ABBREVIATIONS STRUCTURAL NOTES **SECTION 3 - CONCRETE (CONT.): SECTION 1 - GENERAL REQUIREMENTS:** LIGHTWEIGHT CONCRETE ΑT AND MOMENT 3.1 - CAST-IN-PLACE CONCRETE (CONT): NUMBER, POUND 1.1 - CONTRACTOR INSTRUCTIONS: MAXIMUM MOMENT CONNECTION ROUND. DIAMETER A. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE STRUCTURE IS E. REINFORCING DETAILING: MECH **ADDITIONAL** MECHANICAL FULLY COMPLETED. IT IS SOLELY THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION . ALL REINFORCING SHALL BE DETAILED IN ACCORDANCE WITH ACI 318 "BUILDING CODE ARCHITECTURALLY EXPOSED STR. STEE MEZZ MEZZANINE PROCEDURES, SEQUENCE, MEANS, AND METHODS TO ENSURE THE STABILITY OF THE BUILDING REQUIREMENTS FOR STRUCTURAL CONCRETE" AND ACI 315 "DETAILS AND DETAILING OF ABOVE FINISHED FLOOR **MANUFACTURER** AND ITS COMPONENT PARTS AND THE ADEQUACY OF TEMPORARY OR INCOMPLETE CONNECTIONS CONCRETE REINFORCEMENT" PER PUBLICATION SP-66. HORIZONTAL MOMENT AIR HANDLING UNIT SPLICES OF REINFORCING BARS ARE PERMITTED ONLY AS DETAILED OR AUTHORIZED BY THE DURING ERECTION. SECTION 2.0: DESIGN LOADS ALTERNATE MINIMUM B. THE STRUCTURAL ENGINEER IS NOT ENGAGED IN CONSTRUCTION ENGINEERING. THE STRUCTURAL ENGINEER. LAP SPLICES, WHERE PERMITTED, SHALL BE CLASS 'B', UNLESS APPROX **APPROXIMATE** MISC MISCELLANEOUS CONTRACTOR IS RESPONSIBLE FOR FURNISHING, INSTALLING, AND REMOVING ALL TEMPORARY SHOWN OTHERWISE IN DETAILS, NOTES, OR SCHEDULES. ARCHITECT, ARCHITECTURAL METAL BRACING AND SUPPORTS NECESSARY TO FACILITATE BUILDING ERECTION. ERECTION AID BRACING 3. REFER TO DEVELOPMENT LENGTH AND LAP SPLICE TYPICAL DETAILS AND SCHEDULES ON ALLOWABLE STRENGTH DESIGN MWFRS MAIN WIND FORCE RESISTING SYSTEM SHEET CS-002 FOR DEVELOPMENT LENGTHS, SPLICE LENGTHS, AND ADDITIONAL AND SUPPORTS ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS. BUILDING NEAR FACE C. $\,$ THE CONTRACTOR'S ERECTION METHODS SHALL CONSIDER THE EFFECTS OF THERMAL REQUIREMENTS. NON-REDUCIBLE BEAM 4. MAKE ALL REINFORCING BARS CONTINUOUS AROUND CORNERS OR PROVIDE CORNER BARS MOVEMENTS OF STRUCTURAL ELEMENTS THAT MAY OCCUR DURING THE CONSTRUCTION PHASE. **BOTTOM OF** NOT IN CONTRACT D. IT IS SOLELY THE RESPONSIBILITY OF EACH CONTRACTOR TO FOLLOW APPLICABLE SAFETY CODES OF EQUAL SIZE AND SPACING. SEE TYPICAL DETAILS. NEAR SIDE WHERE ALL REINFORCING WITHIN A STRUCTURAL ELEMENT WILL BE SPLICED AT THE SAME AND REGULATIONS DURING ALL PHASES OF CONSTRUCTION. THE ENGINEER IS NOT ENGAGED IN. BEARING NOT TO SCALE AND DOES NOT SUPERVISE, CONSTRUCTION. LOCATION THE SPLICES SHALL BE STAGGERED UNLESS SHOWN OTHERWISE IN DETAILS OR **BETWEEN** NWC NORMAL WEIGHT CONCRETE 1.2 - USE OF DRAWINGS AND COORDINATION: CAMBER **OVERALL HEIGHT** 6. AT ENDS OF BEAMS, SLABS, JOISTS, WALLS, AND GRADE BEAMS TERMINATE TOP CANTIL CANTILEVER ON CENTER A. DRAWINGS SHALL NOT BE SCALED. CONTACT THE ENGINEER IN THE EVENT THAT DIMENSIONS REINFORCING WITH STANDARD HOOKS, UNLESS SHOWN OTHERWISE. COLD FORMED METAL FRAMING **OUTSIDE DIAMETER** NECESSARY TO PROCEED WITH