Bulletin Number - BP3 Goldwalk - 04

| Project | Steamboa | t Base Village Re | develo | opment | | Date 07/15/20 | 021 |
|--|---|--|---------------------------|---|---------------------|--|--|
| Project Location | Steamboa | t, Colorado | | | | Architect's Project | Number 03.7835.000 |
| Owner/Client | Alterra Mo | ountain Company | / East | t West Partners | | File 6BL This | is page 1 of 2 |
| То | Saunders | Construction Inc. | | | | Attention Brya | an Sculthorpe |
| Address | 86 Invern | ess Place North | | | | | |
| City | Englewood | t | | | | | ip 80112 ode |
| Delivered via: | | Messenger | | Hand carried | | Facsimile | |
| Delivered via: | | | | | | | |
| | | Express | | Pick-up | | E-mail Address | |
| | | 🗌 Mail | | UPS | | Website Address | BIM360 |
| This Bulletin Convey | s to Contracto | or (Check one of the f | ollowing | five choices.): | | | |
| Architect's Author Architect recomme | | linor Changes ons to the Work as des | scribed | below. | | | |
| | | plemental Instruction rk in accordance with | | | | Architect's Supplemental is. | Instructions form.) |
| This confirms Arch Note: The above th | itect's verbal in ree choices are | ield Order (Use this structions to (individu e each subject to the for contract Documents | ial's nar ollowing | me) on (date) _ terms: The change(s | , a s), clarific | s described below. ation(s) and/or confirmat | cion(s) described below |
| described herein. proposal. This is r modifications. | itemized prop Submit proposi not a Change (| osal for changes in al within da | the Cor ys or n | ntract Sum and/or T otify the Architect in | Time for writing | proposed modifications of the date on which yo | to the Contract Documents u anticipate submitting your k described in the proposed |
| Other: As describe | ed below. | | | | | | |
| Attachments | | BP3 GOLDWALK | - BUL | LETIN 04 (MICRO | OPILES |) | |
| Requested by | | | | | | | |
| Architect | Owner | 🛛 Contractor | | Other (specify): | _ | | |
| Issued by Gensler by | / | Jacob Apple | | | | Date Signed | 07/15/2021 |
| Issued by Owner by | | | | | | Date Signed | , , |
| Required; Please | return signed co | opy to Gensler | | Not Required | | | |
| Accepted by Contrac | tor by | | | · | | Date Signed | |
| Required; Please | return signed co | opy to Gensler | | Not Required | | | |
| Distribution | | Bryan Sculthorp Greg Morgan Adam Cleveland Ryan Stone Mike Schmidt John Albright Gregg Riker | e | | | | |
| Prepared by Gensler | by | Jacob Apple | | | | Date Signed | 07/15/2021 |
| Instructions / Descr | iption / Refer | ences / Dates | | | | | |
| | | | | | | | |
| | | | | | | 1225 17th Street Suite 150 Denver CO 80202 Tel: +1 303.595.85 | |
| BL_050615 | | | | | | Fax: +1 303.825.68 | 23 |

Gensler

Jegensler.ad/projects/03/03.7835.000/documentation/6 - construction administration/6bl - bulletins/6bl-bp3 - goldwalk/2021-0714-bp3 gw - bulletin 04 - micropiles/working/2021.0625 bp3 w - bulletin - anative.docx

Description of Changes: General: This bulletin addresses revising foundations to a micropile design at the escalator.

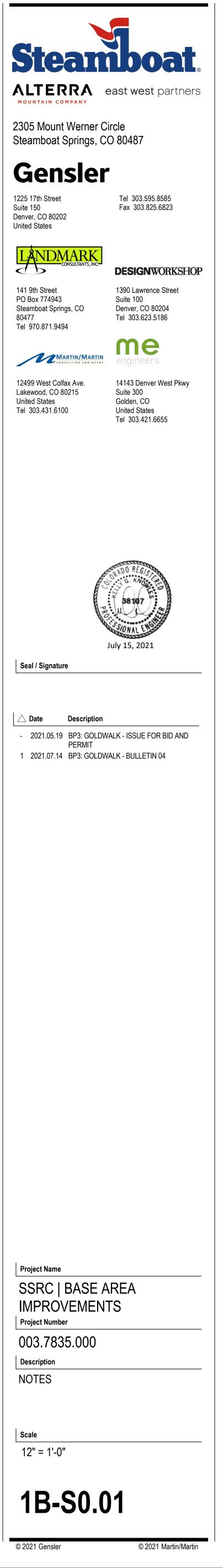
Structural Drawing Changes:

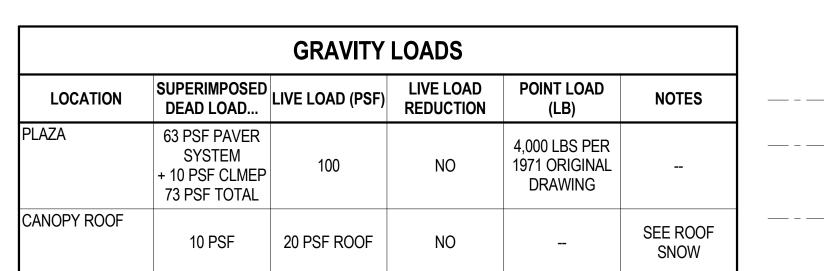
- 1. 1B-S0.01
 - a. Micropiles are added to the 'deferred submittals'.
- 2. 1B-S0.02
 - a. Micropiles are defined in the 'Foundation Notes'.
- 3. 1B-S0.10
- a. Micropile special inspections are added.
- 4. 1B-S1.01
 - a. Plan 1:
 - i. Micropile locations are added.
 - ii. Bottom of wall elevations are revised.
 - iii. New foundation to existing wall details are clarified.
- b. Elevation 3:
- i. Bottom of foundation elements are revised. 5. 1B-S2.00
 - a. Cross section 2:
 - i. Bottom of foundation elements are revised.
 - b. Cross section 3:
 - i. Bottom of foundation elements are revised.
- 6. 1B-S3.01:
 - a. Detail 4:i. Bottom of foundation is revised for micropiles.
 - b. Detail 9:
 - i. Bottom of foundation is revised for micropiles.
 - c. Detail 10:
 - i. Bottom of foundation is revised for micropiles.
 - d. Detail 11:
 - i. Bottom of foundation is revised for micropiles.
 - e. Detail 16:
 - i. Bottom of foundation is revised for micropiles.
 - f. Detail 17:
 - i. Bottom of foundation is revised for micropiles.
 - g. Detail 19:
 - i. Pilaster geometry is revised and the pile cap is added for micropiles.
 - h. Detail 20:
 - i. Bottom of foundation is revised for micropiles.
- 7. 1B-S3.02
 - a. Detail 7:
 - i. Bottom of foundation is revised for micropiles.
 - b. Detail 12:
 - i. Detail is added.
 - c. Detail 13:
 - Detail is added.
 - d. Detail 16:
 - i. Detail is added.
 - e. Detail 18:
 - Detail is added.
 - f. Detail 20:
 - i. Detail is added.

