

CAMPBELL RESIDENCE LOT #5 - EAGLES VISTA STEAMBOAT SPRINGS, CO. #1907 VE REVISIONS 10.14.2019



ARCHITECTURE PLANNING LANDSCAPE INTERIORS



CAMPBELL RESIDENCE LOT #5 - EAGLES VISTA STEAMBOAT SPRINGS, CC #1907





PROPOSED PLANT LIST								
No.	Sym.	Common Name/ Biological Name	Planting Size/ Remarks	Mature Size				
Decidu	ous Trees:							
	ASP	Aspen/ Populus tremuloides	10' Tall/ Clump/ B&B	50' Ht. & 40' Spd.				
	SSC	Spring Snow Crab	2" Caliper/ B&B	15' Ht. & 15' Spd.				
Decidu	ous Shrubs:							
	GCU	Golden Currant/ Ribes aureum	18"-24" Spread/ #5	4' Ht. & 4' Spd.				
	CHC	Native Chokecherry/ Prunus virginiana	18"-24" Spread/ #5	5' Ht. & 5' Spd.				
	SRB	Serviceberry/ Amelanchier alnifolia	18"-24" Spread/ #5	6' Ht. & 6' Spd.				
Spruce	e Trees:							
	CBS	Colorado Blue Spruce/ Picea pungens	8' Tall/ B&B	50' Ht. 25' Spd.				

*LANDSCAPE CONTRACTOR TO SUBMIT FINAL PLANT LIST AND PERENNIAL SELECTIONS FOR OWNER APPROVAL.

LANDSCAPE AND IRRIGATION NOTES

1. PLANTING BEDS ARE TO HAVE 3" OF WESTERN RED CEDAR MULCH OR LANDSCAPE ROCK OVER

- LANDSCAPE FABRIC. 2. AN UNDERGROUND, PRESSURIZED IRRIGATION SYSTEM WILL BE PROVIDED. ALL PLANTING BEDS
- ARE TO BE IRRIGATED WITH AN AUTOMATIC DRIP SYSTEM AND ALL TURF AND NATIVE SEEDED AREAS ARE TO BE IRRIGATED WITH A POP-UP SPRAY SYSTEM.
- 3. CUT STONE EDGING IS TO BE INSTALLED ALONG THE EDGE OF THE PLANTING BEDS.
- 4. CONTRACTOR WILL MAKE EVERY EFFORT TO MINIMIZE DISRUPTION TO THE EXISTING VEGETATION OUTSIDE THE IMMEDIATE CONSTRUCTION AREA.
- 5. LOCATE ALL UTILITIES PRIOR TO CONSTRUCTION. ALL DISTURBED AREAS ARE TO BE RE-VEGETATED.



<u>}</u>	BOULDER WALLS ORNAMENTAL GRASSES
}	- CRAB TREE
	-LANDSCAPE ROCK & MULCH PLANTING BEDS (ROCK_APPROVED BY OWNER)
$\overline{)}$	-PERENNIALS
	- ASPEN TREE · CUT STONE EDGER – IRRIGATED TURF
	- DECIDUOUS SHRUB - DRIP IRRIGATED - LANDSCAPE ROCK & MULCH PLANTING BEDS (ROCK APPROVED BY OWNER) - LANDSCAPE BOULDERS
\mathcal{A}	- SPRUCE TREE



20'











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AMPBEL

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LOT #5 -STEAMBO/

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(4) PERSPECTIVE #4



3 PERSPECTIVE #3





2 PERSPECTIVE #2



ARCHITECTURE PLANNING LANDSCAPE INTERIORS



AGLES VISTA SPRINGS, CC 1907 RESIDENCE #1 CAMPBEL AMBO. **9**# C Ш Н . ()

ISSUE NAME DATE 50% DD MNOR ADJUSTMENT BUILDING PERMIT VE REVISIONS 08.23.201 DRAWING TITLE PERSPECTIVES SHEET NO. A3.1







SIDENCE V ≥ S S LES VI PRING 7 \geq ШК .06 # AMPBEI C #5 MB С \bigcirc

ISSUE NAME	DATE
50% DD	07.03.2019
	07.12.2019
VE REVISIONS	10.14.2019
DRAWING TITL	E
BUILDIN ELEVATIO	G NS
SHEET NO.	
A3.	2

SNOW FENCING - VERIFY LAYOUT AND INSTALL AS PER MFR.

TPO WATERPROOFING
 MEMBRANE ROOFING WITH
 CONTOUR RIB PROFILE - 18"
 EXPOSURE (CARLISLE ROOFING)

PAINTED COMPOSITE
 SIDIND - DIAMOND
 KOTE LP SMARTSIDE
 (6" EXPOSURE)

RECLAIMED VERTICAL WOOD SIDING (6" EXPOSURE)

DASHED LINES INDICATE



 $1 \frac{\text{BUILDING ELEVATION - EAST}}{1/4" = 1'-0"}$



- FIREPLACE FLUE TERMINATION - INSTALL AND SEAL AS PER MFR.

- SLOPED STEEL CHIMNEY CAP

PLATE STEEL SIDING PANELS ON TREATED WOOD FURRING STRIPS

6x12 TIMBER RAFTERS (REF. STRUCT.)

BUILT-UPWOOD FASCIA METAL FASCIA TRIM BUILT-UPWOOD FASCIA STEEL BEAM (REF. STRUCT.)

SLOPED STONE CAP

METAL CLAD WOOD
 FOLDING DOOR SYSTEM -INSTALL AS PER MFR.

- 6x12 TIMBER COLUMNS (REF. STRUCT.)

— +36" STEEL / WOOD RAILING

- METAL DOWNSPOUT (REF. SITE / LANDSCAPE FLAN FOR TERMINATION)

6" NOM. STONE VENEER WITH WALLS TIES AND ANCHORS AS REQUIRED

VERTICAL METAL PANEL
 SIDING WITH 18"
 EXPOSURE (BRIDGER
 STEEL ULTRA BATTEN)

- CONCRETE FOUNDATION WALL (REF. STRUCT.)

CONCRETE PIER (REF. STRUCT.)





ARCHITECTURE PLANNING LANDSCAPE INTERIORS



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ISSUE NAME PRICING SET REVISED PRICING SET 50% DD MNOR ADJUSTMENT BUILDING PERMIT VE REVISIONS	DATE 05212019 06.142019 07.032019 07.122019 08.232019 10.142019	
DRAWING TITL BUILDIN ELEVATIO	E G NS	
SHEET NO.	3	019 7:36:41 AM

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+36" STEEL / WOOD RAILING





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BUILDING ELEVATIONS

A3.4

05.21.2019

07.12.2019 08.23.2019 10.14.2019

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T.O. PLYWOOD MAIN LEVEL

T.O. PLYWOOD LOWER LEVEL 87' - 10 1/4"

SIDENCE <u>≺</u> ט S S \geq **N**N S Ш SPF 907 Ц Ш С С #1 Ш AMPBEL DT #5 -AMBO O Щ S S

ARCHITECTURE PLANNING LANDSCAPE INTERIORS









SNOW FENCING - VERIFY LAYOUT AND INSTALL AS PER MFR.

TPO WATERPROOFING MEMBRANE ROOFING WITH CONTOUR RIB PROFILE - 18" EXPOSURE (CARLISLE ROOFING)

BUILT-UP WOOD FASCIA

- METAL FASCIA

TRIM

METAL DOWNSPOUT (REF. SITE / LANDSCAPE FLAN FOR TERMINATION)

STEEL BEAM

(REF. STRUCT.)

- 6x12 TIMBER COLUMNS (REF. STRUCT.)

VERTICAL METAL PANEL
 SIDING WITH 18"
 EXPOSURE (BRIDGER
 STEEL ULTRA BATTEN)



DRAWING TITLE

BUILDING SECTIONS

A4.1

SHEET NO.

BUILDING SECTIONS

SHEET NO.

