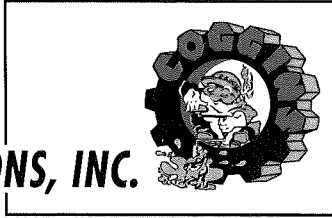


COGGINS & SONS, INC.



Caisson Drilling, Excavation Shoring, Tieback Anchors

**RCRBD
Record Set
TC
06/07/2021**

CALCULATIONS ANCHORS THROUGH EXISTING WALL

for

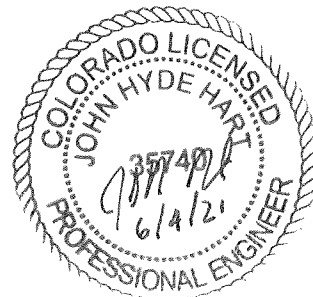
PROJECT NO. - 5776
SSRA - BASE AREA IMPROVEMENTS
2305 MOUNT WERNER CIRCLE
STEAMBOAT SPRINGS, CO 80487

Prepared for

CLIENT: SAUNDERS CONSTRUCTION, INC.
ADDRESS: 86 INVERNESS PLACE NORTH
CITY: ENGLEWOOD STATE: COLORADO

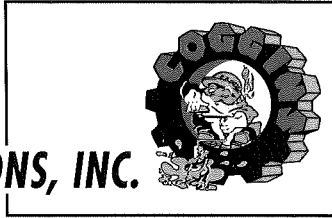
PH. 303-699-9000

Prepared By:
JOHN H. HART, PE, D.GE
COGGINS & SONS, INC.



DATE: JUNE 4, 2021

COGGINS & SONS, INC.



Caisson Drilling, Excavation Shoring, Tieback Anchors

**CALCULATIONS
SUBMITTAL INDEX**

for

PROJECT NO. - 5776
PROJECT: STEAMBOAT BASE AREA

ITEM NO.	DESCRIPTION	PAGES
1	GEOTECHNICAL INFORMATION	S1.0 - S1.5
2	EARTH PRESSURE AND ANCHOR LOADING	S2.0 - S2.2
3	BEARING PLATE	S3.0
4		
5		
6		
7		
8		
9		
10		

APPENDIX "A" REFERENCE MATERIAL AND CODES



December 30, 2020

Steamboat Ski & Resort Corp.
 Attn: Jim Schneider
 2305 Mt. Werner Circle
 Steamboat Springs, CO 80487

Job Number: 20-12000

Subject: Subsoil and Foundation
 Investigation, Steamboat Base Area
 Redevelopment, Steamboat Springs,
 Colorado.

Jim,

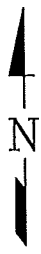
This report presents the results of the Subsoil and Foundation Investigation for the proposed Steamboat Base Area Redevelopment project within the Steamboat Ski & Resort located at 2305 Mt. Werner Circle in Steamboat Springs, Colorado. The approximate location of the project site is shown in Figure #1.

NWCC, Inc. (NWCC) scope of our work included obtaining data from observations made at the site, logging of seven test holes, sampling of the probable foundation soils, and laboratory testing of the samples obtained. This report presents recommendations for economically feasible and safe type foundations, as well as allowable soil pressures and other design and construction considerations that are advisable, but not necessarily routine to quality design and building practices.

For design purposes, NWCC has assumed that building loads will be moderate to high, typical of this type of commercial construction. If loadings or conditions are significantly different from those above, NWCC should be notified to reevaluate recommendations in this report.


Proposed Construction: NWCC understands the first phase of the project will consist of the demolition of two of the existing buildings within the existing ski area base and the construction of two new buildings to be named the Promenade and Building B.

The existing Gondola building will be demolished and reconstructed as the Promenade. We understand the Promenade will be a composite steel-framed or mild reinforced concrete elevated slab structure with the main level at the existing grade and one level below expected to extend approximately 14 feet below the existing grade. It is anticipated the Promenade may have additional floors built above the main level in the future. We understand the maximum column dead load for this building is anticipated to be approximately 375 kips and the maximum anticipated column dead load plus live load for this building is approximately 750 kips. Wall loads are anticipated to be on the order of 15 kips per foot.



NOT TO SCALE



Title: SITE PLAN-LOCATION OF TEST HOLES	Date: 12/18/2020	
Job Name: Steamboat Base Area Redevelopment	Job No. 20-12000	
Location: Steamboat Ski Area, Steamboat Springs, Colorado	Figure #2	

(970)879-7888 Fax (970)879-7891
 2580 Copper Ridge Drive P.O. Box 775226
 Steamboat Springs, Colorado 80477

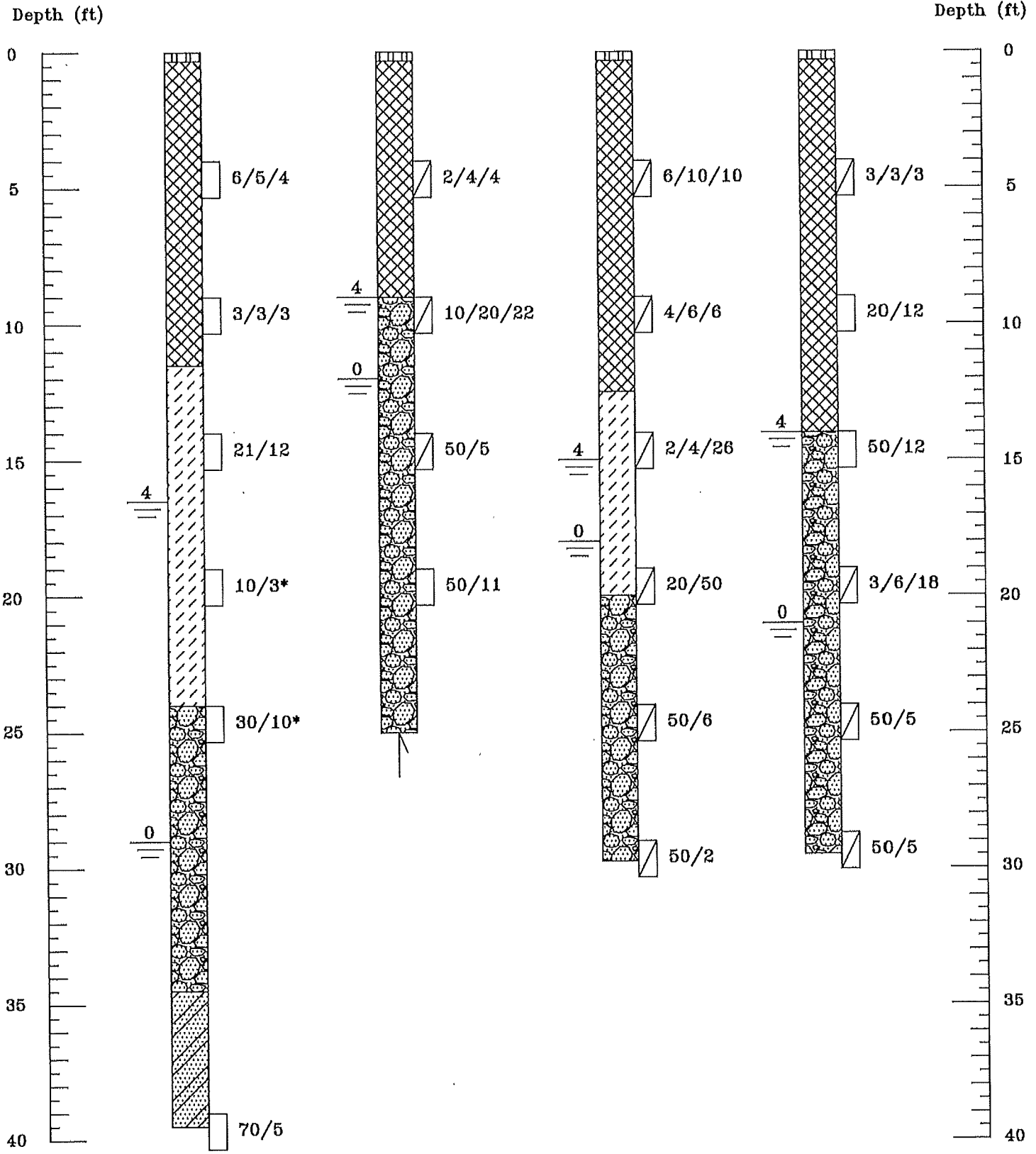
S.I.2

Test Hole 4
Elevation: 6,905.5'

Test Hole 5
Elevation: 6,892.2'

Test Hole 6
Elevation: 6,904.8'

Test Hole 7
Elevation: 6,905.7'



Title: LOGS OF EXPLORATORY TEST HOLES	Date: 12/18/2020	
Job Name: Steamboat Base Area Redevelopment	Job No.: 20-12000	
Location: Steamboat Ski Area, Steamboat Springs, Colorado	Figure #4	

(970)870-7888 • Fax (970)870-7891
2580 Copper Ridge Drive • P.O. Box 775226
Steamboat Springs, Colorado 80417

LEGEND:



BRICK PAVER UNDERLAIN BY: WELDED WIRE MESH, SNOWMELT LINE AND LEVELING SAND.