THE WORK ARE MISSING FROM THE DRAWINGS. PROVIDE THE FOLLOWING CLEAR COVERS MEASURED FROM SURFACE OF CONCRETE TO OPH OF CAST-IN-PLACE OPPOSITE HAND B. ANY DIFFERENCES BETWEEN THE ARCHITECTURAL AND STRUCTURAL DRAWINGS SHALL BE OUTER FACE OF REBAR UNLESS SHOWN OTHERWISE ON PLANS OR IN DETAILS: OPNG **COMPLETE JOINT PENETRATION** OPFNING REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. **OPPOSITE DESCRIPTION OF REBAR USE** CENTERLINE C. CONFLICTS WITHIN THE STRUCTURAL DRAWINGS OR BETWEEN THE STRUCTURAL DRAWINGS, COVER (INCHES) **ORIENTED STRAND BOARD** CLEAR COVER NOTES, AND/OR SPECIFICATIONS SHALL BE REPORTED TO THE STRUCTURAL ENGINEER FOR **CONCRETE POURED AGAINST EARTH** CMU CONCRETE MASONRY UNIT OVERSIZE, OVERSIZED RESOLUTION BEFORE PROCEEDING WITH THE WORK. CONCRETE POURED IN FORMS AND EXPOSED TO WEATHER OR EARTH COL COLUMN AXIAL LOAD D. EXISTING CONDITIONS: THE STRUCTURAL DRAWINGS HAVE BEEN PREPARED USING AVAILABLE 1 1/2" #5 BAR OR SMALLER CONC CONCRETE POWDER ACTUATED FASTENER INFORMATION OF EXISTING CONDITIONS. NO ATTEMPT HAS BEEN MADE TO VERIFY EXISTING CONN CONNECTION PARALLEL CONDITIONS AGAINST INFORMATION RECEIVED FROM THE CLIENT OR OTHER SOURCES. IT IS THE CONCRETE POURED IN FORMS AND EXPOSED TO WEATHER OR EARTH CONSTR CONSTRUCTION PRECAST, PRECAST CONCRETE RESPONSIBILITY OF THE CONTRACTOR TO COMPARE THE EXISTING CONDITIONS TO THE #6 BAR OR LARGER POUNDS PER CUBIC FOOT CONTINUOUS CONT INFORMATION SHOWN ON THE DRAWINGS AND NOTIFY THE ARCHITECT OF ANY DIFFERENCES SLABS, WALLS, AND JOISTS (UNLESS NOTED OTHERWISE) 3/4" POUNDS PER CUBIC YARD COORD COORDINATE BEFORE PROCEEDING WITH THE WORK. PERP COVFR PERPENDICULAR . THE STRUCTURAL DRAWINGS ARE INTENDED TO BE USED IN CONJUNCTION WITH DRAWINGS FROM K. PLACING OF REINFORCING: PROVIDE ALL ACCESSORIES AND ADDITIONAL SUPPORT BARS BAR DIAMETER PLATE CIVIL, LANDSCAPE, ARCHITECTURAL, INTERIORS, MECHANICAL, ELECTRICAL, PLUMBING, AND ANY NECESSARY TO SUPPORT AND SECURE REINFORCING AT POSITIONS SHOWN ON DRAWINGS AND DEFORMED BAR ANCHOR POUNDS PER LINEAR FOOT OTHER DISCIPLINE. THE REQUIREMENTS OF ALL DISCIPLINES SHALL BE REFLECTED IN THE TO MAINTAIN REQUIRED CONCRETE COVER. DCW PARTIAL JOINT PENETRATION DEMAND CRITICAL WELD SUBMITTED SHOP DRAWINGS AND THE CONSTRUCTED WORK. L. CONTROL JOINTS IN CONCRETE: PRELIM PRELIMINARY PROVIDE CONTROL JOINTS IN CONCRETE WALLS AT A MAXIMUM SPACING OF 30'-0" ON CENTER. 1.3 - SPECIAL INSPECTION REQUIREMENTS AND QUALITY CONTROL: PROP DIAMETER PROPERTY SEAL CONTROL JOINTS EXPOSED TO EARTH OR WEATHER WITH JOINT SEALANT. A. SPECIAL INSPECTIONS SHALL BE PROVIDED IN ACCORDANCE WITH THE IBC. INCLUDING SECTION 110 PSF DRAWING POUNDS PER SQUARE FOOT M. CONSTRUCTION JOINTS: AND CHAPTER 17. SEE SCHEDULE THIS SHEET FOR STRUCTURAL SPECIAL INSPECTION POUNDS PER SQUARE INCH REINFORCING SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS, UNLESS DETAILED REQUIREMENTS **EACH FACE** PARALLEL STRAND LUMBER OTHERWISE. ALTERNATE SPLICE CONFIGURATIONS SHALL BE SUBMITTED FOR ENGINEER 