A4.2

CAMPBELL RESIDENCE LOT #5 - EAGLES VISTA STEAMBOAT SPRINGS, C #1907

 $1 \frac{\text{BUILDING SECTION}}{1/4" = 1'-0"}$

CAMPBELI Ś

 $1 \frac{\text{BUILDING SECTION}}{1/4" = 1'-0"}$

<u>T.O. PLYWOOD LOWER LEVEL</u> 87' - 10 1/4"

<u>T.O.</u> PLYWOOD MAIN LEVEL 100' - 0"

ISSUE NAME	DATE
	08.23.2019
VE REVISIONS	10.14.2019
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DRAWING TITL	E
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 $\bigcirc 1 \\ \hline \frac{\text{BUILDING SECTION}}{1/4" = 1'-0"}$

			ROOF, WALL & FLOOR ASSEMBLY LEGEND	
			R1 ROOF ASSEMBLY - 1 • FLEECEBACK TPO WATERPROOFING MEMBRANE ROOFING (CARLISLE ROOFING) a. PROVIDECONTOUR RIB PROFILE - 18" EXPOSURE • ICE AND WATERSHIEID • ROOF FRAMING (REF. STRUCT.) • INSULATION	
	*		a. 4" CLOSED CELL SPRAY FOAM INSULATION b. FILL REMAINING FRAMING CAVITIES WITH BLOWN IN BATT INSULATION (BIBBS) c. OVERALL INSULATION VALUE: R-49 MIN. AS PER 2015 I.E.C.C.	
3	VARIES		R2 ROOF ASSEMBLY - 2 • TPO WATERPROOFING MEBRANE ROOFING • TAPERED RIGID INSULATION • PLWOOD SHEATHING (REF. STRUCT.) • INSULATION a. 4" CLOSED CELL SPRAY FOAM INSULATION	VERTICAL A R T S
1.1.2			 b. FILL REMAINING FRAMING CAVITIES WITH BLOWN IN BATT INSULATION (BIBBS) c. OVERALL INSULATION VALUE: R-49 MIN. AS PER 2015 I.E.C.C. 	ARCHITECTURE
1'-2"	\	T.O. STEEL 109' - 10 1/2"	R3 ROOF ASSEMBLY - 3 • TPO WATERPROOFING MEBRANE ROOFING • TAPERED RIGID INSULATION • PLWOOD SHEATHING (REF. STRUCT.) • ROOF FRAMING (REF. STRUCT.) • FINISHED SOFFIT	LANDSCAPE INTERIORS Copyright VERTICAL ARTS, INC. All Rights Reserved
9' - 10 1/2"	11' - 0 1/2"		 W1 EXTERIOR WALL ASSEMBLY - 1 EXTERIOR FINISH MATERIAL a. REFER ELEVATIONS FOR MATERIAL TYPES AND LOCATIONS INSULATED SHEATHING SYSTEM (1 1/2" THICK) a. INTEGRAL WEATHER BARRIER b. R-5 MIN. AS PER I.E.C.C. WOOD FRAMING (REF. STRUCT.) a. FIREBLOCKING AS REQUIRED b. 2x4 WOOD FURRING AS APPLICABLE AT LOWER LEVEL BLOW-IN BLANKET INSULATION (BIBS) a. R-21 MIN. AS PER I.E.C.C. VAPOR BARRIER INTERIOR FINISH 	incorporated herein, as an instrument of professional service, is the property of Vertic Arts Inc. and is not to be used, in whole or in p for any other project without the written authorization from a principal of Vertical Arts, I
		T.O. PLYWOOD MAIN LEVEL	 W2 EXTERIOR WALL ASSEMBLY - 2 SPARAY APPLIED WATERPROOFING	ISTA ISTA S, CO.
	12' - 1 3/4"		 CONCRETE FOUNDATION WALL (REF. STRUCT.) RIGID INSULATION a. R-15 (AT CRAWL SPACE LOCATIONS W4 EXTERIOR WALL ASSEMBLY - 4 1 1/2" INSULATED DRAINAGE / PROTECTION BOARD a. 10'-0" DEPTH MIN. OR TO LOWER LEVEL FLOOR b. R-5 MIN. AS PER I.E.C.C. SPRAY APPLIED WATERPROOFING CONCRETE FOUNDATION WALL (REF. STRUCT.) WOOD FRAMING (REF. STRUCT.) a. FIREBLOCKING AS REQUIRED BATT INSULATION a. R-13 MIN. AS PER I.E.C.C. b. INTEGRAL VAPOR BARRIER AT INTERIOR SIDE 5/8" GYP BOARD (AS APPLICABLE) 	MPBELL RESIDE T #5 - EAGLES V AMBOAT SPRING #1907
			 F1 FLOOR ASSEMBLY - 1 PLYWOOD SHEATHING (REF. STRUCT.) WOOD FLOOR FRAMING (REF. STRUCT.) UNDER FLOOR STAPLE UP RADIANT HEAT SYSTEM a. ALUMINUM REFLECTIVE BARRIER BATT INSULATION (R-19) FINISHED CEILING 	CA STEP
	_	T.O. PLYWOOD LOWER LEVEL 87' - 10 1/4"	 F2 FLOOR ASSEMBLY - 2 PLYWOOD SHEATHING (REF. STRUCT.) WOOD FLOOR FRAMING (REF. STRUCT.) UNDER FLOOR STAPLE UP RADIANT HEAT SYSTEM a. ALUMINUM REFLECTIVE BARRIER BATT INSULATION a. R-19 MIN. AS PER I.E.C.C. 	ISSUE NAME DATE BUILDING PERMIT 08.23.2019 PERMIT RE-SUBMITTAL 09.25.2019 VE REVISIONS 10.14.2019
			 F3 FLOOR ASSEMBLY - 3 4" CONCRETE SLAB DRAINAGE BOARD EPDM WATRPROOFING MEMBRANE 3/4" PLYWOOD SHEATHING TAPERED WOOD SLEEPERS a. 1/4": 12" SLOPE MIN. PLYWOOD SHEATHING (REF. STRUCT.) WOOD FLOOR FRAMING (REF. STRUCT.) UNDER FLOOR STAPLE UP RADIANT HEAT SYSTEM 	DRAWING TITLE
			 a. ALUMINUM REFLECTIVE BARRIER BATT INSULATION (R-19) 5/8" GYP. BOARD (AS APPLICABLE) 	WALL SECTIONS
			 F4 FLOOR ASSEMBLY - 4 PRE-FABRICATED COMPOSITE DECKING WOOD FRAMING (REF. STRUCT.) 	SHEET NO.
			 F5 FLOOR ASSEMBLY - 5 CLASS I VAPOR RETARDER a. OVERLAP ALL JOINTS 6" MIN., SEAL AND TAPE b. EXTEND EDGES 6" MIN. UP STEM WALL AND ATTACH RIGID INSULATION a. R-15 MIN. AS PER I.E.C.C. b. EXTEND HORIZONTALLY 2'-0" AT GRADE 	A5.1

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•	CLASS I VAPOR RETARDER
	a. OVERLAP ALL JOINTS 6" MIN., SEAL AND TAPE
	b. EXTEND EDGES 6" MIN. UP STEM WALL AND
	ATTACH
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RING S Ш SP 907 #1 Ш LOT #5 -TEAMBO/ S ISSUE NAME DATE

 BUILDING PERMIT
 08.23.2019

 PERMIT RE-SUBMITTAL
 09.25.2019

 VE REVISIONS
 10.14.2019
 DRAWING TITLE DETAILS SHEET NO. A9.4

ISSUE NAME DATE JILDING PERMIT 08.23.201 Æ REVISIONS 10.14.20 **DRAWING TITLE** DETAILS SHEET NO. A9.5

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Ш О SIDEN S S ES VI RING SPF 907 Ш Ш #1 AT CAMPBELI Ш LOT #5 -STEAMB0/

3" = 1'-0"

5 JAMB DETAIL 3" = 1'-0"

C	DULE								
NOMINAL		DETAILS							
	HEIGHT	TYPE	HEAD	JAMB	SILL	REMARKS			
	•		•	•					
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6	PROVIDE WEATHERSTRIPPING AND DOOR SWEEP			
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6	PROVIDE WEATHERSTRIPPING AND DOOR SWEEP			
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	Т	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	A	6/A10.5	-	5/A10.5	PROVIDE WEATHERSTRIPPING - MULLED WITH ADJACENT WINDOW			
	9' - 0"	E	9/A10.5	8/A10.5	7/A10.5	PROVIDE WEATHERSTRIPPING			
	9' - 0"	E	9/A10.5	8/A10.5	7/A10.5	PROVIDE WEATHERSTRIPPING			
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	6/A9.6	20 MINUTE RATED DOOR WITH CLOSER. PROVIDE WEATHERSTRIPPING AND DOOR SWEEP			
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	6/A10.6	5/A10.6	4/A10.6				
	8' - 0"	С	12/A10.4	8/A10.6	7/A10.6	PROVIDE WEATHERSTRIPPING AND DOOR SWEEP			
	6' - 2"	В	3/A10.6	2/A10.6	1/A10.6				