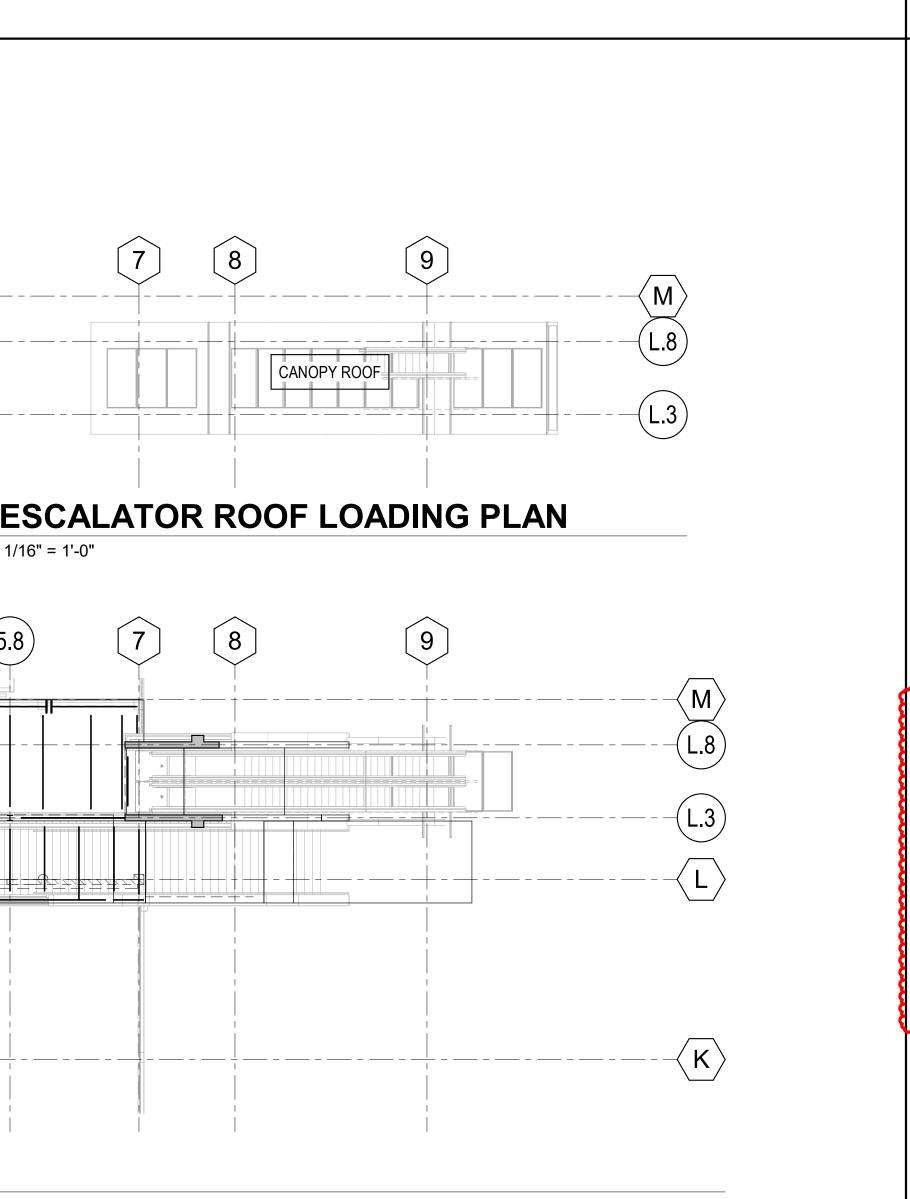
| SYMBOLS LEGEND | DEFERRED SUBMITTALS | DESIGN CRITERIA | STRUCTURAL DRAWING LIST |
|---|--|--|--|
| <u>SYMBOL</u> <u>DESCRIPTION</u> <u>SYMBOL</u> <u>DESCRIPTION</u> | 1) GENERAL: | 1) CODES AND STANDARDS: | SHEET NUMBER SHEET TITLE |
| - GRID LINES MECH UNIT (XX.Xk = MECH UNIT OPERATING WEIGHT | 1A) THE FOLLOWING PORTIONS OF THE STRUCTURAL DESIGN WILL NOT BE SUBMITTED AT THE TIME OF PERMIT APPLICATION. WHEN RECEIVED AND REVIEWED, THESE DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL BY THE CONTRACTOR: | 1A) GENERAL DESIGN - INTERNATIONAL BUILDING CODE 2018 | 1B-S0.01 NOTES 1B-S0.02 NOTES |
| SECTION OR DETAIL OUT | - EXCAVATION SHORING - MICROPILES - METAL STAIRS | 2) SEISMIC LOADS - SEISMIC DESIGN CATEGORY = C SEISMIC DESIGN CATEGORY = C | 1B-S0.03NOTES1B-S0.10QUALITY ASSURANCE |
| DETAIL CUT XX SHEET NUMBER XT SHEET NUMBER CONCRETE PAD PAD THICKNESS | METAL STAIRS ARCHITECTURAL/METAL CLADDING PANEL LIGHT GAGE METAL STUDS | RISK CATEGORY = II EARTHQUAKE IMPORTANCE FACTOR, le = 1.00 DESIGN SPECTRAL RESPONSE COEFFICIENT, SDs = 0.333 | 1B-S0.11QUALITY ASSURANCE1B-S1.01GOLDWALK - LEVEL 1 FRAMING PLAN |
| ELEVATION CUT | METAL RAILINGS ANCHORAGE, BRACING AND ATTACHMENT OF REQUIRED ARCHITECTURAL, MECHANICAL, ELECTRICAL, | DESIGN SPECTRAL RESPONSE COEFFICIENT, SD1 = 0.133 SOIL SITE CLASS = C | 1B-S1.02GOLDWALK - LEVEL 2 FRAMING PLAN1B-S1.03GOLDWALK - LEVEL 3 FRAMING PLAN |
| XX STRUCTURAL MEMBER | PLUMBING, FIRE SPRINKLER, AND OTHER EQUIPMENT AND SYSTEMS. - PERMANENT EARTH SHORING SYSTEMS | BASIC STRUCTURAL SYSTEM: STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE STRUCTURAL SEISMIC LATERAL SYSTEM: STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC | 1B-S1.04GOLDWALK - CANOPY ROOF FRAMING PLAN1B-S1.05GOLDWALK CANOPY DETAILS |
| XXX'-XX" ELEVATION CALLOUT ELEVATION CALLOUT CAST-IN-PLACE CONCRETE | 1B) CONNECTION OF DEFERRED SUBMITTAL ITEMS TO PRIMARY STRUCTURE BY DEFERRED SUBMITTAL SUPPLIER TO PROVIDE CONNECTIONS AND FRAMING ARRANGEMENT TO | STRUCTURAL SEISMIC LATERAL SYSTEM: STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC RESISTANCE RESPONSE MODIFICATION FACTOR. R = 3 | 1B-S2.00GOLDWALK CROSS SECTIONS1B-S3.00TYPICAL CONCRETE DETAILS |
| DRAWING REVISION OVERFRAMING | AVOID LOADING WHICH EXCEEDS THE CAPACITY OF THE ELEMENT BEING ATTACHED TO. REFERENCE LOAD MAPS FOR MECHANICAL, ELECTRICAL, PLUMBING AND FIRE SPRINKLER LOAD ALLOWANCES. | SEISMIC RESPONSE COEFFICIENT, Cs = 0.111 SYSTEM OVERSTRENGTH FACTOR, OMEGA = 3 | 1B-S3.01FOUNDATION DETAILS1B-S3.02CONCRETE DETAILS |
| NUMBER SYMBOL DESCRIPTION CURRENT REVISION CLOUD SLAB TYPE | 1C)ALL DEFERRED SUBMITTALS TO BE ATTACHED TO PRIMARY STRUCTURE WITH A PINNED CONNECTION. MOMENT CONNECTIONS TO PRIMARY STRUCTURE NOT PERMITTED UNLESS NOTED ON DRAWINGS OR | SEISMIC RESISTING SYSTEM NOT CONSIDERED FOR ANCILLARY STRUCTURE ADDITION IN LEVEL B1 AND NEW SLAB REPLACEMENT AT LEVEL 1. ESCALATOR CANOPY STRUCTURE: | 1B-S3.10TYPICAL SOG DETAILS1B-S3.50CONCRETE SUPPORTING STEEL DETAILS |
| | APPROVED BY ENGINEER IN WRITING PRIOR TO SUBMITTAL OF DRAWINGS OR CALCULATIONS. | DESIGN BASE SHEAR EAST-WEST DIRECTION = 30 K DESIGN BASE SHEAR NORTH-SOUTH DIRECTION = 30 K | 1B-S4.00MASONRY DETAILS1B-S5.00TYP STEEL BEAM CONNS - LRFD |
| ROUGHENED SURFACE, INTENTIONALLY ROUGHEN TO 1/4" F1- FTG MARK (SF = STRIP FTG) | 1D)LOADING AND LOCATION FOR ATTACHMENT OF DEFERRED SUBMITTAL ITEMS ARE NOTED ON DRAWINGS AND ARE NOT TO BE RE-LOCATED OR INCREASED WITHOUT WRITTEN APPROVAL. | SEISMIC ANALYSIS PROCEDURE: EQUIVALENT LATERAL-FORCE ANALYSIS 3) WIND LOADS | 1B-S5.01 TYP STEEL BEAM CONNS - LRFD 1B-S5.02 STEEL DETAILS |
| AMPLITUDE, UNO | 1E) GC / METAL STUD FRAMING DESIGNER / CLADDING DESIGNER COORDINATION: - METAL STUD FRAMING AND FRAMING ATTACHMENT IS DESIGNED FOR THE TRIBUTARY WIND AND GRAVITY LOAD OF THE STUD SPACING. CLADDING SUPPLIER TO DESIGN CLADDING TO ATTACH AT EACH STUD. | RISK CATEGORY = II BASIC ULTIMATE WIND SPEED, Vult = 115 MPH BASIC NOMINAL WIND SPEED, Vasd = 89.1 MPH | 1B-S5.03STEEL DETAILS1B-S5.31TYPICAL SLAB ON METAL DECK DETAILS1B-S5.60PERFORMANCE SPECIFIED FRAMING |
| SERVICE LOAD PROVIDED FOR SPECIALTY DESIGNER SYMBOL DESCRIPTION Image: Step STEP STEEL BEAM SHEAR STUD QUANTITY | CLADDING ATTACHMENT SPACING WHICH EXCEEDS THE STUD SPACING IS NOT ACCEPTABLE WITHOUT APPROVAL FROM THE METAL STUD SUPPLIER/DESIGNER AND THE PROJECT EOR. | EXPOSURE CATEGORY = C INTERNAL PRESSURE COEFFICIENT, Gcpi = +/-0.18 | |
| XXk BEAM SIZE [XX] c=X" XXk | - IF THE CLADDING SUPPLIER DOES NOT WANT OR CANNOT ATTACH TO EACH STUD THE LOADS FROM THE CLADDING SUPPLIER MUST BE PROVIDED TO THE METAL STUD FRAMING SUPPLIER. THE METAL STUD FRAMING DESIGN. | INTERNAL PRESSURE COEFFICIENT, Gcpi = +/-0.0 FOR CANOPY TOPOGRAPHIC ALTITUDE ADJUSTMENT, Ke = 0.78 | |
| Image: SLOPE (XXX'-XX") Image: Ultimate billing light of the state of the sta | - GC TO COORDINATE BETWEEN METAL STUD FRAMING SUPPLIER AND CLADDING SUPPLIER AS REQUIRED. 1F) FLOOR FRAMING AND EDGE ANGLE ARE DESIGNED TO SUPPORT ONE LEVEL OF CURTAIN WALL OR METAL | <u>4) DESIGN WIND PRESSURE FOR COMPONENTS AND CLADDING AND ELEMENTS DESIGNED BY THE CONTRACTOR</u> 4A) LISTED COMPONENT AND CLADDING WIND PRESSURES ARE INCLUDED FOR REFERENCE ONLY. FINAL CALCULATIONS SHALL BE COMPLETED BY CONTRACTOR | |
| Image: Subgrade Image: Subgrade | STUD WALL FRAMING AND EDGE ANGEL ARE DESIGNED TO SUPPORT ONE LEVEL OF CORTAIN WALL OR METAL STUD WALL FRAMING. SUPPORTING MULTIPLE LEVELS OF CURTAIN WALL OR METAL STUD WALL FROM ONE FLOOR LEVEL IS NOT PERMITTED. | 4B) PRESSURES LISTED BELOW ARE ULTIMATE 4C) SEE 'WALL CORNER AND SPECIAL ROOF ZONES DIAGRAM' | |
| FORM SAVER DECK TYPE | 1G)WALLS, GRADE BEAMS AND THE UNDERSIDE OF CONCRETE ON METAL DECK SHALL BE CONSIDERED CRACKED FOR THE PURPOSE OF DESIGNING ANCHORS FOR ATTACHMENT OF DEFERRED SUBMITTAL ITEMS. | 4D)COMPONENT AND CLADDING SURFACE PRESSURES (PSF) - WALLS PRESSURES | |
| NOTES: ABOVE DECK CONCRETE THICKNESS 1. ITEMS NOT DESIGNED BY M/M ARE SHOWN HALFTONED, ZE3.25A | 1H) SUBMIT STAMPED STRUCTURAL CALCULATIONS FOR ALL DEFERRED SUBMITTAL ITEMS PRIOR TO OR | WALLS AREA 10 SF 200 SF 500 SF WALLS INTERIOR NEG (ZONE 4) -24.5 -21.8 -20.6 -19.0 | |