3. THE CONTRACTOR SHALL MAINTAIN A LOG OF DISCREPANCIES NOTED BY THE INDEPENDENT **EXPANSION JOINT** POST-TENSIONED, POST-TENSIONING REVIEW AND APPROVAL PRIOR TO PROCEEDING WITH WORK. TESTING AGENCY FOR THE DURATION OF THE PROJECT. EACH ITEM IN THE LOG SHALL BE **ELEVATOR** QUANTITY N. CONSTRUCTION TOLERANCES SHALL CONFORM TO REQUIREMENTS GIVEN IN ACI 117. REFERENCED BY AN ITEM NUMBER WITH AN APPROPRIATE DESCRIPTION OF THE DISCREPANCY, THE EMBEDMENT, EMBEDDED REACTION CONTRACTOR SHALL FURTHER CONSIDER EFFECTS OF SLAB SHRINKAGE DUE TO CONCRETE DATE THE DISCREPANCY WAS NOTED. A DESCRIPTION OF THE CORRECTIVE ACTION AND THE DATE REDUCIBLE **ENGR ENGINEER** CURING AND APPLICATION OF POST-TENSIONING FORCES, AS APPLICABLE, AND COORDINATE WITH THE CORRECTIVE ACTION WAS TAKEN. **EQUAL** RADIUS SLAB EDGES AND OTHER DIMENSIONS THAT MAY BE AFFECTED. C. PRE-CONSTRUCTION MEETINGS SHALL BE CONDUCTED BY THE CONTRACTOR PRIOR TO **EQUIP EQUIPMENT** REFER. REFERENCE O. CONCRETE PLACEMENT: COMMENCEMENT OF EACH PRIMARY STRUCTURAL SYSTEM, INCLUDING, BUT NOT LIMITED TO, **EQUIV EQUIVALENT** REINF REINFORCEMENT, REINFORCING 1. CONSOLIDATE ALL CONCRETE DURING PLACEMENT AND THOROUGHLY WORK AROUND REM FOUNDATIONS AND SLABS-ON-GRADE. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND **EACH WAY** REMAINDER REINFORCING AND EMBEDDED ITEMS AND INTO CORNERS OF FORMS FOLLOWING ACI ENGINEER A MINIMUM OF ONE WEEK IN ADVANCE OF EACH PRE-CONSTRUCTION MEETING. REQ'D **EXIST EXISTING** REQUIRED **EXPANSION** REVISION WHEN CONCRETE PLACEMENT IS INTERRUPTED, NOTIFY STRUCTURAL ENGINEER FOR **SECTION 2 - FOUNDATION CONSTRUCTION: EXTERIOR ROUGH OPENING** RECOMMENDATIONS. UNLESS DIRECTED OTHERWISE, PROVIDE A CONSTRUCTION JOINT BY **FABRICATE ROOF TOP UNIT** INTENTIONALLY ROUGHENING THE CONCRETE SURFACE TO 1/4" AMPLITUDE. COAT THE JOINT CONCRETE STRENGTH SLIP CRITICAL 2.1 - FOUNDATION DESIGN CRITERIA: WITH AN APPROVED BONDING AGENT PRIOR TO POURING CONCRETE ON THE OPPOSITE SIDE SCHED MASONRY STRENGTH SCHEDULE, SCHEDULED A. GEOTECHNICAL FOUNDATION DESIGN CRITERIA WAS TAKEN FROM RECOMMENDATIONS OF THE JOINT. SDS FLOOR DRAIN SELF DRILLING SCREW ESTABLISHED IN SOILS REPORT PREPARED BY NORTHWEST COLORADO CONSULTANTS, INC., SECT SECTION 3.2 - POST-INSTALLED ANCHORS INTO CONCRETE: FOUNDATION UNDER PROJECT NUMBER 23-13103, DATED MARCH 29, 2024. FAR FACE SHT SHEET A. PROVIDE POST-INSTALLED CONCRETE ANCHORS AS SHOWN IN THE PLANS, DETAILS, AND B. ALLOWABLE FOOTING PRESSURE USED FOR DESIGN: FINISH, FINISHED SIMILAR SPECIFICATIONS. ALTERNATE PRODUCTS TO BE SUBMITTED FOR REVIEW AND APPROVAL BY THE .2,500 PSF . FOUNDATIONS PLACED OVER NATURAL SAND/GRAVEL SLBB SHORT LEG BACK TO BACK FI OOR STRUCTURAL ENGINEER OF RECORD. BASIS OF DESIGN SHALL BE ANCHORS AND FASTENERS AS 2. FOUNDATIONS PLACED DIRECTLY ON BEDROCK... .5,000 PSF SLFS FAR SIDE SEISMIC LOAD FORCE SYSTEM MANUFACTURED BY HILTI, INC. OR AS OTHERWISE SPECIFIED. C. MINIMUM DEAD LOAD FOOTING PRESSURE USED FOR DESIGN: SOG FOOTING SLAB ON GRADE B. ANCHOR