SBT	SBR				
Capacity (veh/h)	423	-	-	-	16	314	-	-				
HCM Lane V/C Ratio	0.415	-	-	-	1.721	0.025	-	-				
HCM Control Delay (s)	19.4	-	-	-	\$ 840.1	16.8	-	-				
HCM Lane LOS	C	-	-	-	F	C	-	-				
HCM 95th %tile Q(veh)	2	-	-	-	4	0.1	-	-				
Notes												
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined		*: All major volume in platoon						

Long Term PM Build
16: Elk River Rd & Curve Plaza/Access #1

Intersection													
Int Delay, s/veh	37												
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR	
Lane Configurations													
Traffic Vol, veh/h	257	0	45	0	0	16	110	166	0	0	72	193	
Future Vol, veh/h	257	0	45	0	0	16	110	166	0	0	72	193	
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0	
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free	
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None	
Storage Length	-	-	-	-	-	0	-	-	-	-	-	0	
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-	
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-	
Peak Hour Factor	88	88	88	92	92	92	59	59	59	85	85	85	
Heavy Vehicles, %	1	1	1	0	0	0	4	4	4	1	1	1	
Mvmt Flow	292	0	51	0	0	17	186	281	0	0	85	227	
Major/Minor	Minor2		Minor1		Major1		Major2						
Conflicting Flow All	747	738	85	-	-	281	312	0	0	-	-	0	
Stage 1	85	85	-	-	-	-	-	-	-	-	-	-	
Stage 2	662	653	-	-	-	-	-	-	-	-	-	-	
Critical Hdwy	7.11	6.51	6.21	-	-	6.2	4.14	-	-	-	-	-	
Critical Hdwy Stg 1	6.11	5.51	-	-	-	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.11	5.51	-	-	-	-	-	-	-	-	-	-	
Follow-up Hdwy	3.509	4.009	3.309	-	-	3.3	2.236	-	-	-	-	-	
Pot Cap-1 Maneuver	330	347	977	0	0	763	1237	-	-	0	-	-	
Stage 1	925	826	-	0	0	-	-	-	-	0	-	-	
Stage 2	453	465	-	0	0	-	-	-	-	0	-	-	
Platoon blocked, %								-	-	-	-	-	
Mov Cap-1 Maneuver	~ 279	285	977	-	-	763	1237	-	-	-	-	-	
Mov Cap-2 Maneuver	~ 279	285	-	-	-	-	-	-	-	-	-	-	
Stage 1	760	826	-	-	-	-	-	-	-	-	-	-	
Stage 2	364	382	-	-	-	-	-	-	-	-	-	-	
Approach	EB		WB		NB		SB						
HCM Control Delay, s	117.9		9.8		3.4		0						
HCM LOS	F		A										
Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1WBLn1	SBT	SBR							
Capacity (veh/h)	1237	-	-	312	763	-	-						
HCM Lane V/C Ratio	0.151	-	-	1.1	0.023	-	-						
HCM Control Delay (s)	8.4	0	-	117.9	9.8	-	-						
HCM Lane LOS	A	A	-	F	A	-	-						
HCM 95th %tile Q(veh)	0.5	-	-	13.5	0.1	-	-						
Notes													
~: Volume exceeds capacity		\$: Delay exceeds 300s		+: Computation Not Defined				*: All major volume in platoon					

Long Term PM Build
20: Shield Dr & Access #2

Intersection						
Int Delay, s/veh	0.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations						
Traffic Vol, veh/h	3	2	291	2	23	198
Future Vol, veh/h	3	2	291	2	23	198
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage, #	0	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	79	79	74	74
Heavy Vehicles, %	2	2	4	4	1	1
Mvmt Flow	3	2	368	3	31	268
Major/Minor	Minor1	Major1		Major2		
Conflicting Flow All	700	370	0	0	371	0
Stage 1	370	-	-	-	-	-
Stage 2	330	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.11	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.209	-
Pot Cap-1 Maneuver	405	676	-	-	1193	-
Stage 1	699	-	-	-	-	-
Stage 2	728	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	392	676	-	-	1193	-
Mov Cap-2 Maneuver	392	-	-	-	-	-
Stage 1	699	-	-	-	-	-
Stage 2	705	-	-	-	-	-
Approach	WB	NB		SB		
HCM Control Delay, s	12.7	0		0.8		
HCM LOS	B					
Minor Lane/Major Mvmt	NBT	NBRWBLn1		SBL	SBT	
Capacity (veh/h)	-	-	471	1193	-	
HCM Lane V/C Ratio	-	-	0.012	0.026	-	
HCM Control Delay (s)	-	-	12.7	8.1	0	
HCM Lane LOS	-	-	B	A	A	
HCM 95th %tile Q(veh)	-	-	0	0.1	-	

Long Term PM Build
22: Car Dealership /Access #3 & Curve Ct

Intersection												
Int Delay, s/veh	1.4											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		↕			↕			↕			↕	
Traffic Vol, veh/h	0	120	2	17	151	22	3	0	26	9	0	0
Future Vol, veh/h	0	120	2	17	151	22	3	0	26	9	0	0
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Free	Free	Free	Free	Free	Free	Stop	Stop	Stop	Stop	Stop	Stop
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage, #	-	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	92	92	92	92	92	92	92	92	92	92	92	92
Heavy Vehicles, %	0	0	0	4	4	4	2	2	2	2	2	2
Mvmt Flow	0	130	2	18	164	24	3	0	28	10	0	0
Major/Minor	Major1		Major2			Minor1			Minor2			
Conflicting Flow All	188	0	0	132	0	0	343	355	131	357	344	176
Stage 1	-	-	-	-	-	-	131	131	-	212	212	-
Stage 2	-	-	-	-	-	-	212	224	-	145	132	-
Critical Hdwy	4.1	-	-	4.14	-	-	7.12	6.52	6.22	7.12	6.52	6.22
Critical Hdwy Stg 1	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Critical Hdwy Stg 2	-	-	-	-	-	-	6.12	5.52	-	6.12	5.52	-
Follow-up Hdwy	2.2	-	-	2.236	-	-	3.518	4.018	3.318	3.518	4.018	3.318
Pot Cap-1 Maneuver	1398	-	-	1441	-	-	611	571	919	598	579	867
Stage 1	-	-	-	-	-	-	873	788	-	790	727	-
Stage 2	-	-	-	-	-	-	790	718	-	858	787	-
Platoon blocked, %		-	-		-	-						
Mov Cap-1 Maneuver	1398	-	-	1441	-	-	604	563	919	573	571	867
Mov Cap-2 Maneuver	-	-	-	-	-	-	604	563	-	573	571	-
Stage 1	-	-	-	-	-	-	873	788	-	790	717	-
Stage 2	-	-	-	-	-	-	779	708	-	832	787	-
Approach	EB		WB			NB			SB			
HCM Control Delay, s	0		0.7			9.3			11.4			
HCM LOS						A			B			
Minor Lane/Major Mvmt	NBLn1	EBL	EBT	EBR	WBL	WBT	WBR	SBLn1				
Capacity (veh/h)	872	1398	-	-	1441	-	-	573				
HCM Lane V/C Ratio	0.036	-	-	-	0.013	-	-	0.017				
HCM Control Delay (s)	9.3	0	-	-	7.5	0	-	11.4				
HCM Lane LOS	A	A	-	-	A	A	-	B				
HCM 95th %tile Q(veh)	0.1	0	-	-	0	-	-	0.1				