| 2. ITEMS INCLUDE: - EXISTING CONSTRUCTION - PERFORMANCE SPECIFIED ITEMS (STAIRS, RAILINGS, ETC.) - PERFORMANCE SPECIFIED ITEMS (STAIRS, RAILINGS, ETC.) | CONCURRENTLY WITH DRAWINGS OR PRODUCT DATA. INCLUDE ANALYSIS OF ATTACHMENT TO PRIMARY STRUCTURE. INCLUDE CURRENT ICC REPORT WITH ALL PROPRIETARY STRUCTURAL ELEMENTS AND ANCHORS/FASTENERS. | WALLS CORNER NEG (ZONE 5) -49.0 -38.1 -33.4 -27.2 WALLS POSITIVE ZONE 4 & 5 24.5 20.4 18.6 16.3 | |
| - PERFORMANCE SPECIFIED ITEMS (STAIRS, RAILINGS, ETC.) | 1I) POWDER ACTUATED FASTENERS (PAF) INTO CONCRETE OR CMU SHALL NOT BE USED TO RESIST TENSION | - ROOF PRESSURES ROOF AREA DOOF INTERIOR NEC (ZONE 1) 10 SF 100 SF 500 SF | |
| (E) or EXIST Existing EA Each LOC(s) Location(s) or Locate REINF Reinforce(ing)(d)(ment) | LOADS. POWDER ACTUATED FASTENERS SHALL NOT BE USED TO RESIST GRAVITY LOADS WHICH INCLUDE BRICK VENEER. | ROOF INTERIOR NEG (ZONE 1)-46.2-35-54.4- USE THESE FOR JOIST UPLIFT WIND DESIGN FORCES UNO-24.5-24.5-16.0ROOF INTERIOR NEG (ZONE 1')-24.5-24.5-16.0 | |
| | 1J) REFERENCE COLD-FORMED STEEL FRAMING NOTES FOR ADDITIONAL DEFERRED SUBMITTAL DESIGN CRITERIA. | ROOF NEGATIVE (ZONE 2) -62.6 -48.2 -76.2 - EAVES, RAKES, RIDGES | |
| @AtEFEach FaceLSLLaminated Strand LumberRETReturnABAnchor BoltEJExpansion JointLSLLaminated Strand LumberRORough Opening | | ROOF CORNERS NEG (ZONE 3) -62.6 -48.2 -76.2 ROOF POSITIVE ALL ZONES -16 -16 -16.0 ROOF NEGATIVE OVERHANG ZONE 1 & 1' -46.2 -43.5 -54.4 | |
| ACI American Concrete Institute EL Elevation LT Light ROF Random Oriented Fiber ACI ELEV Elevator LT Light South | | ROOF NEGATIVE OVERHANG ZONE 2 -62.6 -43.4 -59.8 | |
| ADDNL Additional EMBED Embedded LTS Tension Lap Splice SC Slip Critical AESS Architecturally Exposed EN Edge Nail LTWT Lightweight SC Schedule | | ROOF NEGATIVE OVERHANG ZONE 3' -62.6 -43.4 -59.8 - PARAPET PRESSURES | |
| AFF Above Finish Floor ALT Alternate | | PARAPET QP = 27.2 psf SOLID PARAPET PRESSURE 10 SF 100 SF 500 SF | |
| ALUM Aluminum ADA American Discont EQ Equal EQ Equally Spaced MACH Machine | | PARAPET CASE A: ZONE 2: 87.0 68.6 54.4 ZONE 3: 87.0 68.6 54.4 PARAPET CASE B: INTERIOR ZONE: -49.0 -42.2 -35.4 | |
| All A All Charles Equipment Association EQUIP Equipment APPROX Approximate | | CORNER ZONE: -49.0 -42.2 -35.4 CORNER ZONE: -73.4 -58.5 -43.5 PARAPET CASE A = PRESSURE TOWARDS BUILDING (POS) | |
| ARCH Architect or Architectural EW Each Way B/ or BO Bottom of EXP Expansion | | PARAPET CASE B = PRESSURE AWAY FROM BLDG (NEG) PARAPET CAP UPLIFT PRESSURES ZONE 2: -68.0 PSF | |
| BAL Balance BD Board EXP ANCH Expansion Anchor EXT Exterior MBS Metal Building Supplier MCL Masonry Control Joint | | ZONE 268.0 PSF ZONE 3: -68.0 PSF | |
| BF Braced Frame F Fluid Load MECH Mechanical BG Backgouge Fa Flood Load MEP Mech/Elect/Plumb | | 5) LATERAL LOAD RESISTING SYSTEM DESCRIPTION: SEISMIC RESISTING SYSTEM NOT CONSIDERED FOR ANCILLARY STRUCTURE ADDITION IN LEVEL B1 AND NEW SLAB REPLACEMENT AT LEVEL 1. | |
| BL Brick Ledge BLDG Building FF Finished Floor | | - ESCALATOR CANOPY STRUCTURE - ROOF HSS FRAME AS DIAPHRAGM AND WOOD GLULAM COLUMN, HSS BEAM, AND ROD BRACE AS LATERAL FRAMES IN SHORT DIRECTION, WOOD GLULAM COLUMN MOMENT FRAMES IN LONG | |
| BLKG Blocking BM Beam FLG Flange | | DIRECTION. 6) GRAVITY LOADS | |
| BN Boundary Nail BOS Bottom of Steel | | 6A) SEE LOAD KEY ON SHEET 1B-S0.02 FOR SUPERIMPOSED DEAD LOAD AND LIVE LOADS USED IN DESIGN. | |
| BOT or BBottomFOFace ofMOMasonry OpeningTTop or Thermal LoadBRGBearingFPFull Penetration or FireMDMetalTTop and Bottom | | 6B) DRIFTING, SLIDING AND UNBALANCED SNOW - GROUND SNOW LOAD = 132.0 PSF - SNOW EXPOSURE FACTOR, Ce = 1.0 | |
| BSMT Basement Proofing BTWN Between FRAM Framing | | SNOW EXPOSORE FACTOR, CE = 1.0 SNOW LOAD IMPORTANCE FACTOR, Is = 1.0 THERMAL FACTOR, Ct = 1.20 (ESCALATOR CANOPY) | |
| CC Center to Center FS Far Side NIC Not in Contract TL Total Load CF Cold Formed FT Foot or Feet NM TOC Top of Concrete | | UNIFORM ROOF SNOW LOAD = 110.9 PSF FLAT ROOF SNOW LOAD = 110.9 PSF | |
| CGCenter of GravityFTGFootingNO OR #NumberTOFTop of FootingCIPCast-In-PlaceFVField VerifyNOMNominalTOMTop of Masonry | | 7) RAIN LOADS: 7A) DESIGN RAIN INTENSITY = 2.5 INCHES PER HOUR | |
| CJ Control Joint CJP Complete Joint GALV Galvanized | | 7B) DESIGN RAIN ROOF PRESSURE = 21 PSF | |
| Penetration GC General Contractor CL Centerline GL Glu-lam NWC Normal Weight Concrete TOW Top of Wall TRANS Transverse | | | |
| CLG Ceiling CLMS Ceiling/Light/Mechanical/ GR Grade or Grind OAE Or Approved Equivalent TWS Two-Way Slab | | | |
| Superimposed Load GR MB Grade Beam CLR Clear CMU Concrete Masonry Unit GR MB Grade Beam H Soil Lateral Load OD Outside Diameter OH Opposite Hand UNO Unless Noted Otherwise | | | |
| COL Column CONC Concrete | | | |
| OWS Oversized VERT Vertical VERT Vertical With the second se | | | |
| Image: Second | | | |
| Image: Second structure Image: HT Height Image: Second structure Image: HT Height Image: Second structure Image: HT Image: Second structure Image: Second structure Image: | | | |
| CTR(D) Center(ed) Inside Face PD Pier Dowel W/O Without d Penny Inch Inch PEMB Pre-Engineered Metal WD Without | | | |
| The interior Initerior Building With the interior D or DL Dead Load IS Inside Diameter Building With the interior DAS Deformed Anchor Stud IT Precast Inverted Tee Department With the interior | | | |
| DBL Double Intervention recust invented recustor PERP Perpendicular Waterproofing DCW Demand Critical Weld UST Joist Intervention recustor PL Plate (Steel) WPS Welding Procedure | | | |
| Di Gravity Ice Load JI JOINT PREFAB Prefabricated WT Weight | | | |
| Nice with the second | | | |
| DN Down LCF Compression | | | |
| DO Ditto DP Drilled Pier or Deep LCS Compression Lap Splice | | | |
| DT Precast Double Tee LDH Hook Development Length Radius or Rain Load | | | |
| BWB(s) Drawing(s) LG Length V DWL(s) Dowels(s) LL Live Load RB Precast Rectangular Beam | | | B/~ |
| E Earthquake Load LLH Long Leg Horizontal E-W East-West LLV Long Leg Vertical | | | |
| | | | |
| | | | |