CAPACITY USED IN DESIGN SHALL BE BASED ON THE TECHNICAL DATA PUBLISHED BY HILTI, FOUNDATIONS PLACED OVER NATURAL SAND/GRAVEL FIELD VERIFY SPACING 2. FOUNDATIONS PLACED DIRECTLY ON BEDROCK.. INC. OR SUCH OTHER METHOD AS APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. .1,000 PSF SPEC YIELD STRENGTH **SPECIFICATION** SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE D. ALLOWABLE COEFFICIENT OF FRICTION FOR RESISTING SLIDING = 0.40. SPRT GALVANIZE, GALVANIZED SUPPORT STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS . MINIMUM FROST DEPTH IS FOUR FEET. BOTTOM OF FOOTING SHALL BE AT OR BELOW FROST **GENERAL** SQUARE DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE DEPTH AS MEASURED FROM GRADE. GLUED LAMINATED BEAM STAINLESS STEEL VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN LATERAL EARTH PRESSURE FOR ON-SITE MATERIAL (DOES NOT INCLUDE HYDROSTATIC OR GRADE SHORT SLOTTED HOLE PARALLEL ICC-ESR REPORT SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE FOR SEISMIC USES, SURCHARGE LOADS) HORIZONTAL SHORT SLOTTED HOLE TRANSVERSE LOAD RESISTANCE, INSTALLATION CATEGORY, AND AVAILABILITY OF COMPREHENSIVE INSTALLATION 1. ON-SITE SOILS: HANGER STANDARD INSTRUCTIONS. ADHESIVE ANCHORS SHALL CONSIDER CREEP AS WELL AS BOTH IN-SERVICE AND .55 PCF a. AT-REST PRESSURE HEADED ANCHOR STUD STIF STIFFENER INSTALLATION TEMPERATURES. ..45 PCF b. ACTIVE PRESSURE STIR **HOLLOW STRUCTURAL SECTION** STIRRUP ANCHORS ARE TO BE INSTALLED PER THE MANUFACTURER'S WRITTEN INSTRUCTIONS, AS INCLUDED 2. FREE-DRAINING GRANULAR FILL: INSIDE DIAMETER STEEL IN THE ANCHOR PACKAGING. ALL ANCHORS ARE TO BE INSTALLED IN DRY CONDITIONS IN HAMMER .45 PCF a. AT-REST PRESSURE STRUCT STRUCTURE, STRUCTURAL INFORMATION DRILLED HOLES, UNLESS PRIOR WRITTEN APPROVAL HAS BEEN RECEIVED FROM THE STRUCTURAL 35 PCF b. ACTIVE PRESSURE INTERIOR SHEAR WALL ENGINEER OF RECORD ALLOWABLE PASSIVE PRESSURE. 250 PCF SYMM **SYMMETRIC** JOINT EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC 2.2 - FOUNDATION SLAB: TORSION ANCHOR LOCATIONS. EXISTING REINFORCING IS NOT TO BE CUT WITHOUT PRIOR WRITTEN KIP, KIPS (1 KIP = 1000 POUNDS) TO BE DETERMINED A. FOUNDATION SLAB SHALL BEAR ON UNDISTURBED SOIL OR COMPACTED STRUCTURAL FILL GOOD APPROVAL FROM THE STRUCTURAL ENGINEER OF RECORD. KIPS PER LINEAR FOOT THD THREAD, THREADED FOR THE ALLOWABLE BEARING PRESSURE AS INDICATED IN SECTION 2.1. E. ALL PERSONS RESPONSIBLE FOR THE INSTALLATION OF POST-INSTALLED MECHANICAL ANCHORS KIPS PER SQUARE FOOT TOP OF B. EXCAVATIONS SHALL BE KEPT FREE OF WATER AT ALL TIMES. EXCAVATIONS SHALL BE INSPECTED ARE TO BE PROPERLY TRAINED BY A MANUFACTURER'S REPRESENTATIVE OR AN ALTERNATE KIPS PER SQUARE INCH T.O.C TOP OF CONCRETE AND APPROVED BY THE GEOTECHNICAL ENGINEER IMMEDIATELY PRIOR TO PLACING FOUNDATION EQUIVALENT TRAINING FOR EACH TYPE OF INSTALLATION INCLUDED IN THE PROJECT. T.O.S TOP OF SLAB, TOP OF STEEL POUNDS CONCRETE. ALL PERSONS RESPONSIBLE FOR THE INSTALLATION OF ADHESIVE ANCHORS SHALL BE CERTIFIED **TRANS** DEVELOPMENT LENGTH TRANSVERSE C. AREAS OF LOOSE OR SOFT SOIL MATERIAL ENCOUNTERED AT THE BOTTOM OF FOOTING IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR LONG LEG BACK TO BACK TYP TYPICAL EXCAVATION SHALL BE REMOVED AND REPLACED WITH NON-EXPANSIVE STRUCTURAL FILL OR EQUIVALENT. PROGRAM CERTIFICATES OF INSTALLATION PERSONNEL SHALL BE SUBMITTED TO THE UNLESS NOTED OTHERWISE LONG LEG HORIZONTAL U.N.O. CONCRETE FLOW-FILL IN ACCORDANCE WITH EARTHWORK REQUIREMENTS AND AS APPROVED BY STRUCTURAL ENGINEER OF RECORD. VERT, V LONG LEG VERTICAL VERTICAL THE GEOTECHNICAL ENGINEER. LONG LONGITUDE, LONGITUDINAL WIDE FLANGE D. EARTH CUTS SHALL NOT BE USED AS FORMWORK FOR FOOTINGS WITHOUT APPROVAL FROM LOAD AND RESISTANCE FACTOR DESIGN **WORK POINT** GEOTECHNICAL ENGINEER AND STRUCTURAL ENGINEER. WATERSTOP LONG SIDE HORIZONTAL 2.3 - GENERAL FOUNDATION REQUIREMENTS: LONG SLOTTED HOLE PARALLEL WEIGHT WELDED WIRE REINFORCEMENT LONG SLOTTED HOLE TRANSVERSE WWR A. PREPARE SUB-BASE BELOW ALL SLAB-ON-GRADE CONSTRUCTION PER THE GEOTECHNICAL REPORT EXTRA STRONG LONG SIDE VERTICAL UNLESS NOTED OTHERWISE IN PLANS OR DETAILS. OVER-EXCAVATE BELOW FOUNDATION SLAB AND LAMINATED VENEER LUMBER DOUBLE EXTRA STRONG REPLACE WITH COMPACTED STRUCTURAL FILL SHOULD SOFT, LOOSE SOIL BE ENCOUNTERED AS INDICATED IN THE GEOTECHNICAL REPORT. B. ALL EARTHWORK AND SITE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE **SYMBOLS & PATTERNS** PROJECT SPECIFICATIONS AND GEOTECHNICAL REPORT. ALL EXCAVATION AND BACKFILL OPERATIONS SHALL BE OBSERVED BY THE GEOTECHNICAL ENGINEER. ELEVATION MARKER CAST-IN-PLACE STRUCTURAL C. $\,$ EXCAVATING ADJACENT TO EXISTING STRUCTURES SHALL ONLY BE DONE UNDER THE SUPERVISION STEEL MASONRY CONCRETE OF THE GEOTECHNICAL ENGINEER. UNDER NO CIRCUMSTANCES SHALL EXCAVATIONS UNDERMINE EXISTING FOUNDATIONS. PROVIDE SHORING AND/OR ADEQUATE SOIL LAYBACK AS NECESSARY TO **PRECAST** ALUMINUM COBBLE STEP INDICATOR ENSURE THE STABILITY OF EXISTING STRUCTURES. CONCRETE **SECTION 3 - CONCRETE:** PLYWOOD EARTH SLOPE INDICATOR GROUT 3.1 - CAST-IN-PLACE CONCRETE: UNIT >. 4. WOOD GRAVEL FOOTING STER MASONRY ∐n∐ INDICATOR PERFORMED IN ACCORDANCE WITH THE REQUIREMENTS SPECIFIED IN ACI 318 AND ACI 301, CODE → BRICK REFERENCED EDITIONS, EXCEPT AS MODIFIED BY THE PROJECT CONTRACT DOCUMENTS. CMU / STUD WALL SAND BRICK LEDGE STEP MASONRY FRAMING THE MINIMUM STRENGTHS AS FOLLOWS: INDICATOR **EXPLANATION OF DRAWING CALL-OUTS ON PLANS:** STRUCTURAL SHEET --DETAIL, SECTION, PARTIAL WHERE DETAIL, SECTION, PLAN, OR ELEVATION IDENTIFIER # PARTIAL PLAN, OR **ELEVATION IS DRAWN** S-XXX REFER TO ACT 318 FOR ADDITIONAL INFORMATION ON DURABILITY REQUIREMEN C. ALL CONCRETE IN CONTACT WITH ON-SITE SOILS SHALL CONTAIN TYPE IL CEMENT. D. ALL REINFORCING STEEL IN CONCRETE SHALL HAVE A MINIMUM YIELD STRENGTH OF 60,000 PSI AND SHALL MEET THE REQUIREMENTS OF ASTM A615 OR ASTM A706. REINFORCING SHOWN AS GRADE 75 SHALL HAVE A MINIMUM YIELD STRENGTH OF 75,000 PSI AND MEET THE REQUIREMENTS