ASPHALT PAVEMENT.



FILL: Aggregate Base Course and Subbase Aggregate.



FILL: Sandy clays to clayey sands, low to moderately plastic, fine to coarse grained with gravels and occasional cobbles and boulders, soft to very stiff to loose to medium dense, slightly moist to very moist and gray to dark brown.



CLAYS: Slightly sandy to very sandy, low to moderately plastic, fine to coarse grained with gravels and occasional cobbles, soft to stiff, moist to wet and light brown to brown.



SANDS AND GRAVELS: Silty to clayey, low to non-plastic, fine to coarse grained with cobbles and boulders, medium dense to very dense, moist to wet and brown to gray.



CLAYSTONE BEDROCK: Sandy to very sandy to silty, low to moderately plastic, fine to medium grained with occasional gravels, weathered to very hard, slightly moist to moist and light brown in color.



Drive Sample, 2-inch I.D. California Liner Sampler.



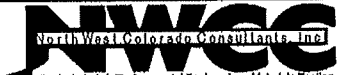
Drive Sample, Split Spoon Sampler.

35/12 Drive Sample Blow Count, indicates 35 blows of a 140-pound hammer falling 30 inches were required to drive the sampler 12 inches.
*Indicates hammer was bouncing on a suspected cobble or boulder.

3/5/20 Drive Sample Blow Count, indicates split spoon sampler with 3,5, and 20 blows of a 140-pound hammer falling 30 inches were required to drive the sampler 6 inches.

0,4,5 Indicates depth at which groundwater was encountered when measured at time of drill and when measured 4 or 5 days after drilling.

↑ Indicates depth at which practical rig refusal was encountered in very dense cobbles and boulders.

Title: LEGEND AND NOTES	Date: 12/18/2020	
Job Name: Steamboat Base Area Redevelopment	Job No. 20-12000	
Location: Steamboat Ski Area, Steamboat Springs, Colorado	Figure #5	

(970)879-7888 • Fax (970)879-7891
2680 Copper Ridge Drive • P.O. Box 775226
Steamboat Springs, Colorado 80477

NWCC, Inc.

TABLE 1

SUMMARY OF LABORATORY TEST RESULTS

SAMPLE LOCATION		NATURAL MOISTURE CONTENT (%)	NATURAL DRY DENSITY (pcf)	ATTERBERG LIMITS			GRADATION		PERCENT PASSING No. 200 SIEVE	UNCONFINED COMPRESSIVE STRENGTH (psf)	SOIL or BEDROCK DESCRIPTION	UNIFIED SOIL CLASS.
TEST HOLE	DEPTH (feet)			LIQUID LIMIT (%)	PLASTICITY INDEX (%)	GRAVEL (%)	SAND (%)					
1	9	14.6	103.1	27	9	1	29	70		Claystone Bedrock	CL	
2	19	10.7	118.0	36	22	1	27	72	26,350	Claystone Bedrock	CL	
3	4	13.8	114.2	39	23	0	19	81		Claystone Bedrock	CL	
4	14	22.7	102.3	39	23	0	20	80		Sandy Clay	CL	
4	24	11.4		22	3	4	76	20		Silty Sand	SM	
4	39	16.0	110.3	28	10	0	36	64	16,190	Claystone Bedrock	CL	
5	14	14.5			NP	7	80	13		Silty Sand	SM	
5	19	11.4			NP	38	56	6		Very Gravelly Sand	SP	
6	4	20.8		26	9	21	45	34		FILL: Clayey Gravelly Sand	SC	
6	14	18.0		26	10	11	50	39		Very Clayey Sand	SC	
7	9	19.1	104.1	28	10	4	20	76		FILL: Sandy Clay	CL	
7	19	16.5		24	9	18	52	30		Gravelly Clayey Sand	SC	
7	29	14.1		22	3	34	51	15		Gravelly Silty Sand	SM	

51.4

NP = Non Plastic

JOB NUMBER: 20-12000

COGGINS & SONS, INC.

PH. (303) 791-9911 FAX (303) 791-0967
CAISSON DRILLING
EARTH RETENTION
TIE BACK ANCHORS

Name J. HANT

PROJECT: STEAMBOAT

PROJECT NO.: 5776

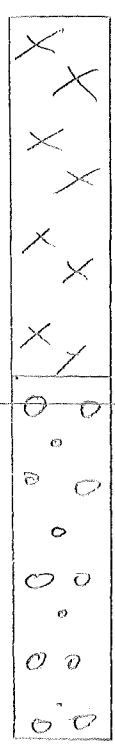
SUBJECT: GEO TECH

DATE: _____

REFER TO: NORTHWEST COLORADO CONSULTANTS, INC.
SUBSOIL AND FOUNDATION INVESTIGATION 12/30/20
JOB # 20-12000

TEST HOLE #7

6906



$$\gamma_{sat} = 104.1 \text{ #/ft}^3$$

$$W = 19.1$$

$$- 200 = 76$$

$$\gamma_{dust} = 124 \text{ #/ft}^3$$

6892

6876

NO SPECIFIC TESTS TO DETERMINE SHEAR STRENGTH OF SOIL. WILL HAVE TO USE CORRELATIONS FROM BLOW COUNTS AND EXPERIENCE IN THE AREA.

WILL USE THE FOLLOWING:

$$\gamma_{dust} = 125 \text{ #/ft}^3$$

$$\phi = 34^\circ$$

$$C = 0 \text{ #/ft}^2$$

Name J. HUNT

52.0

PROJECT: STEAM BOAT

PROJECT NO.: 5776

SUBJECT: GEOTECH

DATE: _____

EARTH PRESSURE

$$EP = .65 K \gamma H$$

$$K_a = \tan^2 (45 - \phi/2) = .283$$

$$\gamma = 125 \text{ #/FT}^3$$

$$H = 16 \text{ FT}$$

$$EP = .65 (.283) (125 \text{ #/FT}^3) (16 \text{ FT})$$

$$EP = 368 \text{ #/FT}^2$$

$$EP = 368 \text{ #/FT}^2 (8 \text{ FT}) = \underline{\underline{2,944 \text{ #/FT}}}$$