Ste Ste 2 8 $\leq \leq 2$ DESIGNERS: NIC MA LEAD REVIT TECH:C DATE PRINTED:7/15.

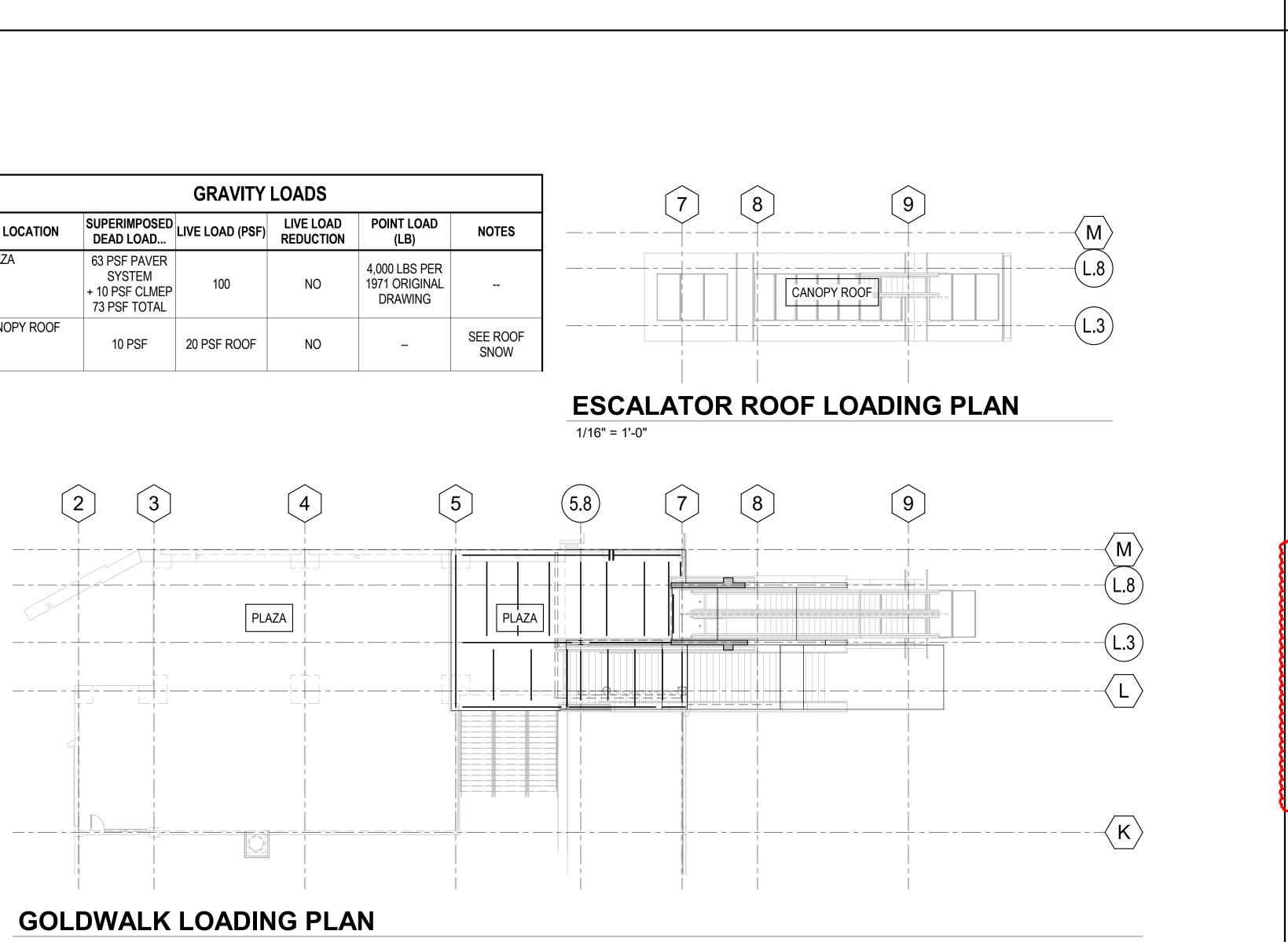
MM JOB #: 20.1411.S.01 PRINCIPAL: KELLY KNOW EOR: KELLY KNOWLES

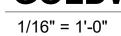






1/16" = 1'-0"





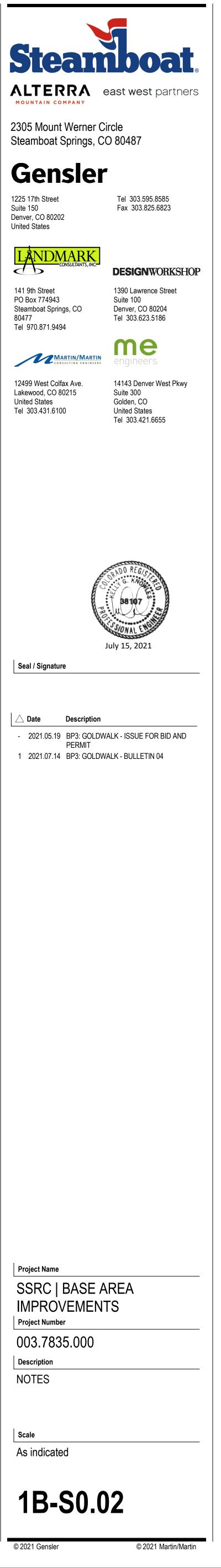


| | FOUNDAT | ION NOTE | S |
|---|-------------------|------------------------|----------|
| 1) DESIGN CRITERIA: THE GEOTECHNICAL REPORT PREPARED APRIL 22, 2021 PROVIDED CRITERIA FOR T | | | |
| 2) FOOTINGS: 2A) FOOTING DESIGN CRITERIA: MAXIMUM TOTAL LOAD BEARING PRES MINIMUM CONTINUOUS FOOTING WIDT MINIMUM SPREAD FOOTING WIDTH = 1 ULTIMATE COEFFICIENT OF FRICTION | H = 1 FT FT | L LOADS = 0.4 | |
| FROST DEPTH TO BOTTOM OF FOUND, <u>FOUNDATION WALLS:</u> EQUIVALENT FLUID PRESSURES USED | ATION = 48 IN | | |
| "ACTIVE" CONDITION = 45 PCF "AT REST" CONDITION = 55 PCF "PASSIVE" CONDITION = 275 PCF LATERAL PRESSURE DUE TO SURCHAI ULTIMATE COEFFICIENT OF FRICTION ¹ | RGE = 250 PSF | | |
| 3B) WALL DESIGN BASED ON IN-SITU SOILS REQUIREMENTS. | S ADJACENT TO FO | UNDATION WA | LLS. |
| 4) SITE RETAINING WALLS: 4A) EQUIVALENT FLUID PRESSURES USED "ACTIVE" CONDITION = 45 PCF "AT REST" CONDITION = 55 PCF "PASSIVE" CONDITION = 275 PCF LATERAL PRESSURE DUE TO SURCHART | | : | |
| 4B) WALL DESIGN BASED ON IN-SITU SOILS | TO RESIST LATERA | | WALLS |
| REQUIREMENTS. | | ~~~~~ | |
| 5A) MICROPILE CAPACITY CRITERIA: - COMPRESSION CAPACITY = 54K (SERV - UPLIFT CAPACITY = 0K (SERVICE) | ICE, MINIMUM) FOR | VERTICAL AN | D BA1 |
| 5B) MICROPILE MATERIALS: - THREADED ROD, BY CONTRACTOR - GROUT, BY CONTRACTOR | | | |
| 5C) MICROPILES ARE A PERFORMANCE SP | ECIFIED ITEM TO B | E DESIGNED E | BY THE |
| 5D)BEDROCK ELEVATIONS - SEE GEOTECHNICAL REPORT. | | | |
| 5E) MICROPILE TESTING - SEE MICROPILE TESTING TABLE AND N | ICROPILE SUPPLIE | R FOR ADDITI | ONAL |
| | | | |
| | | | |
| | | | |
| | | | |
| | | | |
| | | | |
| | WOOD | NOTES | |
| GLUED LAMINATED MEMBERS: 1A) COMBINATION SYMBOL: SINGLE SPAN: 24F-V4 MULTI- SPAN: 24F-V8 | | | |
| 1B) MINIMUM DESIGN VALUES ARE BASED | ON THE 2018 NDS. | | |
| 2) METAL CONNECTORS: 2A) FRAMING CONNECTORS SHALL CONF | | | |
| 2B) ALL CONNECTOR HOLES SHALL BE FI UPLIFT. ALL BOLT HOLES SHALL BE DRIL THE BOLT DIAMETER. | | | |
| 3) OPENINGS: 3A) OPENING, POCKETS, ETC., SHALL NO | FBE PLACED UNLES | SS DETAILED (| ON TH |
| MAS | SS TIMBER M | ATERIAL | TAE |
| WOOD ELEMENT | SPECIES/ GRADE | Fb TOP/BOT (PSI) | Fv (P |
| GLULAMS - SINGLE SPAN GLULAMS - MULTI-SPAN/COLS | 24F-V4 24F-V4 | 1850/2400 2400/2400 | 26 26 |
| NOTES: | | | - |

