GENERAL CRITERIA

THERMAL FACTOR (Ct)

1.		SING INTERNATIONAL BUILDING CODE (IBC) 20 ⁻ DESIGN LOADS FOR BUILDING AND OTHER
	STRUCTURES.	
	A. SNOW LOAD DESIGN DATA	
	GROUND SNOW LOAD, Pg	110 psf (Below 5300')
	FLAT-ROOF LOAD, Pf = 0.7*Ce*Ct*I*PG	110 psf
	SNOW EXPOSURE FACTOR (Ce)	1.0
	SNOW IMPORTANCE FACTOR (Is)	1.0

B. WIND LOAD DATA 115 MPH 3SEC-GUST BASIC WIND SPEED, V*ULT* WIND IMPORTANCE FACTOR 1.0 WIND EXPOSURE

BUILDING CLASSIFICATION TOPOGRAPHIC FACTOR, KZT

TOTAL: 270, TRANS: 155 KIPS, LONG: 118 KIPS WIND BASE SHEAR. V C. EARTHQUAKE DESIGN DATA RISK CATEGORY IMPORTANCE FACTOR (Ie)_ 0.3240 0.0830 SITE CLASS 0.333 0.133 SEISMIC DESIGN CATEGORY SEISMIC FORCE-RESISTING SYSTEM LIGHT FRAMED WALLS W/ WOOD STRUCT. PANELS SEISMIC COEFFICIENT, CS 0.0512

6.5

200 KIPS

D. <u>DESIGN LOADS FLOOR & ROOF</u>

SEISMIC COEFFICIENT, R

SEISMIC BASE SHEAR. V

DEAD LOAD _15 psf 110 psf (Snow) LIVE LOAD

FLOOR DEAD LOAD 40 psf (OFFICE & RESIDENTIAL AREA) LIVE LOAD LIVE LOAD 100 psf (CORRIDOR, STAIRS & STORAGE)

2. CAST-IN-PLACE CONCRETE A. F'c = 4000psi @ 28 DAYS (AIR ENTRAINED) - ALL EXTERIOR BUILDING B. F'c = 3000psi @ 28 DAYS (NONE AIR ENTRAINED) - ALL BUILDING

- 3. REINFORCING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615 OR A706, GRADE 60.
- 4. UNLESS NOTED OTHERWISE, LAP SPLICED OR EMBEDMENT LENGTHS SHALL CONFORM TO TABLE A, CLASS B SPLICE. SEE THIS SHEET, TABLE A.
- 5. UNLESS NOTED OTHERWISE, CONCRETE COVER OVER STEEL REINFORCEMENT SHALL CONFORM TO THE MINIMUMS REQUIRED BY CURRENT ADDITION OF ACI 318.
- 6. REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315, EXCEPT WHERE OTHERWISE INDICATED.
- 7. COVER: UNLESS OTHERWISE NOTED OR DETAILED, THE FOLLOWING MINIMUM CONCRETE

	,,	
COVER SHALL BE PROVIDED FOR REINFORCE	EMENT	
EXPOSURE	MINIMUM COVER (IN.)	
A. CONCRETE CAST AGAINST AND		
PERMANENTLY EXPOSED TO EARTH:	3	
B. CONCRETE EXPOSED TO EARTH OR W	EATHER:	
#6 - #18 BARS	2	
#5 - AND SMALLER	1 1/2	
C. CONCRETE NOT EXPOSED TO WEATHE	R	
OR NOT IN CONTACT WITH GROUND:		
SLABS, WALLS, JOISTS:		
#11 AND SMALLER	3/4	
BEAMS, COLUMNS:		

FOUNDATION NOTES - GEO-TECHNICAL REPORT GOVERNS ALL REQUIREMENTS FOR GEO-TECHNICAL /SOIL AND FOUNDATION PIERS REQUIREMENTS

1 1/2

- 1. FOR COMPACTED FILL SOIL AND EXCAVATION REQUIREMENTS, SEE GEO-TECHNICAL REPORT BY JESIK JOB # 20-8206, DATED 4/23/20 AND 2018 IBC CHAPTER 18. CONTRACTOR SHALL BE RESPONSIBLE TO FOLLOW THE RECOMMENDATIONS SPECIFIED THEREIN.
- 2. DESIGN FOUNDATION BEARING PRESSURE (NET) 3000 PSF DEAD + LIVE LOAD INCREASED BY 1/3 FOR COMBINED VERTICAL AND WIND/SEISMIC LOADS.
- 3. REINFORCEMENT SHALL BE PLACED MID-DEPTH OF SLAB, U.N.O.

TIES, STIRRUPS, PRIMARY REINFORCEMENT

- 4. CHAMFER EXPOSED EDGES OF CONCRETE 3/4", AT EXPOSED EDGES UNLESS OTHERWISE NOTED.
- 5. SUB-GRADE PREPARATION:
- A. EXISTING FOUNDATIONS AND UTILITIES AT ANY POINT BENEATH OR WITHIN 3'-0" OF THE NEW STRUCTURES SHALL BE REMOVED ENTIRELY. ANY FILL MATERIAL FROM PREVIOUS CONSTRUCTION ACTIVITIES WHICH IS ENCOUNTERED WITHIN THE BUILDING FOOTPRINT SHOULD ALSO BE REMOVED ENTIRELY. EXPOSED SUB-GRADE AT THE BASE OF REQUIRED EXCAVATION WHICH IS TO RECEIVE FILL SHALL BE COMPACTED TO NOT LESS THAN 90% MAXIMUM LAB DENSITY FOR COHESIVE MATERIAL, AND 95% MAXIMUM LAB DENSITY FOR COHESION-LESS MATERIAL, TO A MINIMUM DEPTH OF 8". SEE EXCAVATION/FILL DETAIL THIS SHEET.
- 5. FILL: A. ALL FILL PLACED UNDER BUILDING SLABS SHALL BE NON-EXPANSIVE AND SHALL BE COMPACTED TO NOT LESS THAN 95% MAXIMUM DENSITY ACCORDING TO ASTM D-1557.

STRUCTURAL STEEL

- ALL STRUCTURAL STEEL MEMBERS SUCH AS COLUMNS, BEAMS, GIRTS AND BRACES SHALL BE PER SCHEDULE OF CONSTRUCTION MATERIALS THIS SHEET. MISCELLANEOUS STEEL ITEMS SHALL BE ASTM A36. MISCELLANEOUS STEEL TUBES SHALL BE ASTM A500, GRADE B.
- 2. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AWS CODE
- ALL BOLTS FOR BEAM CONNECTIONS SHALL BE ASTM A325 WITH A MINIMUM DIAMETER OF 3/4", UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS. WASHERS SHALL BE INSTALLED UNDER NUTS OF FASTENERS WHEN REQUIRED BY THE SPECIFICATION FOR JOINTS.
- 4. ALL FIELD WELDS SHALL BE INSPECTED PER SHEET S0.2.
- 5. STEEL BEAMS SHALL BE CONCENTRIC WITH COLUMNS, UNLESS OTHERWISE NOTED.
- 6. ALL ANCHOR BOLTS SHALL BE ASTM A36 OR A307, UNLESS NOTED OTHERWISE.
- 7. NO OPENINGS SHALL BE CUT IN STRUCTURAL MEMBERS UNLESS SHOWN ON THE DRAWINGS.
- 8. BUILDING STEEL ROOF BEAMS AND SUPPORT POST ARE NON-SELF-SUPPORTING. THE ROOF METAL DECK, VERTICAL STEEL BRACING AND MASONRY WALLS ARE REQUIRED TO PROVIDE LATERAL STABILITY FOR THE STEEL BEAMS AND COLUMNS. CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING REQUIRED TO MAINTAIN STABILITY OF THE POST AND BEAM SYSTEM UNTIL THESE BRACING ELEMENTS ARE IN PLACE.

TABLE A - REINFORCEMENT TENSION LAPS. EMBEDMENT AND HOOK LENGTHS fy = 60000 psi f'c = 3000 psiCLASS B LAP (IN) EMBEDMENT AND CLASS CLEAR SPACING A LAP (IN) (5)(6)(7)TOP BAR $\,$ (10) $\,$ | OTHER BARS $\,$ | TOP BAR $\,$ (10) $\,$ | OTHER BARS $\,$ 11 | 2.82 | 4.23 | 7.05 | 125 | 89 | 71 | 96 | 69 | 55 | 162 | 116 | 93 |

NOTES FOR TABLE A

- LENGTHS SHOWN CONFORM WITH NON SEISMIC PROVISIONS OF ACI 318-95 FOR UNCOATED BARS NOT ENCLOSED BY CLOSELY SPACED SPIRALS OR TIES. DEVELOPMENT OF REINFORCEMENT NOT COVERED BY THE TABLE SHALL CONFORM WITH ACI 318-95.
- 2. MULTIPLY LENGTHS SHOWN BY 0.87 FOR 4000 PSI. CONCRETE, BUT LENGTH OF LAP SHALL NOT BE LESS THAN 12 INCH.
- 3. MULTIPLY LENGTHS SHOWN BY 1.3 FOR LIGHTWEIGHT AGGREGATE CONCRETE.
- 4. BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS WHEN ALL BARS ARE LAPPED AT THE SAME LOCATION. WHEN BAR LAPS ARE STAGGERED, AND LAP HALF THE BARS ARE LAPPED AT THE SAME LOCATION. THE BAR CLEAR SPACING IS TWICE THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS. WHEN ALL BARS ARE EMBEDDED AT THE SAME LOCATION, THE BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS ONE BAR DIAMETER.
- 5. CLASS A LAP LENGTHS APPLY ONLY WHERE NOTED ON THE DRAWINGS.
- 6. LAP AND EMBEDMENT LENGTHS SHOWN APPLY WHEN MINIMUM CONCRETE COVER OVER BARS CONFORMS WITH VALUES GIVEN IN THE TABLE FOR "CONCRETE COVER". THESE COVER VALUES CONFIRM WITH ACI 318-95.
- 7. CLASS A LAP AND EMBEDMENT LENGTH HAVE SAME VALUE.
- 8. CLASS B LAP LENGTHS APPLY FOR ALL SPLICES UNLESS NOTED OTHERWISE.
- 9. HOOK LENGTH GIVEN IS THE STRAIGHT LINE DISTANCE FROM THE LOCATION OF MAXIMUM STRESS IN THE BAR TO THE OUTSIDE END OF THE HOOK. MULTIPLY LENGTHS GIVEN BY 0.7 FOR HOOKS WITH SIDE COVER NORMAL TO THE HOOK NOT LESS THAN 2-1/2 INCH AND FOR 90 DEGREE HOOKS COVER ON BAR EXTENSION BEYOND HOOK NOT LESS THAN 2 INCH.
- 10. TOP BARS ARE HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12 INCHES OF CONCRETE IS CAST BELOW THE REINFORCEMENT.
- 11. MULTIPLY LAP AND EMBEDMENT LENGTHS GIVEN BY 2.0 FOR BARS WITH CLEAR SPACING OF TWO BAR DIAMETERS OR LESS OR CONCRETE COVER OF ONE BAR DIAMETER OR LESS.
- 12. MINIMUM CONCRETE COVER FROM FACE OF MEMBER TO EDGE BAR SHALL NOT BE LESS THAN TWO AND ONE HALF BAR DIAMETERS.

REBAR TYPICAL LAPS & NOTES

Scale: N.T.S.

	SCHEDULE OF CONSTRUCTION N	MATERIALS	
	LOCATION	28-DAY COMPRESSIVE STRENGTH	
CONCRETE	EXTERIOR CONCRETE (EXPOSED TO FREEZING AND/OR DE-ICERS)	4,000 P.S.I. MIX TYPE D	
	EXTERIOR CONCRETE (NOT EXPOSED TO FREEZING)	3,000 P.S.I. MIX TYPE A	
	FOOTINGS	3,000 P.S.I. MIX TYPE A	
	FOUNDATION WALLS	3,000 P.S.I. MIX TYPE D	
	INTERIOR SLABS ON GRADE	3,000 P.S.I. MIX TYPE E	
	NOTE: CONCRETE STRENGTH USED IN DESIGN IS 3,000 P.	S.I.	
STRUCTURAL	APPLICATION	MATERIAL	
STEEL	SQUARE OR RECTANGULAR HSS	ASTM A500 (46ksi) GRADE B	
	WIDE FLANGES SECTIONS	ASTM A992 (50ksi)	
	OTHER SHAPES AND PLATES	ASTM A36 (36ksi)	
REINFORCING STEEL	BARS SHOWN ON DRAWING TO BE FIELD BENT	ALL OTHER BARS	
	ASTM A615, GRADE 40 OR GRADE 60 (SEE LAP SPLICE SCHEDULE D/S003 FOR LAP LENGTHS	ASTM A615, GRADE 60 (SEE LAP SPLICI SCHEDULE D/S003 FOR LAP LENGTHS)	

Type A - Water reducing admixture. This is used to reduce the quantity of mixing water at a given workability or increase workability
at a given water content.
Type B - Retarding admixture used for increasing setting time of concrete.
Type C - Accelerating admixture used for decreasing setting time and to develop early strength gain.
Type D - Water reducing and retarding admixture has the effects of both A and B.
Type E - Water reducing and accelerating admixtures has the effects of both A and C.
Type F - Water reducing, high range admixture used to reduce the quantity of mixing water required to produce concrete of a
given consistency by 12% or more, and can be used to produce high slump or flowing concrete.
Type G - Water reducing, high range, and retarding admixtures are used to reduce the quantity of mixing water required to produce
concrete of given consistency by 12% or more and retard setting times of concrete.

Type S - Specific performance admixtures used for shrinkage reduction, ASR mitigation, viscosity modification or any other

	APPLICATION	S	PECIES AND MIN	IMUM GRADE	
	TOP PLATES STRUTS ROOF JOISTS FLOOR JOISTS MISC. FRAMING HEADERS BEAMS LEDGERS	DOUGLAS FIR-LARCH HEM FIR SPRUCE-PINE-FIR MSR 1650F	#2 OR BETTER #2 OR BETTER		
3ER	BLOCKING	DOUGLAS FIR-LARCH HEM FIR SPRUCE-PINE-FIR MSR 1650F	#2 OR BETTER		
JD LUMBER		DOUGLAS FIR-LARCH SOUTHERN PINE			
MOOD SION L	SILL PLATES	<u>2x4's</u>		2x6's OR GREATER	
DIMENSION		HEM FIR	STANDARD OR BETTER STANDARD OR BETTER STANDARD OR BETTER 1.3E	HEM FIR	#2 OR BETTER
	TRUSSED RAFTERS (CHORDS AND WEBS)	DOUGLAS FIR-LARCH HEM FIR SPRUCE-PINE-FIR MSR 1650F	#2 OR BETTER	•	
	EXTERIOR WALL STUDS & INTERIOR STRUCTURAL WALL STUDS	DOUGLAS FIR-LARCH HEM FIR SPRUCE-PINE-FIR	STUD GRADE OR BETTE STUD GRADE OR BETTE STUD GRADE OR BETTE	R. SEE NOTE 3	
	INTERIOR NON-STRUCTURAL WALL STUDS	DOUGLAS FIR-LARCH HEM FIR SPRUCE-PINE-FIR			

A DDI IO A TIONI		IONI	DESIGN VALUES (SEE NOTE 1) - P.S.I.					
APPLICATION			Fb	Fv	Fc⊥	Fc	E x 10 6	
STRUCTURAL	1-1/2" x=-	< 5-1/2"	1700	285	680	1400	1.3	
COMPOSITE LUMBER (SCL)	1-1/2" x A	LL DEPTHS	2,250	285	750	2,175	1.5	
(,	1-3/4" x A	LL DEPTHS	2,600	285	750	2,510	1.9	
	3-1/2" x A	ALL DEPTHS	1,700	285	680	1,400	1.3	
GLUED LAMINATED				ALLOWABLE STRESSES - P.S.I.				
BEAMS				Fb TENSION ZONE	Fb COMPRESSION ZONE STRESSED IN TENSION	Fv	Fc	E x 10 6
	ALL BEAMS	24F-V4 DF/DF O SP/SP (W/ STRE OR 24F-1.8E		2,400 (MIN.)	1450 (MIN.) SEE NOTE 2	265 (MIN.)	1,600 (MIN.)	1.8 (MIN.)

specific requirement.

DESIGN VALUES ARE FOR NORMAL DURATION. REPETITIVE FRAMING FACTORS AND SIZE FACTORS HAVE NOT

1850 FOR DF/DF COMBINATION AND 1950 FOR SP/SP COMBINATION.

ANCHOR DETAILS

3. FOR 2x6 AND LARGER, USE #2 GRADE OR BETTER FOR ANY OF THE THREE WOOD TYPES.

	TYPICAL ANCHOR BOLT SCHEDULE					
(1)	INSTALLATION TYPE CAST-IN-PLACE (PRE AUTHORIZED) [2]				DRILL-IN-OPTIONS (SUBMITTAL REQUIRED) [3]	
PROJECTION NOTE (1)	BOLT TYPE	STANDARD J-BOLT	THREADED HEADED ROD ANCHOR ANCHOR	SIMPSON "SSTB" ANCHOR BOLT	ADHESIVE ANCHOR	
ı			A		—	
0.7	TENT AENTS		12x DIA.		4	
MIN. EMBED UNO PER PLAN	EMBEDMENT REQUIREMENTS	7 1/2" —	1/4" MIN.	PER MFR	NOT ALLOWED AT P-T SLAB	
Š	LIMITS	5/8"Ø MAX.	5/8"Ø THRU 2 1/2"Ø	FOR WOOD FRAME ONLY	5/8"Ø THRU 1"Ø	
	ANCHOR	BOLT MATER	RIAL - A325 OR F1554 GR	ADE 36 MIN.		
DIA = ANCHOR BOLT DIAMETER (NOMINAL)						

NOTES:

- 1. CONTRACTOR SHALL DETERMINE THE REQUIRED THREAD PROJECTION SUITABLE FOR THE THICKNESS OF MATERIAL BEING FASTENED PLUS GROUT ALLOWANCE, IF ANY, AND CONSTRUCTION TOLERANCES, UNO.
- 2. CONTRACTOR MAY SELECT APPROPRIATE CAST-IN-PLACE ANCHOR BOLT OPTION WITHOUT SUBMITTAL.
- 3. DRILL-IN OPTIONS ARE NOT APPROPRIATE AT ALL CONDITIONS. IF DRILL-IN METHOD IS PREFERRED, SUBMIT MANUFACTURER'S INFORMATION, ALLOWABLE LOAD VS. EMBEDMENT DATA AND LOCATIONS OF WHERE SUBSTITUTIONS ARE REQUESTED. ENGINEER WILL DETERMINE IF SUBSTITUTION IS APPROPRIATE FOR LOCATION AND LOADING.
- 4. EMBEDMENT OF DRILL-IN ANCHORS SHALL BE PER ENGINEER'S SUBMITTAL REVIEW COMMENTS. EMBEDMENT SHALL BE (9) NINE TIMES THE NOMINAL ANCHOR DIAMETER, UNO.
- 5. AT PRESSURE TREATED SILLS, PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL ANCHORS.



Scale: N.T.S.

STRUCTURAL GENERAL NOTES S0.1

SHEET: 7 OF 140

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THE FOLLOWING DEFINITIONS COVER THE MEANINGS OF CERTAIN TERMS USED IN THESE NOTES:

'ARCHITECT/ENGINEER' - THE ARCHITECT OF RECORD AND THE STRUCTURAL ENGINEER OF RECORD.

- 'STRUCTURAL ENGINEER OF RECORD' (SER) THE STRUCTURAL ENGINEER WHO IS LICENSED TO STAMP AND SIGN THE STRUCTURAL DOCUMENTS FOR THE PROJECT. THE SER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM.
- 'SUBMIT FOR REVIEW' SUBMIT TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION.
- 'PER PLAN' INDICATES REFERENCES TO THE STRUCTURAL PLANS, ELEVATIONS AND STRUCTURAL GENERAL NOTES.
- 'SPECIALTY STRUCTURAL ENGINEER' (SSE) A PROFESSIONAL ENGINEER (PE OR SE), LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, (TYPICALLY NOT THE SER), WHO PERFORMS SPECIALTY STRUCTURAL ENGINEERING SERVICES FOR SELECTED SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS, AND WHO HAS EXPERIENCE AND TRAINING IN THE SPECIALTY. DOCUMENTS STAMPED AND SIGNED BY THE SSE SHALL BE COMPLETED BY OR UNDER THE DIRECT SUPERVISION OF THE SSE.
- 'BIDDER-DESIGNED' COMPONENTS OF THE STRUCTURE THAT REQUIRE THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR SUPPLIER WHO IS RESPONSIBLE FOR THE DESIGN, FABRICATION AND INSTALLATION OF SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS TO RE-RETAIN THE SERVICES OF AN SSE. SUBMITTALS OF 'BIDDER-DESIGNED' ELEMENTS SHALL BE STAMPED AND SIGNED BY THE SSE.