DETERMINE ANCHOR LOAD ON WALL

REFER TO FOLLOWING SPREADSHEET

ANCHOR LOAD = 30 K WILL USE 40 K ∴ OK

ANCHOR DIA. = 1.00 in WILL USE 1.00 in ∴ OK

BOND LENGTH = 12 FT WILL USE 20 FT ∴ OK

UNBOND LENGTH = 7 FT WILL USE 12 FT ∴ OK

STEAMBOAT SPRINGS ANCHORS THROUGH EXISTING WALL
H = 16 FEET

LOAD PER FOOT (K/FT)	TOTAL HEIGHT (FT)	DEPTH TO TIEBACK (FT)	SPAN LENGTH (FT)	DIP ANGLE
2.9	16	7	9	20

$\phi = 34$
Ka = 0.283
$\gamma = 125$
Ep = 23
SURCHARGE =
SPACING = 8

34.2

TIEBACK LOAD (K)	EXCAVA. LOAD (K)	ZERO SHEAR (ft. from bot)
28.5	2.6	3.2

MOMENT SUPPORT (K-ft)	MOMENT CANT. (K-ft)	SECTION MODULUS (in ³)
5.8	34.2	11.14

HEIGHT (FT)
16

USE SHAPE

USE STRAND
1

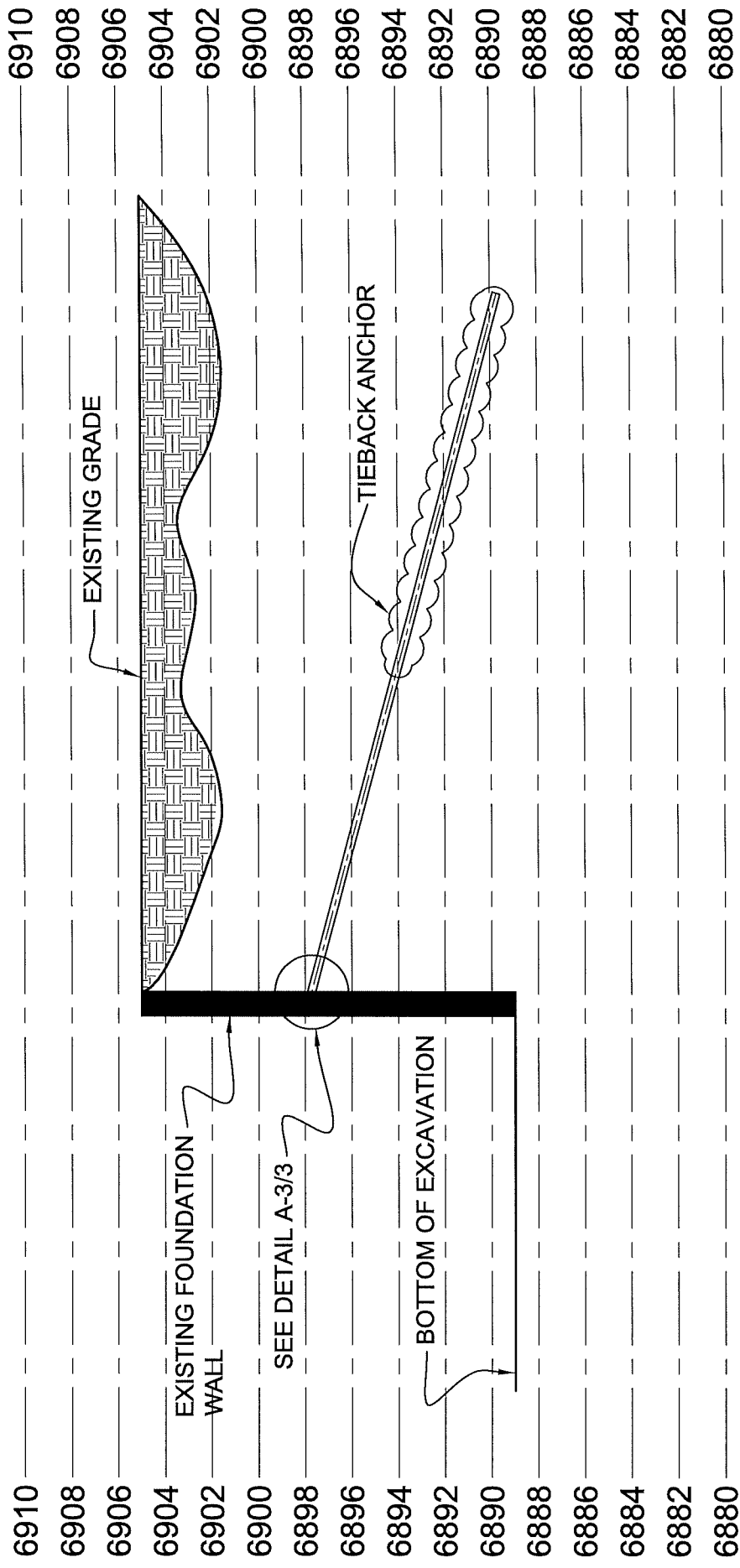
USE BAR (GRADE 150)
1.000

TIEBACK LOAD (K)
30

LOAD
35
70
105
140

SOIL SKIN FRICTION (psi)	DIA. OF HOLE (in.)	DIA. INCREASE	ADJ. DIA. HOLE (in)	TIEBACK	
				LOAD (K)	1.2 LOAD (K)
15	5.25	1.0	5.25	30.28	36.33
				BOND LENGTH (ft)	12
				LOAD PER FOOT (K/ft)	3.0

WALL HT. (ft)	ANCHOR (ft)	DIP ANGLE	FAILURE ANG.	BETA ANGLE	THETA ANGLE	GAMMA ANGLE	LENGTH B (ft)	LENGTH B' (ft)	UNBOND LEN. (ft)
16	9	20	62	28	70	82	4.27	2.85	7



CROSS SECTION ANCHOR #8

NOT TO SCALE

COGGINS & SONS, INC.

PH. (303) 791-9911 FAX (303) 791-0967
CAISSON DRILLING
EARTH RETENTION
TIE BACK ANCHORS

Name J. HART

93.0

PROJECT: STEAMBOAT

PROJECT NO.: 5776

SUBJECT: BEARING PLATE

DATE: _____

$$\text{ANCHOR LOAD} = 40 \text{ k}$$

$$\text{BEARING PLATE} = 15 \text{ in} \times 15 \text{ in}$$

$$\text{CORE HOLE} = 7 \text{ in}$$

$$\text{PLATE AREA} = (15 \text{ in})(15 \text{ in}) = 225 \text{ in}^2$$

$$\text{CORE HOLE AREA} = \frac{\pi (7 \text{ in})^2}{4} = 38 \text{ in}^2$$

$$225 \text{ in}^2$$

$$- 38 \text{ in}^2$$

$$\hline 187 \text{ in}^2$$

$$\frac{40000 \#}{187 \text{ in}^2} = 213 \#/\text{in}^2$$

$$\text{CONCRETE STRENGTH} = 3000 \#/\text{in}^2 \text{ (minimum)}$$

$$3000 \#/\text{in}^2 > 213 \#/\text{in}^2 \therefore \text{OK}$$

APPENDIX “A”

REFERENCE MATERIAL AND CODES

- I. EARTH RETENTION DESIGN REFERENCE DOCUMENTS & BIBLIOGRAPHY
- II. REFERENCE DESIGN CODES AND STANDARDS

**COGGINS & SONS, INC., EARTH RETENTION DESIGN REFERENCE MATERIALS
DECEMBER 31, 2015,
BY JOHN H. HART, P.E., D.GE, JAMES V. WARNICK, JR., P.E.**

EARTH RETENTION DESIGN REFERENCE DOCUMENTS & BIBLIOGRAPHY

- 1) PECK, HANSON & THORNBURN, "FOUNDATION ENGINEERING", SECOND EDITION, 1974.
- 2) TERZAGHI, PECK, MEARI, "SOIL MECHANICS IN ENGINEERING PRACTICE", THIRD EDITION, 1996.
- 3) LAMBE, WHITMAN, "SOIL MECHANICS", FIRST EDITION, 1969.
- 4) SPECIAL REPORT 247 "LANDSLIDE: INVESTIGATION AND MITIGATION", TRANSPORTATION RESEARCH BOARD
- 5) DRILLED SHAFTS: CONSTRUCTION PROCEDURES AND LRFD DESIGN METHODS FHWA-NHI-10-016 MAY 2010
- 6) GROUND ANCHORS AND ANCHORED SYSTEMS, GEOTECHNICAL ENGINEERING CIRCULAR NO.4, FHWA OFFICE OF BRIDGE TECHNOLOGY, JUNE 1999.
- 7) JOSEPH E. BOWLES, "FOUNDATION ANALYSIS AND DESIGN", FOURTH AND FIFTH EDITIONS, 1988 & 1996.
- 8) BRAJA M. DAS, "PRINCIPLES OF FOUNDATION ENGINEERING", SECOND EDITION, 1990.
- 9) HOLTZ AND KOVACS, "AN INTRODUCTION TO GEOTECHNICAL ENGINEERING", 1981.
- 10) ROBERT M. KOERNER, "DESIGNING WITH GEOSYNTHETICS", THIRD EDITION, 1994.
- 11) PTI, "RECOMMENDATIONS FOR PRESTRESSED ROCK AND SOIL ANCHORS", FOURTH EDITION, 2004.
- 12) ALAN MACNAB, "EARTH RETENTION SYSTEMS HANDBOOK", 2002.
- 13) XANTHAKOS, ABRAMSON & BRUCE, "GROUND CONTROL AND IMPROVEMENT", 1994.
- 14) PETROS P. XANTHAKOS, "GROUND ANCHORS AND ANCHORED STRUCTURES", 1991.
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- 16) LEE W. ABRAMSON, THOMAS S. LEE, SUNIL SHARMA, GLENN M. BOYCE, "SLOPE STABILITY AND STABILIZATION METHODS", 1996.
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- 20) RISA TECHNOLOGIES, "RISA-2D"
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- 22) ENSOFT, INC., "L-PILE"
- 23) AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "MANUAL OF STEEL CONSTRUCTION – ALLOWABLE STRESS DESIGN", NINTH EDITION, 1989.
- 24) AMERICAN INSTITUTE OF STEEL CONSTRUCTION, "MANUAL OF STEEL CONSTRUCTION - LOAD AND RESISTANCE FACTOR DESIGN", THIRD EDITION, 2001.
- 25) ACI 381-05/318R-05, "BUILDING CODE AND COMMENTARY", 2005.
- 26) ANSI/ASCE 7-95, "MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES."
- 27) AF&PA ASD/LRFD -2005, "WOOD DESIGN PACKAGE