1. PROPERTIES LISTED SHALL BE MET OR EXCEEDED.

2. MULTI-SPAN CONDITIONS INCLUDE GLULAM MEMBERS WITH CANTILEVERS

| S | GENERAL NOTES |
|---|--|
| NSULTANTS, INC., NUMBER 20-12000, DATED | 1) GENERAL: 1A) ENGINEER: REFERENCES ON THE STRUCTURAL DRAWINGS TO 'ENGINEER' MEAN THE STRUCTURAL ENGINEER OF |
| IE PROJECT. | RECORD. OTHER ENTITIES ARE SPECIFICALLY NOTED AS "CONTRACTOR'S ENGINEER", "MECHANICAL ENGINEER", ETC. 1B) THESE NOTES SUPPLEMENT THE SPECIFICATIONS, WHICH SHALL BE REFERENCED FOR ADDITIONAL REQUIREMENTS. |
| | 1C) UNDERGROUND UTILITIES: LOCATE EXISTING UTILITIES AND NOTIFY ARCHITECT OF EXISTING UTILITIES OR SUBGRADE CONDITIONS WHICH INTERFERE WITH WORK. |
| | 1D) STRUCTURAL ELEMENTS ARE CENTERED ON GRID LINES AND GRID LINE INTERSECTIONS UNLESS DIMENSIONED OTHERWISE. |
| | 2) USE OF DRAWINGS: 2A) DO NOT SCALE DRAWINGS. |
| | 2B) DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. |
| | 2C) DETAILS NOTED TYPICAL APPLY TO ALL SIMILAR CONDITIONS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ELSEWHERE ON THE PROJECT. |
| ALLS. SEE EARTHWORK SPECIFICATION FOR | 2D) WHERE DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL NOTES AND SPECIFICATIONS: - CONTACT THE ARCHITECT PRIOR TO PROCEEDING WITH CONSTRUCTION - THE MORE STRINGENT REQUIREMENTS SHALL GOVERN FOR BIDDING / PRICING |
| | 3) EXISTING STRUCTURES: 3A) CONTRACT DOCUMENTS HAVE BEEN PREPARED USING AVAILABLE DRAWINGS AND SITE OBSERVATION AS PERMITTED BY ACCESS RESTRICTIONS DURING DESIGN. |
| WALLS. SEE EARTHWORK SPECIFICATION FOR | 3B) DURING CONSTRUCTION, THE CONTRACTOR MAY ENCOUNTER EXISTING CONDITIONS WHICH ARE NOT KNOWN OR ARE AT VARIANCE WITH PROJECT DOCUMENTATION. CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ALL CONDITIONS NOT PER THE CONTRACT DOCUMENTS. EXAMPLES INCLUDE: SIZES OR DIMENSIONS OTHER THAN THOSE SHOWN DAMAGE OR DETERIORATION TO MATERIALS AND COMPONENTS CONDITIONS OF INSTABILITY OR LACK OF SUPPORT ITEMS NOTED AS EXISTING ON THE DRAWINGS BUT NOT FOUND IN THE FIELD |
| ID BATTERED PILES | 3C)PREPARE DIMENSIONAL DRAWINGS OF ALL DISCOVERED ITEMS. |
| | 3D) CONTRACTOR SHALL FIELD VERIFY ALL EXISTING STRUCTURAL CONDITIONS PRIOR TO SUBMITTING SHOP DRAWINGS. 3E) CONTRACTOR SHALL MAKE ALLOWANCE FOR THE RESOLUTION OF SUCH DISCOVERIES IN THE CONSTRUCTION SCHEDULE. |
| BY THE CONTRACTOR. | 3F) SUBMIT A DIMENSIONED DRAWING OF ALL NEW OPENINGS THROUGH EXISTING STRUCTURE AND SECURE APPROVAL PRIOR TO CUTTING. NEW OPENING MAY BE EITHER SHOWN ON THE CONTRACT DOCUMENTS OR PROPOSED BY THE CONTRACTOR. DRAWING SHALL SHOW: VERTICAL & HORIZONTAL LOCATION AND SIZE OF NEW OPENING(S) ALL EXISTING OPENINGS IN THE VICINITY OF THE NEW OPENING(S) ALL EXISTING STRUCTURE (BEAMS, COLUMNS, SLABS, WALLS, ETC) IN THE VICINITY OF THE NEW OPENING(S) |
| ONAL INFORMATION. | ALL REINFORCING BAR SIZES AND POSITIONS (LAYOUT LOCATION AND DEPTH) CONFLICTING WITH OR IN THE VICINITY OF THE NEW OPENING(S). <u>4) COORDINATION:</u> |
| | 4A) STRUCTURAL DRAWINGS ARE NOT STAND-ALONE DOCUMENTS AND ARE INTENDED TO BE USED IN CONJUNCTION WITH CIVIL, ARCHITECTURAL, MECHANICAL, ELECTRICAL, AND DRAWINGS FROM OTHER DISCIPLINES. THE CONTRACTOR SHALL COORDINATE ALL REQUIREMENTS OF THE CONTRACT DOCUMENTS INTO SHOP DRAWINGS AND WORK. |
| | 4B) COORDINATE DIMENSIONS OF ALL OPENINGS, BLOCKOUTS, DEPRESSIONS, ETC., WITH ARCHITECTURAL DRAWINGS, DRAWINGS FROM OTHER DISCIPLINES, AND FIELD CONDITIONS PRIOR TO SHOP DRAWING SUBMITTAL. |
| | 4C) SEE ARCHITECTURAL PLANS FOR INTERIOR PARTITIONS. PARTITION FRAMING SHALL BE CONNECTED TO THE PRIMARY STRUCTURE IN SUCH A WAY SO AS TO ALLOW FOR VERTICAL LIVE LOAD DEFLECTIONS OF SPAN/360 AT FLOOR FRAMING OR SPAN/240 AT ROOF FRAMING. DO NOT MAKE RIGID VERTICAL AND HORIZONTAL CONNECTIONS TO THE PRIMARY STRUCTURE IN THE PLANE OF THE PARTITION. |
| | 5) SUBMITTALS AND SUBSTITUTIONS: 5A) SUBMITTALS: REFER TO SPECIFICATIONS FOR DETAILED REQUIREMENTS. - IF THE CONTRACTOR REQUESTS A CHANGE FROM THE STRUCTURAL DRAWINGS, IT SHALL BE APPROVED BY THE ARCHITECT AND DESIGNED BY MARTIN/MARTIN, INC. PRIOR TO SUBMITTING SHOP DRAWINGS. VARIATION SHALL BE INDICATED ON THE SHOP DRAWINGS. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN, INC. FOR MAKING THE CHANGE. |
| | CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE IN SUBMITTALS ALL SHOP DRAWINGS SHALL REFERENCE THE STRUCTURAL DRAWING NUMBER AND DETAIL USED TO PREPARE THE SUBMITTAL SUBMIT A STATEMENT OF RESPONSIBILITY FOR CONSTRUCTION OF THE LATERAL LOAD RESISTING SYSTEM IDENTIFIED IN THE DESIGN CRITERIA IN ACCORDANCE WITH IBC 2018 SECTION 1704 |
| | 5B) SUBSTITUTIONS: ARCHITECT'S APPROVAL SHALL BE SECURED FOR ALL SUBSTITUTIONS |
| 5. SEE DETAILS FOR REQUIREMENTS. S INCLUDING OPTIONAL NAIL LOCATIONS FOR MAXIMUM HOLE DIAMETER IS 1/16" LARGER THAN | 5C)NONCONFORMANCE: NOTIFY ARCHITECT OF CONDITIONS NOT CONSTRUCTED PER THE CONTRACT DOCUMENTS PRIOR TO PROCEEDING WITH CORRECTIVE WORK. SUBMIT PROPOSED REPAIR TO THE ARCHITECT FOR ACCEPTANCE. CONTRACTOR SHALL COMPENSATE MARTIN/MARTIN, INC. FOR DESIGNING THE REPAIR. |
| ON THE STRUCTURAL DRAWINGS. | 6) TEMPORARY CONDITIONS, CONSTRUCTION ENGINEERING, AND OSHA STANDARDS: 6A) THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION AND ONLY FOR LOADS ANTICIPATED DURING THE STRUCTURE'S SERVICE LIFE. |
| | 6B) THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES. REFER TO "LATERAL LOAD RESISTING SYSTEM DESCRIPTION" IN DESIGN CRITERIA FOR ADDITIONAL INFORMATION. CONTRACTOR SHALL |
| TABLE | PROVIDE ALL REQUIRED ENGINEERING AND OTHER MEASURES TO ACHIEVE THE MEANS, METHODS, AND SEQUENCES OF WORK WHICH MAY INCLUDE, BUT IS NOT LIMITED TO: - LAYOUT |
| Fv (PSI) E (PSI) REMARKS | DESIGN FOR FORMWORK, SHORING, AND RESHORING DESIGN OF CONCRETE MIXES ERECTION PROCEDURES WHICH ADDRESS STABILITY OF THE FRAME DURING CONSTRUCTION |
| 265 1,800,000 - 265 1,800,000 SEE NOTE 2 | WELD PROCEDURES DESIGN OF TEMPORARY BRACING OF WALLS FOR WIND, SEISMIC, OR SOIL LOADS SURVEYING TO VERIFY CONSTRUCTION TOLERANCES |
| | EVALUATION OF TEMPORARY CONSTRUCTION LOADS ON STRUCTURE DUE TO EQUIPMENT AND MATERIALS STRUCTURAL ENGINEERING TO RESIST ANY OTHER LOADS NOT IDENTIFIED ON DESIGN DRAWINGS |
| RS. | 6C) FOUNDATION WALLS SHALL NOT BE BACKFILLED UNTIL THE SLABS-ON-GRADE AND UPPER SLABS ARE IN-PLACE AND REACH FULL STRENGTH UNLESS ADEQUATE BRACING IS PROVIDED. USE ONLY HAND OPERATED TOOLS FOR COMPACTION ADJACENT TO FOUNDATION WALLS AND GRADE BEAMS. GRADE BEAMS SHALL BE BACKFILLED EVENLY ON BOTH SIDES. |
| | 6D) NOTHING SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE CONSTRUED AS ELIMINATING THE NEED FOR THE CONTRACTOR TO COMPLY WITH ALL OSHA REQUIREMENTS. WHERE THE STRUCTURAL DRAWINGS APPEAR TO CONFLICT WITH OSHA REQUIREMENTS, THE STRUCTURAL DRAWINGS REPRESENT FINAL CONDITIONS ONLY. THE CONTRACTOR SHALL ADD ALL ERECTION FRAMING NECESSARY TO COMPLY WITH OSHA. THE CONTRACTOR SHALL ADD ALL NECESSARY BOLTS, ANCHOR BOLTS, PLATES, STIFFENER PLATES, STABILIZER PLATES, BRACING, ETC., AS WELL AS CLOSURES FOR OPENINGS. IN ADDITION, FIELD WELD ANYTHING THAT MAY BE CONSIDERED A TRIP HAZARD, SUCH AS SHEAR STUDS, AFTER PROTECTIVE DECKING IS INSTALLED. WASHERS OR RINGS MAY BE WELDED TO COLUMNS TO PROVIDE FOR SAFETY CABLES. HOLES IN COLUMNS FOR SAFETY CABLES SHALL BE SHOP INSTALLED AND SHALL BE INDICATED ON SHOP DRAWINGS. ADJUST COLUMN SPLICE LOCATIONS OR ADD COLUMN SPLICES AS NECESSARY TO COMPLY WITH OSHA REQUIREMENTS. SUBMIT PROPOSED LOCATIONS. |
| | |
| | |
| | |
| | |
| | |