STRUCTURAL DESIGN CRITERIA **SECTION 1.0: CODES AND STANDARDS:**

.100 PSF

. 250 PSF

.6000 L

.118 PS

. 0.333

115 MPF

A. 2021 INTERNATIONAL BUILDING CODE (IBC-21).

a. SURFACE PAVING / FINISH ASSEMBLY.

b. HORIZONTAL VEHICLE IMPACT LOAD..

SEISMIC IMPORTANCE FACTOR. le...

1. MAPPED SPECTRAL ACCELERATION (SHORT PERIOD), Ss.

ALLOWABLE STRESS DESIGN WIND SPEED, Vasd.

2. MAPPED SPECTRAL ACCELERATION (1 SECOND PERIOD), S1.

SOIL SITE CLASS *CONFIRMED BY GEOTECHNICAL REPORT*

A. GRAVITY DESIGN LOADS:

1. GROUND SNOW LOAD, Pa...

7. SEISMIC DESIGN CATEGORY.

BASIC DESIGN WIND SPEED, V..

3. WIND IMPORTANCE FACTOR, IW

4. WIND EXPOSURE CATEGORY

DEAD LOADS:

LIVE LOADS:

SNOW LOADS:

WIND LOADS

SEISMIC LOADS:

B. 2023 ROUTT COUNTY AMENDMENTS TO THE 2021 INTERNATIONAL BUILDING CODE.

a. SIDEWALK AND VEHICULAR DRIVEWAYS SUBJECT TO TRUCKING.

DESIGN SPECTRAL ACCELERATION (SHORT PERIOD), S_{DS} *ROUTT COUNTY MAX*

6. DESIGN SPECTRAL ACCELERATION (1 SECOND PERIOD), SD1*SITE SPECIFIC VALUE*...

MINIMUM DESIGN LOADS AND ASSOCIATED CRITERIA FOR BUILDINGS AND OTHER STRUCTURES (ASCE

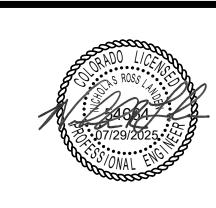
D. CONCRETE DESIGN - BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-19).

ORTIS STRUCTURAL, LLC

7935 E. PRENTICE AVE., SUITE 305 GREENWOOD VILLAGE, CO 80111 P: 720.593.3800

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STRUCTURAL FORTIS STRUCTURAL ENGINEER: LLC

ISSUES & REVISIONS: Date Description

DATE:

DRAWN BY:

SHEET TITLE:

WEST PROMENADE **SLAB - NOTES**

02/12/25

A. ALL CONCRETE WORK INCLUDING FABRICATION AND PLACEMENT OF REINFORCING SHALL BE

3. $\,$ CONCRETE MIXES SHALL SATISFY THE REQUIREMENTS GIVEN IN THE TABLE BELOW AND MEET

DESCRIPTION OF	28 DAY	DURABILITY EXPOSURE CLASS REQUIREMENTS				
CONCRETE USE	STRENGTH (PSI)	FREEZE/THAW	SULFATE	PERMEABILITY	CORROSIO	
FDN SLAB AND WALL	5,000	F3	S2	W0	C2	
NOTES: 1. ALL CONCRETE IS NORMAL WEIGHT CONCRETE = 145 PCF, U.N.O. 2. REFER TO ACL 318 FOR ADDITIONAL INFORMATION ON DURABILITY REQUIREMENTS						

-WALL VERTICAL REINFORCING, RE: PLAN

DEVELOPMENT AND SPLICE LENGTHS FOR GRADE 60 REINFORCING BAR IN TENSION NOTES: TO THE CLASS A LAP SPLICE LENGTH. OF FRESH CONCRETE BELOW THEM. SMALLER BAR. FOR SPLICE LENGTHS. CONCRETE MIXES.