ARCHITECTURE PLANNING LANDSCAPE INTERIORS

 \bigcirc SIDEN ဟ RIN Ц Ш SPI 907 #1 AT AMPBELI Ш LOT #5 -TEAMBO/ Ú Ś

STRUCTURAL GENERAL NOTES

1. DESIGN LOADS: 2015 INTERNATIONAL BUILDING CODE WITH ROUTT COUNTY AMENDMENTS, ASCE 7-10 2. RISK CATEGORY: II STANDARD

- 3. SITE LOCATION:
- A. ELEVATION: 7070.0'
- 4. ROOFS:
- A. ROOF DEAD LOAD 20 PSF B. ROOF LIVE LOAD 20 PSF, 300 LBS
- C. GROUND SNOW LOAD, Pg 115 PSF (PER ROUTT COUNTY REGIONAL BLDG DEPT)
- D. FLAT-ROOF SNOW LOAD, Pf 90 PSF (FOR DESIGN) E. SNOW EXPOSURE FACTOR, Ce 1.0
- F. SNOW IMPORTANCE FACTOR, Is 1.0
- G. THERMAL FACTOR, Ct 1.1 5. FLOOR LOADS:

OCCUPANCY OR USE	UNIFORMLY DISTRIBUTED (PSF)	CONCENTRATED LOAD (LBS)	LIVE LOAD REDUCTION
RESIDENTIAL LIVE LOAD	40	N/A	YES
BALCONIES & DECKS (COVERED) LIVE LOAD	1.5 TIMES LL FOR THE OCCUPANCY SERVED (100 MAX)	N/A	NO
BALCONIES & DECKS (UNCOVERED) LIVE LOAD	90	N/A	NO
RESIDENTIAL DEAD LOAD	20 (NO GYPCRETE)	N/A	NO
RESIDENTIAL DECK DEAD LOAD	15	N/A	NO
RESIDENTIAL GARAGE LIVE LOAD	40	3000	NO
RESIDENTIAL GARAGE DEAD LOAD	65	N/A	NO

6. WIND:

- A. ULTIMATE DESIGN WIND SPEED, VULT, (3-SECOND GUST) 115 MPH
- B. NOMINAL DESIGN WIND SPEED, VASD, (3-SECOND GUST) 90 MPH C. INTERNAL PRESSURE COEFFICIENT 0.18 (ENCLOSED)
- D. WIND EXPOSURE
- E. AIR DENSITY COEFFICIENT
- F. COMPONENTS AND CLADDING ULTIMATE DESIGN WIND PRESSURES 1. WALLS:
- a. WITHIN 12 FEET OF CORNERS +23 PSF -31 PSF
- b. AWAY FROM CORNERS +23 PSF -25 PSF 2. ROOFS:
- a. WITHIN 12 FEET OF CORNERS +16 PSF -43 PSF b. WITHIN 12 FEET OF EDGES +16 PSF -38 PSF c. AWAY FROM EDGES +16 PSF -28 PSF
- 3. OVERHANGS:
- a. WITHIN 6 FEET OF CORNERS +16 PSF -36 PSF b. AWAY FROM CORNERS +16 PSE -23 PSE
- 4. PRESSURES MAY BE REDUCED FOR EFFECTIVE WIND AREAS LARGER THAN 10 SQUARE FEET, BUT NOT BELOW 16 PSF
- 7. SEISMIC:
- A. SPECTRAL RESPONSE ACCELERATION PARAMETERS
- 1. SHORT PERIOD 0.27<u>ợ</u> a. S_S
- b. S_{DS} 0.285g
- 2. ONE SECOND a. S₁ 0.074g
- b. S_{D1} 0.119g
- B. SOILS SITE CLASS C. SEISMIC IMPORTANCE FACTOR
- 1.0 D. SEISMIC DESIGN CATEGORY B
- E. BASIC SEISMIC-FORCE-RESISTING SYSTEM(S)
- PER IBC SECTION 1613.1 EXCPETION 1- SEISMIC DESIGN NOT REQUIRED F. ANALYSIS PROCEDURE EQUIVALENT LATERAL FORCE

FOUNDATION DESIGN:

1. REFER TO SOILS REPORT NO. 17-10640 BY NORTHEST COLORADO COSULTNACTS (NWCC), DATED SEPTEMBER 5,2017. 2. GEOTECHNICAL ENGINEER SHALL VERIFY SOIL CONDITIONS AND TYPES DURING EXCAVATION AND PRIOR TO

- PLACEMENT OF FORMWORK OR CONCRETE
- 3. MINIMUM FROST DEPTH SHALL BE 4'-0" BELOW EXTERIOR GRADE

1. DESIGN OF FOOTINGS IS BASED ON

- A. MAXIMUM ALLOWABLE BEARING PRESSURE 3,000 PSF
- B. MINIMUM DEAD LOAD PRESSURE 700 PSF. 2. BEAR ON THE NATURAL UNDISTURBED SOIL OR COMPACTED STRUCTURAL FILL. EXTERIOR FOOTINGS SHALL BEAR BELOW FROST DEPT
- EARTH RETAINING STRUCTURES

1. EARTH EQUIVALENT FLUID LATERAL PRESSURE: A. WALLS RESTRAINED AT TOP (AT REST) 55 PCF - ON-SITE SOILS

- B. WALLS RESTRAINED AT TOP (AT REST) 45 PCF IMPORTED FREE DRAINING MATERAIL
- C. CANTILEVERED WALLS (ACTIVE) 45 PCF ON-SITE SOILS
- D. CANTILEVERED WALLS (ACTIVE) 35 PCF IMPORTED FREE DRAINING MATERAIL E. PASSIVE RESISTING 250 PCF (ASSUMED)
- 2. COEFFICIENT OF SLIDING FRICTION 0.4

REINFORCED CONCRETE:

1. DESIGN IS BASED ON ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE." 2. CONCRETE WORK SHALL CONFORM TO ACI 301 "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE." 3. STRUCTURAL CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES.

INTENDED USE	EXPOSURE CLASS	fc, PSI 28 DAYS	MAX W/CM RATIO	MAXIMUM AGGREGATE	SLUMP, INCHES (+/- 1")	AIR CONTENT PERCENT (+/- 1.5%)	CEMENT TYPE	ADMIXTURES / COMMENTS
FOOTINGS	F0-S0-W0-C1	3000	0.52	3/4" STONE	5	2%	I/II	
STEM WALLS	F2-S0-W0-C1	4500	0.45	3/4" STONE	4	6%	I/II	
WALLS	F0-S0-W0-C0	4000	0.45	3/4" STONE	4	3%	I/II	
INTERIOR SLAB - GARAGE	F0-S0-W0-C0	4000	0.45	3/4" STONE	4	3%	I/II	
EXTERIOR SLAB ON GRADE	F3-S0-W0-C2	5000	0.40	3/4" STONE	4	6%	1/11	25% MAX FLY ASH

4. DETAILING, FABRICATION, AND PLACEMENT OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 315 "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT." 5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

- 6. REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60, EXCEPT TIES OR BARS SHOWN TO BE FIELD-BENT, WHICH SHALL BE GRADE 40.
- 7. EPOXY COATED REINFORCING BARS SHALL CONFORM TO ASTM A775 (STRAIGHT BARS) AND ASTM A934 (PRE-

FABRICATED BARS). 8. ZINC COATED (GALVANIZED) REINFORCING BARS SHALL CONFORM TO ASTM A767.

9. BARS TO BE WELDED SHALL CONFORM TO ASTM A706.

10. UNLESS NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS, LAP BARS 50 DIAMETERS (MINIMUM). 11. AT CORNERS AND INTERSECTIONS, MAKE HORIZONTAL BARS CONTINUOUS OR PROVIDE MATCHING CORNER BARS FOR EACH LAYER OF REINFORCEMENT.

12. TRIM OPENINGS IN WALLS AND SLABS WITH (2) #5 FOR EACH LAYER OF REINFORCEMENT, FULLY DEVELOPED BY EXTENSION OR HOOK. 13. IN CONTINUOUS MEMBERS, SPLICE TOP BARS AT MID-SPAN AND SPLICE BOTTOM BARS OVER SUPPORTS.