| | | SPECIAL INS | | ITEM |
|---|----------------|----------------|---|--|
| ITEM | FREQUENCY | STANDARD | CRITERIA | REINFORCING STEEL |
| EXPANSION ANCHORS, SLE | EVE ANCHORS, S | CREW ANCHORS | | - WELDING |
| PRIOR TO START OF WORK | - | ICC-ES REPORT | REVIEW CONTRACTOR'S INSTALLATION PROCEDURE | CONCRETE - COMPOSITE SAMPL |
| PRIOR TO INSTALLATION OF ANCHOR | EACH ANCHOR | ICC-ES REPORT | VERIFY TYPE, DIAMETER, LENGTH, FINISH, AND BASE MATERIAL. VERIFY SOLID GROUTED AREA AROUND ANCHORS IN GROUTED MASONRY. VERIFY MAXIMUM IMPACT WRENCH TORQUE RATING FOR SCREW ANCHORS | 1. fc < 5000 PSI 2. fc ≥ 5000 PSI AN SHOTCRETE |
| DURING INSTALLATION OF ANCHOR | С | ICC-ES REPORT | CONTINUOUS INSPECTION REQUIRED REGARDLESS IF PERIODIC INSPECTION IS PERMITTED BY ICC-ES REPORT. VERIFY HOLE DIMENSIONS, HOLE CLEANING, ANCHOR EMBEDMENT, EDGE DISTANCES AND SPACING | - SLUMP/SLUMP FLO |
| AFTER INSTALLATION OF ATTACHED ASSEMBLY | 100% VISUAL | - | VERIFY NUMBER, EDGE DISTANCES, AND ANCHOR FLUSH WITH AND PERPENDICULAR TO THE RECEIVING SURFACE | ENTRAINMENT IS SPECIFIED AND LIGHTWEIGHT CON |
| DHESIVE ANCHORS, REINF | ORCING STEEL A | NCHORED INTO H | IARDENED CONCRETE | |
| PRIOR TO START OF | - | ICC-ES REPORT | REVIEW CONTRACTOR'S INSTALLATION PROCEDURE | - TEMPERATURE |
| VVUKN | 1 | | | |
| WORK PRIOR TO INSTALLATION OF ANCHOR | EACH ANCHOR | ICC-ES REPORT | VERIFY TYPE, DIAMETER, LENGTH, FINISH, AND BASE MATERIAL. VERIFY SOLID GROUTED AREA AROUND ANCHORS IN GROUTED MASONRY | |
| PRIOR TO INSTALLATION | EACH ANCHOR | | BASE MATERIAL. VERIFY SOLID GROUTED AREA | - UNIT WEIGHT FOR STRUCTURAL LIGHTWEIGHT |
| PRIOR TO INSTALLATION OF ANCHOR DURING INSTALLATION | | | BASE MATERIAL. VERIFY SOLID GROUTED AREA AROUND ANCHORS IN GROUTED MASONRY CONTINUOUS INSPECTION REQUIRED REGARDLESS IF PERIODIC INSPECTION IS PERMITTED BY ICC-ES REPORT. VERIFY HOLE DIMENSIONS, HOLE CLEANING, ANCHOR | - UNIT WEIGHT FOR STRUCTURAL |

| ITEM | FREQUENCY | STANDARD | CRITERIA | | | |
|--|--------------------------------|----------------|--|--|--|--|
| EXPANSION ANCHORS, SLEEVE ANCHORS, SCREW ANCHORS | | | | | | |
| - TORQUE TEST | 100% | - | TEST ANCHOR WITH CALIBRATED TORQUE WRENCH TO 100% OF THE INSTALLATION TORQUE NOTED IN ICC-ES REPORT. ATTAIN SPECIFIED TORQUE WITHIN 1/2 TURN OF THE NUT | | | |
| ADHESIVE ANCHORS, REINF | FORCING STEEL A | NCHORED INTO H | IARDENED CONCRETE | | | |
| - TENSION TEST | FIRST 3 AND 1% OF REMAINING | | TEST THE INSTALLATION OF THE FIRST 3 OF EACH TYPE, BASE MATERIAL, AND POSITION (DOWN, HORIZONTAL, OVERHEAD). OBSERVE ASTM E488 MINIMUM EDGE DISTANCES FOR DETERMINING TEST LOCATIONS. SUBMIT PROPOSED TEST LOCATIONS AND REQUESTS FOR REQUIRED TENSION TEST LOAD VALUES TO ENGINEER | | | |

| JOB #: 20.1411.S.01 | DESIGNERS: NIC MARTIN |
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IGHTWEIGHT CON EMPERATURE NIT WEIGHT FOR TRUCTURAL IGHTWEIGHT OLD WEATHER (OMPRESSIVE TRENGTH SHOTCRETE (ADDITI COMPRESSIVE

FLOOR FLATNESS R MEASURE CONCRE FLOOR FLATNESS AND FLOOR LEVEL (FL) STRUCTURAL CONCRETE TESTING NOTES:

CURING

| STRUCTURAL CONCRETE TESTING | | | | | | | | |
|---|---|---|--|--|--|--|--|--|
| ITEM | FREQUENCY | STANDARD | CRITERIA | | | | | |
| INFORCING STEEL, BOLTS AND EMBEDMENTS | | | | | | | | |
| WELDING | - | - | PER STRUCTURAL STEEL TESTING | | | | | |
| DNCRETE | | | | | | | | |
| COMPOSITE SAMPLE | | | OBTAIN AT POINT OF PLACEMENT. FOR DRILLED PIERS OBTAIN NEAR BEGINNING OF LOAD PRIOR | | | | | |
| 1. fc < 5000 PSI | 100 CY/MIX/DAY | ASTM C172 | TO PLACEMENT IN SHAFT. ADJUST FREQUENCY AS REQUIRED TO PROVIDE MINIMUM 5 TOTAL | | | | | |
| 2. f'c ≥ 5000 PSI AND SHOTCRETE | 50 CY/MIX/DAY | | TESTS PER MIX BUT NOT MORE THAN ONE SAMPLE PER TRUCK LOAD | | | | | |
| SLUMP/SLUMP FLOW | EACH COMPOSITE SAMPLE | ASTM C143 (SLUMP) OR ASTM C1611 (SLUMP FLOW) | SPECIFIED SLUMP SHALL BE AS SUBMITTED IN THE MIX DESIGN ± 1 1/2". PERFORM ADDITIONAL TESTS WHEN CONCRETE CONSISTENCY APPEARS TO CHANGE | | | | | |
| AIR CONTENT WHEN AIR ENTRAINMENT IS SPECIFIED AND LIGHTWEIGHT CONCRETE | EACH COMPOSITE SAMPLE | ASTM C231 PRESSURE METHOD (NWC) OR ASTM C173 VOLUMETRIC METHOD (LWC) | _ | | | | | |
| TEMPERATURE | EACH COMPOSITE SAMPLE AND 60 MINUTE INTERVALS | ASTM C1064 | REQUIRED WHEN AIR TEMPERATURE IS 40 °F AND BELOW OR 80°F AND ABOVE | | | | | |
| UNIT WEIGHT FOR STRUCTURAL LIGHTWEIGHT | EACH COMPOSITE SAMPLE | ASTM C138 | _ | | | | | |
| COLD WEATHER CURING | - | ASTM C1074 | RECORD MAXIMUM AND MINIMUM CONCRETE TEMPERATURE DURING CURING PERIOD, WHEN DAILY AVERAGE AIR TEMPERATURE OF 40 °F OR BELOW IS EXPECTED FOR 3 SUCCESSIVE DAYS DURING CURING PERIOD | | | | | |
| COMPRESSIVE STRENGTH | EACH COMPOSITE SAMPLE | ASTM C31 ASTM C39 EITHER: (4)6x12 OR (6)4x8 CYLINDERS | TEST PER SCHEDULE BELOW: - 7 DAYS: (1) 6x12 OR (1) 4x8 - 28 DAYS: (2) 6x12 OR (3) 4x8 - 56 DAYS: (1) 6x12 OR (2) 4x8 (IF 28 DAY TESTS DO NOT ACHIEVE SPECIFIED 28 DAY STRENGTH) ACCEPTANCE CRITERIA PER ACI 318 | | | | | |
| IOTCRETE (ADDITIONAL R | EQUIREMENTS) | | | | | | | |
| COMPRESSIVE | - | IBC 2018 - 1908.10 | - | | | | | |
| CURING | - | IBC 2018 - 1908.9 | - | | | | | |
| OOR FLATNESS REQUIRE | MENTS | | 1 | | | | | |
| MEASURE CONCRETE FLOOR FLATNESS (FF) AND FLOOR LEVELNESS (FL) | - | ASTM E1155 | PERFORM MEASUREMENTS WITHIN 48 HOURS OF FINISHING OPERATIONS AND PRIOR TO REMOVAL OF SHORES OR FORMS. MEASURE AREAS INDICATED IN THE SPECIFICATIONS | | | | | |

1. NONDESTRUCTIVE TESTING MAY BE PERMITTED BY THE ARCHITECT, BUT WILL NOT BE USED AS SOLE BASIS FOR APPROVAL OR REJECTION OF DEFICIENT CONCRETE.