ALL LENGTHS IN TABLE ARE PROVIDED IN INCHES. LAP SPLICES OF REINFORCING BARS SHALL BE CLASS B SPLICES TYPICAL UNLESS NOTED OTHERWISE. REINFORCING BAR DEVELOPMENT LENGTH (Ld) IS EQUAL BOTTOM HORIZ. = HORIZONTAL BARS WITH 12" OR LESS OTHER HORIZ. = HORIZONTAL BARS WITH MORE THAN 12" OF FRESH CONCRETE PLACED BELOW THEM.

WHEN BARS OF DIFFERENT SIZE ARE LAP SPLICED IN TENSION, SPLICE LENGTH SHALL BE THE LARGER OF Ld OF LARGER BAR AND TENSION LAP SPLICE LENGTH OF

LENGTHS PROVIDED IN THIS TABLE ARE NOT APPLICABLE TO EPOXY-COATED BARS; CONTACT STRUCTURAL ENGINEER FOR EPOXY-COATED BARS. FOR BUNDLED BARS CONTAINING TWO BARS PER BUNDLE, NO INCREASE IN LENGTHS IS REQUIRED. FOR DETERMINING SPACING, COVER, AND CONFINEMENT A UNIT OF BUNDLED BARS SHALL BE TREATED AS A SINGLE BAR OF A DIAMETER DERIVED FROM THE EQUIVALENT TOTAL AREA AND HAVING A CENTROID THAT COINCIDES WITH THAT OF THE BUNDLED BARS. FOR BUNDLED BARS CONTAINING THREE OR MORE BARS PER BUNDLE, CONTACT STRUCTURAL ENGINEER THIS TABLE IS FOR NORMAL WEIGHT CONCRETE,

CONTACT STRUCTURAL ENGINEER FOR LIGHTWEIGHT

TABLE VALUES ASSUME MINIMUM BAR COVER (MEASURED FROM EDGE OF CONCRETE TO EDGE OF BAR) = 3/4" AND MINIMUM BAR SPACING (MEASURED FROM BAR CENTER TO BAR CENTER) = 6". CONTACT STRUCTURAL ENGINEER FOR OTHER CASES.

DEVELOPMENT AND SPLICE LENGTHS OF GRADE 60 SLAB REINFORCING BARS IN TENSION DETAIL VIEW

03-CONC-201

CLASS "B" LAP SPLICE, TYP. -WALL HORIZONTAL REINFORCING, RE: PLAN CONTINUOUS KEYWAY, -3/4" EDGE CHAMFER, FILL WITH SEALANT, RE: SCHEDULE FOR RE: ARCH. **CONSTRUCTION JOINT** DIMENSIONS -WALL VERTICAL REINFORCING, RE: PLAN SPACING SOIL SIDE OF WALL--WALL HORIZONTAL REINFORCING, RE: PLAN

CONTROL JOINT ₹ 5 1. CONTINUOUS KEYWAYS ARE TO BE PROVIDED ACCORDING TO THE FOLLOWING SCHEDULE:

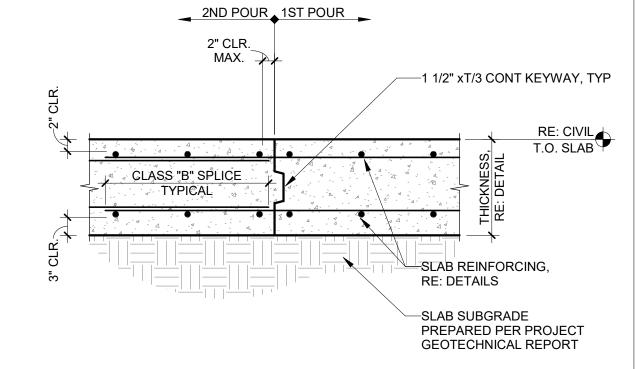
11	TETWATO AILE TO BET HOVIBED ACCORDING TO								
		SHEAR KEY SCHEDULE							
	T (IN.)	8"	10"	12"	14"	16"			
	W (IN.)	3 1/2"	3 1/2"	5 1/2"	5 1/2"	5 1/2"			
	D (IN.)	1 1/2"	1 1/2"	1 1/2"	1 1/2"	1 1/2"			