14. FORM INTERMITTENT SHEAR KEYS AT ALL CONSTRUCTION JOINTS AND AS SHOWN ON THE STRUCTURAL DRAWINGS. 15. EXCEPT AS NOTED ON THE DRAWINGS, CONCRETE PROTECTION FOR REINFORCEMENT IN CAST-IN-PLACE CONCRETE SHALL BE AS FOLLOWS:

- A. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
- 1. EXPOSED TO EARTH OR WEATHER: a. #6 THROUGH #18 BARS 2"
- b. #5 BAR, W31 OR D31 WIRE, AND SMALLER 1-1/2"

B. NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: 1. SLABS, WALLS, JOISTS: #11 BARS AND SMALLER 3/4"

- 2. BEAMS AND COLUMNS:
- a. PRIMARY REINFORCEMENT 1-1/2 b. STIRRUPS, TIES, SPIRALS 1-1/2

16. ANCHOR BOLTS AND RODS FOR BEAM AND COLUMN-BEARING PLATES SHALL BE PLACED WITH SETTING TEMPLATES.

POST-INSTALLED ANCHORS

- 1. ALL CAST IN PLACE ANCHORS DESIGNED IN ACCORDANCE WITH ACI 318. 2. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONSTRUCTION DOCUMENTS. THE CONTRACTOR SHALL OBTAIN APPROVAL FROM THE ENGINEER-OF-RECORD PRIOR TO INSTALLING POST-INSTALLED ANCHORS IN PLACE OF MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. 3. CARE SHALL BE TAKEN IN PLACING POST-INSTALLED ANCHORS TO AVOID CONFLICTS WITH EXISTING REBAR. EXISTING REINFORCING BARS SHALL NOT BE CUT UNLESS APPROVED BY THE EOR.
- INFORMATION (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT DEPTH AS INDICATED ON THE DRAWINGS. HOLES SHALL BE DRILLED AND CLEANED IN ACCORDANCE WITH THE MPII. 5. SUBSTITUTION REQUESTS, FOR PRODUCTS OTHER THAN THOSE SPECIFIED, SHALL BE SUBMITTED BY THE
- CONTRACTOR TO THE ENGINEER-OF-RECORD ALONG WITH CALCULATIONS THAT ARE PREPARED & SEALED BY A PERFORMANCE VALUES (MINIMUM) OF THE SPECIFIED PRODUCT USING THE APPROPRIATE DESIGN PROCEDURE
- AND/OR STANDARD(S) AS REQUIRED BY THE AUTHORITY HAVING JURISDICTION. TRAINING FOR ALL PRODUCTS TO BE USED. PRIOR TO THE ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND MADE AVAILABLE TO THE EOR/ SPECIAL INSPECTOR AS REQUESTED TENSION LOADS SHALL BE DONE BY A CERTIFIED ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI 318-
- PRIOR TO COMMENCEMENT OF INSTALLATION. 8. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS (ACI 318-11 D 2.2, ACI 318-14
- 9. ALL POST INSTALLED ANCHORS SHALL BE INSTALLED IN DRY HOLES THAT HAVE BEEN DRILLED, CLEANED, AND PREPARED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION INFORMATION AND THE
- RESPECTIVE ICC-ES EVALUATION REPORTS AND PER THE CURRENT ICC-ES REPORT (IBC 2012/2015 TABLE 1705.3 NOTE B)

	CONCRETE
ANCHOR TYPE	DEWALT
EXPANSION	POWER-STUD+ SD2 (ICC ESR-250
CONCRETE SCREW	SCREW-BOLT+ (ICC ESR 3889)
ADHESIVE	AC200+ (ICC ESR-4027)

STRUCTURAL STEEL:

- STRUCTURAL STEEL BUILDINGS" (AISC 360) AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" (AISC 303) BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC). 2. STRUCTURAL STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A992, 50 KSI YIELD 3. ROLLED STEEL FLOOR PLATES SHALL CONFORM TO ASTM A786, COMMERCIAL GRADE.
- 4. OTHER ROLLED SHAPES, INCLUDING PLATES, CHANNELS, WTS, AND ANGLES SHALL CONFORM TO ASTM A36, 36 KSI
- 6. HSS ROUND SHAPES SHALL CONFORM TO ASTM A500, GRADE C, 46 KSI YIELD.
- 7. PIPE SHAPES SHALL CONFORM TO ASTM A53, GRADE B, 35 KSI YIELD. A325 BOLTS, DETAILED IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND THE "STEEL CONSTRUCTION
- ASTM A325 OR A490 BOI TS" 9. ALL BEAMS SHALL HAVE FULL DEPTH WEB STIFFENERS EACH SIDE OF WEBS ABOVE AND BELOW COLUMNS.
- NOTED ON THE STRUCTURAL DRAWINGS. WITH EQUIPMENT APPROVED BY THE STUD MANUFACTURER ACCORDING TO THE STUD MANUFACTURER'S
- RECOMMENDATIONS.
- WELD E70 ELECTRODES. WHERE NOT SPECIFICALLY NOTED, MINIMUM WELD SHALL BE 3/16" FILLET BY LENGTH OF CONTACT EDGE 13. GROUT BENEATH COLUMN BASE AND BEAM BEARING PLATES SHALL HAVE A MINIMUM 28-DAY, COMPRESSIVE

STRUCTURAL WOOD FRAMING:

- 1. IN-GRADE BASE VALUES HAVE BEEN USED FOR DESIGN. 2. DIMENSIONAL LUMBER FRAMING SHALL BE S4S DOUG FIR NO. 2 AND BETTER UNO.
- 3. SOLID TIMBER BEAMS AND POSTS SHALL BE DOUGLAS FIR-LARCH NO. 1 AND BETTER UNO. 4. STUDS SHALL BE DOUG FIR STUD GRADE AND BETTER UNO.
- 5. TOP AND BOTTOM PLATES SHALL BE DOUGLAS FIR-LARCH NO. 2 AND BETTER UNO.
- ALL LUMBER SHALL BE 19% MAXIMUM MOISTURE CONTENT AT THE TIME OF INSTALLATION UNO.
- DOUGLAS FIR-LARCH OR SOUTHERN YELLOW PINE. PRESERVATIVE-TREATED WOOD SHALL BE TREATED IN FOLLOWING USE CATEGORY:
- A. UC2 AT INTERIOR
- B. UC3B AT EXTERIOR WITH NO GROUND CONTACT C. UC4B AT EXTERIOR WITH GROUND CONTACT 8. FASTENERS FOR USE WITH TREATED WOOD SHALL BE CORROSION RESISTANT IN ACCORDANCE WITH SECTION
- 2304.10.5 IN 2015 IBC.
- 10. ALL IRON AND STEEL PRODUCTS ATTACHED TO TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A123 OR SHALL BE TYPE 304 OR 316 STAINLESS STEEL
- 11. STRUCTURAL MEMBERS SHALL NOT BE CUT FOR PIPES, ETC. UNLESS SPECIFICALLY NOTED OR DETAILED ON THE STRUCTURAL DRAWINGS.
- 12. ALL BOLTS SHALL BE RETIGHTENED PRIOR TO CLOSING IN OF WALLS. FLOORS, AND ROOFS. 13. ALL BOLTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS UNDER HEAD AND/OR NUT, UNO.
- CONNECTORS AND INSTALLED WITH ALL HOLES FILLED (ROUND AND TRIANGULAR) WITH THE MAXIMUM SIZE NAIL RECOMMENDED BY THE MANUFACTURER TO DEVELOP THE MAXIMUM RATED CAPACITY. 15. CONNECTOR BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME B18.2.1 AND ASTM SAE J429 GRADE 1.
- 16. NAILS AND SPIKES SHALL CONFORM TO ASTM F1667. 17. WOOD SCREWS SHALL CONFORM TO ANSI/ASME B18.6.1.
- TO THE SHANK DIAMETER AT THE UNTHREADED SECTION. 19. CONVENTIONAL LIGHT FRAMING SHALL COMPLY WITH IBC SECTION 2308. 20. COLUMNS / MULTIPLE STUDS IN BEARING WALLS SUPPORTING ALL BEAMS AND HEADERS SHALL OCCUR CONTINUOUSLY THROUGH EACH FLOOR LEVEL DOWN TO THE FOUNDATION OR ANOTHER SUPPORT BEAM. SOLID
- SQUASH BLOCKING EQUIVALENT IN AREA TO THE COLUMN/MULTIPLE STUDS ABOVE SHALL BE PROVIDED WITHIN THE JOIST SPACE BENEATH THE COLUMN/MULTIPLE STUDS. 21. ALL BEAMS AND TRUSSES SHALL BE BRACED AGAINST ROTATION AT POINTS OF BEARING.
- 22. 2X BLOCKING SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS, UNO.

SEATS, CAPS, ETC.

SCHEDULE" IN 2015 IBC.

HORIZONTALLY @ 12" PER PLY.

DRAWINGS.