2. REPORTS OF COMPRESSIVE STRENGTH TESTS SHALL CONTAIN THE FOLLOWING INFORMATION: DATE OF CONCRETE PLACEMENT, LOCATION OF CONCRETE BATCH IN WORK, DESIGN 28-DAY COMPRESSIVE STRENGTH, SLUMP, CONCRETE SUPPLIER AND MIXTURE ID NUMBER, TIME OF BATCH AND PLACEMENT, AMBIENT AIR TEMPERATURE, SITE ADDED WATER AND ADMIXTURES, UNIT WEIGHT, AND AS REQUIRED BY ASTM C39.

| STRU | CTURAL CO | NCRETE SP | ECIAL |
|---|------------|---------------------|---|
| ITEM | FREQUENCY | STANDARD | |
| REINFORCING STEEL | | | |
| - DURING PLACEMENT | Р | -ACI 301-16 3.2-3.3 | VERIFY LOCATIO SPLICE |
| - PRIOR TO PLACEMENT OF CONCRETE | 100% | -ACI 301-10 3.2-3.3 | DIAMET |
| - WELDING | С | AWS D1.4 | VERIFY |
| - FIELD BENDING | Р | ACI 301-16 3.3.2.8 | |
| - COATED REINFORCING | Р | ACI 301-16 3.2.1.2 | |
| - MECHANICAL CONNECTORS | С | ICC-ES REPORT | |
| BOLTS AND EMBEDMENTS | | | |
| - PRIOR TO PLACEMENT OF CONCRETE | 100% | - | VERIFY QUANTI EDGE D TEMPLA |
| - WELDING | - | - | INSPEC ⁻ |
| CONCRETE | | | |
| - MIX DESIGN | EACH TRUCK | - | VERIFY FOR EA |
| - FORMWORK PRIOR TO PLACEMENT OF CONCRETE | Р | ACI 301-16 2.2-2.3 | INSPEC ⁻ BEAM, C SLAB-OI |
| - PLACEMENT OF CONCRETE | С | ACI 301-16 5.3.2 | |
| - CURING | Р | ACI 301-16 5.3.6 | |
| - SHORE/FORM REMOVAL | Р | ACI 301-16 2.3.2 | FOR BE |

QUALITY ASSURANCE GENERAL NOTES L INSPECTIONS STATEMENT OF STRUCTURAL SPECIAL INSPECTIONS AND TESTING CRITERIA **GENERAL:** A. SCOPE OF WORK THE OWNER WILL ENGAGE A QUALIFIED INSPECTION AND TESTING AGENCY(S) TO PERFORM GRADE, FINISH, SIZE, BAR QUANTITY, SPECIAL INSPECTIONS AND TESTING FOR ALL STRUCTURAL MEMBERS AND ASSEMBLIES AS TION, SPACING, COVER, HOOK LENGTHS, NOTED HEREIN. E LENGTH, SPLICE LOCATIONS, BEND SPECIAL INSPECTIONS ARE IN ADDITION TO INSPECTIONS BY THE AUTHORITY HAVING TERS, COATING, SURFACE CONDITION, JURISDICTION REQUIRED BY IBC 2018 SECTION 110. UPPORT REFER TO THE SPECIFICATIONS FOR REPORTING AND PROCEDURAL REQUIREMENTS FOR QUALITY ASSURANCE AND QUALITY CONTROL. ASTM A706 REINFORCING STEEL REFER TO ARCH/MECH/ELEC/CIVIL SPECIFICATIONS AND DRAWINGS FOR ADDITIONAL SPECIAL INSPECTION AND TESTING THAT MAY BE REQUIRED. -B. SPECIAL INSPECTIONS AND TESTING ARE APPLICABLE TO ALL REVISIONS AND/OR FUTURE WORK -ADDED BY AMENDMENTS TO THESE DOCUMENTS. C. DEFINITIONS - SPECIAL INSPECTOR: THE AGENCY ENGAGED BY THE OWNER AND APPROVED BY THE AUTHORITY HAVING JURISDICTION TO ACT AS THE DESIGNATED REPRESENTATIVE TO PERFORM INSPECTIONS. TYPE, FINISH, DIAMETER, LENGTH, SPECIAL INSPECTION: INSPECTION PERFORMED BY THE SPECIAL INSPECTOR ACCORDING TO IBC TITY, EMBEDMENT LENGTH, SPACING AND 2018 SECTION 1704 TO ENSURE COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENTS DISTANCES. VERIFY USE OF PLACING AND REFERENCED STANDARDS. LATE WHERE SPECIFIED • (P) PERIODIC INSPECTION: THE PART-TIME OR INTERMITTENT OBSERVATION BY THE SPECIAL CT PER THE STRUCTURAL STEEL TABLE INSPECTOR OF WORK BEING PERFORMED. SPECIAL INSPECTOR SHALL BE PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. OBSERVATION OF ALL WORK (100% VISUAL) SHALL BE MADE AT THE COMPLETION OF THE WORK. (C) CONTINUOUS INSPECTION: THE FULL-TIME OBSERVATION BY THE SPECIAL INSPECTOR OF Y USE OF APPROVED DESIGN MIXTURE WORK BEING PERFORMED. SPECIAL INSPECTOR SHALL BE PRESENT IN THE AREA WHERE THE EACH TRUCK LOAD WORK IS BEING PERFORMED. OBSERVATION OF ALL WORK (100% VISUAL) SHALL BE MADE AT CT FIRST POUR OF EACH TYPE (GRADE THE COMPLETION OF THE WORK. I, COLUMN, STRUCTURAL SLAB, -ON-DECK, ETC.) D. DEFICIENCIES IN WORK CORRECT DEFICIENCIES IN WORK THAT TESTS AND INSPECTIONS INDICATE DO NOT COMPLY -WITH THE CONTRACT DOCUMENTS AND REFERENCED STANDARDS. ALL COST OF ADDITIONAL TESTING AND/OR INSPECTIONS FOR CORRECTIVE WORK SHALL BE -BORNE BY THE CONTRACTOR. EAMS AND STRUCTURAL SLABS 2. SHOP FABRICATIONS: A. GENERAL PERFORM INSPECTIONS AND TESTING FOR ALL SHOP FABRICATED STRUCTURAL MEMBERS AND ASSEMBLIES AS NOTED HEREIN. SPECIAL INSPECTOR SHALL PERFORM SPECIAL INSPECTIONS AND TESTING UNLESS THE FABRICATOR IS REGISTERED AND APPROVED BY THE AUTHORITY HAVING JURISDICTION TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION OR

APPROVED.

2018 SECTION 1704.2.5.1.

B. SHOP FABRICATIONS INCLUDED

SOILS SPECIAL INSPECTIONS ITEM FREQUENCY STANDARD CRITERIA SUBGRADE - EXCAVATION VERIFY EXCAVATIONS ARE EXTENDED TO THE PROPER DEPTH AND HAVE REACHED THE PROPER BEARING MATERIAL SOILS REPORT VERIFY BEARING MATERIAL IS ADEQUATE TO **BEARING MATERIAL** ACHIEVE THE DESIGN BEARING CAPACITY CONTROLLED FILL - PRIOR TO PLACEMENT VERIFY SUBGRADE HAS BEEN PROPERLY PREPARED - PLACEMENT VERIFY USE OF PROPER MATERIALS, DENSITIES, COMPACTION, AND LIFT С THICKNESSES

FABRICATION HAS A CURRENT ICC-ES EVALUATION REPORT.

 SPECIAL INSPECTOR SHALL VERIFY THE FABRICATOR MAINTAINS AND FOLLOWS DETAILED SHOP FABRICATION AND QUALITY CONTROL PROCEDURES, UNLESS FABRICATOR IS REGISTERED AND

CERTIFICATE OF COMPLIANCE TO THE AUTHORITY HAVING JURISDICTION ACCORDING TO IBC

QUALIFICATIONS THAT MEET OR EXCEED THE CRITERIA OF AWS D1.1 SUBCLAUSE 6.14.6 AND

AMERICAN SOCIETY FOR NONDESTRUCTIVE TESTING (ASNT) SNT-TC-1A OR ASNT CP-189.