- 28) IBC 2009 AND 2012
- 29) INSTITUTION OF CIVIL ENGINEERS, "GROUND ANCHORAGES AND ANCHORED STRUCTURES", PROCEEDINGS OF THE INTERNATIONAL CONFERENCE ORGANIZED BY THE INSTITUTION OF CIVIL ENGINEERS AND HELD IN LONG, UK, ON 20-21 MARCH 1997, EDITED BY G.S. LITTLEJOHN.
- 30) ADSC-IAF – MICROPILE COMMITTEE, "BUCKLING OF MICROPILES – A REVIEW OF HISTORIC RESEARCH AND RECENT EXPERIENCES", ALLEN CADDEN, P.E. AND JESÚS GÓMEZ, Ph.D., SCHNABEL ENGINEERING ASSOCIATES, WEST CHESTER, PA.
- 31) POST-TENSIONING INSTITUTE, PREPRINT OF CHAPTER VIII, POST-TENSIONING MANUAL, SIXTH EDITION, "ANCHORAGE ZONE DESIGN", GREGOR P WOLLMANN AND CARIN L. ROBERTS-WOLLMAN, 2000.
- 32) U.S. DEPARTMENT OF TRANSPORTATION, FEDERAL HIGHWAY ADMINISTRATION, PUBLICATION NO. FHWA-SA-93-068, "SOIL NAILING FIELD INSPECTORS MANUAL, SOIL NAIL WALLS-DEMONSTRATION PROJECT 103", APRIL 1994.
- 33) MICROPILE DESIGN AND CONSTRUCTION PUBLICATION NO. FHWA NHI-05-039 DECEMBER 2005
- 34) VARIOUS TECHNICAL PAPERS OBTAINED FROM CONFERENCES AND SEMINARS
- 35) PROPRIETARY DATA GATHERED BY COGGINS THROUGH IN-HOUSE RESEARCH

Saunders Construction, LLC
86 Inverness Place North
Englewood, CO 80112

PROJECT: Steamboat - BV BP Promenade and Goldwalk

DATE: 05/27/2021

TO: Coggins & Sons, Inc.
9512 Titan Park Cir
Littleton, CO 80125

RE: 3-315000-1-Excavation and Support

ATTN: John Hart

JOB: 2001502104

WE ARE SENDING:	SUBMITTED FOR:	ACTION TAKEN:
<input type="checkbox"/> Shop Drawings	<input type="checkbox"/> Review	<input type="checkbox"/> Reviewed
<input type="checkbox"/> Letter	<input type="checkbox"/> Your Use	<input type="checkbox"/> Reviewed w/ Corrections Noted
<input type="checkbox"/> LEED	<input type="checkbox"/> As Requested	<input type="checkbox"/> Resubmit
<input type="checkbox"/> Product Data		<input type="checkbox"/> Submit
<input type="checkbox"/> Plans		<input type="checkbox"/> Returned
<input type="checkbox"/> Samples	SENT VIA:	<input type="checkbox"/> Returned for Corrections
<input type="checkbox"/> Specifications	<input checked="" type="checkbox"/> Attached	<input checked="" type="checkbox"/> Due Date: 06/17/2021
<input type="checkbox"/> Other:	<input type="checkbox"/> Separate Cover Via:	

Item	Package	Code	Rev.	Copies	Date	Description	Status
Submittal	3-315000-1	BP3_315000.01	1		05/27/2021	Excavation Support and Protection - Shop Drawings	No Exception Taken

NOTES: Please review (BP3_315000_01_Excavation_and_Support_NET). Please release all material and communicate immediate to SCI if schedule poses any issue.



SUBMITTED FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS EXCEPT AS NOTED. REVIEW OF THIS SUBMITTAL DOES NOT RELEASE THE SUBCONTRACTOR OF ITS RESPONSIBILITY TO CONFORM WITH THE PLANS AND SPECIFICATIONS NOR DOES THIS REVIEW RELEASE THE SUBCONTRACTOR FROM VERIFYING THE ACCURACY OF QUANTITIES AND FIELD DIMENSIONS.

CC: Reno Romagnoli / Saunders Construction, LLC

Submittal Package Number:
BP3_315000_01_Excavation_and_Support_NET for
Reviewed/No Exceptions Taken
Signed: Alec Hallman
Date: 05/27/2021
SAUNDERS CONSTRUCTION, INC.

Alec Hallman

Saunders Construction, LLC
 86 Inverness Place North
 Englewood, CO 80112

PROJECT: Steamboat - BV BP Promenade and Goldwalk

DATE: 05/17/2021

TO: M. Arthur Gensler, Jr. and Associates,
 Inc.
 1225 17th Street, # 150
 Denver, CO 80202

RE: 3-315000-1-Excavation and Support

ATTN: Jacob Apple

JOB: 2001502104

WE ARE SENDING:	SUBMITTED FOR:	ACTION TAKEN:
<input checked="" type="checkbox"/> Shop Drawings	<input checked="" type="checkbox"/> Review	<input type="checkbox"/> Reviewed
<input type="checkbox"/> Letter	<input type="checkbox"/> Your Use	<input type="checkbox"/> Reviewed w/ Corrections Noted
<input type="checkbox"/> LEED	<input type="checkbox"/> As Requested	<input type="checkbox"/> Resubmit
<input type="checkbox"/> Product Data		<input type="checkbox"/> Submit
<input type="checkbox"/> Plans		<input type="checkbox"/> Returned
<input type="checkbox"/> Samples	SENT VIA:	<input type="checkbox"/> Returned for Corrections
<input type="checkbox"/> Specifications	<input checked="" type="checkbox"/> Attached	<input checked="" type="checkbox"/> Due Date: 06/07/2021
<input type="checkbox"/> Other:	<input type="checkbox"/> Separate Cover Via:	

Item	Package	Code	Rev.	Copies	Date	Description	Status
Submittal	3-315000-1	BP3_315000.01	1		05/17/2021	Excavation Support and Protection - Shop Drawings	For Review

NOTES: Please review (BP3_315000_01_Excavation_and_Support_FR)

CC: Geoffrey Brooksher / M. Arthur Gensler, Jr. and Ass

Signed: _____
 Alec Hallman

Submittal Review Sheet

PLEASE USE THE SPACES BELOW FOR COMMENTS AND STAMPS



SUBMITTED FOR GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS EXCEPT AS NOTED. REVIEW OF THIS SUBMITTAL DOES NOT RELEASE THE SUBCONTRACTOR OF ITS RESPONSIBILITY TO CONFORM WITH THE PLANS AND SPECIFICATIONS NOR DOES THIS REVIEW RELEASE THE SUBCONTRACTOR FROM VERIFYING THE ACCURACY OF QUANTITIES AND FIELD DIMENSIONS.

**Submittal Package Number:
BP3_315000_01_Excavation_and_Support_FR for Review
Signed: Alec Hallman
Date: 05/17/2021
SAUNDERS CONSTRUCTION, INC.**

MARTIN/MARTIN, INC

RECEIVED FOR
RECORD ONLY

Date Received: 5/17/2021

Reviewer: achen

Gensler

SUBMITTAL REVIEW

- NO EXCEPTIONS TAKEN.**
- MAKE CORRECTIONS AS NOTED.** Resubmittal not required unless Contractor cannot comply with corrections noted.
- REVISE AS NOTED AND RESUBMIT.**
- RESUBMIT PROPERLY.** Submittal not reviewed for reasons noted.
- NOT REVIEWED.** Submittal not required by Contract Documents.
- RECEIVED FOR CLIENT'S RECORD ONLY.** Submittal not reviewed.

Gensler has reviewed this Submittal, but only for the purpose of checking for conformance with the design intent expressed in the Contract Documents. Gensler's action on a specific item does not indicate approval of an assembly of which the item is a component, nor of an item as delivered and installed if it does not conform to the Contract Documents.

Contractor is responsible for checking for deviations between this Submittal and differing information or conditions in the Contract Documents and field conditions; for determining or substantiating the accuracy and completeness of other details such as dimensions and quantities; for substantiating instructions for installation or performance of equipment or systems designed by Contractor; for construction means, methods, techniques, schedules, sequences, procedures, and fabrication processes; for errors and omissions in Submittals; for coordination of the Work of the trades, safety precautions and performing the Work in a safe and satisfactory manner and in conformance with the Contract Documents.