OTHER DRAWINGS: REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL AND PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING BUT NOT LIMITED TO: DIMENSIONS, ELEVATIONS, SLOPES, DOOR AND WINDOW OPENINGS, NON-BEARING WALLS, STAIRS, FINISHES, DRAINS, WATERPROOFING, RAILINGS, MECHANICAL UNIT LOCATIONS, AND OTHER NON-STRUCTURAL ITEMS.

STRUCTURAL DETAILS: THE STRUCTURAL DRAWINGS ARE INTENDED TO SHOW THE GENERAL CHARACTER AND EXTENT OF THE PROJECT AND ARE NOT INTENDED TO SHOW ALL DETAILS OF THE WORK.

STRUCTURAL RESPONSIBILITIES: THE STRUCTURAL ENGINEER (SER) IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE PRIMARY STRUCTURE IN ITS COMPLETED FORM.

COORDINATION: THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS; FOR SELECTING FABRICATION PROCESSES; FOR TECHNIQUES OF ASSEMBLY; AND FOR PERFORMING WORK IN A SAFE AND SECURE MANNER.

MEANS, METHODS AND SAFETY REQUIREMENTS: THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH).

BRACING/SHORING DESIGN ENGINEER: THE CONTRACTOR SHALL, AT HIS DISCRETION EMPLOY AN SSE, A REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING.

TEMPORARY SHORING, BRACING: THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

CONSTRUCTION LOADS: LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

CHANGES IN LOADING: THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL / STRUCTURAL / MECHANICAL / ELECTRICAL OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

NOTE PRIORITIES: PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THE STRUCTURAL GENERAL NOTES.

DISCREPANCIES: IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, PLAN/DETAILS OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.

SITE VERIFICATION: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

ADJACENT UTILITIES: THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATIONS, SHORING, AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY COMPLETE

ALTERNATES: ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE, AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.

SUBMITTALS

SUBMIT FOR REVIEW: SUBMITTALS OF SHOP DRAWINGS, AND PRODUCT DATA ARE REQUIRED FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS AND FOR BIDDER DESIGNED ELEMENTS.

SUBMITTAL REVIEW PERIOD: SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF TWO WEEKS FOR REVIEW BY THE ARCHTECT/ENGINEER PRIOR TO THE ONSET OF FABRICATION.

GENERAL CONTRACTOR'S PRIOR REVIEW: PRIOR TO SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE 'SER', AND THEREFOR, MUST BE VERIFIED BY THE GENERAL CONTRACTOR. CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE DETAILER AND PROVIDE THE CONTRACTOR'S REVIEW STAMP AND SIGNATURE BEFORE FORWARDING TO THE ARCHITECT/ENGINEER.

SHOP DRAWING REVIEW: ONCE THE CONTRACTOR HAS COMPLETED THE REQUIRED PRIOR REVIEW, THE 'SER' WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS, NOR THE DEPARTURES THERE FROM.

SHOP DRAWING DEVIATIONS: WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS THEY SHALL BE DESIGNED AND STAMPED BY THE RESPONSIBLE 'SSE'.

DEFERRED SUBMITTALS

BIDDER-DESIGNED ELEMENTS

SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND 'SER' FOR REVIEW PRIOR TO SUBMISSION TO THE APPROVING AGENCY.

DESIGN OF PREFABRICATED, "BIDDER-DESIGNED", MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:

- 1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND AND EARTHQUAKE LOADS IN COMBINATIONS REQUIRED BY IBC.
- 2. DESIGN WITHIN THE DEFLECTION LIMITS NOTED HEREIN AND AS SPECIFIED OR REFERENCED IN THE IBC.
- 3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERENCE STANDARDS OF THE
- GOVERNING CODE.

 4. SUBMITTAL SHALL INCLUDE:
 - CALCULATIONS PREPARED, STAMPED AND SIGNED BY THE 'SSE' DEMONSTRATING CODE CONFORMANCE.
 - b. ENGINEERED COMPONENT DESIGN DRAWINGS ARE PREPARED, STAMPED AND SIGNED BY THE 'SSE'.
 - c. PRODUCT DATA, TECHNICAL INFORMATION AND MANUFACTURER'S WRITTEN REQUIREMENTS AND AGENCY APPROVALS AS APPLICABLE.
 - d. 'SSE' MAY SUBMIT TO THE ARCHITECT/ENGINEER, A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERALLY EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AUTHORITY HAVING JURISDICTION. SUBMIT ADEQUATE DOCUMENTATION OF DESIGN.

DEFLECTION LIMITS:	VERTICAL	LIMIT
	ROOF MEMBERS, DEAD + LIVE OR SNOW OR WIND, TOTAL LOAD (TL) DEFLECTION	L / 240, WHERE (L IS SPAN LENGTH, INCHES)
	ROOF, LIVE OR SNOW OR WIND LOAD (RLL)	L/360
	FLOOR MEMBERS, TOTAL LOAD (TL) uno	L / 240
FLOOR LIVE LOAD (LL) uno		L/360
HORIZONTAL		LIMIT AND FOOTNOTE
	MEMBERS SUPPORTING BRITTLE FINISHES	L / 240 (1)
MEMBERS SUPPORTING FLEXIBLE FINISHES		L / 180 (1)

(1) WIND LOAD IS REDUCIBLE TO 0.6 TIMES THE COMPONENT AND CLADDING LOADS PER TABLE 1604.3 FOOTNOTE f.

GENERAL CONTRACTOR'S PRIOR REVIEW: ONCE THE CONTRACTOR HAS COMPLETED THE REVIEW OF THE 'SSE' COMPONENT DRAWINGS, THE 'SER' WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. REVIEW OF THE SPECIALTY STRUCTURAL ENGINEER'S 'SSE' SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE 'SSE' OF RESPONSIBILITY FOR THAT DESIGN. ALL NECESSARY BRACING, TIES, ANCHORAGE, PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE 'SSE'S' DESIGN DRAWINGS AND CALCULATIONS. THESE ELEMENTS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:

"BIDDER-DESIGNED" DEFERRED SUBMITTALS

- PRE-FABRICATED METAL PLATED ROOF & FLOOR TRUSSES
- TEMPORARY SHORING SYSTEMS
- STAIR HANDRAILS, GUARDRAILS, BALCONY RAIL ANCHORAGES

QUALITY ASSURANCE (TESTING AND INSPECTION)

QUALITY ASSURANCE (TESTING AND INSPECTION) AS REQUIRED BY THE OWNER AND SECTIONS 1704 THRU 1709 OF THE IBC, SHALL BE PROVIDED BY A QUALIFIED AGENCY SELECTED BY THE OWNER. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE REQUIRED INSPECTIONS. ALL TESTING AND INSPECTION REPORTS SHALL BE SENT WITHIN 24 HOURS OF THE TEST OR INSPECTION TO THE OWNER, ARCHITECT, APPLICABLE ENGINEER, BUILDING OFFICIAL AND GENERAL CONTRACTOR. ITEMS REQUIRING QUALITY ASSURANCE (TESTING AND INSPECTION) ARE:

- 1. SOILS/ENGINEERED FILL (IBC SECTION 1705.6 AND TABLE 1705.6):
- A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
- B. PRIOR TO PLACEMENT OF THE PREPARED FILL, THE INSPECTOR SHALL DETERMINE THAT
- THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE SOILS REPORT.

 C. DURING PLACEMENT AND COMPACTION OF THE FILL MATERIAL, THE INSPECTOR SHALL DETERMINE THAT THE MATERIAL BEING USED AND THE MAXIMUM LIFT THICKNESS
- COMPLIES WITH THE GEO-TECHNICAL'S REPORT.

 D. THE INSPECTOR SHALL DETERMINE THAT THE IN-PLACE DRY DENSITY OF THE
- ENGINEERED FILL MATERIAL COMPLIES WITH THE GEO-TECHNICAL REPORT.

 1. CONTINUOUS FOOTING ENGINEERED FILL: SEE SPECIFICATIONS.
- SPOT FOOTING ENGINEERED FILL: SEE SPECIFICATIONS.
 SITE WORK ENGINEERED FILL (PAVED AREAS, SIDEWALKS, TRENCHES, ETC.): SEE
- SPECIFICATIONS.
- 4. BUILDING PAD ENGINEERED FILL: SEE SPECIFICATIONS.
- 2. CONCRETE (IBC SECTION 1705.3 AND TABLE 1705.3):
- A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
- B. CYLINDERS, SLUMP, TEMPERATURE AND AIR-ENTRAINMENT SHALL BE DONE. PROVIDE SIX SAMPLES EACH FOR EXTERIOR SITE WORK CONCRETE, FOOTINGS, FOUNDATION
- WALLS AND INTERIOR SLABS ON GRADE.

C. PROTECTION OF CONCRETE DURING COLD AND HOT WEATHER.

- 3. PREFABRICATED METAL PLATE WOOD TRUSSES (IBC SECTIONS 1705.5)
- A. REFER TO SPECIFICATION SECTION 06 1753 SHOP-FABRICATED WOOD TRUSSES: TRUSSED RAFTERS FOR CERTIFICATION REQUIREMENTS.
- B. THE INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED
 FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS OF
- FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS OF INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATORS ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS
- 4. WOOD DIAPHRAGMS AND SHEAR WALLS (IBC SECTION 1705.5)
- A. FOR WALLS AND ROOF AREAS WHERE THE NAIL SPACING IS 4 INCHES AND LESS ON CENTER, THE INSPECTOR SHALL VERIFY WOOD PANEL SHEATHING GRADE, THICKNESS AND NOMINAL SIZE OF FRAMING MEMBERS, ADJOINING PANEL EDGES, NAIL SIZE AND SPACING, BOLTING AND OTHER FASTENING OF COMPONENTS.
- 5. EPOXY ANCHORS (IBC SECTION 1706.1):
- A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
- B. INSPECTION SHALL VERIFY ALL DRILLED HOLES SIZE AND DEPTH PRIOR TO INSTALLATION OF EPOXY AND ANCHOR ROD. SEE SPECIFICATIONS FOR QUANTITY OF TESTING.
- 6. STEEL (IBC SECTION 1705.2):
- A. INSPECTION DURING FABRICATION IS NOT REQUIRED IF THE FABRICATOR IS REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT INSPECTION. SPECIAL INSPECTION OF FIELD WELDS AS REQUIRED BY IBC SECTION 1704.
- 7. MASONRY (IBC SECTION 1705.4):
- A. REFER TO SPECIFICATION SECTION 04 0000 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
- B. SPECIAL INSPECTION OF MASONRY IS NOT REQUIRED.





3400 S. LINCOLN AVE

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S0.2

GENERAL NOTES

SHEET: 8 OF 140

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BEAM OR COLUMN CROSSTIES BEAM STIRRUPS AND COLUMN TIES

d = BAR DIAMETER, D = BEND DIAMETER

REBAR TYPICAL BEND DETAILS

TYPICAL ANCHOR BOLT SCHEDULE DRILL-IN-OPTIONS INSTALLATION CAST-IN-PLACE (PRE AUTHORIZED) [2] (SUBMITTAL TYPE REQUIRED) [3] SIMPSON "SSTB" ANCHOR BOLT BOLT **ADHESIVE** STANDARD THREADED TYPE ROD J-BOLT HEADED ANCHOR ANCHOR ANCHOR 12x DIA. ≺TACK MFR **NOT ALLOWED** AT P-T SLAB FOR WOOD 5/8"Ø THRU 1"Ø 5/8"Ø MAX. | 5/8"Ø THRU 2 1/2"Ø ANCHOR BOLT MATERIAL - A325 OR F1554 GRADE 36 MIN. DIA = ANCHOR BOLT DIAMETER (NOMINAL)

ANCHOR DETAILS

NOTES:

- CONTRACTOR SHALL DETERMINE THE REQUIRED THREAD PROJECTION SUITABLE FOR THE THICKNESS OF MATERIAL BEING FASTENED PLUS GROUT ALLOWANCE, IF ANY, AND CONSTRUCTION TOLERANCES, UNO.
- 2. CONTRACTOR MAY SELECT APPROPRIATE CAST-IN-PLACE ANCHOR BOLT OPTION WITHOUT SUBMITTAL.
- DRILL-IN OPTIONS ARE NOT APPROPRIATE AT ALL CONDITIONS. IF DRILL-IN METHOD IS PREFERRED, SUBMIT MANUFACTURER'S INFORMATION, ALLOWABLE LOAD VS. EMBEDMENT DATA AND LOCATIONS OF WHERE SUBSTITUTIONS ARE REQUESTED. ENGINEER WILL DETERMINE IF SUBSTITUTION IS APPROPRIATE FOR LOCATION AND LOADING.
- EMBEDMENT OF DRILL-IN ANCHORS SHALL BE PER ENGINEER'S SUBMITTAL REVIEW COMMENTS. EMBEDMENT SHALL BE (9) NINE TIMES THE NOMINAL ANCHOR DIAMETER, UNO.
- AT PRESSURE TREATED SILLS, PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL ANCHORS.

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	FOOTING SCHEDULE							
MARK	SIZE LxWxH	REINFORCEMENT	REMARKS					
F25	2'-6" x 2'-6" x 1'-0"	3-#5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1					
F30	3'-0" x 3'-0" x 1'-0"	4- #5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1					
F40	4'-0" x 4'-0" x 1'-0"	5- #5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1					
F50	5'-0" x 5'-0" x 1'-0"	6- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1					
F60	6'-0" x 6'-0" x 1'-0"	7- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1					
F65	6'-6" x 6'-6" x 1'-0"	8- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1					

- 1. TOP OF INTERIOR SPOT FOOTINGS TO BE FF = -6", TYP. UNLESS OTHERWISE NOTED ON FOUNDATION PLAN.
- 2. TOP OF PERIMETER FOOTINGS TO BE FF = -24", TYP. UNLESS OTHERWISE NOTED ON FOLINDATION PLAN

	FOUNDATION PLAN
3.	SPOT PERIMETER FOOTINGS MAY BE COMBINED WITH PERIMETER WALL FOOTINGS, BUT
	SPECIFIED REINFORCING IS IN ADDITION TO PERIMETER WALL FOOTINGS.

COLUMN FOOTING PIER SCHEDULE							
MARK	SIZE LxWxH	REINFORCEMENT	T/F ELEV.	REMARKS			
P1	16" x 16" x 8"	4 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT PERIMETER FOUNDATION WALLS			
P2	16" x 16" x 18"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT FOOTINGS NEAR POOL WALLS			
P3	18" x 18" x 54"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT CANOPY FOOTINGS			
P4	16" x 16" x 54"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT CANOPY FOOTINGS			

- 1. PIER HEIGHT SHALL BE FROM TOP OF FOOTING UP TO FINISH FLOOR UNLESS NOTED **OTHERWISE**
- 2. PERIMETER PIERS MAY BE COMBINED WITH PERIMETER FOUNDATION WALLS, BUT SPECIFIED REINFORCING IS IN ADDITION TO THE PERIMETER FOUNDATION WALL REINFORCING.
- 3. ALL HORIZONTAL TIES SHALL HAVE 4" x 4" LAP LOCK AT CORNER, DETAIL 1/S0.3. 4. ALL PIER REINFORCING SHALL HAVE THE MINIMUM COVER SPECIFIED ON GENERAL CRITERIA "GENERAL CRITERIA" NOTE 7, S0.1.

	BE/	AM SCHEDULE	
MARK	TYPE	SIZE	NOTES
B1	STL. A-50	W 6 x 12	
B2	STL. A-50	W 8 x 18	
B3	STL. A-50	W 10 x 30	
B4	STL. A-50	W 18 x 50	
B 5	STL. A-50	W 21 x 83	
B6	STL. A-50	W 16 x 26	
B7	STL. A-46	TS 6 x 12	
B8	STL. A-36	MC 8 x 8.5	
B9	STL. A-50	W 8 x 58	

Scale: N.T.S.

JOIST SCHEDULE						
MARK	SIZE	SPACING	NOTES			
	2 X 8	16" O.C. MAX.	SEE DETAILS			
J2	11 7/8 TJI360	16" O.C. MAX.	SEE DETAILS			
<u>J3</u>	2 x 6	16" O.C. MAX.	SEE DETAILS			
J4	2 x 10	12" O.C. MAX.	SEE DETAILS			
<u>J5</u>	4X8	32" O.C. MAX.	SEE DETAILS			

	С	OLUMN SCHEDULE	
MARK	TYPE	SIZE	NOTES
(C1)	STL. A-46	TS 4 x 4 x 5/16	SEE S3.1
C2 >	STL. A-46	TS 3 x 3 x 5/16	SEE S3.1

W 6x4X5/16

MARK

EB1

TYPE

C3 STL. A-46

⟨C4⟩ | STL. A-46

Scale: N.T.S.

STL. A-50

ELEVATOR BEAM SCHEDULE

NOTES

BRACE BEAM SIZED FOR VERT & HORZ.

LOADS FROM CAR RAIL PER MANUFACTURER

HOIST BEAM SIZED FOR 15,000LBS LOCATED AT THE CENTER OF THE BEAM

SEE S3.1

SEE S3.1

SEE FOUNDATION PLAN AND DETAIL 10/S5.3 FOR BASE PLATES.

TS 6 x 6 x 5/16

TS 10 x 10 x 5/16

	WOOD BEARING WALL SCHEDULE														
WALLS	TYPICAL EXT	ERIOR WALL	- WW1	TYPICAL COR	RIDOR WALL	- WW2	TYPICAL INTER		•	DOUBLE :		OOR/ROOF & Y WALLS		WW5	
FLOOR LEVEL	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE
3&4	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2
1&2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@12"OC	SPF NO. 2	SPF NO. 2

		Н	EADER S	CHEDUL	E 1,2			
		FIRST & SEC	OND FLOOR	THIRD	FLOOR	ROOF BEARING		
MARK	HEADER	JACK STUDS	KING STUDS	JACK STUDS	KING STUDS	JACK STUDS	KING STUDS	
(H1)	2-2x8	2	2	2	2	2	1	
H2	3-2x8	2	1	2	1	2	1	
(H3)	3-2x10	3	1	2	1			
(H4)	3-2x8	2	1	2	1			
(H5)	3-2x12					2	1	
(H6)	3-2x12	2	2	2	1			
(H7)	4-2x8	2	1	2	1	2	1	
(H8)	1-6X12 SCL	2	2	2	1	2	1	
(H9)	6X10	3	1					
(H10)	1-6X14 SCL	3	1					
(H11)	5.25x9.5 SCL					2	1	
(H12)	3-1.75X14 SCL					2	1	
H13	3-1.75X9.5 SCL					2	1	

1. SEE SHEETS S1.1 THRU S1.4 FOR BUILDING LEVEL KEY 2. SEE SHEET S0.1 FOR SPECIES AND MINIMUM GRADE

Scale:	ľ

					SHE	AR WALL SCH	EDULE						
		SHEATHING		ATTA	CHMENT AT PANEL	EDGES	SOLE F	PLATE CONNEC	CTION		HOLD DO\	٧N	DEMA DICO
MARK	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	REMARKS
S1	5/8" GYP. SHEATHING, BLOCKED, ONE FACE	5/8" GYP. SHEATHING, UNBLOCKED, ONE FACE	5/8" GYP. SHEATHING, UNBLOCKED, ONE FACE	6d COOLER NAILS @ 7"OC	6d COOLER NAILS @ 7"OC	6d COOLER NAILS @ 7"OC	1/2"Ø ANCHORS @72"OC	10d NAILS AT 8"OC	10d NAILS AT 8"OC	1/2"Ø ANCHOR EACH END			SDPWS TABLE 4.3C
S2	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	BASE PLY 6d COOLER NAILS @ 9"OC	BASE PLY 6d COOLER NAILS @ 9"OC	BASE PLY 6d COOLER NAILS @ 9"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	1/2"Ø ANCHOR EACH END			SDPWS TABLE 4.3C
				FACE PLY 8d COOLER NAILS @ 7"OC	FACE PLY 8d COOLER NAILS @ 7"OC	FACE PLY 8d COOLER NAILS @ 7"OC				W/SIMPSON BPS BEARING PLATE			
S3	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	6d COOLER NAILS @ 6"OC	6d COOLER NAILS @ 6"OC	6d COOLER NAILS @ 6"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	HDU5-SDS2.5 SEE NOTE 10	CS18	CS22	SDPWS TABLE 4.3A
S4	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	BASE PLY 6d COOLER NAILS @ 9"OC	BASE PLY 6d COOLER NAILS @ 9"OC	BASE PLY 6d COOLER NAILS @ 9"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	HDU11-SDS2.5	HDU8-SDS2.5	HDU4-SDS2.5	5 SDPWS TABLE 4.3C
				FACE PLY 8d COOLER NAILS @ 7"OC	FACE PLY 8d COOLER NAILS @ 7"OC	FACE PLY 8d COOLER NAILS @ 7"OC							

UNLESS NOTED OTHERWISE:

SHEATHING TO BE CONTINUOUS FOR LENGTH OF SHEAR WALL

- SEE 8/S5.1 FOR ANCHORAGE OF TIE DOWN CONNECTORS TO SLAB ON GRADE.
- PROVIDE MIN. OF 2-2x STUDS AT EACH END OF SHEAR WALL PANEL. SEE MANUFACTURER AND SCHEDULE FOR MIN.IMUM STUD NUMBER AND SIZE AT TIE DOWN LOCATIONS. PROVIDE RSP STRAP TIES ON EACH SIDE OF TOP/SILL PLATES FOR BORED HOLE/NOTCH GREATER THAN 1 1/2" FOR 2x4 OR 2 1/2" FOR 2x6.
- PROVIDE 6d @ 6" OC BETWEEN STUDS AT LOCATIONS WHERE WALL STUD SIZE CHANGES.
- AT GYPSUM SHEATHING, ATTACHMENT AT INTERMEDIATE FIELD STUDS EQUALS ATTACHMENT AT PANEL EDGES. AT WOOD SHEATHING, ATTACHMENT AT INTERMEDIATE FIELD STUDS EQUALS 12" O.C.
- ALTERNATE FASTENERS ARE PERMITTED TO BE USED IF THEIR DIMENSIONS ARE NOT LESS THAN THE SPECIFIED DIMENSIONS. DRYWALL SCREWS ARE PERMITTED TO SUBSTITUTE FOR THE 8d (2 1/2" X 0.131"), AND 6d (1 7/8" X 0.092") NAILS LISTED ABOVE. NO. 6 (1 1/4") SCREWS TYPE S OR W ARE PERMITTED TO SUBSTITUTE FOR 6d (1 7/8" X 0.092) NAILS.
- ALL WOOD MEMBERS FASTENER FOR SHEAR WALLS, FLOORS, ROOF WILL BE PER IBC FASTENING SCHEDULE TABLE 2304.9.1
- 10. AT SHEAR WALL 3 AT LEVEL 1 ANCHORS AT EACH END OF SHEAR WALLS WILL BE 1/2" Ø ANCHORS UNLESS OTHERWISE SHOWN ON PLANS. SEE PLAN VIEWS FOR HDU5-SDS2.5 HOLD DOWN LOCATIONS.
- 11. HOLD DOWN ANCHORS BOLT DIAMETER WILL BE PER MANUFACTURE REQUIREMENTS.