CONCRETE FOUNDATION WALL JOINT DETAILS

SOIL SIDE OF WALL-

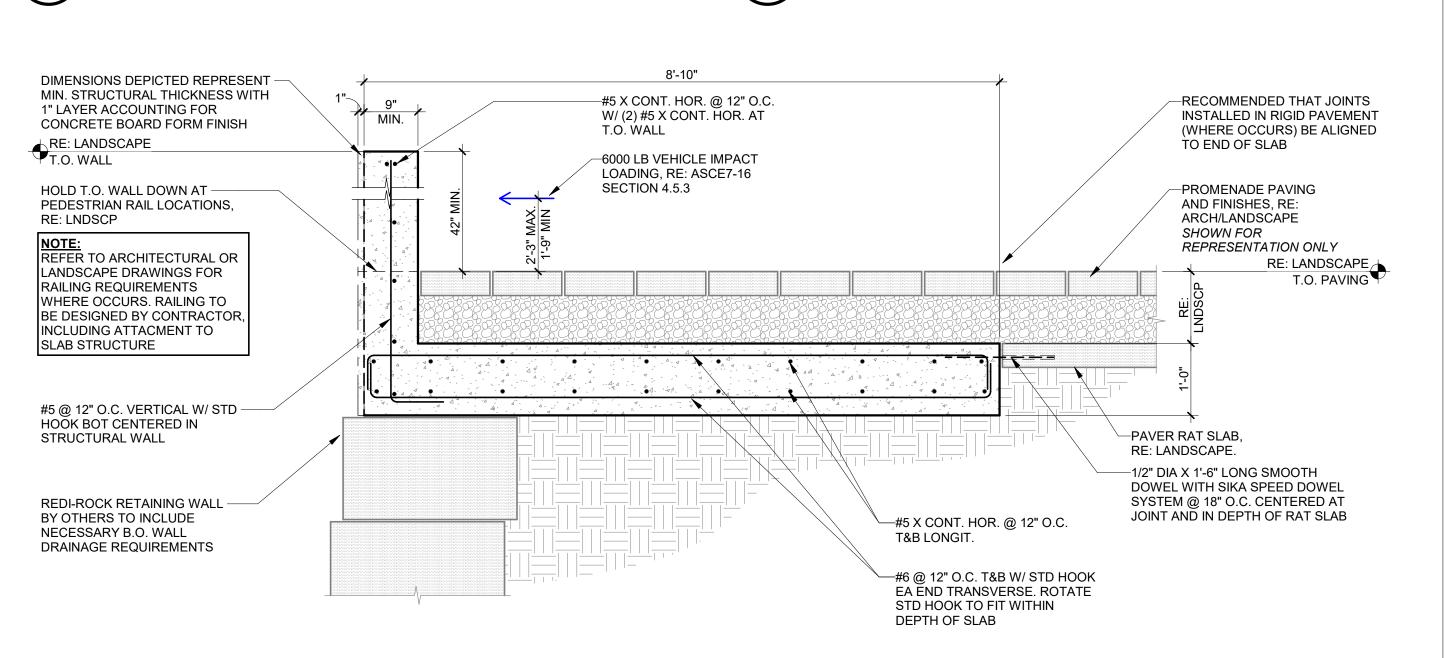
2. CONTINUOUS KEYWAYS ARE TO BE CENTERED WITHIN THE WALL THICKNESS AND SHALL START 1'-0" FROM THE BOTTOM OF THE WALL AND TERMINATE 1'-0" FROM THE TOP OF THE WALL.

3. CONTROL JOINTS IN FOUNDATION WALLS ARE TO BE SPACED AT 30'-0" O.C. MAXIMUM SPACING. 4. CONTRACTOR SHALL COORDINATE CONSTRUCTION AND CONTROL JOINT LOCATIONS AND SUBMIT TO THE ENGINEER FOR REVIEW.



- 1. CONSTRUCTION JOINTS ARE TO BE PROVIDED IN ALIGNMENT WITH ADJACENT UPTURNED WALL
- 2. CONTINUOUS KEYWAYS ARE TO BE CENTERED WITHIN THE SLAB THICKNESS AND SHALL START 1'-0" FROM THE EACH END OF THE SLAB.
- 3. CONTRACTOR SHALL COORDINATE CONSTRUCTION JOINT LOCATIONS AND SUBMIT TO THE ENGINEER FOR REVIEW.

CONCRETE SLAB CONSTRUCTION JOINT DETAILS

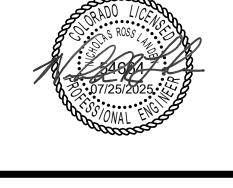


03-CONC-220

SECTION AT RETAINING STRUCTURE UPPER FOUNDATION SLAB

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STRUCTURAL FORTIS STRUCTURAL, ENGINEER: LLC

ISSU	ISSUES & REVISIONS:					
\triangle	Date	Description				

DATE: 07/29/25 DRAWN BY:

SHEET TITLE:

WEST PROMENADE **SLAB - PLANS AND DETAILS**