If more than one submittal review stamp appears on this Submittal, the most stringent action and notations thereon apply. Signature of a submittal review stamp by Gensler or a consultant does not imply that it has reviewed work not within its professional discipline or scope of services.

v1 3/2012

By: 26254 Date: Thu May 27, 2021

Project No: 03.7835.000

Submittal: BP3_315000_01_Excavation_and_Support_FR MM C

\\gensler.ad\Projects\03\03.7835.000\Documentation\6\6S\BP3\

TIEBACK BEAM AND LAGGING GENERAL NOTES

- DESIGN BASIS:
 - 2015 INTERNATIONAL BUILDING CODE.
 - EXCAVATION BRACING IS DESIGNED TO BE TEMPORARY SUPPORT.
 - THE EARTH RETENTION SYSTEM IS DESIGNED TO ACCOMMODATE NORMAL CONSTRUCTION SURCHARGE (250 PSF) OF MATERIAL AND LIGHT TRUCKS AT A MINIMUM DISTANCE OF 4 FEET FROM THE FACE OF THE EARTH RETENTION. THE EARTH RETENTION HAS NOT BEEN DESIGNED FOR HEAVY POINT LOADS SUCH AS: CRANES (TRUCK OR OUTRIGGER), EXCAVATORS, CONCRETE PUMP TRUCKS, OR LARGE STOCKPILES.
 - REFERENCE CODES:
 - AISC STEEL MANUAL, THIRTEENTH EDITION
 - ACI 318-08 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE"
 - PUBLICATION NO. FHWA-IF-99-015 "GROUND ANCHORS AND ANCHORED SYSTEMS"
 - FOUNDATION:
 - SOIL INFORMATION WAS DERIVED FROM THE GEOTECHNICAL ENGINEERING REPORT TITLED PREPARED BY NORTHWEST COLORADO CONSULTANTS, INC. (NWCC JOB NUMBER: 20-12000) DATED 12/30/2020. REVISED 4/22/21
 - MATERIALS:
 - BAR ANCHORS: ALL BARS TO BE GRADE 150 THREADED REINFORCING BARS OR GRADE 75 THREADED REINFORCING BARS. DIAMETER OF BARS SPECIFIED ON TIEBACK SCHEDULES.
 - GROUT: PRESSURE INJECTED GROUT SHALL BE PORTLAND CEMENT TYPE I AND WATER MIX, WITH A WATER TO CEMENT RATIO NOT EXCEEDING 0.45.
 - GRID LINE DIMENSIONS ON EARTH RETENTION DRAWINGS SHALL BE VERIFIED BY CONTRACTOR PRIOR TO BEGINNING WORK.
 - PROCEDURE:
 - ALL UTILITIES WITH EQUIPMENT OR FACILITIES SERVING AN AREA WITHIN 300 FEET OF THE PLANNED EXCAVATION SHALL BE NOTIFIED NOT LATER THAN 2 DAYS PRIOR TO SOLDIER BEAM INSTALLATION THAT SUCH WORK IS TO TAKE PLACE.
 - ENGINEERS APPROVAL MUST BE SECURED FOR ALL SUBSTITUTIONS.
 - THE REQUIREMENTS OF THE LATEST EDITION OF THE "OSHA CONSTRUCTION STANDARDS" SHALL BE COMPLIED WITH BY ALL CONTRACTORS, FABRICATORS AND SUPPLIERS.
 - BACKFILL BETWEEN BRACING AND COMPLETED STRUCTURE SHALL BE PLACED AND COMPACTED AS REQUIRED BY SPECIFICATIONS OF THE PROJECT ENGINEER.
- TEST PROCEDURE
- EQUIPMENT:
- STRESSING EQUIPMENT USED FOR TESTING SHALL HAVE A CURRENT (LESS THAN 12 MONTHS OLD) CALIBRATION CERTIFICATE, WITH CALIBRATION CURVES BEING MADE AVAILABLE FOR REVIEW, UPON REQUEST.
 - ANCHOR MOVEMENTS IN A DIRECTION COLLINER TO THE LINE OF ACTION OF THE ANCHOR SHALL BE RECORDED BY A DIAL GAUGE WITH A MINIMUM RESOLUTION OF 0.001 INCHES.
- METHODOLOGY:
- ALL ANCHORS SHALL BE PROOF TESTED.
 - ONCE A LOAD INCREMENT HAS BEEN OBTAINED, THE LOAD SHALL BE HELD ONLY LONG ENOUGH TO GET A STABLE READING. WHEN 1.33 P TEST LOAD IS REACHED, THE LOAD SHALL BE HELD FOR A TOTAL OF 10 MINUTES WITH MOVEMENT READINGS TAKEN AT 1, 2, 3, 4, 5, 7, & 10 MINUTES.
 - IF, DURING THIS TEN MINUTE INCREMENT, THE LOAD CANNOT BE MAINTAINED OR THE ANCHOR MOVEMENT UNDER CONSTANT LOAD EXCEEDS 0.040 INCHES, THE LOAD TEST SHALL BE CONTINUED TO 60 MINUTES WITH READINGS AT 20, 30, 40, 50, & 60 MINUTES. IF THE DIFFERENCE BETWEEN 6 AND 60 MINUTES IS LESS THAN 0.080 INCHES, THE ANCHOR IS ACCEPTABLE. PROCEED TO THE LOCKOFF LOAD. IF THE DIFFERENCE IS GREATER THAN 0.080 INCHES, THE RATING OF THE ANCHOR SHALL BE RECORDED AS 50% OF THE LARGEST LOAD MAINTAINED, AND ADDITIONAL ANCHORS SHALL BE PLACED TO PROVIDE THE INTENDED CAPACITY OF THE ANCHOR.

PROOF TESTING PROCEDURE

EQUIPMENT AND METHODOLOGY:
 PROOF TESTING SHALL BE CARRIED OUT FOR EACH ANCHOR WHICH IS NOT TO BE PERFORMANCE TESTED. TESTING EQUIPMENT AND METHODOLOGY IS THE SAME AS PERFORMANCE TESTING EXCEPT THAT THE LOADING SCHEDULE BE AS FOLLOWS:

PROOF TEST LOADING SEQUENCE	
INCREMENT	LOAD
1	AL
2	0.25 P
3	0.50 P
4	0.75 P
5	1.00 P
6	1.20 P
LOCKOFF	1.00 P

AS WITH PERFORMANCE TESTING, ANY ANCHOR WHICH FAILS PROOF TESTING SHALL BE RATED AT 50 PERCENT OF THE PREVIOUS LOAD SUCCESSFULLY SUSTAINED, AND ADDITIONAL ANCHORS SHALL BE PLACED AS REQUIRED.

CONSTRUCTION MONITORING

- THE EXISTING WALL WILL BE MONITORED HORIZONTALLY DURING CONSTRUCTION.
- THE CONSTRUCTION MONITORING SHALL BE CONDUCTED ONCE A WEEK WHILE THE EARTH RETENTION IS UNDER CONSTRUCTION.
- AFTER COGGINS COMPLETION, THE GENERAL CONTRACTOR SHALL MONITOR THE WALL AT LEAST ONCE PER WEEK. THE WEEKLY MONITORING SHALL BE CONTINUED UNTIL THE PERMANENT CONCRETE FOUNDATION WALL IS COMPLETED AND BACKFILLED.
- IF ANY SUDDEN MOVEMENT OCCURS DURING THE CONSTRUCTION OF EARTH RETENTION, WORK SHALL BE SUSPENDED IMMEDIATELY AND COGGINS & SONS ENGINEERING STAFF SHALL BE IMMEDIATELY NOTIFIED.
- IF ANY SUDDEN MOVEMENT IS OBSERVED AFTER EARTH RETENTION COMPLETION AND BEFORE FOUNDATION BACKFILL, WORK SHALL BE IMMEDIATELY SUSPENDED AND COGGINS & SONS SHALL BE IMMEDIATELY NOTIFIED.
- LOGS OF ALL MONITORING SHALL BE PROVIDED TO COGGINS & SONS WEEKLY AS PERFORMED.



COGGINS & SONS, INC.