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SHEET: 9 OF 140

5 SCHEDULES

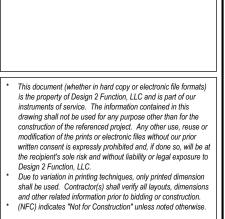
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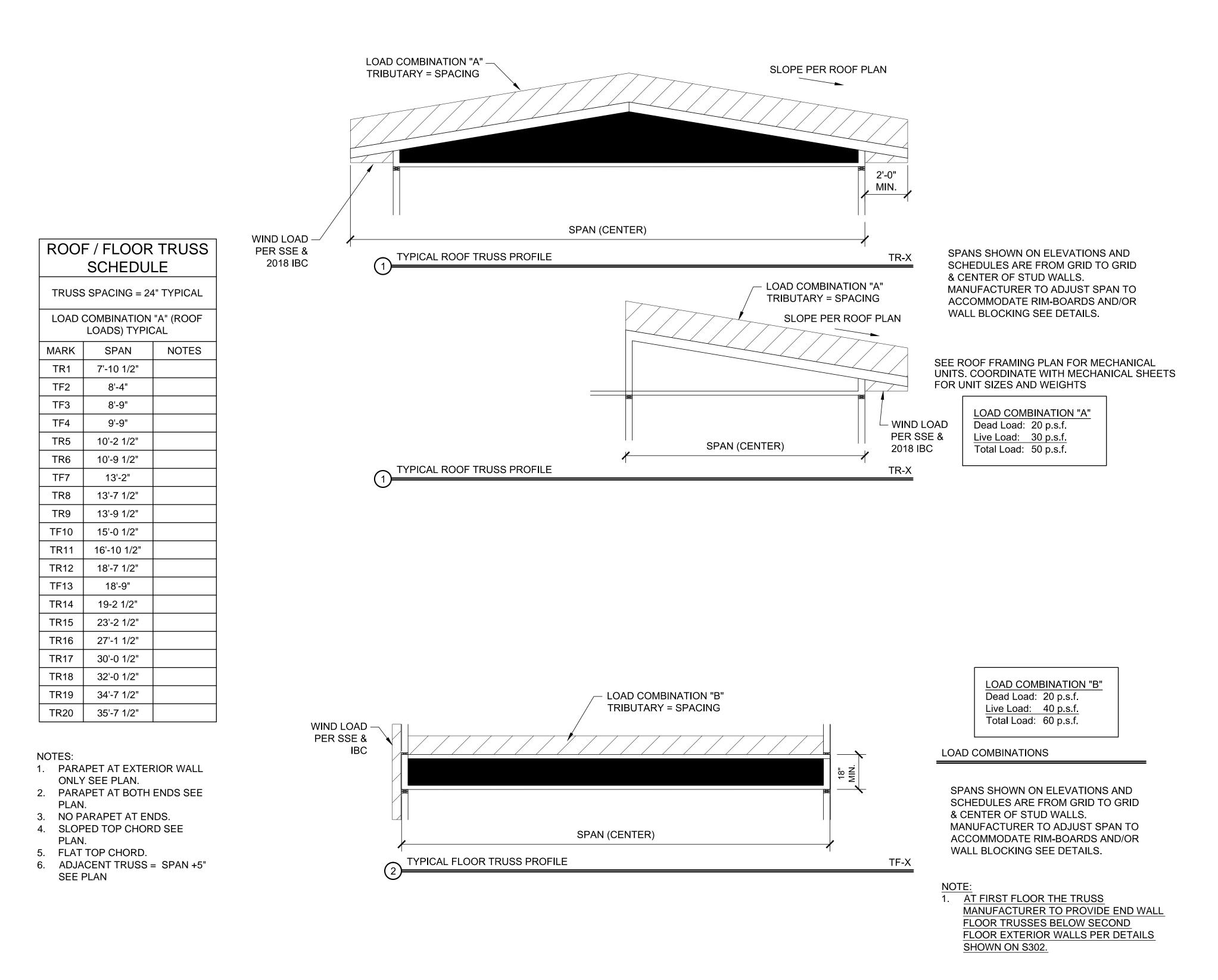
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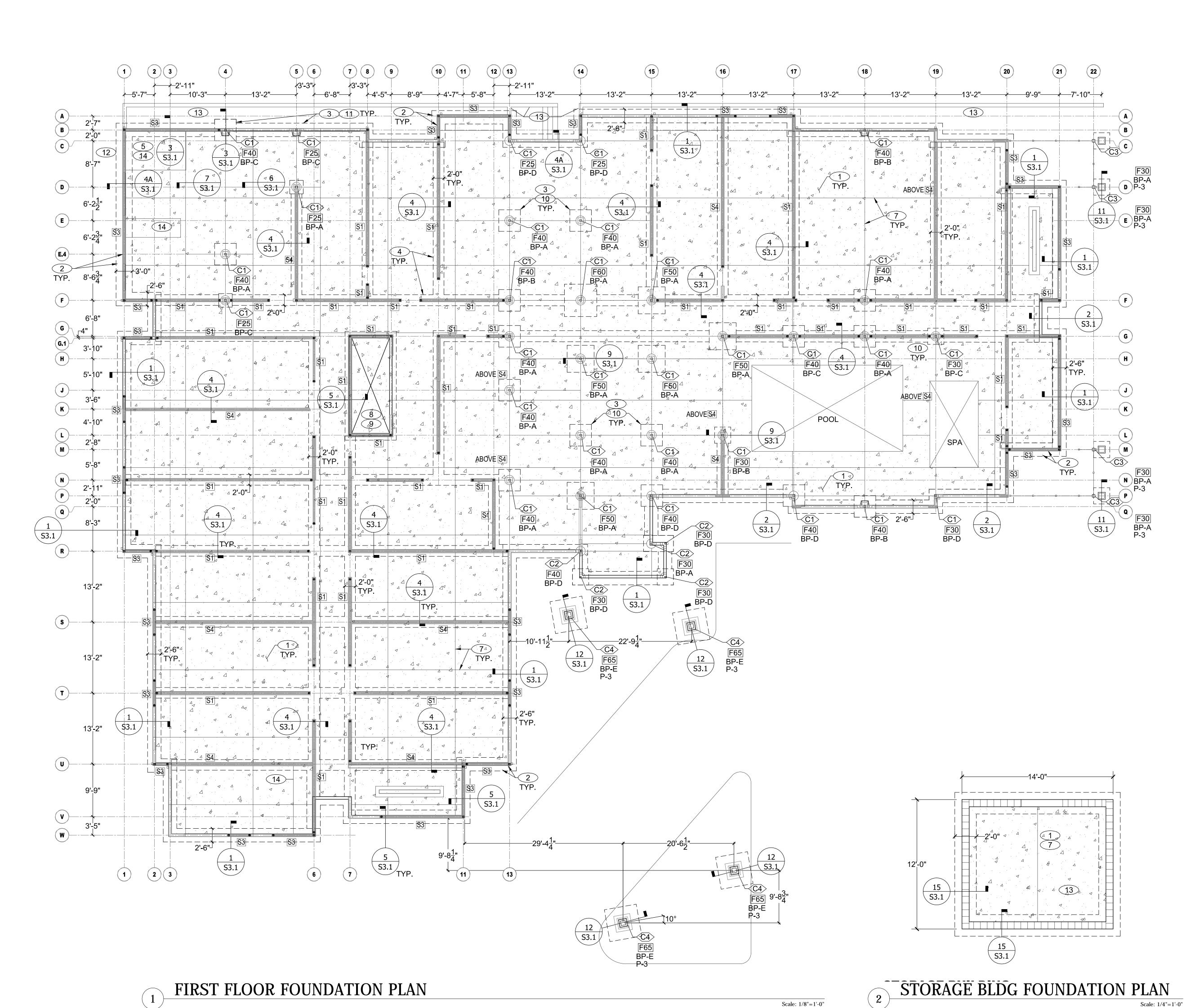
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STRUCTURAL SCHEDULES

SO.4
SHEET: 10 OF 140



PRE-FABRICATED METAL PLATED TRUSSES - ROOF & FLOOR



GENERAL NOTES

- G1. FOOTING ELEVATIONS, IF SHOWN ON THE PLANS, ARE TO THE TOP OF THE FOOTING.
- G2. REFER TO CIVIL DRAWINGS FOR FINISH GRADE ELEVATIONS UNLESS OTHERWISE NOTED.
- G3. OVER-EXCAVATION OF SOIL REMOVED BELOW FOOTINGS SHALL BE REPLACED AND COMPACTED IN LAYERS TO 95% OF MODIFIED PROCTOR DENSITY. SEE DETAIL 13/S3.1
- G4. INTERIOR CONCRETE SLABS ON GRADE, UNLESS OTHERWISE NOTED, SHALL BE REINFORCED WITH WELDED WIRE FABRIC MATS AS FOLLOWS: 4" SLAB - 6 x 6 W1.4 x W1.4
- G5. ELECTRIC CONDUIT AND OTHER PIPES EMBEDDED IN THE CONCRETE FLOOR SHALL BE PLACED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318, PARAGRAPH 6.3.
- G6. ANCHOR BOLTS ARE TO BE FURNISHED PER DETAILS AND AS SHOWN ON SHEET S0.3.
- G7. LOCATE ALL SLEEVES, DRAINS, OPENINGS, EMBEDDED ITEMS, ETC. THAT ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL SUCH ITEMS ARE CORRECTLY POSITIONED & INSTALLED PRIOR TO PLACEMENT OF CONCRETE.
- G8. G.C. SHALL COORDINATE ALL UNDER-SLAB PLUMBING AND ELECTRICAL CONDUIT LOCATIONS & INSTALLATION PER PLUMBING, MECHANICAL AND ELECTRICAL PLANS PRIOR TO POURING OF FLOOR SLAB.
- G9. SEE SHEET S0.1 THRU S0.3 FOR ADDITIONAL NOTES AND LEGEND.
- G10. DATUM FINISH FLOOR ELEVATION = F.F.= 0'-0"

6" SLAB - 6 x 6 W2.1 x W2.1

KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

2. OUTLINE OF CONCRETE STEM WALL AND SPREAD FOOTING OR TURN-DOWN.

1. 4" CONCRETE SLAB WITH 4x4~10/10 WWF - F.F.=0'-0".

- 3. OUTLINE OF CONCRETE ISOLATED CONCRETE PIER AND FOOTING BELOW GRADE FOR COLUMN. SEE FOOTING SCHEDULE S0.3.
- 4. OUTLINE OF BOTTOM OF INTERIOR THICKENED SLAB.
- 5. 12" THICK CONCRETE ISOLATED FOOTING, AT LAUNDRY AREA. SEE LAUNDRY SUPPLIER DRAWINGS FOR DIMENSIONS & DETAIL 7/S3.1
- 6. ADA-COMPLIANT SHOWER, SEE ARCH. DRAWINGS FOR FLOOR
- 7. CONTROL JOINT @ 12'-0" O.C. MAX. EACH WAY. TYPICAL FOR ALL SLAB ON GRADE.
- 8. VERIFY FINAL ELEVATOR PIT DIMENSIONS WITH SUPPLIER DRAWINGS PRIOR TO CONSTRUCTION.
- 9. SUMP PUMP DETAIL 8/S3.1. LOCATE / COORDINATE SUMP PLACEMENT WITH EQUIPMENT AND WITH CONCRETE CONTRACTORS.
- 10. TOP OF FOOTING AT INTERIOR SPOT FOOTINGS TO BE -6" BELOW FF,
- 11. TOP OF FOOTING AT PERIMETER STEM WALLS TO BE -4'-0" BELOW FF, TYP.
- 12. RETAINING STEM WALL. SEE DETAIL 4A/S3.1.
- 13. SITE RETAINING WALL. SEE DETAIL 6/S3.1.
- 14. 4" CONCRETE EQUIPMENT PAD ON TOP OF SLAB.

LEGEND

CJ

EDGE OF FOOTING COLUMN AND BASE PLATE, SEE S3.1

PIER AND ISOLATED SPOT FOOTING BELOW FLOOR SLAB, SEE S3.1 & 10/S5.3

CONCRETE SLAB ON GRADE

DENOTES BASE PLATE TYPE,

CONTROL JOINT

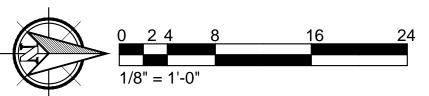
SEE 10/S5.3

DENOTES COLUMN TYPE, PER SCHEDULE ON S0.3

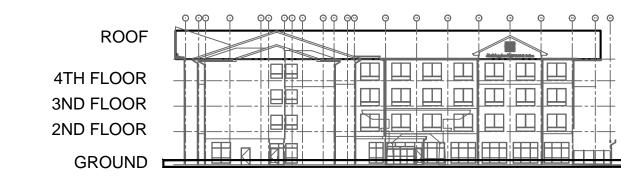
ANCHOR BOLT AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3 & DETAIL 4A/S5.3

HOLD DOWN ANCHOR AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3 & DETAIL 4B/S5.3

SHEAR WALL SEE SCHEDULE SHEET S0.3

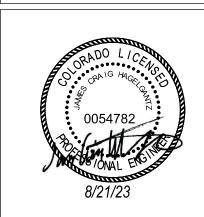


BUILDING LEVEL KEY









drawing shall not be used for any purpose other than for the construction of the referenced project. Any other use, reuse or modification of the prints or electronic files without our prior shall be used. Contractor(s) shall verify all layouts, dimension

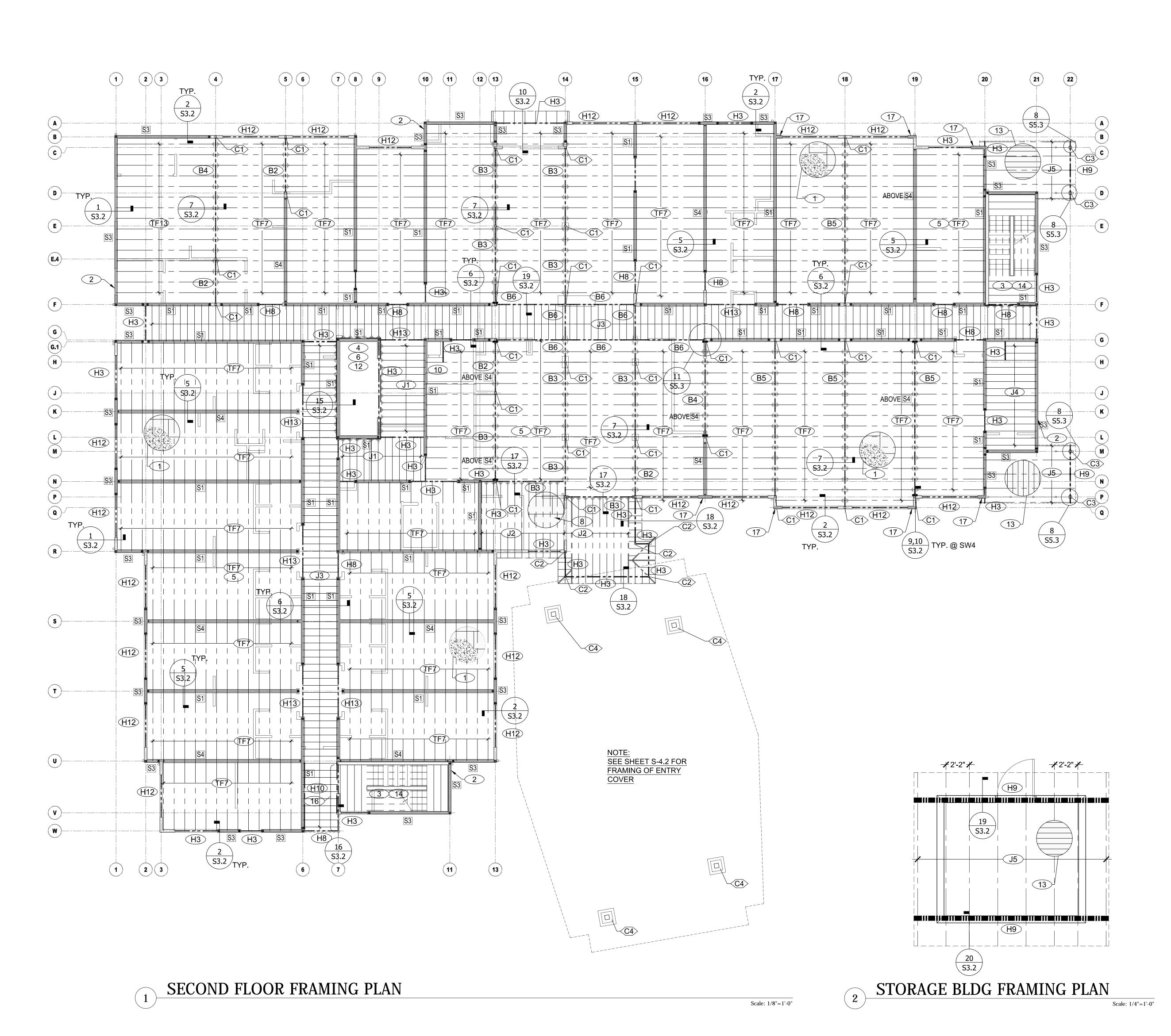
(NFC) indicates "Not for Construction" unless noted otherwis **DRAWING ISSUE DATES:**

REVISION DATES:

PROJECT MANAGER: NICK PIRKL

DRAWN BY: SHEET TITLE: FIRST FLOOR FOUNDATION PLAN

S1.1 SHEET: 11 OF 140



GENERAL NOTES

- G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.
- G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FRAMING.
- G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
- G4. FOR JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.
- G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.
- G6. SEE HEADER SCHEDULE S0.3 FOR BUILT-UP STUDS @ HEADER
- G7. SEE S5 SHEETS FOR TYPICAL WOOD, AND STEEL FRAMING DETAILS.

○ KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

- 3/4" A.P.A RATED FLOOR SHEATHING PANELS (EXPOSURE 1, SPAN RATING 0/24). NAIL @ ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8D @ 6" O.C. AND ALL INTERMEDIATE FRAMING @ 12" O.C. SEE 1, 2/S5.2 SEE DETAILS AND ARCHITECTURAL DRAWINGS FOR GYPCRETE TOPPING THICKNESS.
- 2. 2X WOOD STUDS. SEE S0.3 FOR SHEAR WALL SCHEDULE FOR SIZE AND SPACING. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
- 3. STAIRS, SEE S5.3 AND ARCHITECTURAL DRAWINGS.
- 4. ELEVATOR SHAFT, SEE FOUNDATION PLAN AND DETAILS.
- 5. PRE-MANUFACTURED JOISTS, SEE S0.3 AND DETAILS FOR PROFILE AND LOADS.
- 6. ELEVATOR FRAMING, SEE 14,15/S3.2
- 7. TRUSS MANUFACTURER "SSE" TO INCLUDE DEAD LOADS OF MECHANICAL UNITS IN THE DESIGN OF THE ROOF TRUSSES BELOW THE UNITS AND PANELS SHOWN ON PLAN.
- 8. 23/32" A.P.A. RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8d @ 6" O.C. AND ALL INTERMEDIATE FRAMING AT 12" O.C. SEE 1,2/S5.2
- 9. 2X BLOCKING BETWEEN 2X FLOOR JOIST.
- 10. OPENING IN FLOOR FOR CHUTE, SEE DETAIL 7/S3.3
- 11. 2X BLOCKING AT 4'-0" O.C. SEE DETAIL.2/S3.2
- 12. CONTINUOUS RIM TRUSS, SEE DETAIL 1,2/S3.2
- 13. 2"x6" T&G ROOF DECKING OVER ROOF JOISTS.
- 14. AT INTERIOR WALLS OF ELEV./STAIRWELLS SEE DETAIL 8/S3.2 TYPICAL.
- 15. TRUSS MANUFACTURER TO PROVIDED DUCT CHASES THRU ROOF TRUSSES, COORDINATE LOCATIONS WITH MECHANICAL DRAWINGS.
- 16. HEADER BEAM IS CONTINUOUS FROM GRID LINES "U" TO "W".
- 17. SIMPSON STRONG-WALL SB SWSB24X14 SEE SHEET S3.4 FOR DETAILS.

LEGEND

BEAMS PER HEADER AND BEAM SCHEDULES, SEE S0.3

GRID LINE

JOIST HANGER (LONG LEG DENOTES CONTINUOUS BEAM) SEE DETAIL 12/S5.3

DENOTES JOIST, BEAM, HEADERS PER

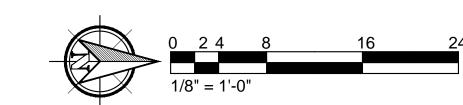
SCHEDULE S0.3

DENOTES SHEAF

DENOTES SHEAR WALL LOCATION/TYPE, SEE S0.3 & DETAIL 4A&B/S5.3

CONTINUOUS RIM JOIST, SEE KEYNOTE 12

MOMENT CONNECTION DETAIL SEE SHEET 10/S3.2 & 9/S3.2

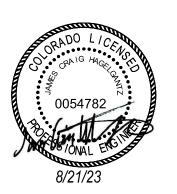


BUILDING LEVEL KEY











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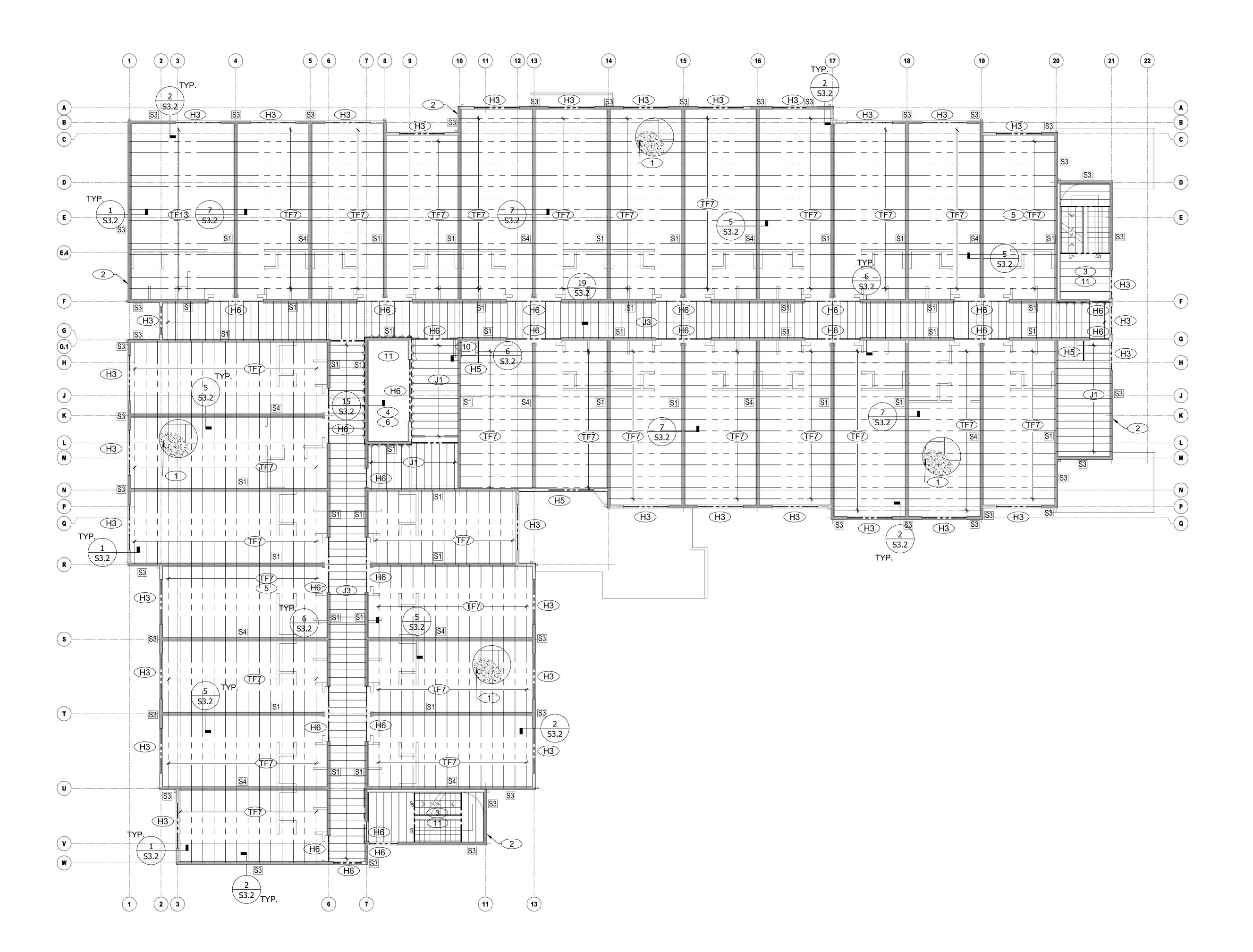
REVISION DATES:

PROJECT MANAGE

SHEET TITLE:

SECOND FLOOR
FRAMING PLAN

S1.2 SHEET: 12 OF 140



THIRD & FOURTH FLOOR FRAMING PLAN

Scale: 1/8"=1'-0"

GENERAL NOTES

- G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.
- G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FRAMING.
- G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
- G4. FOR JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.
- G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.
- G6. SEE HEADER SCHEDULE S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.
- G7. SEE S5 SHEETS FOR TYPICAL TRUSS, WOOD, AND STEEL FRAMING DETAILS.

KEYED NOTES

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- 2. 2X WOOD STUDS. SEE S0.3 FOR SHEAR WALL SCHEDULE FOR SIZE AND SPACING. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
- 3. STAIRS, SEE S5.3 AND ARCHITECTURAL DRAWINGS.
- 4. ELEVATOR SHAFT, SEE FOUNDATION PLAN AND DETAILS.
- PRE-MANUFACTURED TRUSSES, SEE S0.3 AND DETAILS FOR PROFILE AND LOADS.
- 6. ELEVATOR FRAMING, SEE 5,6/S3.3
- 7. TRUSS MANUFACTURER "SSE" TO INCLUDE DEAD LOADS OF MECHANICAL UNITS IN THE DESIGN OF THE ROOF TRUSSES BELOW THE UNITS SHOWN.
- 8. 23/32" A.P.A. RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8d @ 6" O.C. AND ALL INTERMEDIATE FRAMING AT 12" O.C. SEE 1,2/S5.2
- 9. 2X BLOCKING BETWEEN FLOOR TRUSSES.
- 10. OPENING IN FLOOR FOR CHUTE, WITH DOUBLE FLOOR JOIST FRAMING AROUND OPENING, SEE DETAIL 7/S3.3
- 11. AT INTERIOR WALLS OF ELEV./STAIRWELLS SEE DETAIL 8/S3.2 TYPICAL.

LEGEND

JOIST CENTER LINE

BEAMS PER HEADER

BEAMS PER HEADER AND BEAM SCHEDULES, SEE \$0.3

---- GRID LINE

JOIST HANGER (LONG LEG DENOTES CONTINUOUS BEAM) SEE DETAIL 12/S5.3

DENOTES JOIST, BEAM, HEADERS PER SCHEDULE S0.3

DENOTES SHEAR WALL LOCATION/TYPE, SEE

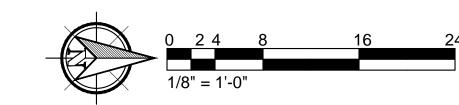
BLOCKING @ EXTERIOR WALL PARALLEL TO FLOOR FRAMING SEE KEYNOTE 11.

S0.3 & DETAIL 4A&B/S5.3

— CONTINUOUS RIM JOIST, SEE KEYNOTE 12

WALL SEE SCHEDULE SHEET S0.3

HOLD DOWN ANCHOR AT END OF SHEAR

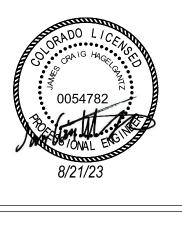


BUILDING LEVEL KEY

<u></u>







3400 S. LINCOLN AVE

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REVISION DATES:

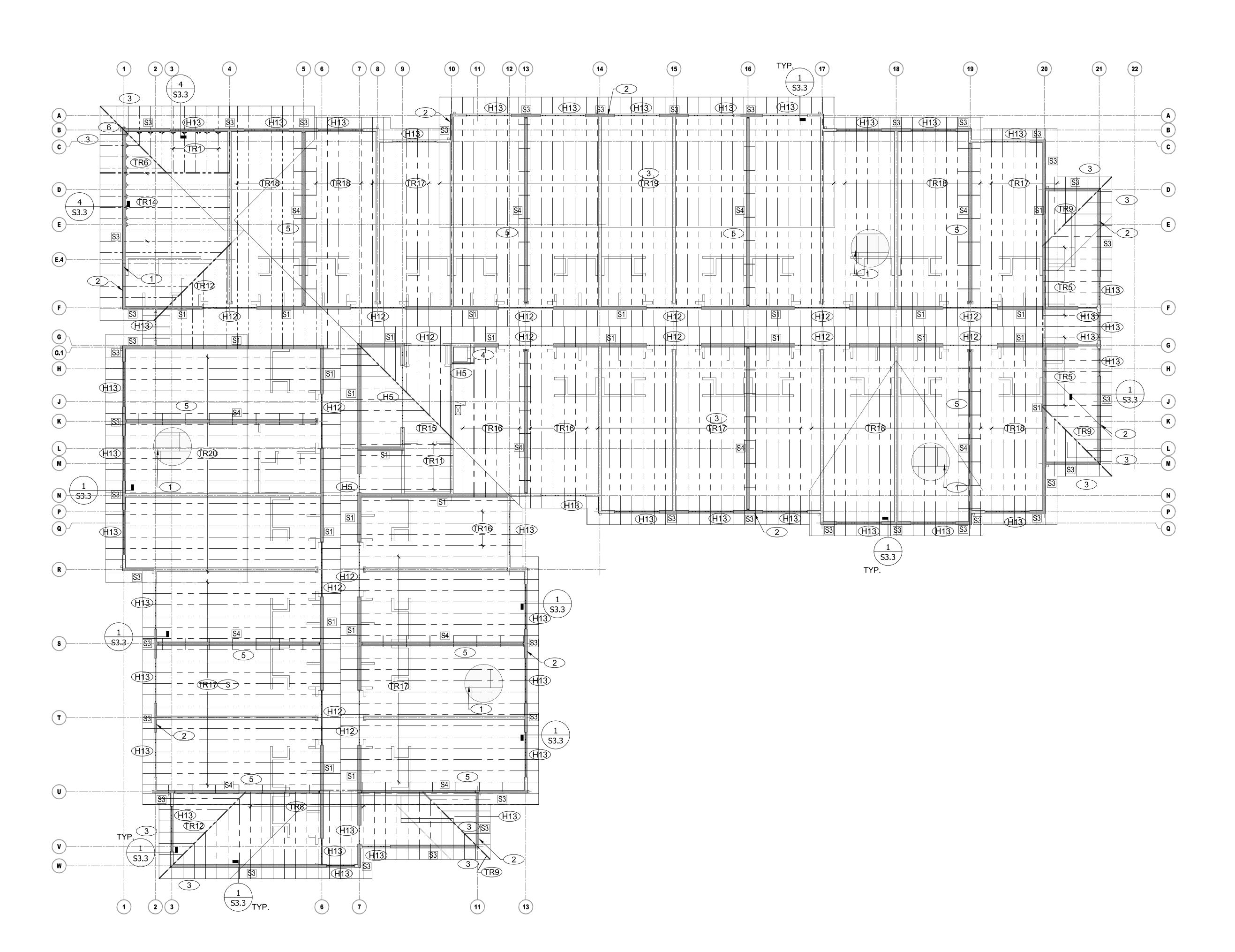
PROJECT MANAGER:

NICK PIRKL

SHEET TITLE:
THIRD & FOURTH

FLOOR FRAMING PLAN

SHEET: 13 OF 140



ROOF FRAMING PLAN Scale: 1/8"=1'-0"

GENERAL NOTES

- G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.
- G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FORMING.
- G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
- G4. FOR TRUSS/JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.
- G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.
- G6. ROOF TRUSS BRACING PER MANUFACTURER REQUIREMENTS. AND "BCSI TRUSS INSTALLATION & BRACING." B5-B9
- G7. SEE HEADER SCHEDULE ON S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.
- G8. SEE S5 SHEETS FOR TYPICAL TJI, WOOD, AND STEEL FRAMING DETAILS.

KEYED NOTES

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- 1. 23/32" A.P.A RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8D @ 6" O.C. AND ALL INTERMEDIATE FRAMING @ 12" O.C. SEE 1, 2/S5.2
- 2x_ WOOD STUDS, SEE S0.3 FOR SHEAR WALL SCHEDULE. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
- 3. PRE-MANUFACTURED WOOD ROOF TRUSSES @ 24" O.C. SEE S0.3 FOR TRUSS PROFILE AND LOADING.
- 4. ROOF CHUTE OPENING WITH DOUBLE 2X12 JOIST FRAMING AROUND OPENING DETAIL 7/S3.3.
- 5. 2X4 BLOCKING AT SHEAR WALL SEE DETAIL 3/S5.1
- SIMPSON TBE CONNECTOR AT RIDGE TRUSS BEARING EACH SIDE OF TRUSS.

LEGEND

BEAMS PER HEADER AND BEAM SCHEDULES, SEE S0.3

GRID LINE

JOIST HANGER (LONG LEG DENOTES

CONTINUOUS BEAM) SEE DETAIL 12/S5.3

DENOTES JOIST, BEAM, HEADERS PER SCHEDULE S0.3

S0.3 & DETAIL 4A&B/S5.3

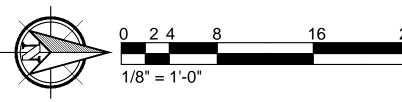
BLOCKING @ EXTERIOR WALL PARALLEL TO FLOOR FRAMING SEE KEYNOTE 11.

CONTINUOUS RIM JOIST, SEE KEYNOTE 12

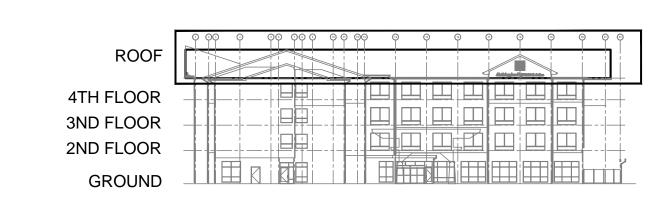
HOLD DOWN ANCHOR AT END OF SHEAR

WALL SEE SCHEDULE SHEET S0.3

DENOTES SHEAR WALL LOCATION/TYPE, SEE



BUILDING LEVEL KEY









3400 S. LINCOLN AVE STEAMBOAT SPRINGS, CO 804

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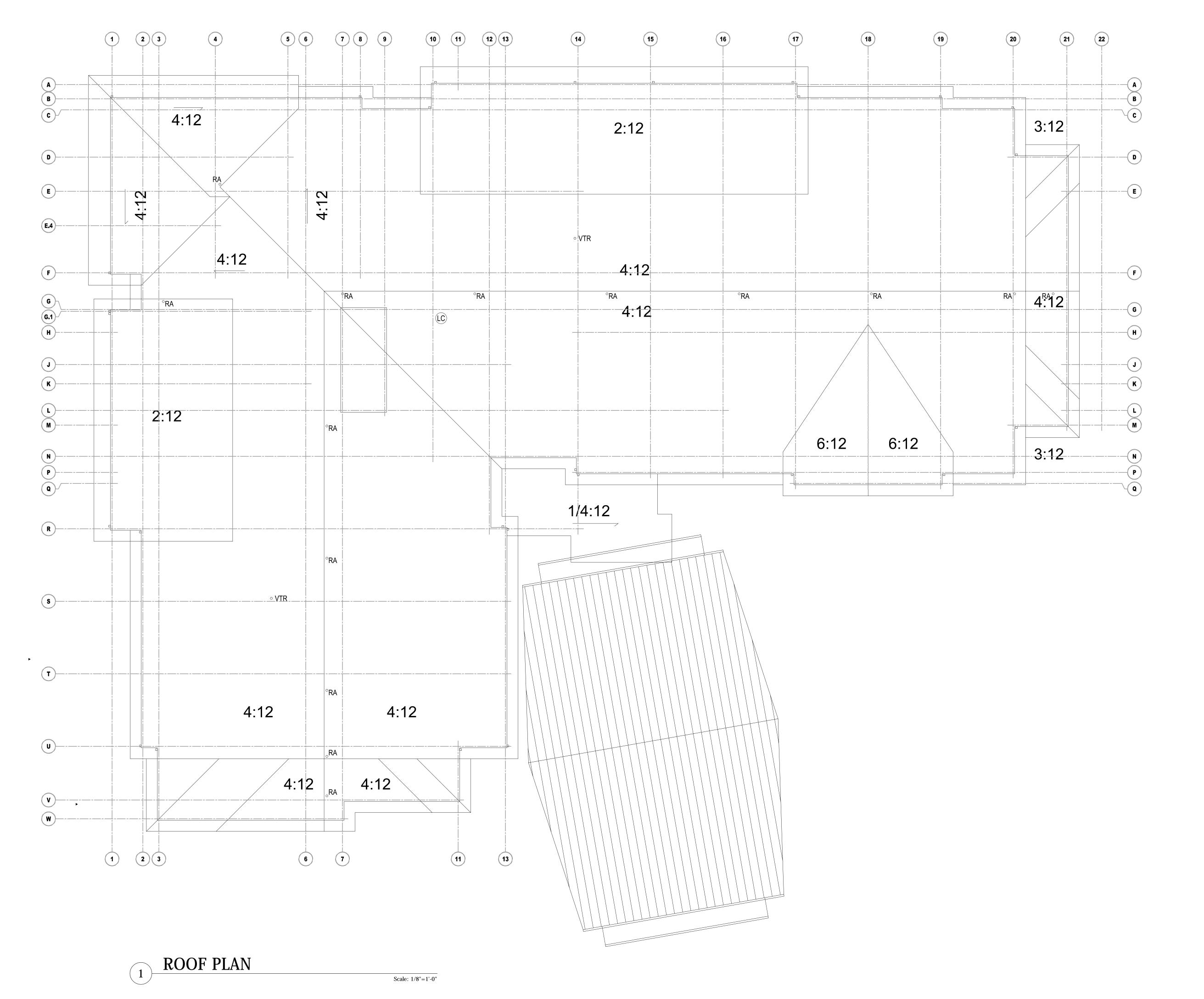
PROJECT MANAGER: NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
ROOF
FRAMING PLAN

S1.4

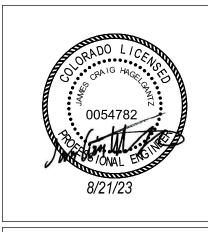
SHEET: 14 OF 140



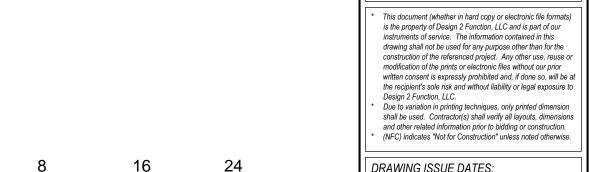
GENERAL NOTES

- G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.
- G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FORMING.
- G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.
- G4. FOR TRUSS/JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.
- G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.
- G6. ROOF TRUSS BRACING PER MANUFACTURER REQUIREMENTS. AND "BCSI TRUSS INSTALLATION & BRACING." B5-B9
- G7. SEE HEADER SCHEDULE ON S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.
- G8. SEE S5 SHEETS FOR TYPICAL TJI, WOOD, AND STEEL FRAMING DETAILS.