4. ALL ANCHORS MUST BE INSTALLED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PRINTED INSTALLATION

REGISTERED PROFESSIONAL ENGINEER: REGISTRATION MUST BE IN THE STATE IN WHICH THE PROJECT IS LOCATED. THE CALCULATIONS SHALL DEMONSTRATE THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING EQUIVALENT

6. THE CONTRACTOR SHALL ARRANGE FOR A MANUFACTURER'S FIELD REPRESENTATIVE TO PROVIDE INSTALLATION

7. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION THAT SUPPORT SUSTAINED

11 D 9.2.2, ACI 318-14 17.8.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE EOR FOR APPROVAL

10. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL AND ADHESIVE ANCHORS PER THE APPLICABLE BUILDING CODE

T INSTALLED ANCHORS					
HILTI	SIMPSON				
KWIK BOLT TZ (ICC ESR-1917)	STRONG-BOLT 2 (ICC ESR-3037)				
KWIK HUS-EZ (ICC ESR-3027)	TITEN HD (ICC ESR 2713)				
HIT-HY 200 (ICC ESR-3187)	AT-XP (UES ER-263)				

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED, AND ERECTED IN ACCORDANCE WITH THE "SPECIFICATION FOR

5. HOLLOW STRUCTURAL SECTION (HSS) RECTANGULAR SHAPES SHALL CONFORM TO ASTM A500, GRADE C, 50 KSI

8. EXCEPT AS NOTED, FRAMED BEAM CONNECTIONS SHALL BE BEARING-TYPE WITH 3/4" DIAMETER, SNUG TIGHT, ASTM MANUAL" BY THE AISC. INSTALL BOLTS IN ACCORDANCE WITH AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING

10. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE (36, 55 WITH WELDABILITY SUPPLEMENT S1, AND/OR 105) AS

11. HEADED ANCHOR STUDS (HAS) SHALL CONFORM TO ASTM A108 AND SHALL BE CONNECTED TO STRUCTURAL STEEL

12. WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH THE AISC DOCUMENTS LISTED ABOVE, THE AMERICAN WELDING SOCIETY (AWS) D1.1: STRUCTURAL WELDING CODE, AND THE RECOMMENDATIONS FOR USE OF

STRENGTH OF 7,500 PSI AND SHALL BE NON-SHRINK, NON-METALLIC, AND TESTED IN ACCORDANCE WITH ASTM C1107.

ALL WOOD EXPOSED TO WEATHER OR IN CONTACT WITH CONCRETE OR MASONRY SHALL BE PRESSURE TREATED ACCORDANCE WITH AWPA STANDARDS U1 AND M4. TREATMENTS SHALL HAVE NO AMMONIA ADDED AND SHALL BE THE

9. ALL CONNECTORS USED WITH PRESSURE-TREATED MATERIAL SHALL BE STAINLESS STEEL ASTM 304 OR 316, OR HAVE A SIMPSON Z-MAX (G185) OR HDG COATING. STANDARD COATING (G90) IS ACCEPTABLE AT INTERIOR CONDITIONS WITH NON PRESSURE-TREATED LUMBER ONLY. CONNECTORS ARE TO BE IN ACCORDANCE WITH ASTM A653 OR ASTM 123.

14. METAL FRAMING ANCHORS SHOWN OR REQUIRED, SHALL BE SIMPSON STRONG-TIE OR EQUAL CODE APPROVED

18. LEAD HOLES FOR LAG SCREWS SHALL BE 40%-70% OF THE SHANK DIAMETER AT THE THREADED SECTION AND EQUAL

23. CROSS-BRIDGING OR SOLID BLOCKING SHALL BE PROVIDED AT 8'-0" MAX. FOR ALL JOISTS AND RAFTERS MORE THAN

10" IN DEPTH, 2X3 OR APPROVED METAL TYPE BRIDGING MAY BE USED. 24. PROVIDE A MINIMUM OF (3) STUDS AT EACH CORNER, UNO.

25. ALL JOISTS AND BEAMS (EXCLUDING I-JOISTS) SHALL BE SEAT-CUT FOR FULL UNIFORM BEARING AT SUPPORTS,

26. VENTING IS REQUIRED IN ALL ENCLOSED ROOF AND CRAWL SPACE FRAMING CAVITIES, SEE ARCHITECTURAL

27. EXCEPT AS NOTED OTHERWISE, MINIMUM NAILING SHALL BE PROVIDED AS SPECIFIED IN TABLE 2304.10.1 "FASTENING 28. ALL MULTIPLE MEMBER BEAMS SHALL BE NAILED TOGETHER WITH MAX NUMBER OF 10D NAILS VERTICALLY @ 3" AND

29. TONGUE AND GROOVE DECKING SHALL BE INSTALLED IN ACCORDANCE WITH THE "STANDARD FOR TONGUE AND

GROOVE HEAVY TIMBER ROOF DECKING", AITC 112. WHERE DECKING MUST BE NAILED FROM THE BOTTOM SIDE, USE (2) 16D GALVANIZED FINISH NAILS AT EACH SUPPORT, COUNTERSUNK AND FILLED. 30. ALL ROOF RAFTERS, JOISTS, TRUSSES, AND BEAMS SHALL BE ANCHORED TO SUPPORTS WITH H2.5A METAL FRAMING ANCHORS UNO. PROVIDE (2) WITHIN 4'-0" OF ALL CORNERS.

WOOD SHEATHING:

1. PLYWOOD AND ORIENTED STRAND BOARD (OSB) FLOOR AND ROOF SHEATHING SHALL BE APA RATED WITH STAMP INCLUDING APA TRADEMARK AND PANEL SPAN RATING.

A. MINIMUM FLOOR SHEATHING: 23/32" APA STURD-I-FLOOR RATED 24 INCH O.C. TONGUE & GROOVE, GLUED AND NAII FD

B. MINIMUM ROOF SHEATHING: 15/32" OSB OR CDX PLYWOOD, APA 32/16, NAILED. C. MINIMUM WALL SHEATHING: 7/16" OSB OR CDX PLYWOOD, APA 24/16, BLOCKED AND NAILED.

- a. OPTIONAL WALL SHEATHING: ZIP SYSTEM R6 SHEATING OR EQUIVALENT, 7/16" APA LAMINATED TO 1" RIGID INSULATION) NAILED WITH 10d SHANK NAIL (0.131"Ø x3") AT 3" PANEL EDGES AND 12" IN FIELD OF PANEL; BLCOK AND NAIL ALL EDGES BETWEEN STUDS 2. NAIL WALL SHEATHING WITH MINIMUM 8D COMMON OR 10D BOX AT 6" AT PANEL EDGES, AND 12" AT INTERMEDIATE
- FRAMING EXCEPT AS NOTED. BLOCK AND NAIL ALL EDGES BETWEEN STUDS. 3. MINIMUM (3) 8D NAILS PER STUD. NAIL ALL PLATES USING EDGE NAIL SPACING INDICATED.
- 4. SHEATHE ALL EXTERIOR WALLS. SHEATHE INTERIOR WALLS AS DESIGNATED ON THE DRAWINGS.

5. SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO TOP PLATE. CUT IN "L" AND "T" SHAPES AROUND OPENINGS. LAP SHEATHING OVER SINGLE 2X PLATE MEMBER AT RIM JOIST. AT RIM JOIST PROVIDE A MINIMUM OF 3" BETWEEN SHEATHING EDGE AND TOP/BOTTOM EDGE OF RIM.

- 6. MINIMUM HEIGHT OF SHEATHING PANELS SHALL BE 16" TO ENSURE THAT PLATES ARE TIED TO STUDS. 7. ALL SHEATHING SHEETS SHALL HAVE 1/8" GAP AT ALL EDGES AND JOINTS.
- 8. FULLY NAIL FLOOR SHEATHING IMMEDIATELY AFTER GLUING (DO NOT SPOT NAIL)

9. PROVIDE (1) PANEL SHEATHING CLIP AT ALL UNSUPPORTED ROOF SHEATHING PANEL EDGES. WHERE SPANS ARE GREATER THAN 32" PROVIDE (2) CLIPS.