Caisson Drilling, Excavation Shoring, Tieback Anchors

9512 TITAN PARK CIRCLE
LITTLETON, COLORADO 80125
TEL: 303-791-9911
FAX: 303-791-0967

PROPOSED TEMPORARY EARTH RETENTION
SSRC - BASE AREA IMPROVEMENTS
2305 MOUNT WERNER CIRCLE
STEAMBOAT SPRINGS, CO 80487

SHEET NO.	INDEX OF SHEETS
XBS - 1	COVER SHEET
XBS - 2	PLAN VIEW & DETAILS
XBS - 3	ELEVATION VIEWS & DETAILS

FOR
SAUNDERS CONSTRUCTION INC.
86 INVERNESS PLACE NORTH
CENTENNIAL, CO 80112
PH: 303-699-9000

Digitally signed by John H. Hart, P.E., D.GE
 DN: C=US,
 E=jhart@cogginsandsons.com,
 OU="Coggins and Sons, Inc.",
 OU=Engineering, CN=John H. Hart, P.E., D.GE
 Date: 2021.05.17 09:33:40-0600'

SIGNATURE FLATTED BY ALECH TO COMBINE PDF WITH TRANSMITTAL



NOT VALID WITHOUT DATE & ORIGINAL SIGNATURE

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NOTE:
 BEFORE CONSTRUCTION COMMENCES, A UTILITY LOCATE MUST BE PERFORMED TO FIELD VERIFY THE LOCATION OF ALL UTILITIES IN THE VICINITY OF THE SITE. THE EARTH RETENTION DESIGN WILL BE REVIEWED AND DRAWINGS REVISED, IF REQUIRED, WHEN FINAL UTILITY LOCATIONS BECOMES AVAILABLE.

DIMENSIONS OBTAINED FROM BID PACK 3: PRICING AND PROCUREMENT DOCUMENTS DATED 2021.04.02 PRODUCED BY:

- GENSLER (ARCH.)
- LANDMARK CONSULTANTS, INC. (CIVIL)
- MARTIN / MARTIN ENGINEERS (STRUCTURAL)

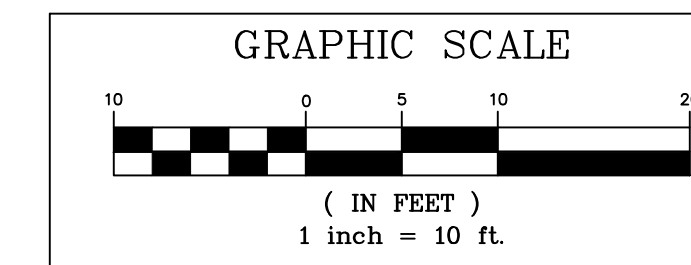
NO.	DATE	REVISIONS DESCRIPTION

This drawing, as an instrument of professional service, is the property of Coggins & Sons, Inc. It is furnished to the General Contractor for information purposes and for approval for construction by Coggins & Sons, Inc. of the excavation retention for the designated project only. It shall be returned to Coggins & Sons upon demand.

JOB NO.: 5776
 ISSUED: 2021.05.17
 DWN BY: JVV
 DWG. NO.
XBS-1
 SHT. 1 OF 3

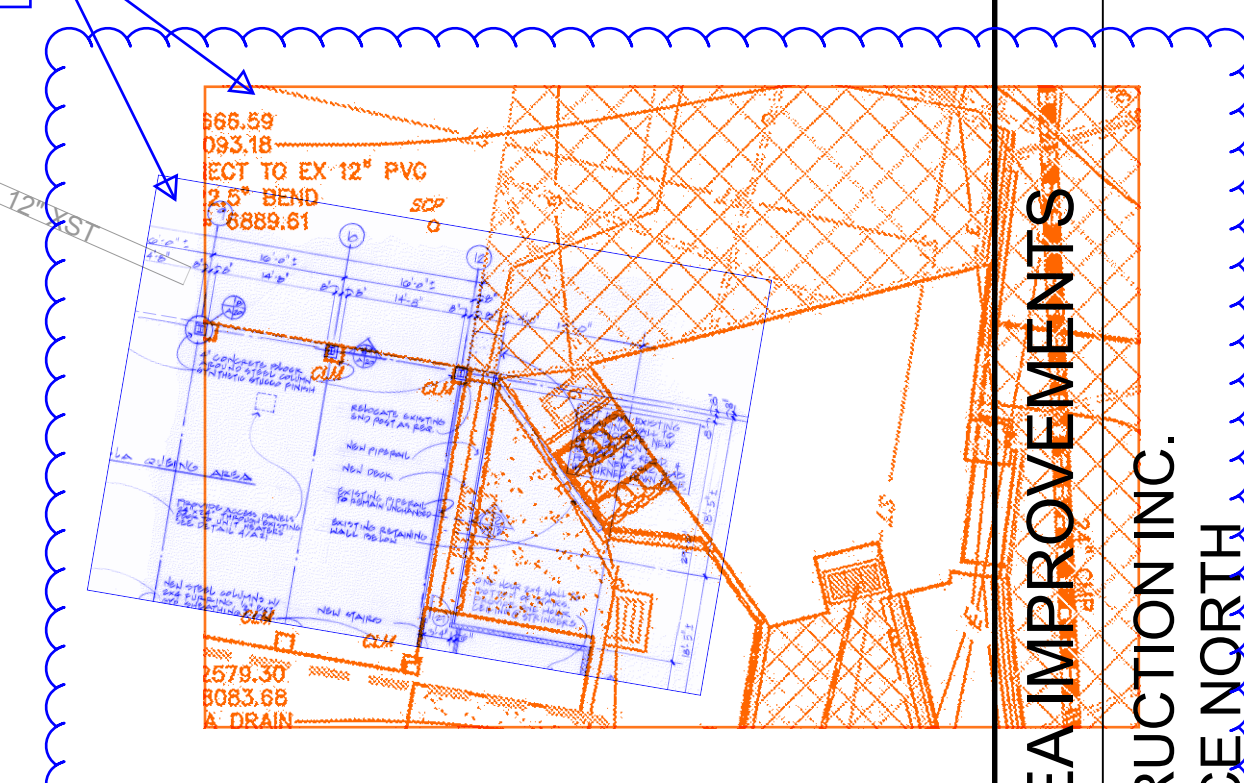
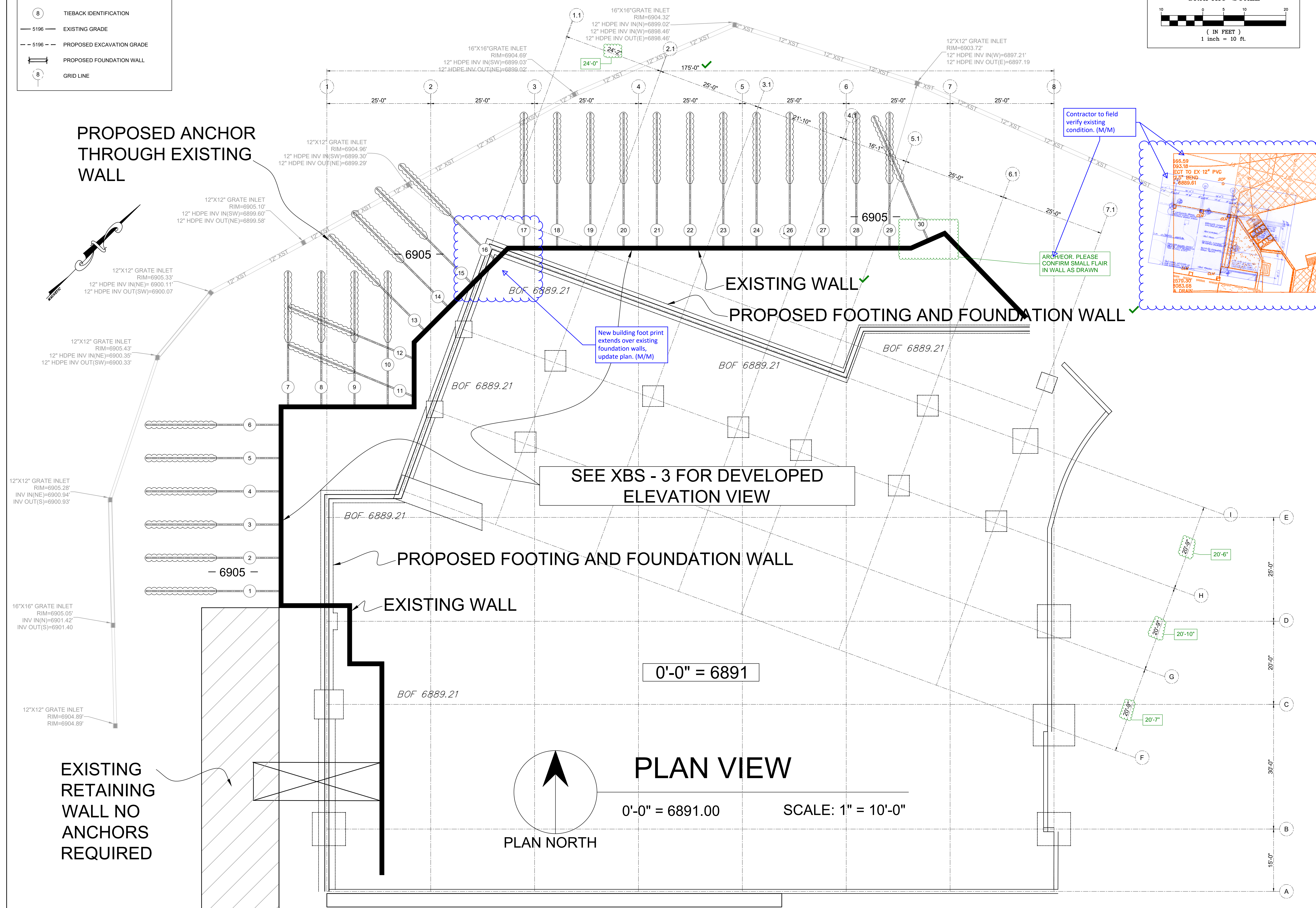
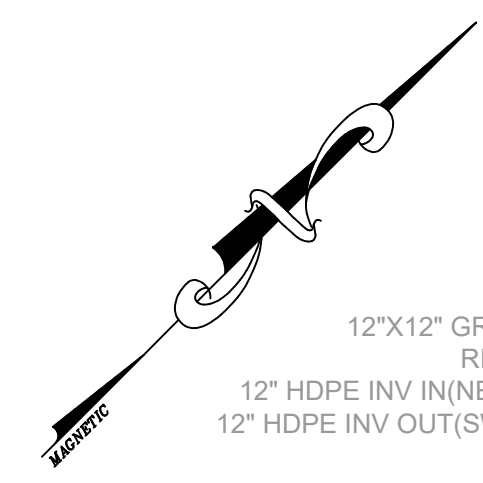
LEGEND