STEAMBOAT SPRINGS CO 80487



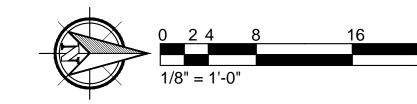
DRAWING ISSUE DATES:

REVISION DATES:

DRAWN BY:
NAP
SHEET TITLE:

SHEET TITLE: ROOF PLAN

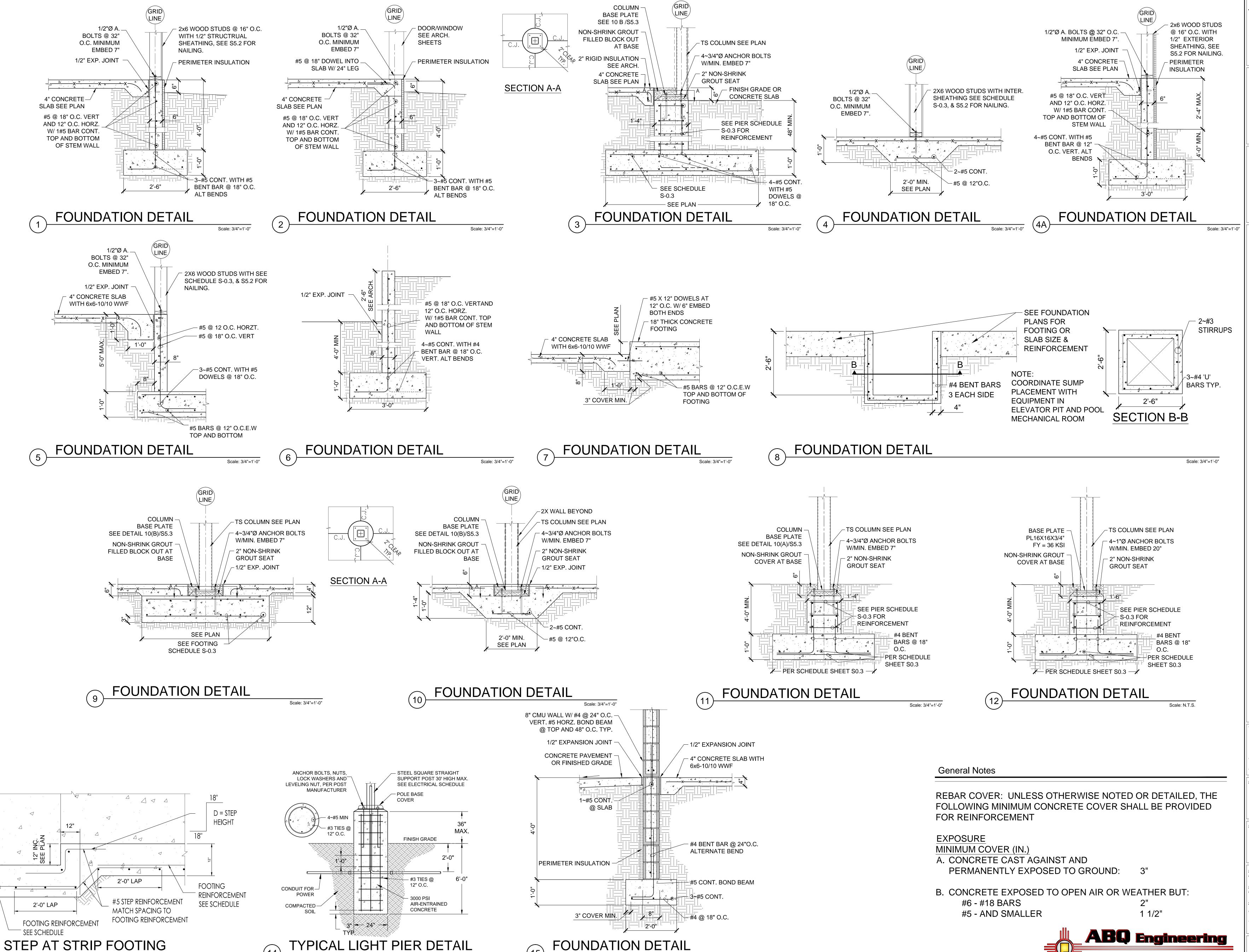
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BUILDING LEVEL KEY

ROOF	
4TH FLOOR	
3ND FLOOR	
2ND FLOOR	
GROUND	



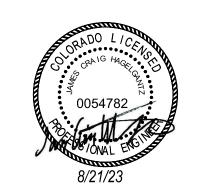


Scale: N.T.S.

Scale: 3/4"=1'-0"

DESIGN2> FUNCTION, LLC
P.O. BOX 93368 ALBUQUERQUE NEW MEXICO 87199-3368

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REVISION DATES

PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP

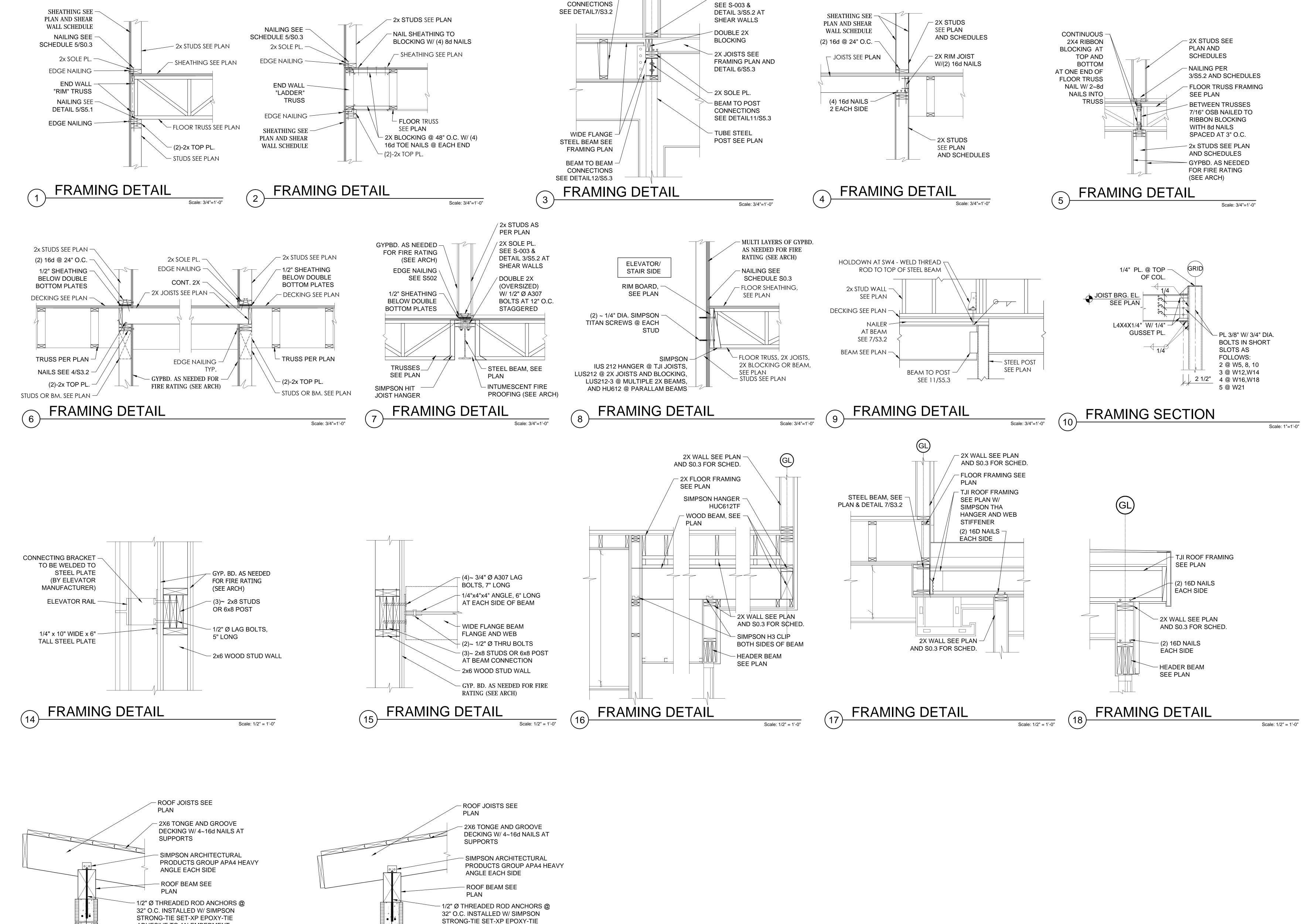
FOUNDATION DETAILS

S3.1 SHEET: 16 OF 140

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tele: 505.255.7802 Proj. No.: 23-015 www.abqeng.com



ADHESIVE TO AN EMBEDMENT

- 8" CMU WALL W/ #4 @ 24" O.C.

Scale: 1/2" = 1'-0"

VERT. #5 HORZ. BOND BEAM

DEPTH OF 6" IN GROUT.

@ TOP AND 48" O.C. TYP.

FRAMING DETAIL

ADHESIVE TO AN EMBEDMENT

- 8" CMU WALL W/ #4 @ 24" O.C.

VERT. #5 HORZ. BOND BEAM

Scale: 1/2" = 1'-0"

DEPTH OF 6" IN GROUT.

@ TOP AND 48" O.C. TYP.

FRAMING DETAIL

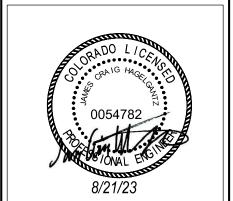
2x STUDS AS PER PLAN

2X SOLE PL.

TRUSS TO BEAM -



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PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP

ABQ Engineering

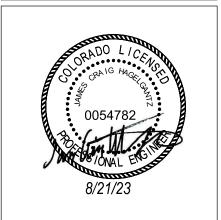
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SECTIONS AND DETAILS

S3.2 SHEET: 17 OF 140



LAUNDRY CHUTE PER

MANUFACTURER SEE

- LAUNDRY CHUTE 2X4

FRAMING SEE PLAN

ROOF SHEATHING

ROOF TRUSSES

STUD WALLS

- 2X BLOCKING

ROOF LEVEL

- ROOF BEAM

SEE PLAN

SEE PLAN

NAILER AT WALL SEE

DETAIL 8/S3.2

Scale: 3/4" = 1'-0"

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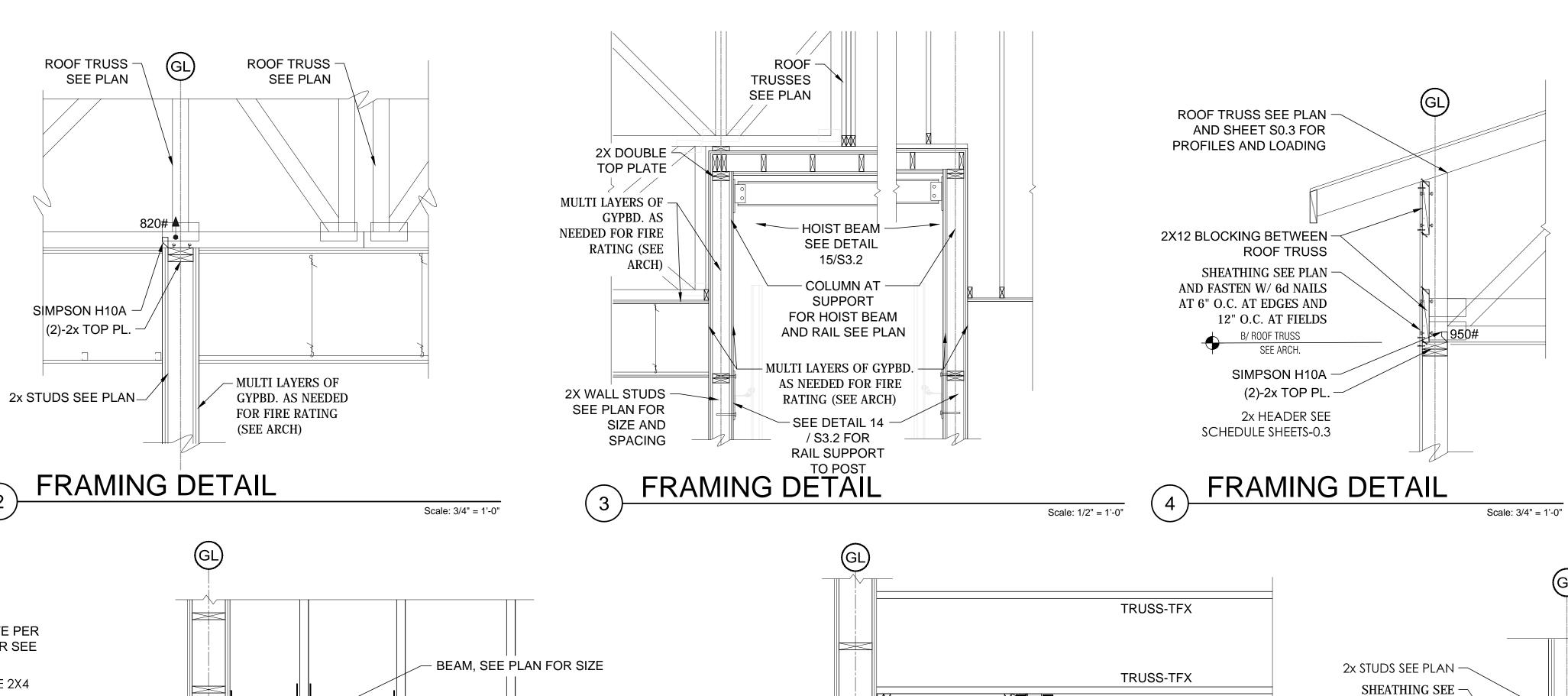
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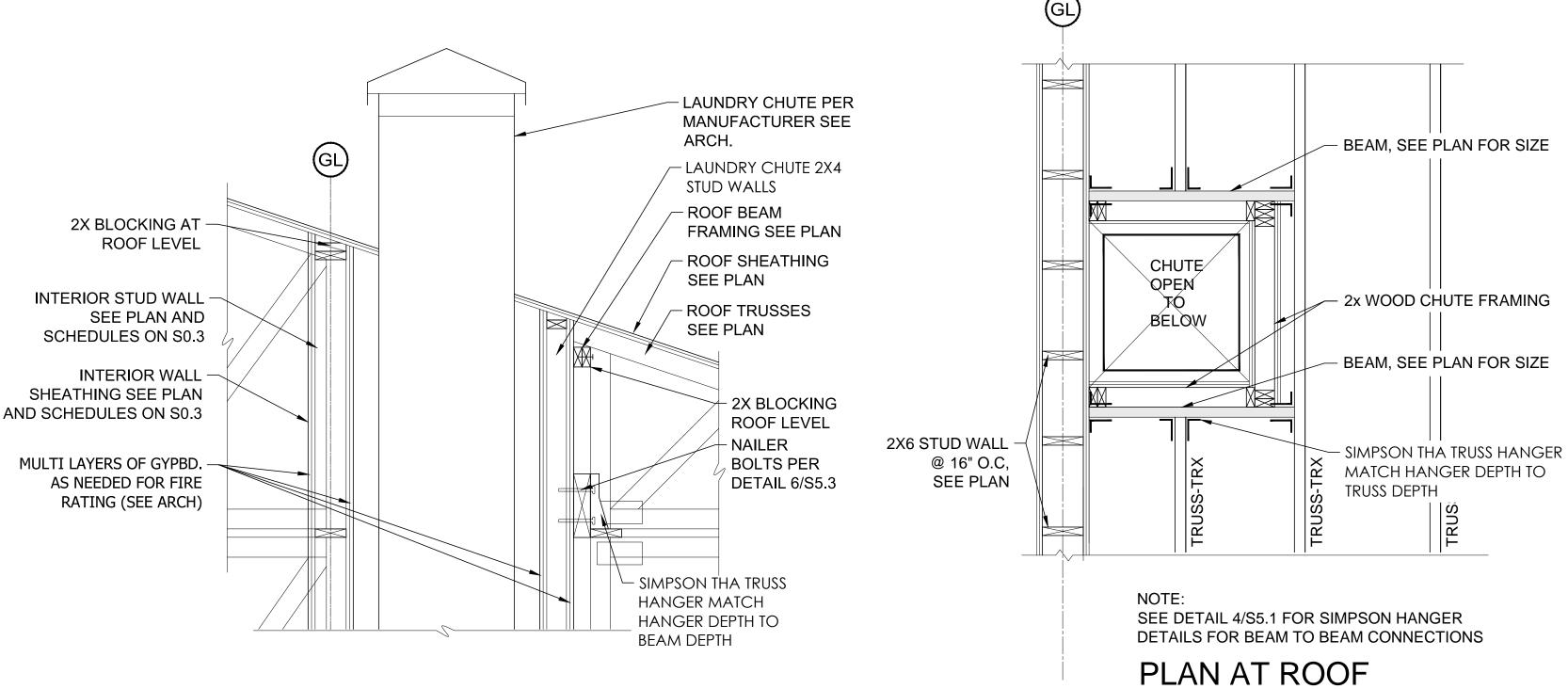
PROJECT MANAGER: NICK PIRKL