ENGINEERED LUMBER

1. STRUCTURAL CAPACITIES OF STRUCTURAL COMPOSITE LUMBER SHALL BE IN CONFORMANCE WITH SECTION 2303.1.9 (2303.1.10 OF THE 2015 IBC) OF THE IBC. 2. MANUFACTURER OF STRUCTURAL COMPOSITE LUMBER PRODUCTS SHALL HAVE PROPER CODE EVALUATION REPORTS

FOR ALL PRODUCTS AND SHALL BE APPROVED BY THE STRUCTURAL ENGINEER. 3. THE CONTRACTOR SHALL NOT CUT, NOTCH, OR OTHERWISE ALTER STRUCTURAL COMPOSITE LUMBER MEMBERS WITHOUT WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER AND THE MANUFACTURER: HOWEVER. HOLES MAY BE

CUT IN MEMBERS IN ACCORDANCE WITH THE MANUFACTURER'S ALLOWABLE HOLE CHART 4. MEMBERS NOTED AS LVL (LAMINATED VENEER LUMBER) ON PLAN SHALL BE 1-3/4" WIDE X DEPTH INDICATED, PLANT-FABRICATED, AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

- A. $F_{b} = 2600 \text{ PSI}$ B. F_v = 285 PSI
- C. $F_{cPAR} = 2460 PSI$
- D. F_{CPERP} = 750 PSI E. E = 1900 KSI
- 5. MEMBERS NOTED AS PSL (PARALLEL STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:
- A. F_b = 2900 PSI B. F_v = 290 PSI
- C. F_{cPAR} = 2900 PSI
- D. $F_{cPERP} = 750 PSI$ E. E = 2000 KSI
- 6. MEMBERS NOTED AS LSL (LAMINATED STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:
- A. F_b = 1700 PSI
- B. F_v = 400 PSI
- C. F_{cPAR} = 1400 PSI D. $F_{cPERP} = 680 PSI$
- E. E = 1300 KSI 7. BRIDGING AND BLOCKING SHALL BE INSTALLED ACCORDING TO THE FABRICATOR'S REQUIREMENTS.
- 8. WOOD I-JOISTS SHALL HAVE THE DEPTH, SPACING, SPAN, AND LAYOUT SHOWN ON THE DRAWINGS. MEMBERS SHALL BE FACTORY MANUFACTURED WITH ORIENTED STRAND BOARD (OSB) WEBS, LAMINATED VENEER LUMBER (LVL) OR
- MACHINE STRESS RATED (MSR) LUMBER FLANGES PER CODE APPROVAL BY ICB OR NER. STRUCTURAL WOOD FLANGES AND WEBS SHALL BE DESIGNED FOR STRUCTURAL CAPACITIES AND DESIGN PROVISIONS ACCORDING TO ASTM D 5055. SUBSTITUTION OF EQUIVALENT SERIES BY OTHERS SHALL BE SUBMITTED TO THE STRUCUTRAL ENGINEER FOR APPROVAL. 9. JOISTS SHALL BE INSTALLED PER THE MANUFACTURER'S RECOMMENDATIONS. HOLES IN WEBS SHALL NOT EXCEED

MANUFACTURER'S PUBLISHED LIMIT CRITERIA. 10. OPEN WEB TRUSSES SHALL HAVE THE DEPTH, SPACING, SPAN, AND LAYOUT SHOWN ON THE DRAWINGS. MEMBERS SHALL BE FACTORY MANUFACTURED WITH TUBULAR STEEL WEBS, AND LAMINATED VENEER LUMBER (LVL) OR MACHINE

STRESS RATED (MSR) LUMBER CHORDS PER CODE APPROVAL BY ICB OR NER. 11. OPEN WEB JOISTS SHALL BE DESIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED TO CARRY THE LOADS INDICATED ON THE STRUCTURAL DRAWINGS.

12. MEMBER FORCES SHALL BE DETERMINED BY THE FABRICATOR. STRESSES SHALL NOT EXCEED THOSE ALLOWED BY THF IBC. 13. DEFLECTION LIMITS FOR WOOD I-JOISTS AND OPEN WEB JOISTS SHALL NOT EXCEED THE FOLLOWING DEFLECTION CRITERIA:

- A. ROOF LIVE LOAD = L/360
- B. ROOF TOTAL LOAD = L/240 (1" MAXIMUM)
- C. FLOOR LIVE LOAD = L/480 D. FLOOR TOTAL LOAD = L/240 (1" MAXIMUM)

- LIGHT-METAL-PLATE-CONNECTED WOOD TRUSSES: . TRUSS MANUFACTURER SHALL COMPLY WITH ALL REQUIREMENTS AS STATED IN SECTION 2303.4 OF THE IBC. 2. ALL PRE-ENGINEERED GABLE END TRUSSES OR TRUSSES WITH INTEGRATED PARAPETS SHALL BE DESIGNED FOR
- WIND FORCES PERPENDICULAR TO THE TRUSS. 3. ALL PRE-ENGINEERED TRUSSES SHALL BE FABRICATED SUCH THAT THEY INCORPORATE ALL ROOF PLANES. AT CONTRACTOR'S OPTION, STANDARD SHAPE TRUSSES MAY BE USED IN CONJUNCTION WITH OVERFRAMING.
- 4. FULL HEIGHT BLOCKING SHALL BE PLACED BETWEEN TRUSSES AT ALL SUPPORTS
- 5. CROSS BRIDGING DESIGN SHALL BE PROVIDED BY TRUSS MANUFACTURER AS REQUIRED FOR LATERAL EFFECTS. 6. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED, SPLICED OR OTHERWISE ALTERED IN ANY WAY WITHOUT WRITTEN APPROVAL OF A REGISTERED DESIGN PROFESSIONAL 7. MANUFACTURE AND INSTALLATION OF METAL PLATED WOOD TRUSSES SHALL COMPLY WITH ANSI/TPI 1 "NATIONAL
- DESIGN STANDARD FOR METAL-PLATE-CONNECTED WOOD TRUSS CONSTRUCTION," BCSI (BUILDING COMPONENT SAFETY INFORMATION) "GUIDE TO GOOD PRACTICE FOR HANDLING, 8. INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES," AND DSB-89 "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES.'
- 9. PRE-ENGINEERED, PREFABRICATED TRUSSES SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE IN WHICH TO PROJECT IS LOCATED TO CARRY THE LOADS INDICATED ON THE STRUCTURAL DRAWINGS IN WHICH THE PROJECT IS LOCATED.
- 10. TRUSSES SHALL BE DESIGNED TO SUPPORT THE FULL DEAD LOADS AND THE SUPERIMPOSED DESIGN LOADS NOTED ABOVE OR ON THE DRAWINGS. 11. STRESSES SHALL NOT EXCEED THOSE LISTED IN THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
- (AF&PA NDS). <NO INCREASES IN STRESS ARE ALLOWED FOR DURATION OF LOAD.> 12. SCISSOR TYPE TRUSSES SHALL BE DESIGNED FOR A MAXIMUM OF 1/2" TOTAL HORIZONTAL DEFLECTION UNDER DEAD PLUS LIVE LOADS 13. THE FABRICATOR SHALL DETERMINE TRUSS WEB ARRANGEMENTS AND MEMBER FORCES.
- 14. TRUSS TO TRUSS CONNECTIONS SPECIFIED SHALL BE BY TRUSS SUPPLIER, UNLESS SPECIFICALLY NOTED ON THE DRAWINGS
- 15. TRUSSES SHALL BE DESIGNED IN BEARING TO NOT EXCEED THE PERPENDICULAR TO GRAIN BEARING VALUES FOR THE TOP PLATE GRADES INDICATED IN THE "STRUCTURAL WOOD FRAMING" GENERAL NOTES. WHERE TRUSS BEARING EXCEED THIS VALUE THE TRUSS MANUFACTURER SHALL PROVIDE BEARING ENHANCERS TO COMPENSATE FOR OVERSTRESSES. TRUSS MANUFACTURER SHALL SPECIFY SIZE, SPECIES, AND NAILING FOR BEARING BLOCKS.
- 16. TRUSS FABRICATOR SHALL SPECIFY ALL FLOOR AND ROOF TRUSS BRACING AND BRIDGING. 17. CALCULATIONS AND SHOP DRAWINGS, INCLUDING MEMBER SIZES, LUMBER SPECIES AND GRADES, AND SUBSTANTIATING DATA FOR CONNECTOR CAPACITIES, SHALL BE SUBMITTED TO THE ARCHITECT, GC, AND
- STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION. 18. TRUSS DESIGN SHALL INCLUDE A 250 LBS LOAD PER NFPA TO SUPPORT SPRINKLER LOADS LOCATED ANYWHERE
- ALONG THE BOTTOM CHORD OF THE TRUSS. 19. DEFLECTION LIMITS FOR TRUSSES SHALL NOT EXCEED THE FOLLOWING DEFLECTION CRITERIA:
- A. FLOOR LIVE LOAD = L/480 B. FLOOR TOTAL LOAD = L/240 (1" MAXIMUM)

RETURNED WITHOUT REVIEW. A. CONCRETE MIX DESIGNS **B. CONCRETE REINFORCING STEEL** E. STRUCTURAL STEEL

Shop drawings:

ELEMENTS ARE IN PLACE. SUPPORTING STRUCTURAL ELEMENTS.

CONSTRUCTION

STRUCTURAL ENGINEER THE START OF CONSTRUCTION. COMPLIANCE LETTER.