- ⑧ TIEBACK IDENTIFICATION
- 5196 — EXISTING GRADE
- - 5196 - - PROPOSED EXCAVATION GRADE
- ▭ PROPOSED FOUNDATION WALL
- ⑧ GRID LINE



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PROPOSED ANCHOR THROUGH EXISTING WALL



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 DWN BY: JWV
 DWG. NO.
XBS-2
 SHT. 2 OF 3

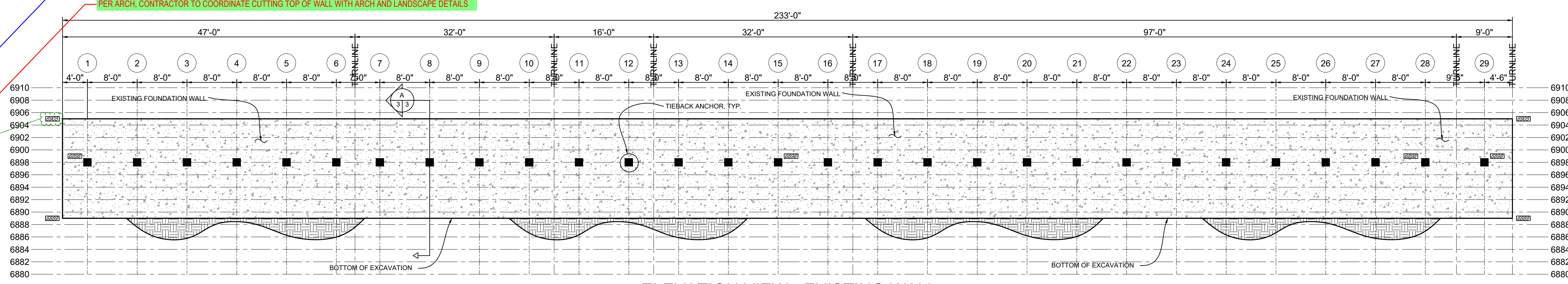


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Top of structural slab at new promenade building = 6904'-8 1/4". Contractor to coordinate with arch and landscape such that remained existing walls are not in conflict with new design. (M/M)

PER ARCH, CONTRACTOR TO COORDINATE CUTTING TOP OF WALL WITH ARCH AND LANDSCAPE DETAILS

ARCH/EOR, PLEASE CONFIRM TOP OF EXISTING WALL REMAINS UNCHANGED



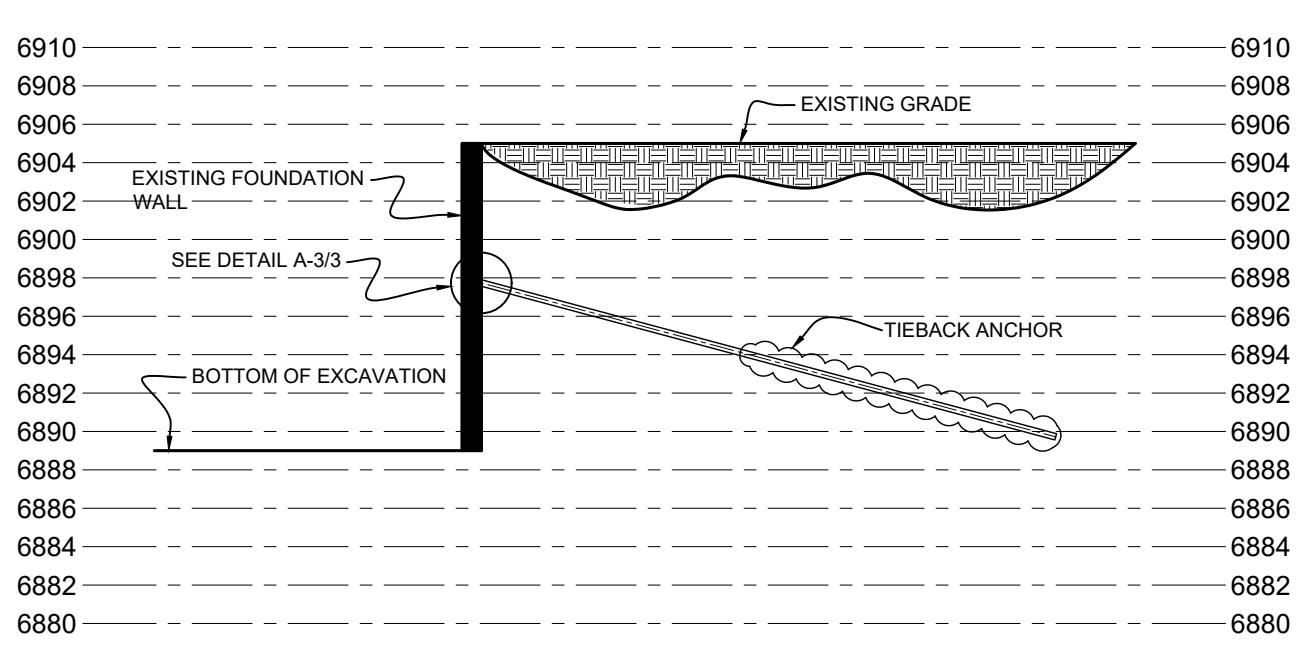
ELEVATION VIEW - EXISTING WALL
NOT TO SCALE

TIEBACK SCHEDULE									
SOLDIER BEAM NO.	ROW #	BAR DIA. (IN.)	TAIL LENGTH (FT.)	UNBONDED LENGTH (FT.)	BONDED LENGTH (FT.)	TOTAL LENGTH (FT.)	LOCKOFF FORCE (KIPS)	DIP ANGLE	HEAD DETAIL TYPE
1 - 29	TOP	1.000	1.00	12.00	20.00	33.00	40.0	15.0	PLATE