DRAWN BY: NAP

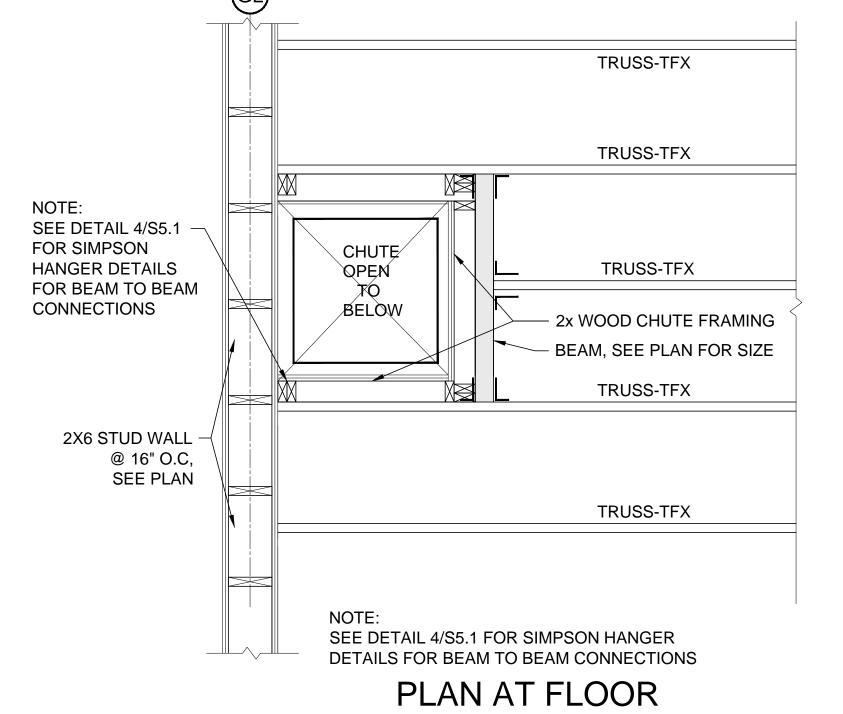
SHEET TITLE: SECTIONS AND DETAILS

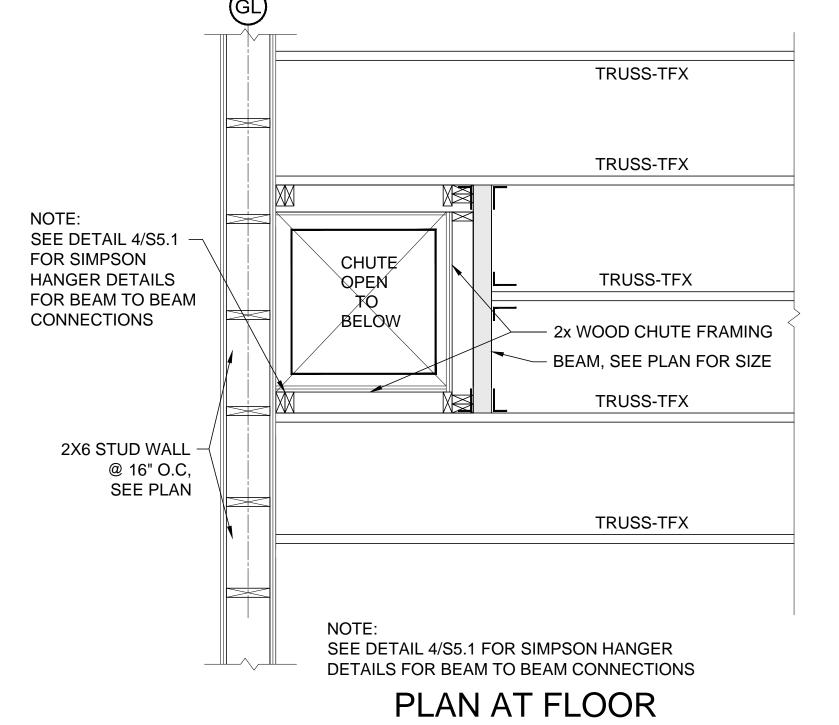
> S3.3 SHEET: 18 OF 140

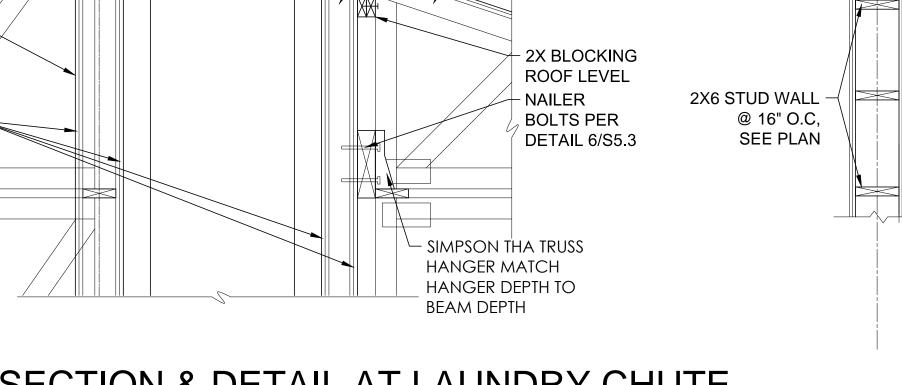




TRUSS-TFX SEE DETAIL 4/S5.1 FOR SIMPSON CHUTE/ HANGER DETAILS TRUSS-TFX QPEN FOR BEAM TO BEAM CONNECTIONS BELOW - 2x WOOD CHUTE FRAMING - BEAM, SEE PLAN FOR SIZE TRUSS-TFX 2X6 STUD WALL @ 16" O.C, SEE PLAN TRUSS-TFX SEE DETAIL 4/S5.1 FOR SIMPSON HANGER DETAILS FOR BEAM TO BEAM CONNECTIONS PLAN AT FLOOR







Scale: 3/4" = 1'-0"

FRAMING SECTION & DETAIL AT LAUNDRY CHUTE

ROOF TRUSS SEE PLAN -

PROFILES AND LOADING

2X12 BLOCKING BETWEEN -

AND SHEET S0.3 FOR

SHEATHING SEE PLAN

12" O.C. AT FIELDS

AND FASTEN W/ 6d NAILS

AT 6" O.C. AT EDGES AND

ROOF TRUSS

SEE ARCH.

SIMPSON H10A -

(2)-2x TOP PL.

2x HEADER SEE

FRAMING DETAIL

SCHEDULE SHEETS-0.3

950#

General Notes

PLAN AND SHEAR

WALL SCHEDULE

NAILER AT -

WALL SEE

DETAIL 8/S3.2

FRAMING SEE

HALLWAY FLOOR

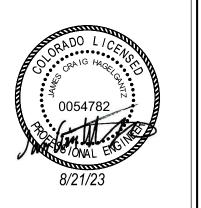
PLAN AND DETAIL

- 1. SEE PLAN AND DETAIL 2/S5.2 FOR ROOF SHEATHING FASTENING PATTERNS FOR FIELD AND EDGES.
- 2. SEE ARCHITECTURAL FOR WALL LAYERED GYP. BOARD FIRE PROTECTION
- 3. SEE S0.3 FOR WALL STUD AND SHEAR WALL SCHEDULE.
- 4. ALL FASTENERS SHALL BE PER IBC 2015 NAILING SCHEDULE FOR WOOD FRAMING MEMBERS UNLESS OTHERWISE SHOWN IN DETAILS.



WSWH ON WOOD FLOOR 11 STRONG-WALL BALLOON-FRAMING 8





day Inn Express

SSW & WSWH ANCHORAGE: CONCRETE STEMWALL

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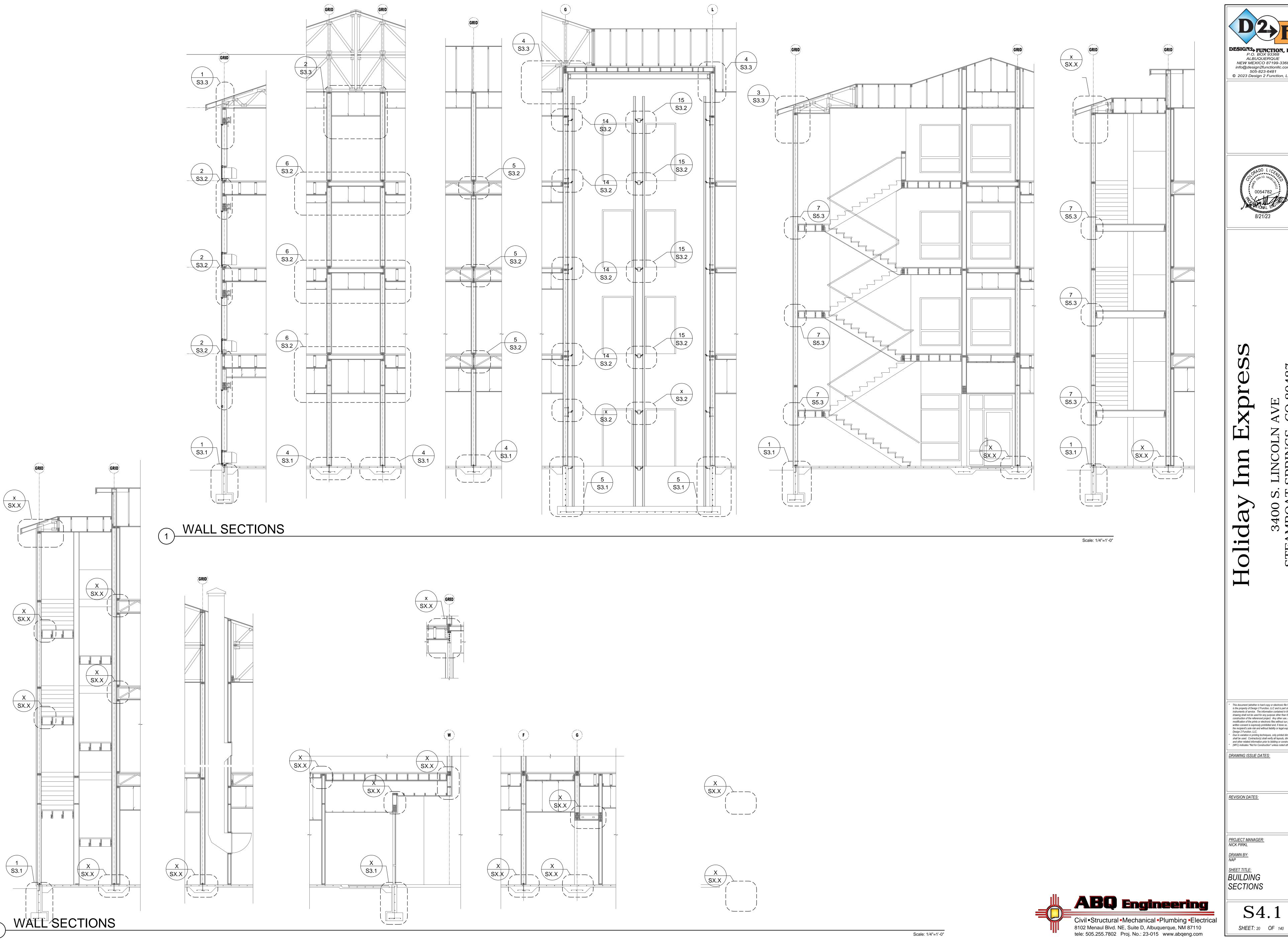
PROJECT MANAGE NICK PIRKL DRAWN BY:

SECTIONS AND DETAILS

ABQ Engineering

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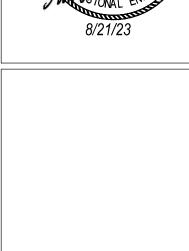
S3.4
SHEET: 19 OF 140





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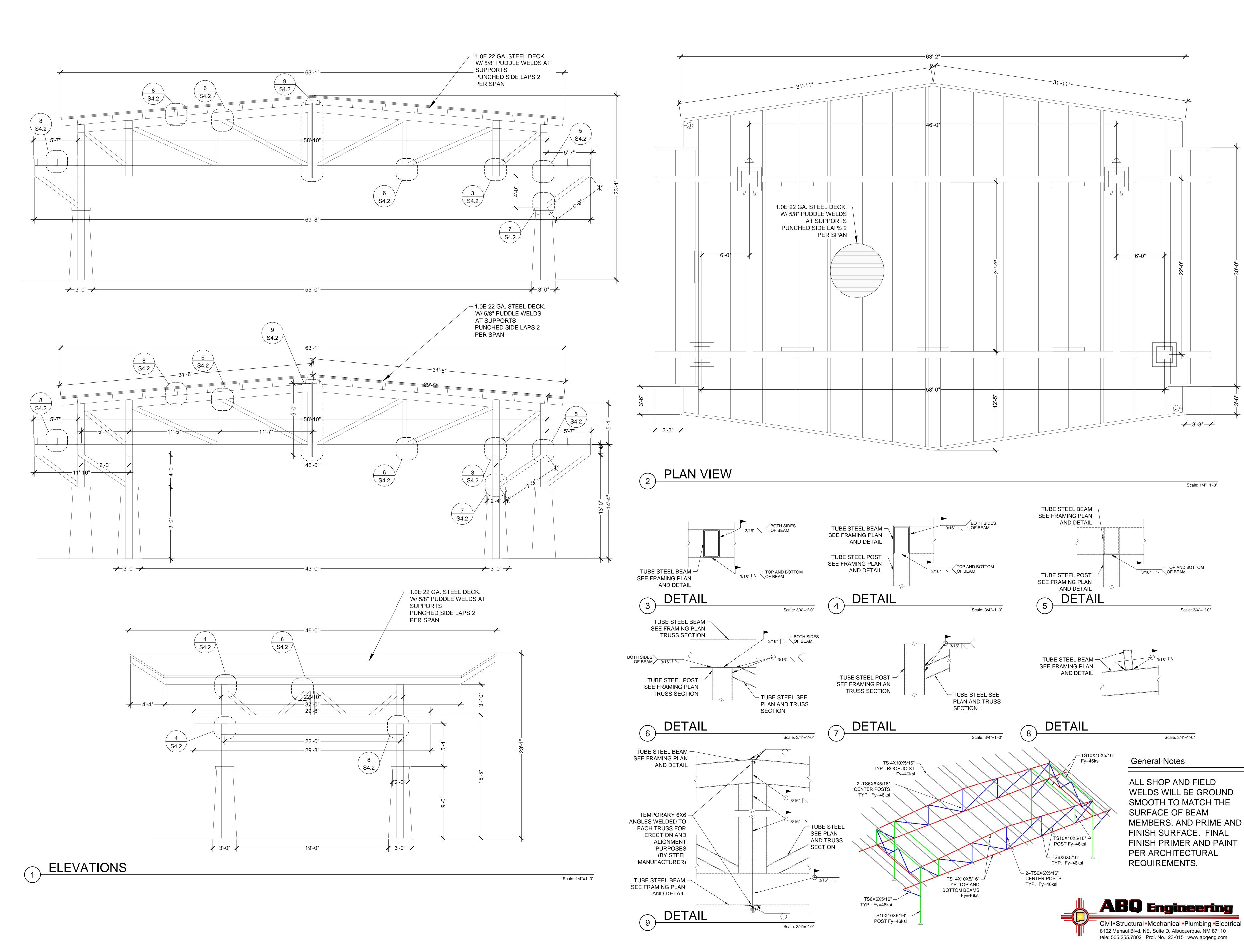
Due to variation in printing techniques, only printed dimension shall be used. Contractor(s) shall verify all layouts, dimensions and other related information prior to bidding or construction.

(NFC) indicates "Not for Construction" unless noted otherwise. DRAWING ISSUE DATES:

REVISION DATES:

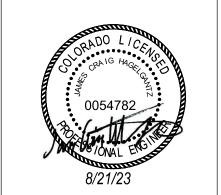
PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP SHEET TITLE: BUILDING SECTIONS

S4.1





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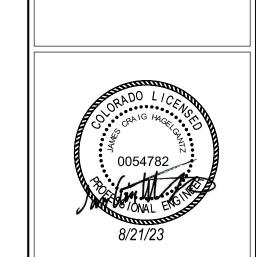
DRAWING ISSUE DATES:

REVISION DATES:

PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP

SHEET TITLE:
BUILDING
SECTIONS

S4.2 SHEET: 21 OF 140



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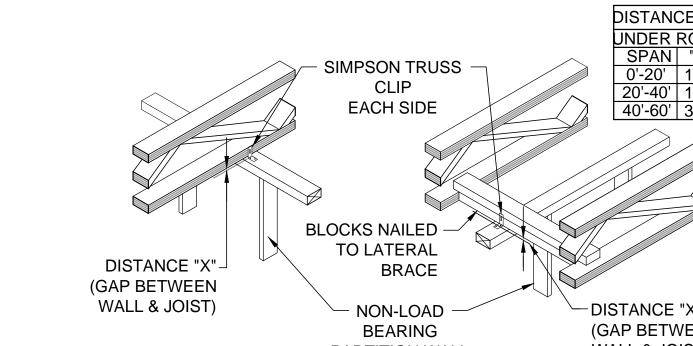
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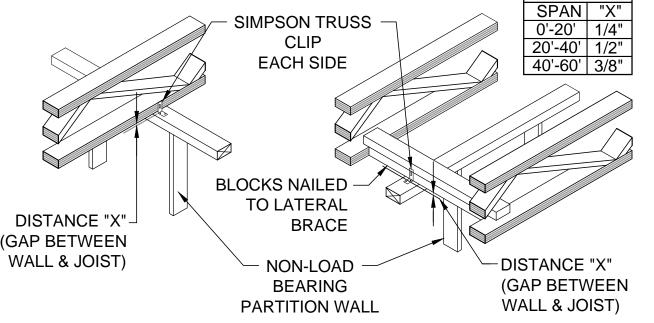
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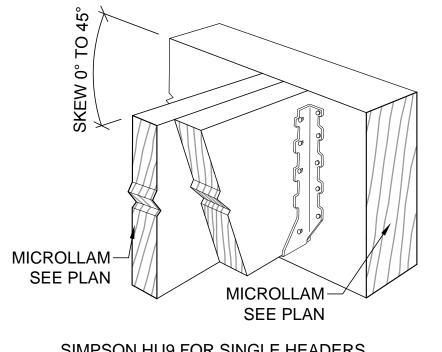
REVISION DATES

STRUCTURAL DETAILS

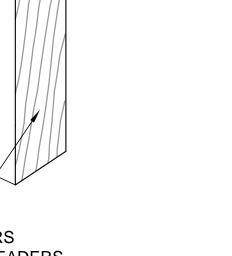
S5.1 SHEET: 22 OF 140

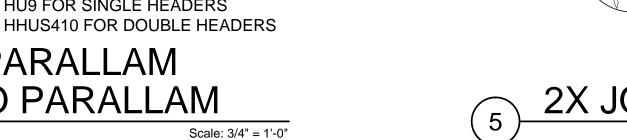


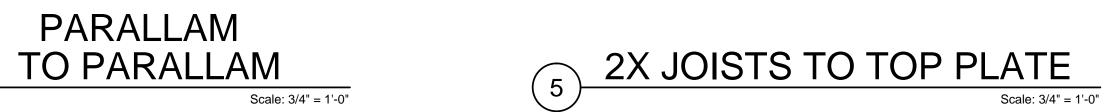








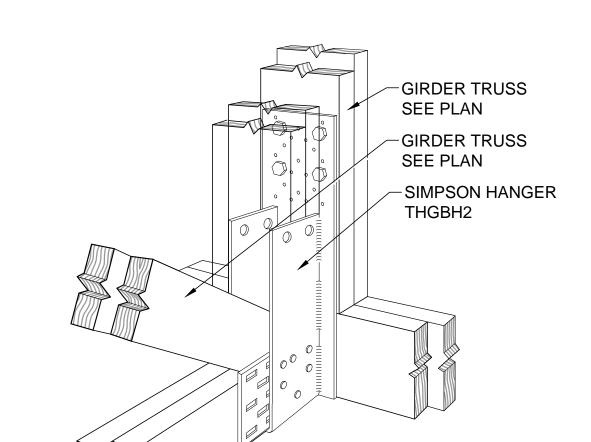


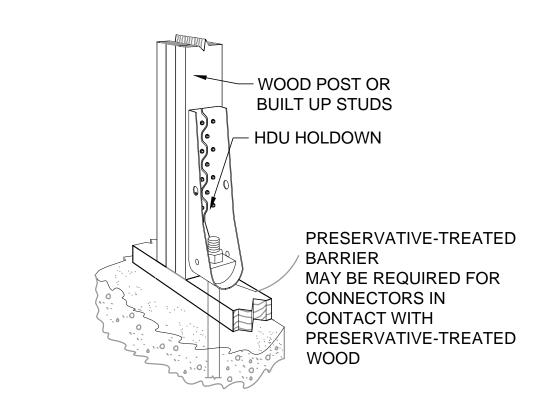


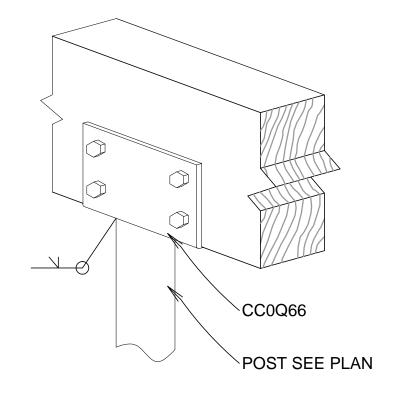
2X RIM BOARD— SEE PLAN

(2)10d ONE— EACH TOP & BOTTOM

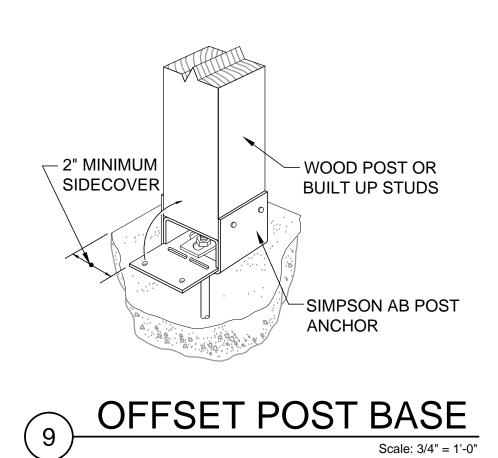
(2)10d— TOE NAIL

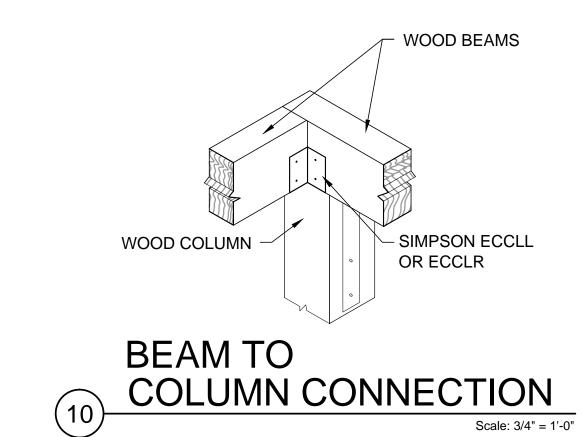






Scale: 3/4" = 1'-0"