LETTER IS NEEDED. SPECIAL INSPECTIONS - 2012:

FOLLOWING SUB-SECTIONS: 1. 1704.2 SPECIAL INSPECTIONS 1. 1705.1.1 SPECIAL CASES 2. 1705.2 STEEL CONSTRUCTION 4. 1705.5 WOOD CONSTRUCTION 5. 1705.6 SOILS

STRUCTURAL ERECTION AND BRACING REQUIREMENTS:

THE STRUCTURAL DRAWINGS ILLUSTRATE AND DESCRIBE THE COMPLETED STRUCTURE WITH ELEMENTS IN THEIR FINAL POSITIONS, PROPERLY SUPPORTED, CONNECTED, AND/OR BRACED.

2. THE STRUCTURAL DRAWINGS ILLUSTRATE TYPICAL AND REPRESENTATIVE DETAILS TO ASSIST THE GENERAL CONTRACTOR. DETAILS SHOWN APPLY AT ALL SIMILAR CONDITIONS UNLESS OTHERWISE INDICATED. ALTHOUGH DUE DILIGENCE HAS BEEN APPLIED TO MAKE THE DRAWINGS AS COMPLETE AS POSSIBLE, NOT EVERY DETAIL IS ILLUSTRATED AND NOT EVERY EXCEPTIONAL CONDITION IS ADDRESSED.

3. ALL PROPRIETARY CONNECTIONS AND ELEMENTS SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURERS' RECOMMENDATIONS. 4. ALL WORK SHALL BE ACCOMPLISHED IN A WORKMANLIKE MANNER AND IN ACCORDANCE WITH THE APPLICABLE CODES

AND LOCAL ORDINANCES. 5. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ALL WORK, INCLUDING LAYOUT AND DIMENSION VERIFICATION, MATERIALS COORDINATION, SHOP DRAWING REVIEW, AND THE WORK OF SUBCONTRACTORS. ANY DISCREPANCIES OR OMISSIONS DISCOVERED IN THE COURSE OF THE WORK SHALL BE IMMEDIATELY REPORTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR RESOLUTION. 6. CONTINUATION OF WORK WITHOUT NOTIFICATION OF DISCREPANCIES RELIEVES THE ARCHITECT AND STRUCTURAL

ENGINEER FROM ALL CONSEQUENCES. 7. UNLESS OTHERWISE SPECIFICALLY INDICATED, THE STRUCTURAL DRAWINGS DO NOT DESCRIBE METHODS OF

8. THE GENERAL CONTRACTOR, IN THE PROPER SEQUENCE, SHALL PERFORM OR SUPERVISE ALL WORK NECESSARY TO ACHIEVE THE FINAL COMPLETED STRUCTURE, AND TO PROTECT THE STRUCTURE, WORKMEN, AND OTHERS DURING CONSTRUCTION, SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO TEMPORARY BRACING, SHORING FOR CONSTRUCTION EQUIPMENT, SHORING FOR EXCAVATION, FORMWORK, SCAFFOLDING, SAFETY DEVICES AND PROGRAMS OF ALL KINDS, SUPPORT AND BRACING FOR CRANES AND OTHER ERECTION EQUIPMENT. 9. DO NOT BACKFILL AGAINST BASEMENT OR RETAINING WALLS UNTIL SUPPORTING SLABS AND FLOOR FRAMING ARE IN PLACE AND SECURELY ANCHORED, UNLESS ADEQUATE TEMPORARY BRACING IS PROVIDED. 10. TEMPORARY BRACING SHALL REMAIN IN PLACE UNTIL ALL FLOORS, WALLS, ROOFS AND ANY OTHER SUPPORTING

11. THE ARCHITECT AND STRUCTURAL ENGINEER BEAR NO RESPONSIBILITY FOR THE ABOVE ITEMS, AND OBSERVATION VISITS TO THE SITE DO NOT IN ANY WAY INCLUDE INSPECTIONS OF THESE ITEMS.

PRECAUTIONARY NOTES ON STRUCTURAL BEHAVIOR:

1. INTERIOR ARCHITECTURAL FINISH DETAILING MUST ACCOMMODATE THE RELATIVE DIFFERENTIAL MOVEMENTS OF

2. WHERE THE ROOF FRAMING ELEMENT SPANS ARE LONG, APPLIED LOADING WILL NATURALLY CAUSE SUBSTANTIAL DEFLECTION. INTERIOR ELEMENTS HUNG FROM THE ROOF STRUCTURE WILL DEFLECT WITH THE ROOF. 3. EXTERIOR/PERIMETER WALL ASSEMBLIES HUNG FROM THE EDGE OF THE BUILDING STRUCTURE WILL BE DIRECTLY AFFECTED (TO SOME DEGREE) BY CHANGES IN EXTERNAL TEMPERATURE AND FLOOR DEFLECTION. 4. EXTERIOR/PERIMETER AND INTERIOR ARCHITECTURAL FINISH DETAILS SHOULD ALLOW FOR RELATIVE MOVEMENTS BETWEEN ELEMENTS WITH DIFFERENT SUPPORT CONDITIONS.

LETTERS OF CONSTRUCTION COMPLIANCE:

1. THE GENERAL CONTRACTOR SHALL DETERMINE FROM THE LOCAL BUILDING AUTHORITY, AT THE TIME THE BUILDING PERMIT IS OBTAINED, WHETHER ANY LETTERS OF CONSTRUCTION COMPLIANCE WILL BE REQUESTED FROM THE

2. THE CONTRACTOR SHALL NOTIFY THE STRUCTURAL ENGINEER OF ALL SUCH REQUIREMENTS IN WRITING PRIOR TO 3. TWO-DAY ADVANCE NOTICE SHALL BE GIVEN WHEN REQUESTING SITE VISITS NECESSARY AS THE BASIS FOR THE

4. THE GENERAL CONTRACTOR SHALL PROVIDE COPIES OF ALL THIRD-PARTY TESTING AND INSPECTION REPORTS TO THE ARCHITECT AND STRUCTURAL ENGINEER A MINIMUM OF ONE WEEK PRIOR TO THE DATE THAT THE COMPLIANCE

1. THE FOLLOWING SPECIAL INSPECTIONS AND TESTING SHALL BE PERFORMED BY A QUALIFIED SPECIAL INSPECTOR, RETAINED BY THE OWNER, IN ACCORDANCE WITH THE FOLLOWING SECTIONS OF IBC CHAPTER 17: A. SECTION 1704 SPECIAL INSPECTIONS, CONTRACTOR RESPONSIBILITY, AND STRUCTURAL OBSERVATIONS AND THE

2. 1704.3 STATEMENT OF SPECIAL INSPECTIONS

B. SECTION 1705 REQUIRED VERIFICATION AND INSPECTION AND THE FOLLOWING SUB-SECTIONS:

3. 1705.3 CONCRETE CONSTRUCTION

C. SECTION 1711 MATERIAL AND TEST STANDARDS

2. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION. THE APPROVED INSPECTOR MUST BE INDEPENDENT FROM THE CONTRACTOR RESPONSIBLE FOR THE WORK BEING INSPECTED.

3. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR SHALL BE TO INSPECT AND/OR TEST THE WORK OUTLINED ABOVE AND WITHIN THE STATEMENT OF SPECIAL INSPECTIONS IN ACCORDANCE WITH CHAPTER 17 OF THE IBC FOR CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS.

4. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. 5. PER SECTION 1704.2.4 THE SPECIAL INSPECTOR SHALL FURNISH REGULAR REPORTS TO THE BUILDING OFFICIAL AND THE STRUCTURAL ENGINEER. PROGRESS REPORTS FOR CONTINUOUS INSPECTION SHALL BE FURNISHED WEEKLY. INDIVIDUAL REPORTS OF PERIODIC INSPECTIONS SHALL BE FURNISHED WITHIN ONE WEEK OF INSPECTION DATES. THE REPORTS SHALL NOTE UNCORRECTED DEFICIENCIES. CORRECTION OF PREVIOUSLY REPORTED DEFICIENCIES. AND CHANGES TO THE APPROVED CONSTRUCTION DOCUMENTS AUTHORIZED BY THE STRUCTURAL ENGINEER OF RECORD. 6. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL SIGNED REPORT WITHIN 10 DAYS OF THE FINAL SPECIAL INSPECTION STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS, TO THE BEST OF THE INSPECTOR'S KNOWLEDGE IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE IBC. WORK NOT IN COMPLIANCE SHALL BE NOTED IN THE REPORT.