AT THE COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A

NONDESTRUCTIVE TESTING (NDT) SHALL ONLY BE PERFORMED BY PERSONNEL WITH

APPROVED FABRICATORS MAY PERFORM TESTING NOTED HEREIN EXCEPT THAT

SHOP FABRICATED STRUCTURAL STEEL INCLUDING STAIRS AND RAILING ELEMENTS

SHOP FABRICATED STEEL CONNECTIONS FOR STRUCTURAL WOOD CONNECTIONS

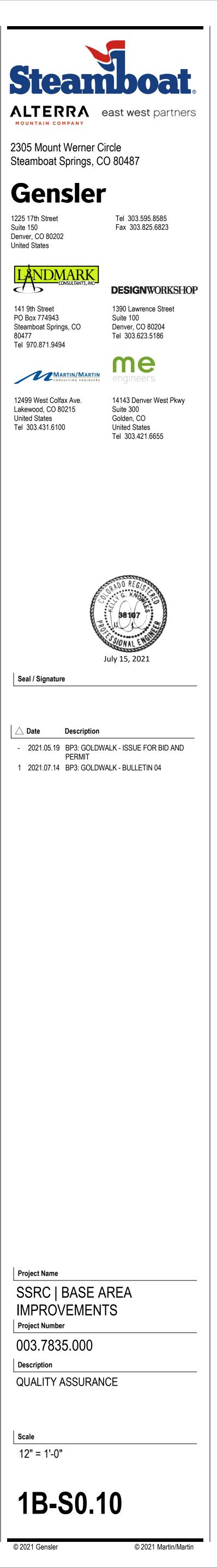
SOILS SPECIAL INSPECTION NOTES:

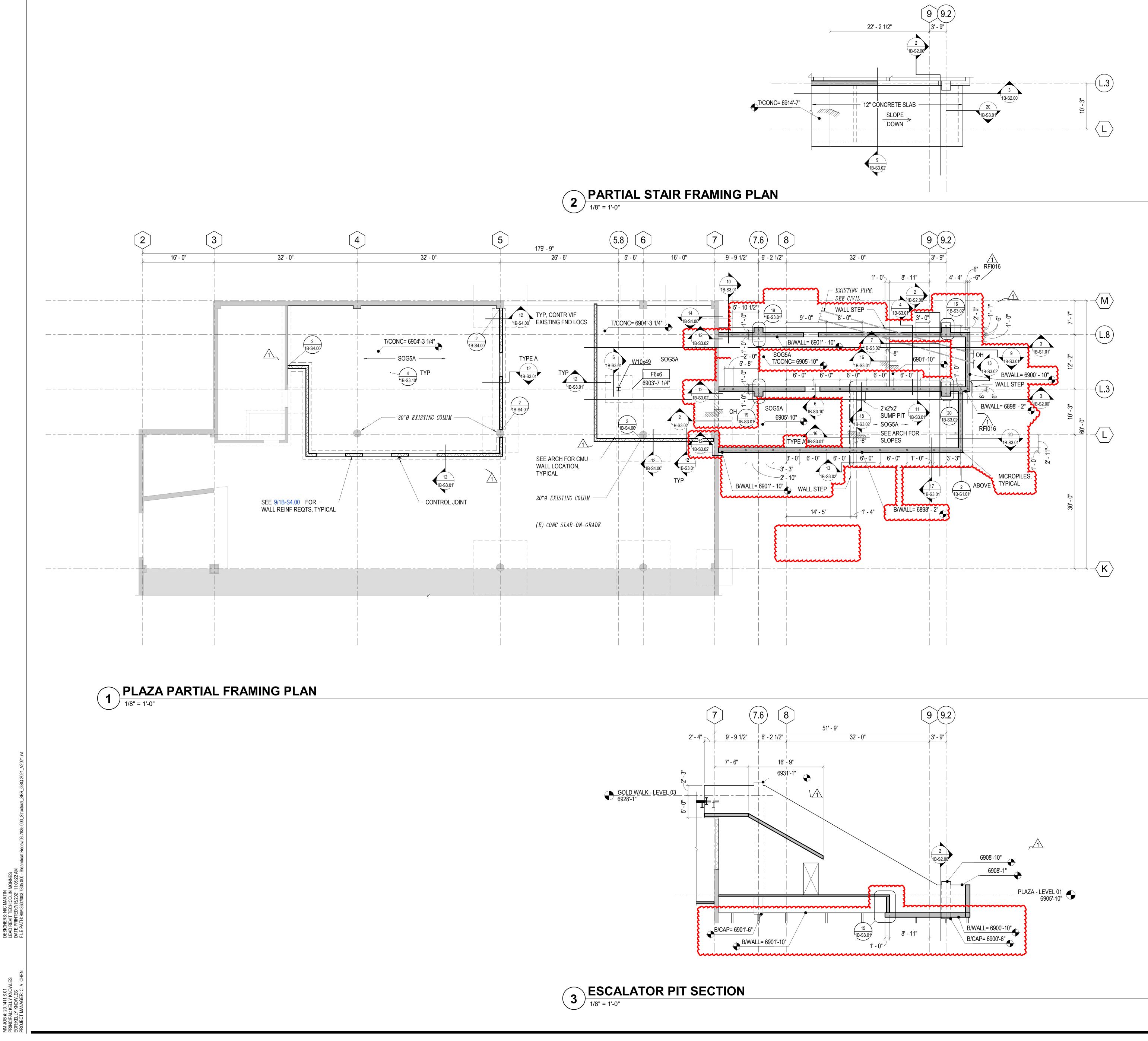
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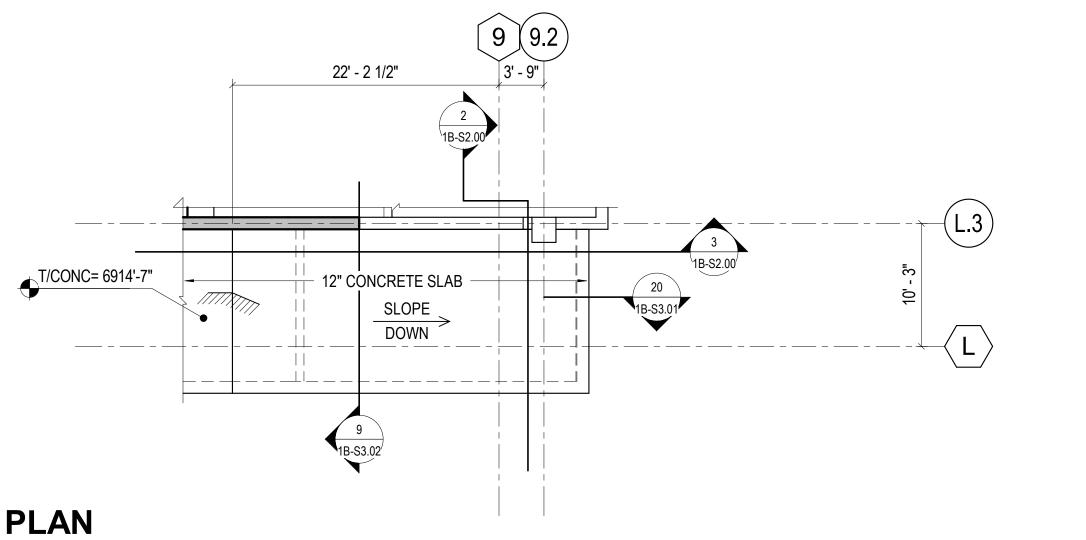
1. SEE CIVIL DRAWINGS AND/OR SPECIFICATIONS FOR ADDITIONAL EARTHWORK AND UTILITY INSPECTION REQUIREMENTS.

2. SEE CIVIL DRAWINGS AND/OR SPECIFICATIONS FOR CLASSIFICATION AND TESTING REQUIREMENTS FOR COMPACTED FILL AND/OR CONTROLLED LOW-STRENGTH MATERIAL.

| ITEM | FREQUENCY | STANDARD | CRITERIA |
|---|-----------|---------------------------------|--|
| VERIFICATION & PROOF TES | TING | | |
| - OBSERVE TESTING | С | - | DETERMINE CAPACITIES OF TEST ELEMENTS |
| DURING DRILLING | | | |
| - DRILLING OPERATIONS | С | - | PROVIDE REPORT FOR EACH MICROPILE |
| - SHAFT PLACEMENT | С | - | VERIFY LOCATION, PLUMBNESS, AND DIAMETE |
| - BEARING MATERIAL | С | PRE- CONSTRUCTION TESTING | VERIFY BEARING MATERIAL IS EQUIVALENT TO THAT USED IN PRECONSTRUCTION TESTING |
| - DEPTH OF PENETRATION | С | - | VERIFY DEPTH OF PENETRATION INTO BEARIN MATERIAL AND OVERALL LENGTH |
| - CONCRETE/GROUT AND REINFORCING STEEL | - | - | PER MICROPILE SUBCONTRACTOR. |







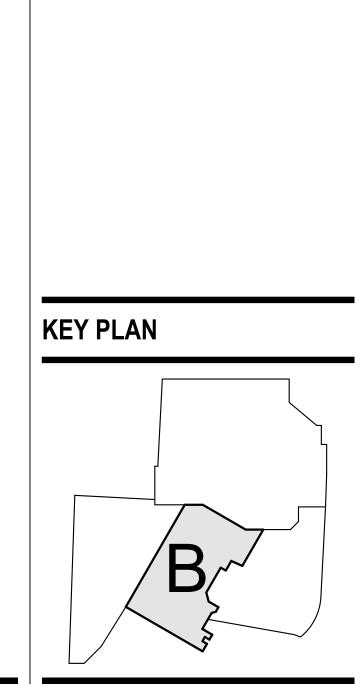
PLAN NOTES

1. CONTRACTOR TO VERIFY ALL EXISTING CONDITION PRIOR TO STEEL FABRICATION.

2. CONTRACTOR TO FIELD LOCATE ALL UTILITIES BELOW GRADE. CONTRACTOR SHALL NOTIFY ARCHITECT BY DIMENSIONED DRAWING OF LOCATIONS WHERE UTILITIES CONFLICT WITH FOUNDATION INSTALLATION. CONTRACTOR SHALL MAKE ALLOWANCE FOR THE RESOLUTION OF SUCH DISCOVERIES PRIOR TO PROCEEDING WITH EFFECTED FOUNDATIONS.

3. SEE ARCH AND MECH DRAWINGS FOR SLAB SLOPES, DEPRESSIONS, FILL, PADS, AND CURBS NOT SHOWN ON THE STRUCTURAL DRAWINGS.

4. SEE 1B-S4.00 FOR TYPICAL MASONRY WALL DETAILS. SEE ARITECTURAL DRAWINGS FOR DIMENSIONS OF ALL MASONRY WALLS.



LEGEND

OUT OF SCOPE