TOE NAIL 10d

@ 6" O.C. LVL TO TOP PLATE

TOP PLATES

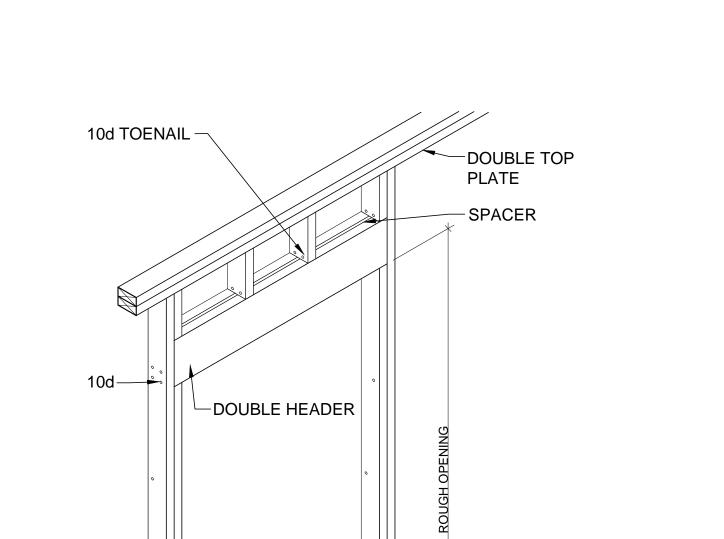
TJI JOISTS











-GIRDER TRUSS

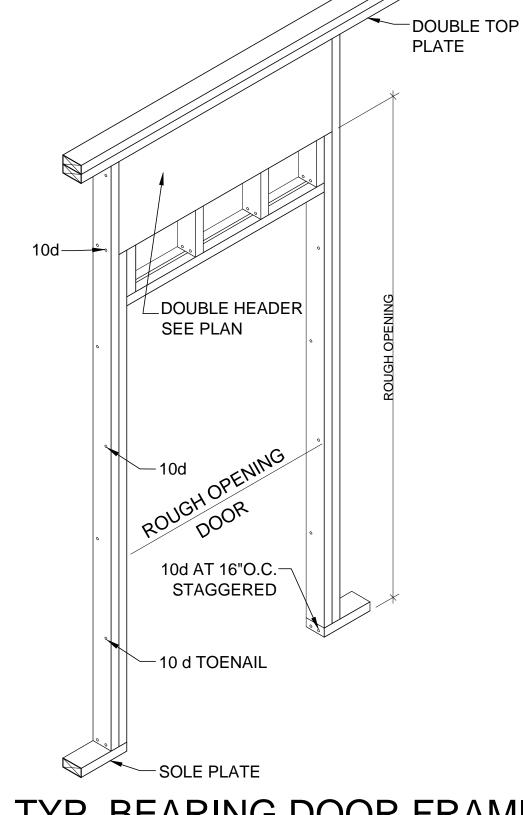
BEAM SEE PLAN

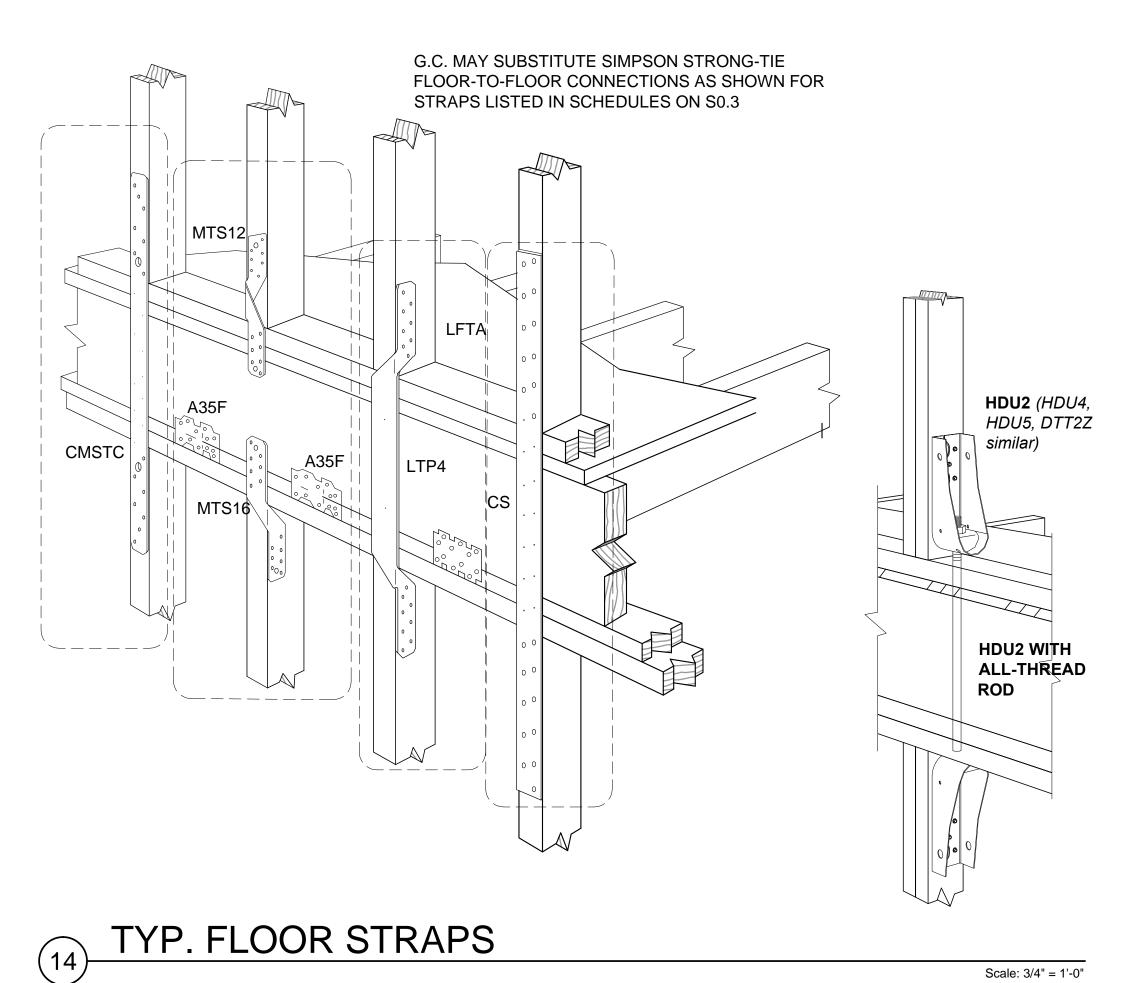
TRUSS/2X BEARING BLOCKING

Scale: 3/4" = 1'-0"

LATERAL SUPPORT

SEE PLAN

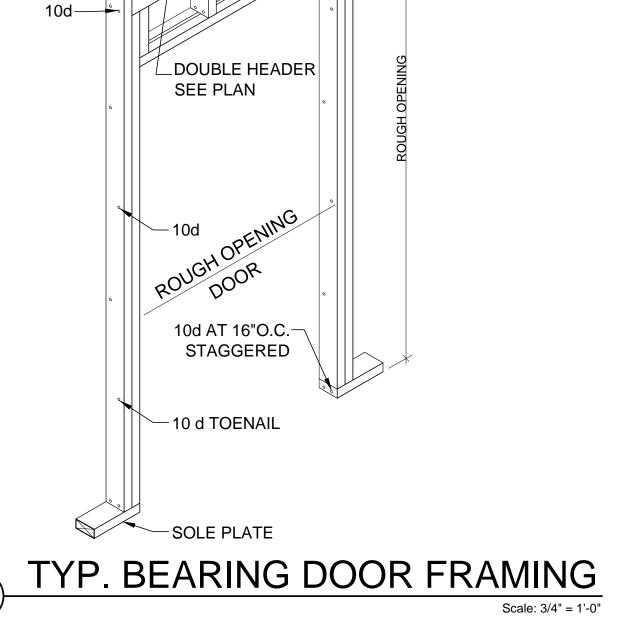




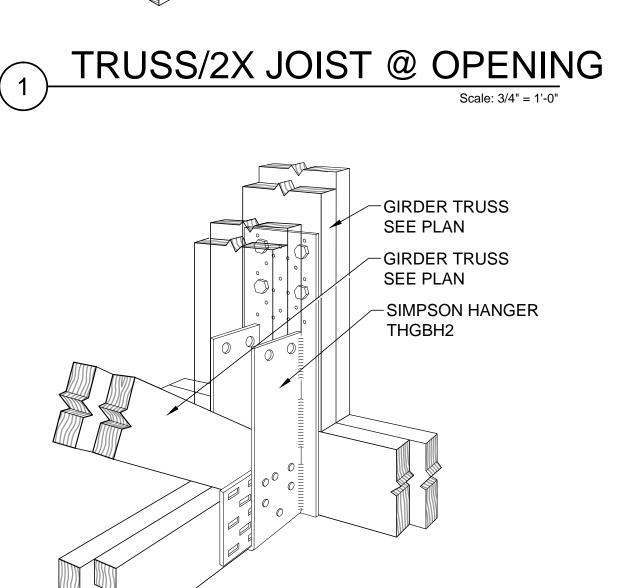


10d AT 16"O.C. STAGGERED

−10 d TOENAIL



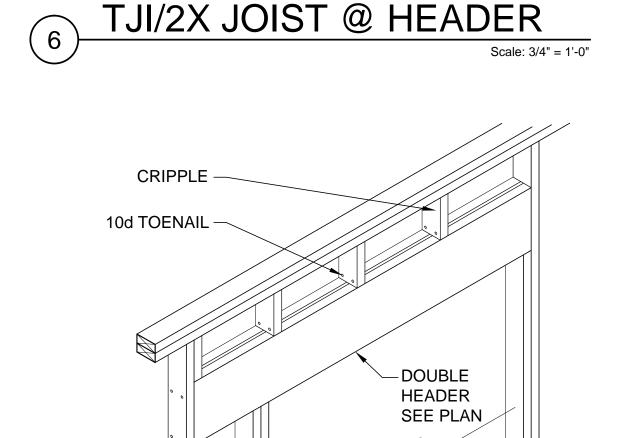
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-GIRDER TRUSS SEE PLAN

BEAM SEE PLAN

-SIMPSON HANGER



10d TOENAIL-BOTH STUDS

-SOLE PLATE

TYPICAL WINDOW FRAMING

Scale: 3/4" = 1'-0"

PLATE NAILING PER

IBC SCHEDULE

SCHEDULE-TYP.

- NAILING PER

- RIM TRUSS

NAILING PER

DETAIL 5/S5.1

PLATE NAILING

- 2x STUDS SEE

SCHEDULES

PER IBC SCHEDULE

PLATE NAILING PER

IBC SCHEDULE

SCHEDULE-TYP.

- NAILING PER

- RIM TRUSS

NAILING PER

DETAIL 5/S5.1

— 2x STUDS SEE

SCHEDULES

-DOUBLE TOP PLATE,

HOLD DOWN AND

AND SCHEDULE

ANCHOR SEE PLAN

-DOUBLE JAMB STUD MIN.

DOUBLE TOP

2X STUDS SEE -

PLAN AND

SCHEDULES

BOUNDARY

2X STUDS SEE -

PLAN AND

SCHEDULES

BOUNDARY

CONTINUOUS PANEL EDGE, SEE

FIELD NAILS 6d @ 12" O.C.

SCHEDULE FOR S0.3 FOR BLOCKING

TYP.

@ ALL OPENINGS.

ASTENERS INTO

DOUBLE PLATE

DIRECTION OF FACE GRAIN

PERPENDICULAR TO SUPPORT

SEE SOLE PLATE CONNECTION FROM

SHEAR WALL SCHEDULE

AND MATCH AT DOUBLE

TOP PLATES BELOW

FASTENERS INTO TOP 2X OF DOUBLE PLATE

SEE SOLE PLATE -

CONNECTION FROM

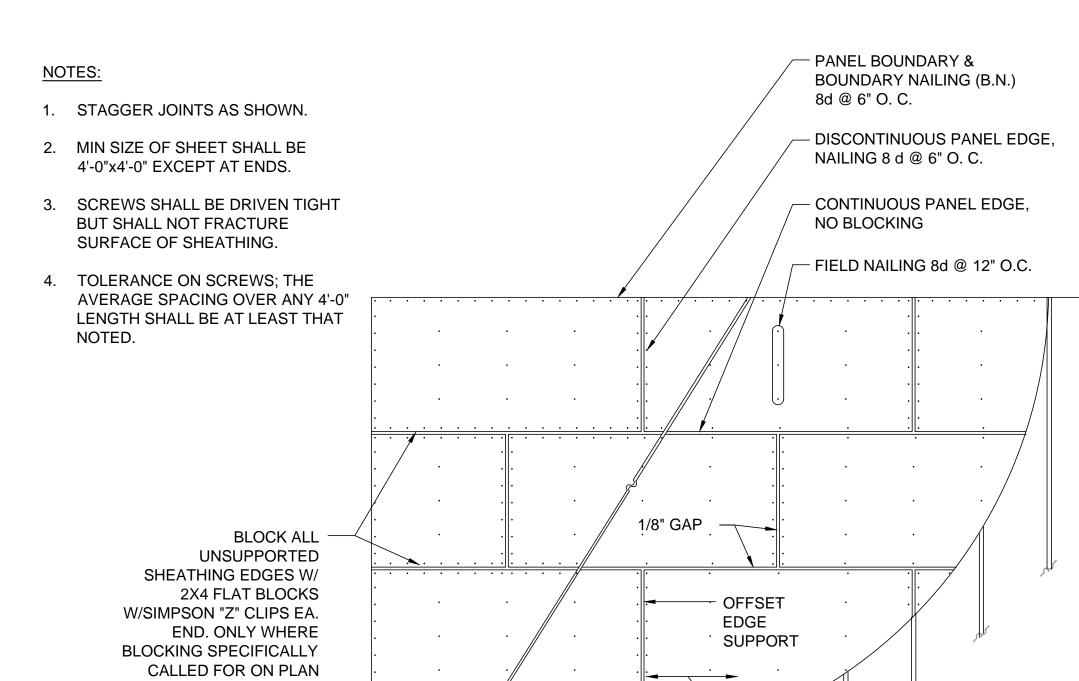
SHEAR WALL SCHEDULE

REVISION DATES

PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP

STRUCTURAL DETAILS

> S5.2 SHEET: 23 OF 140



2ND FLOOR SHEATHING (NAILING)

NOTES:

STAGGER JOINTS AS SHOWN.

EXCEPT AT ENDS.

SHEATHING.

2. MIN SIZE OF SHEET SHALL BE 4'-0"x4'-0"

SHALL NOT FRACTURE SURFACE OF

4. TOLERANCE ON NAILING; THE AVERAGE

SHALL BE AT LEAST THAT NOTED.

BLOCK ALL -

UNSUPPORTED

2X4 FLAT BLOCKS

END. ONLY WHERE

SHEATHING EDGES W/

W/SIMPSON "Z" CLIPS EA.

BLOCKING SPECIFICALLY

CALLED FOR ON PLAN

NAIL SPACING OVER ANY 4'-0" LENGTH

3. NAILS SHALL BE DRIVEN TIGHT BUT

Scale: 3/4" = 1'-0"

PANEL BOUNDARY &

NAILING 8 d @ 6" O. C.

8d @ 6" O. C.

NO BLOCKING

- OFFSET

EDGE

SUPPORT

BOUNDARY NAILING (B.N.)

- DISCONTINUOUS PANEL EDGE,

- CONTINUOUS PANEL EDGE,

FIELD NAILING 8d @ 12" O.C.

DIRECTION OF FACE GRAIN

PERPENDICULAR TO

SUPPORT

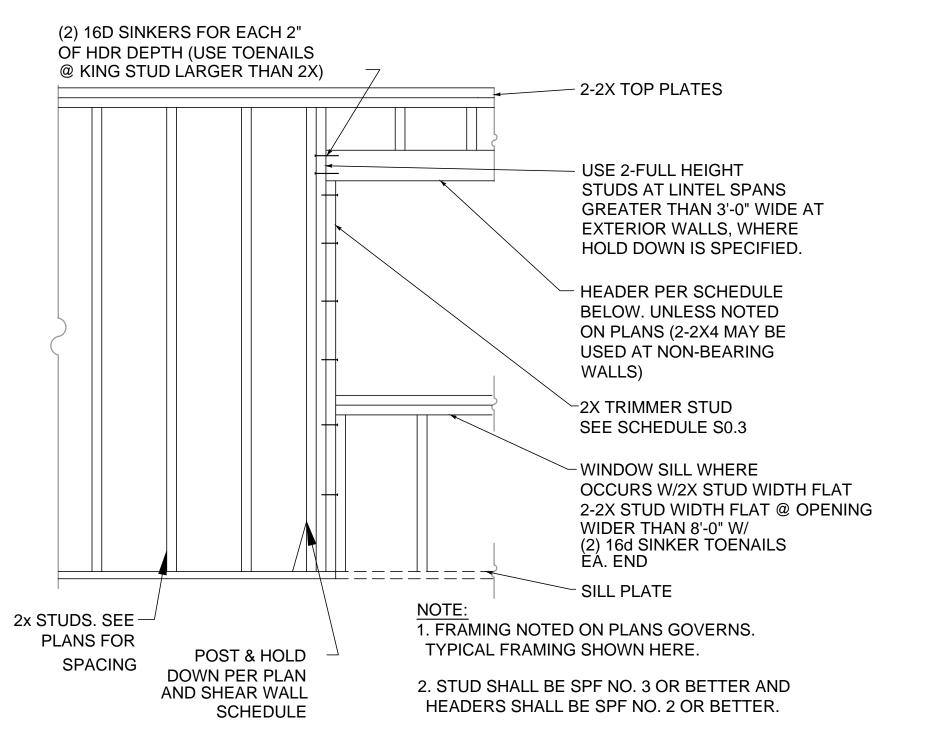
FRAMING, SEE PLANS

3RD, 4TH FLOOR & ROOF SHEATHING (NAILING)

Scale: 3/4" = 1'-0"

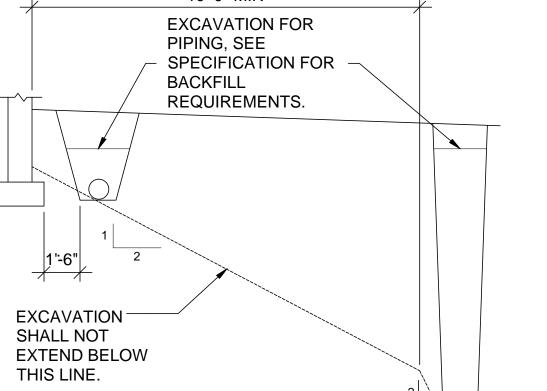
1/2"Ø ANCHOR BOLTS

@ 72" O.C. MIN., 7" EMBED U.O.N

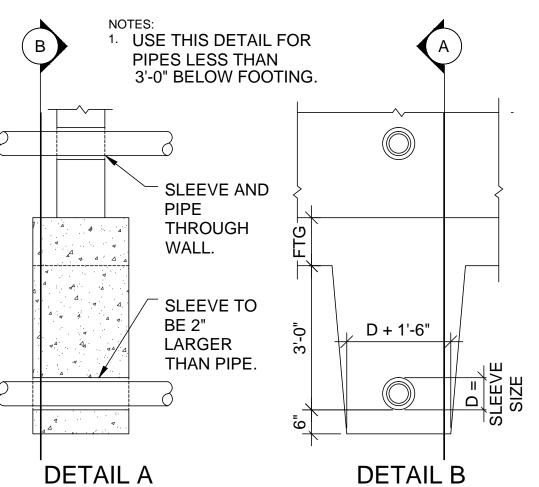




Scale: 3/4" = 1'-0"



IT IS THE CONTRACTORS RESPONSIBILITY TO PROVIDE SHORING, SHEATHING, ETC. TO MAINTAIN THE SIDES OF BACKFILL IS COMPLETE.

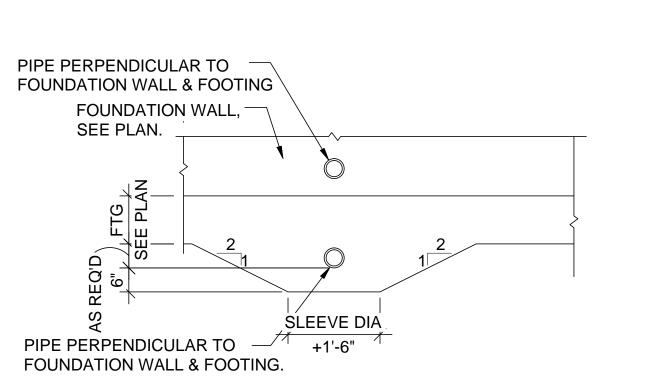


TRENCH AT WALLS, TYP

Scale: N.T.S.