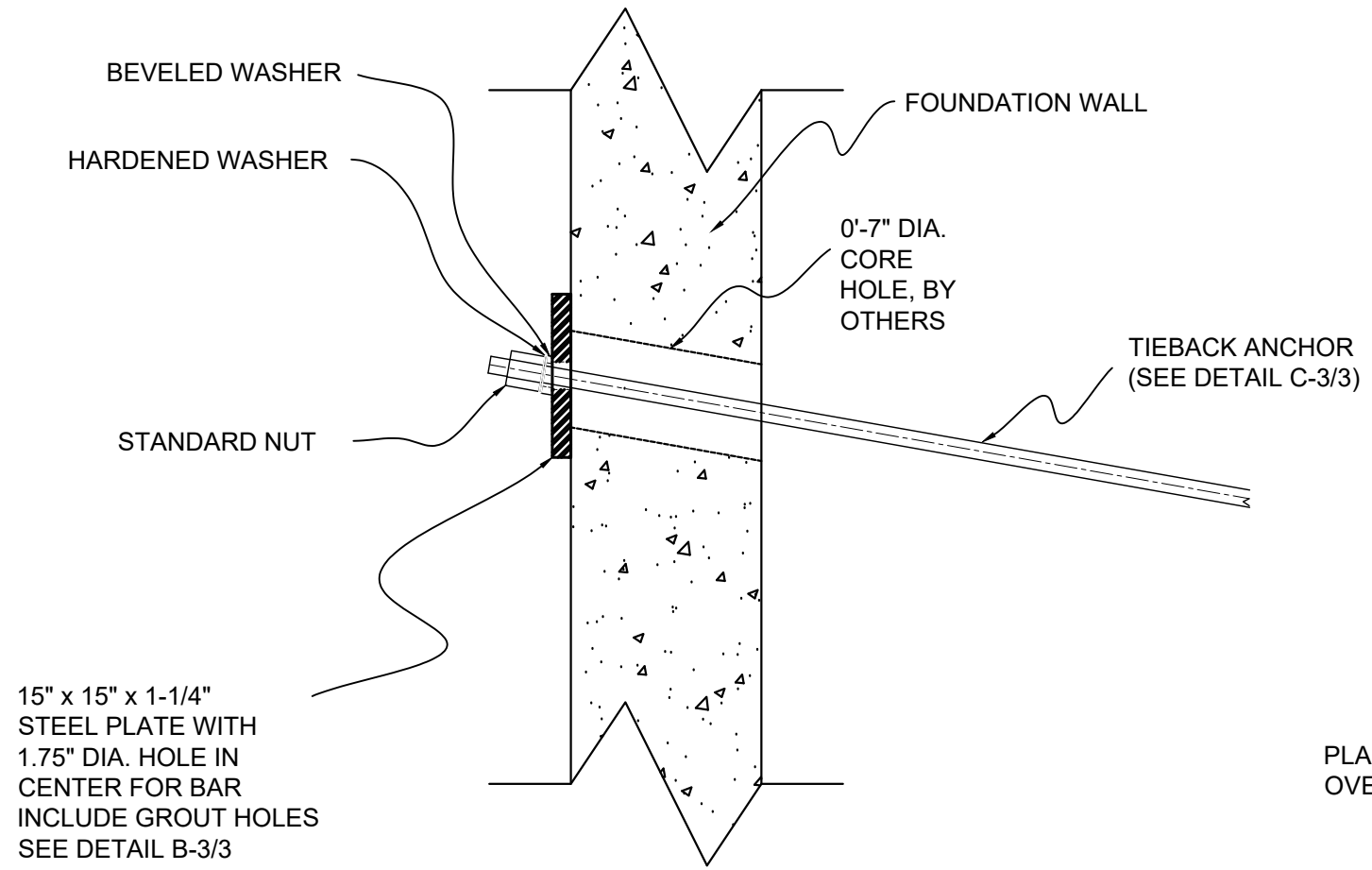
NOTE:
 * SEE ELEVATION VIEW FOR TIEBACK ELEVATION
 * ALL BARS GRADE 150

CONSTRUCTION SEQUENCE

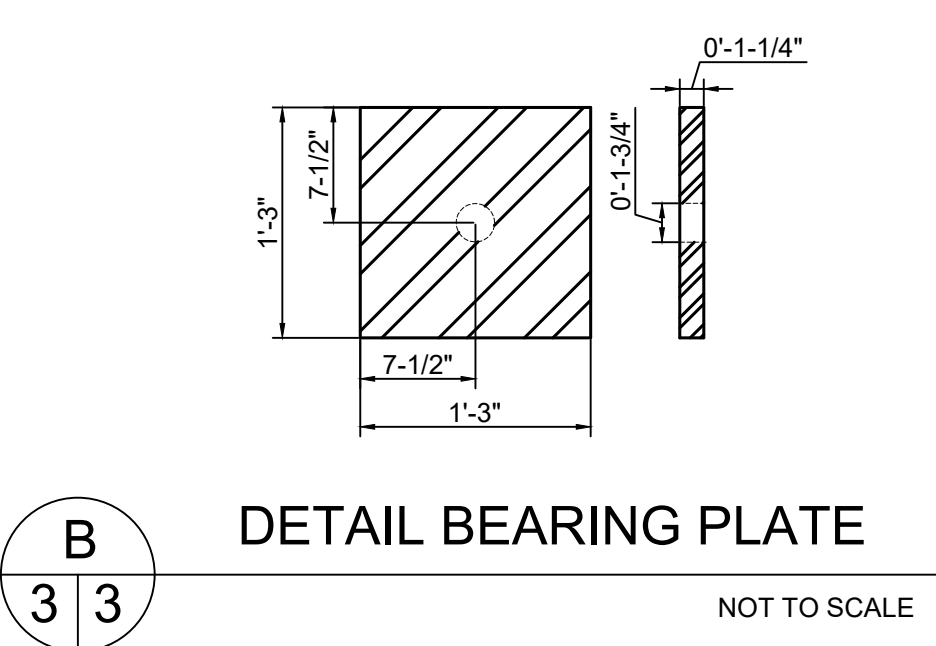
- 1) OTHERS TO DECONSTRUCT EXISTING STRUCTURE
- 2) OTHERS TO CONSTRUCT OR LEAVE BERM IN BOTTOM OF EXCAVATION SO COGGINS DRILL RIG CAN ACCESS ANCHOR HEAD LOCATIONS
- 3) OTHERS TO CORE 7 INCH DIAMETER HOLE THROUGH WALL
- 4) COGGINS TO DRILL AND INSTALL ANCHORS THROUGH EXISTING WALL.
- 5) COGGINS TO STRESS ANCHORS TO EXISTING WALL
- 6) OTHERS TO REMOVE BERM AND / OR EXCAVATE TO BOTTOM OF EXCAVATION.



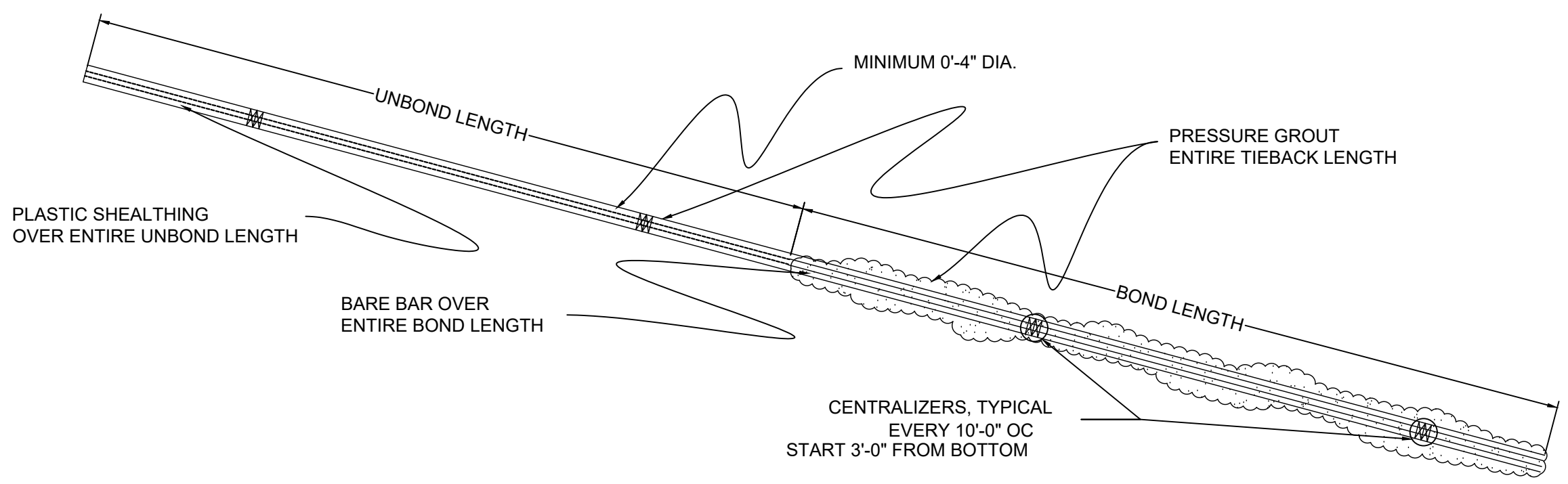
CROSS SECTION ANCHOR #8
NOT TO SCALE



A
3/3
DETAIL ANCHOR HEAD
NOT TO SCALE



B
3/3
DETAIL BEARING PLATE
NOT TO SCALE



C
3/3
DETAIL TIEBACK ANCHOR
NOT TO SCALE

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