PIPE AT STEM WALLS, TYP

Scale: N.T.S.



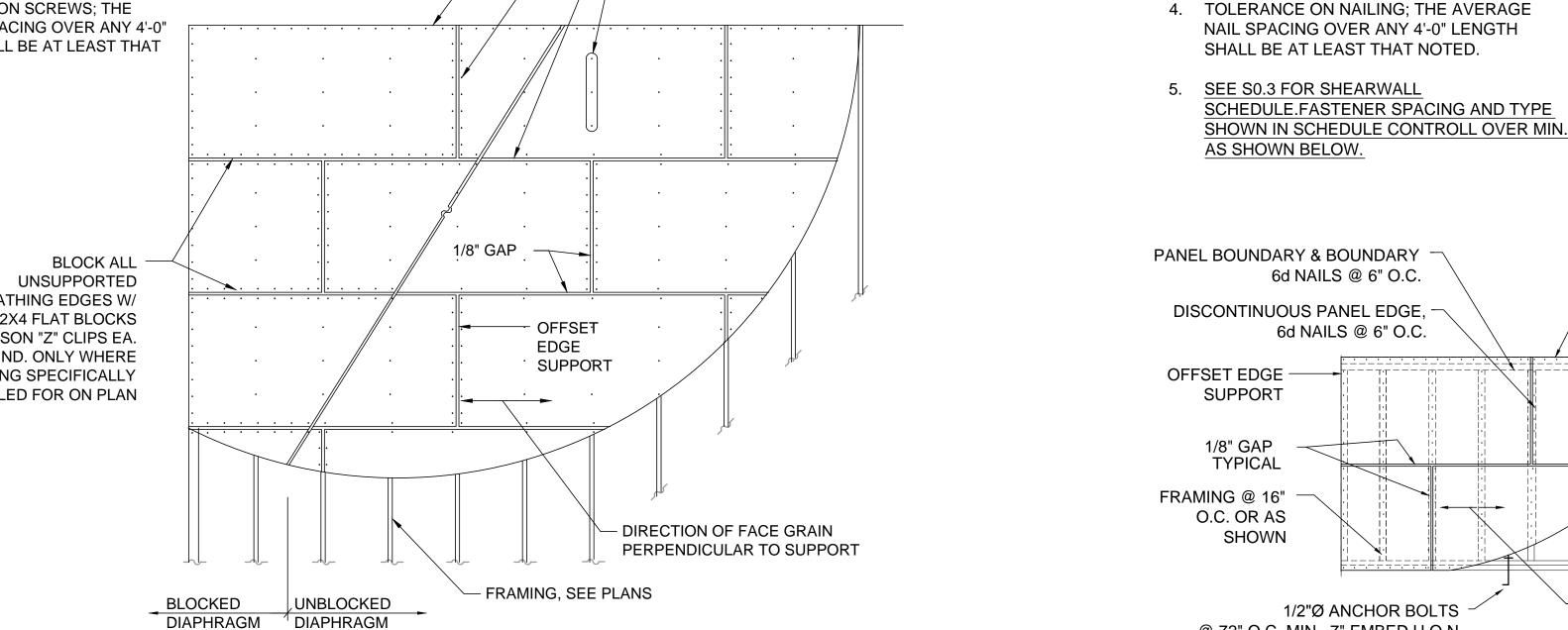
1. FOR PIPES WITHIN FOOTING DEPTH STEP FOOTING PER 1/S500 SO PIPES PASS THROUGH WALL. PROVIDE SLEEVE & GROUT INTO WALL.

2. FOR PIPES BELOW FOOTING PROVIDE SLEEVE & THICKEN CONCRETE FOOTING AS SHOWN OR STEP FOOTING BELOW PIPE.

3. SLEEVE DIAMETER TO BE 2" GREATER THAN PIPE OUTSIDE DIAMETER.

PIPE SLEEVE AT FOOTING, TYP
Scale: N.T.S.

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NOTES:

STAGGER JOINTS AS SHOWN.

EXCEPT AT ENDS.

2. MIN. SIZE OF SHEET SHALL BE 4'-0"x4'-0"

3. NAILS SHALL BE DRIVEN TIGHT BUT SHALL

6d NAILS @ 6" O.C.

6d NAILS @ 6" O.C.

NOT FRACTURE SURFACE OF SHEATHING.

SHEAR WALLS - MINIMUM FASTENER DETAIL

SHEATHING. (OSB/PLYWOOD) CONTINUOUS.

NOTE: EXTERIOR WALLS SHALL HAVE STRUCTURAL GRADE I

— 1/4 D MAX. STUD WIDTH NOTCH EDGE DIST TYP. ALL STUDS — - 4/10 D MAX. STUD WIDTH BORE

Scale: 3/4" = 1'-0"

NOTCH OR BORE 2x4 2x6 2x8 FRACTION OF STUD NOTCH AND BORE 1 3/8" 1 3/4" NOT TO OCCUR ON 4/10 D 1 3/8" 2 1/8" SAME STUD WITHIN 4 1/4" 6/10 D 2" 3 1/4" 4ft OF EACH OTHER

EXT.&BRG. STUDS

STUD NOTCH/HOLE SPEC.

4/10 MAX. STUD

6/10 MAX. STUD

WIDTH BORE

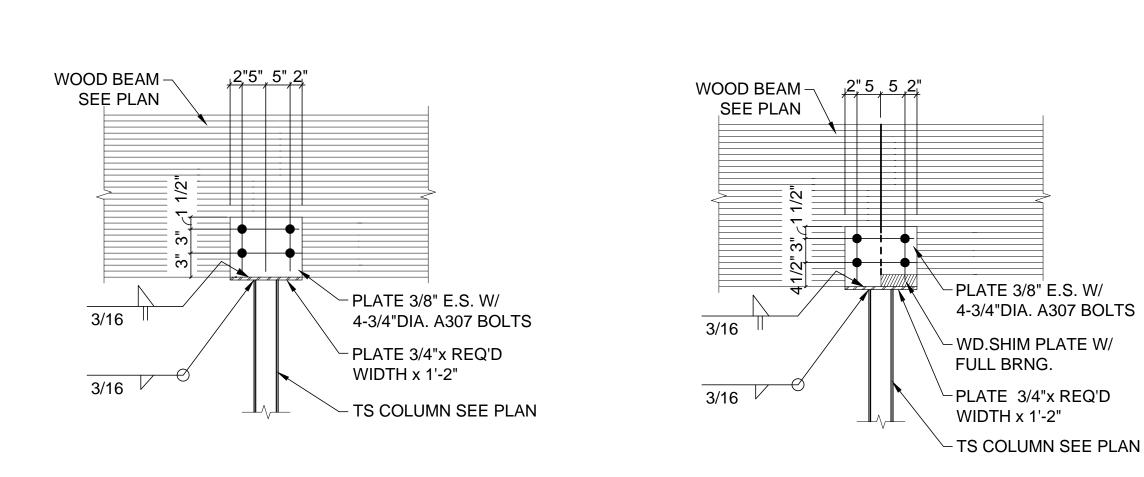
WIDTH NOTCH

− 5/8" MIN.

10'-0" MIN

THE EXCAVATION FROM CAVING IN UNTIL ALL WORK AND

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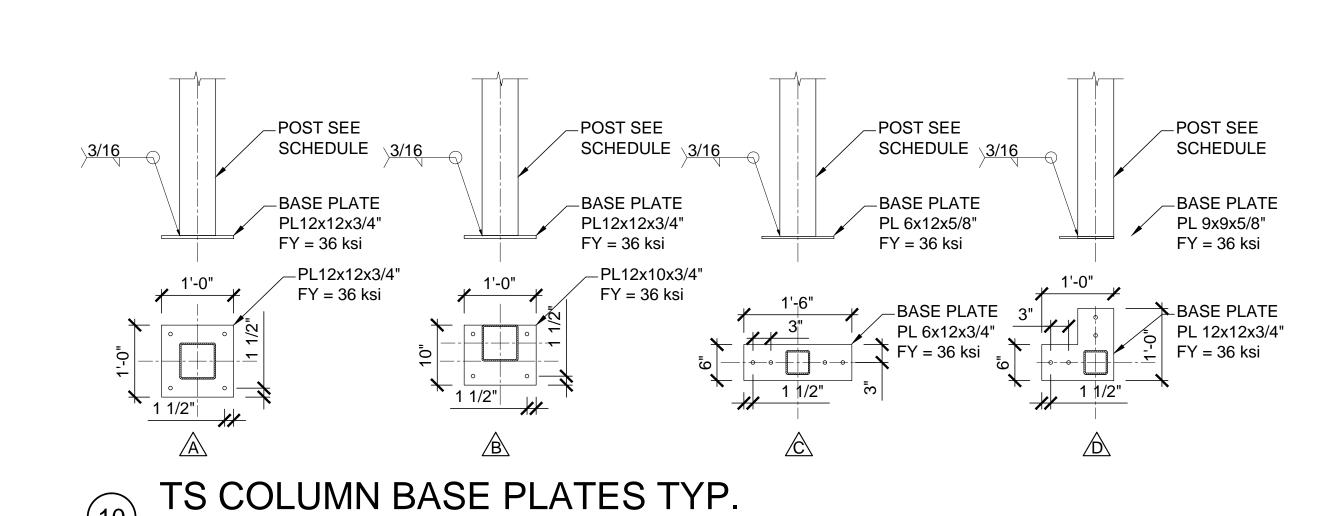


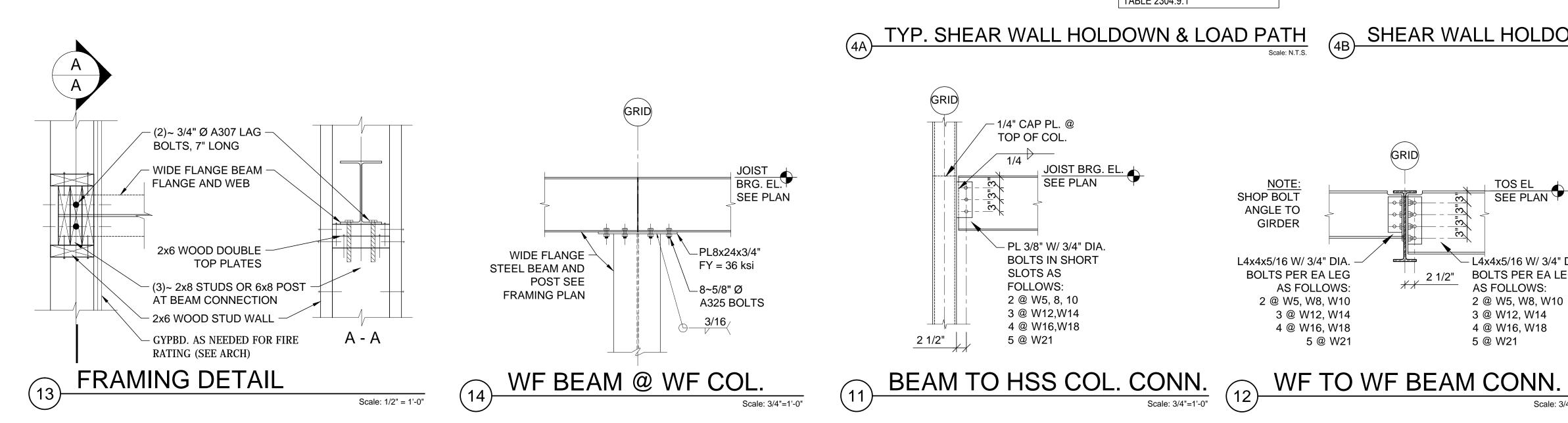
BEAM TO HSS COL.

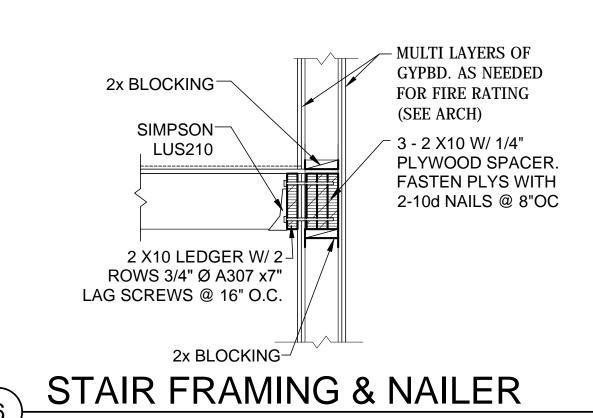
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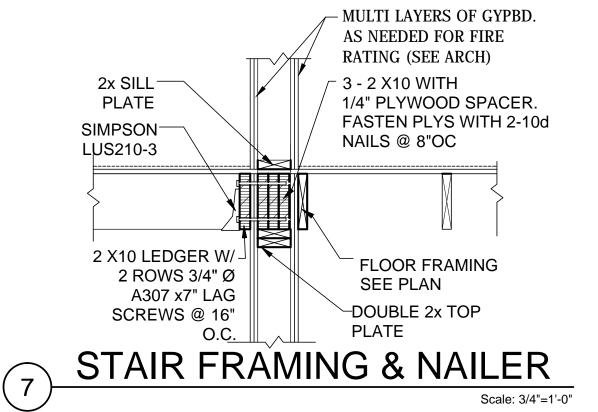
Scale: 3/4"=1'-0"

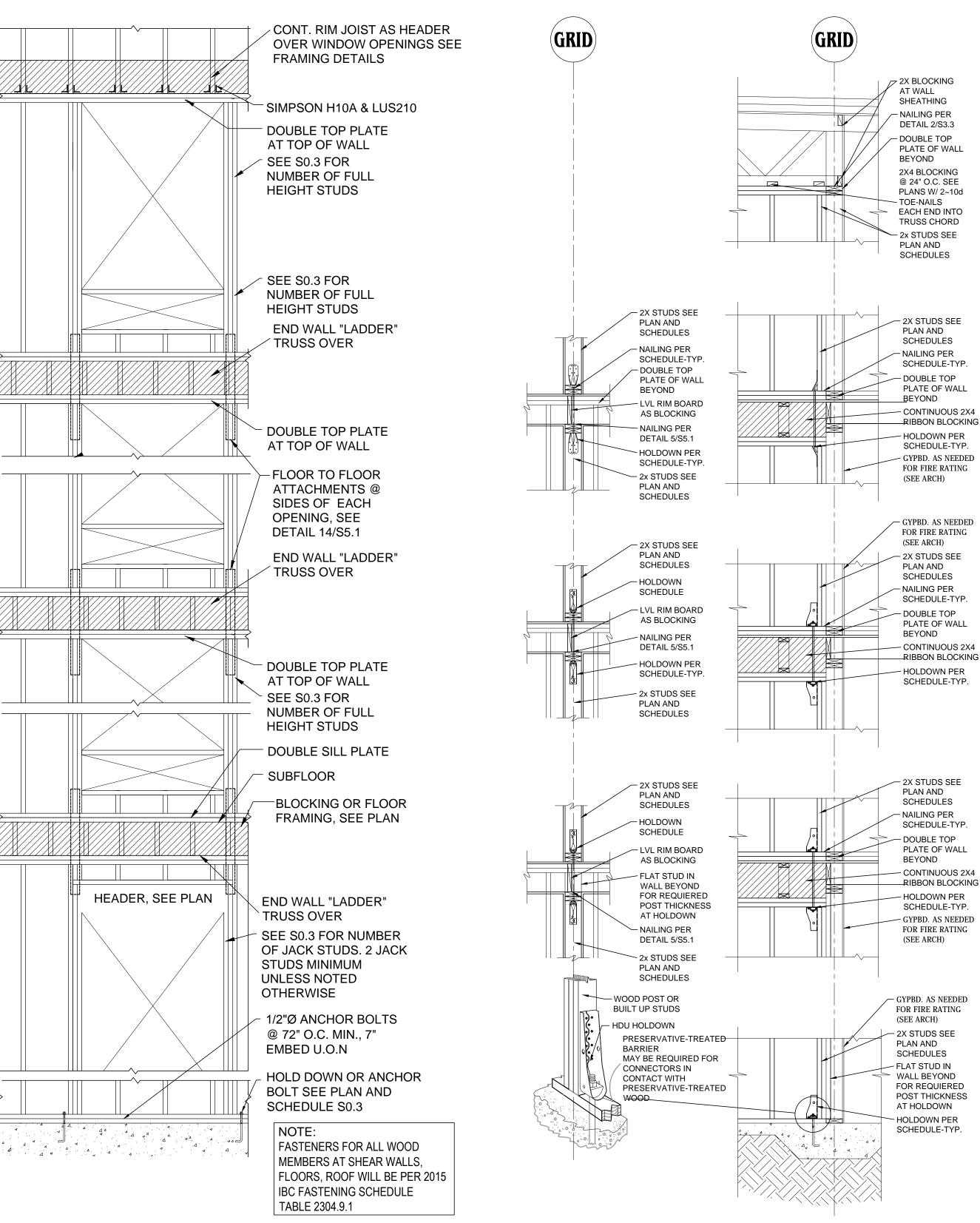
BEAM TO HSS COL. CONN.

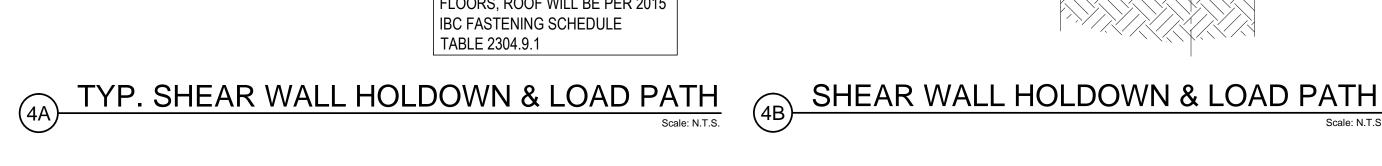


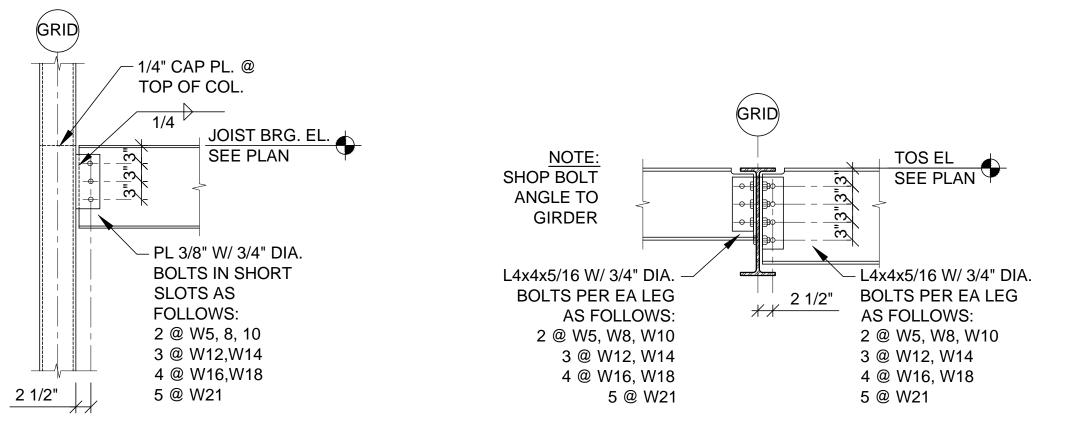


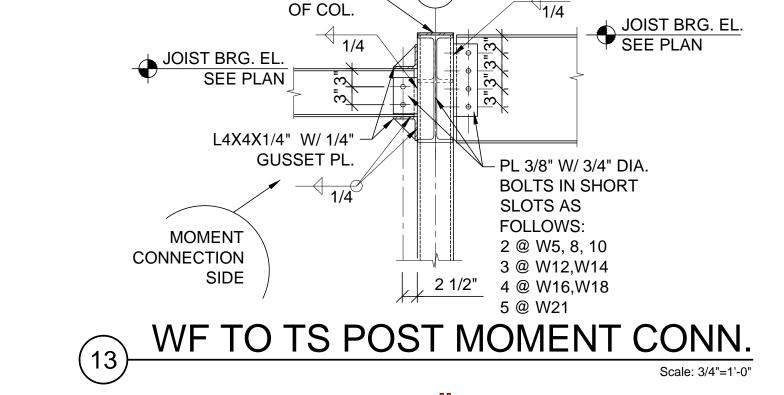








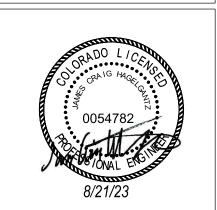




1/4" PL. @ TOP







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DRAWING ISSUE DATES:

REVISION DATES:

PROJECT MANAGER: NICK PIRKL DRAWN BY: NAP

STRUCTURAL DETAILS

S5.3

SHEET:24 OF 140