7. THE CONTRACTOR SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON A MAIN WIND- OR SEISMIC-FORCE-RESISTING SYSTEM PER SECTION 1704 4. THE STATEMENT SHALL ACKNOWLEDGE THE AWARENESS OF THE SPECIAL LISTED REQUIREMENTS OF DESIGNATED SEISMIC SYSTEM OR A WIND- OR SEISMIC-RESISTING COMPONENT IN THE STATEMENT OF SPECIAL **INSPECTIONS PER SECTION 1705.**

8. EXCEPT AS NOTED, THE SPECIAL INSPECTIONS OUTLINED ABOVE ARE IN ADDITION TO, AND BEYOND THE SCOPE OF, PERIODIC STRUCTURAL OBSERVATIONS AS DEFINED IN SECTION 1704.5. STRUCTURAL OBSERVATIONS ARE INCLUDED IN THE STRUCTURAL ENGINEERING DESIGN AND CONSTRUCTION ADMINISTRATION SERVICES PROVIDED BY THE STRUCTURAL ENGINEER.

. THE STRUCTURAL DRAWINGS ARE COPYRIGHTED AND SHALL NOT BE COPIED FOR USE AS ERECTION PLANS OR SHOP DETAILS. USE OF JVA'S ELECTRONIC FILES AS THE BASIS FOR SHOP DRAWINGS REQUIRES PRIOR APPROVAL BY JVA, A SIGNED RELEASE OF LIABILITY BY THE GENERAL CONTRACTOR AND/OR HIS SUBCONTRACTORS, AND DELETION OF JVA'S NAME AND LOGO FROM ALL SHEETS SO USED. 2. THE GENERAL CONTRACTOR SHALL SUBMIT IN WRITING ANY REQUESTS TO MODIFY THE STRUCTURAL DRAWINGS OR

PROJECT SPECIFICATIONS. 3. ALL SHOP AND ERECTION DRAWINGS SHALL BE CHECKED AND STAMPED (AFTER HAVING BEEN CHECKED) BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION FOR STRUCTURAL ENGINEER'S REVIEW; SHOP DRAWING SUBMITTALS NOT CHECKED BY THE GENERAL CONTRACTOR PRIOR TO SUBMISSION TO THE STRUCTURAL ENGINEER WILL BE

4. FURNISH ELECTRONIC VERSION (PDF) OF SHOP AND ERECTION DRAWINGS TO THE STRUCTURAL ENGINEER FOR **REVIEW PRIOR TO FABRICATION FOR:**

C. PLANT FABRICATED WOOD JOISTS

D. PRE-ENGINEERED WOOD TRUSSES

5. SUBMIT IN A TIMELY MANNER TO PERMIT 10 WORKING DAYS FOR REVIEW BY THE STRUCTURAL ENGINEER. 6. SHOP DRAWINGS SUBMITTED FOR REVIEW DO NOT CONSTITUTE "REQUEST FOR CHANGE IN WRITING" UNLESS SPECIFIC SUGGESTED CHANGES ARE CLEARLY MARKED. IN ANY EVENT, CHANGES MADE BY MEANS OF THE SHOP DRAWING SUBMITTAL PROCESS BECOME THE RESPONSIBILITY OF THE ONE INITIATING THE CHANGE.

> STRUCTURAL DRAWING LIST **GENERAL NOTES** ABBREVIATIONS, SYMBOLS KEY & 3D VIEW FOUNDATION PLAN LOWER LEVEL FRAMING PLAN MAIN LEVEL FLOOR FRAMING PLAN LOW ROOF FRAMING PLAN HIGH ROOF FRAMING PLAN SCHEDULES & TYPICAL DETAILS FOUNDATION DETAILS **DETAILS & ELEVATIONS** TYP WOOD DETAILS TYP PE TRUSS JOIST DETAILS FRAMING DETAILS **ROOF DETAILS**

CONSULTING ENGINEERS

Fort Collins, CO 80524 970.225.9099 www.jvajva.com Boulder

Fort Collins
Winter Park

JVA #19872

JVA, Inc. 213 Linden Street, Suite 200

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@	ON CENTER SPACING	DWG	DRAWING	LGS	LIGHT GAGE STEEL	REINF	REINFORCE, -ED, -ING
(E)	EXISTING	DWL	DOWEL	LL	LIVE LOAD	REQ	REQUIRED
(N)	NEW	EA	EACH	LLH	LONG LEG HORIZONTAL	REQMT	REQUIREMENT
(R)	REMOVE	ECC	ECCENTRIC	LLV	LONG LEG VERTICAL	RET	RETAINING
AB	ANCHOR ROD (BOLT)	E-E	END TO END	LOC	LOCATION	RM	ROOM
ADDL	ADDITIONAL	EF	EACH FACE	LP	LOW POINT	RMO	ROUGH MASONRY OPENIN
ADJ	ADJUSTABLE	EJ	EXPANSION JOINT	LSL	LAMINATED STRAND LUMBER (GENERIC TERM)	RO	ROUGH OPENING
AESS	ARCHITECTURALLY EXPOSED	EL	ELEVATION	LT	LIGHT	SC	SLIP-CRITICAL
AFF	ABOVE FINISHED FLOOR	ELEC	ELECTRIC, ELECTRICAL	LVL	LAMINATED VENEER LUMBER (GENERIC TERM)	SCH	SCHEDULE
ALT	ALTERNATE	EMBED	EMBEDMENT	MACH	MACHINE	SDST	SELF-DRILLING/ SELF-TAPPING
AMT	AMOUNT	ENGR	ENGINEER	MASY	MASONRY	SECT	SECTION
ANCH	ANCHOR, ANCHORAGE	EQ	EQUAL	MATL	MATERIAL	SF	SQUARE FEET, SUB-FLOOF
APPROX	APPROXIMATE	EQUIP	EQUIPMENT	MAX	MAXIMUM	SHT	SHEET
ARCH	ARCHITECTURAL	EQUIV	EQUIVALENT	MB	MACHINE BOLT	SHTG	SHEATHING
ATR		FS		MECH	MECHANICAL	SIM	SIMILAR
		LO		ME77			
BC DI			EAST TO WEST	MFK	WANUFACTURE, -ER, -ED	SLV	SHURT LEG VERTICAL
BL	BRICK LEDGE	EXC	EXCAVATE	MIN	MINIMUM	SOG	SLAB ON GRADE
BLK	BLOCK	EXP	EXPANSION	ML	MICROLLAM (TRUS-JOIST BRAND LVL)	SP	SPACES, SPACED
BLKG	BLOCKING	EXT	EXTERIOR	MO	MASONRY OPENING	SPEC	SPECIFICATIONS
BM	BEAM	FD	FLOOR DRAIN	MTL	METAL	SQ	SQUARE
BOT	ВОТТОМ	FDN	FOUNDATION	NF	NEAR FACE	ST	SNUG-TIGHT
BRG	BEARING	FF	FINISHED FLOOR, FAR FACE	NIC	NOT IN CONTRACT	STD	STANDARD
BW		F_F		NS		STIFE	STIFFENER
							STITLINER
			FIGURE			OTDUCT	
CF	CUBIC FOOT	FL	FLUSH	NIS	NOT TO SCALE	STRUCT	STRUCTURE, -AL
CG	CENTER OF GRAVITY	FLG	FLANGE	OCJ	OSHA COLUMN JOIST	SUPT	SUPPORT
CIP	CAST-IN-PLACE	FLR	FLOOR	OD	OUTSIDE DIAMETER	SY	SQUARE YARD
CJ	CONSTRUCTION JOINT, CONTROL JOINT	FO	FACE OF	ОН	OPPOSITE HAND	SYM	SYMMETRICAL
CJP		FP	FULL PENETRATION	OPNG	OPENING	T&B	TOP AND BOTTOM
CL	CENTER LINE	FS	FOOT STEP, FAR SIDE	OPP	OPPOSITE	T&G	TONGUE AND GROOVE
CLG	CEILING	FTG	FOOTING	OSB	ORIENTED STRAND BOARD	ТВ	TOP OF BEAM
CLR	CLEAR	GA	GAGE, GAUGE	PAF	POWDER ACTUATED FASTENER	тс	TOP OF CONCRETE
СМ	CONSTRUCTION MANAGER, -MENT	GALV	GALVANIZED	PC	PRECAST	TCA	TORQUE-CONTROLLED ANCHOR
CMU	CONCRETE MASONRY UNIT	GC	GENERAL CONTRACTOR	PCF	POUNDS PER CUBIC FOOT	TD	TOP OF DECK
COL	COLUMN	GEN	GENERAL	PE	PRE-ENGINEERED	THD	THREAD
COM	COMMON	GL	GLUED LAMINATED, GLULAM	PEN	PENETRATION	THK	THICK, -NESS
COMB	COMBINATION	GND	GROUND	PERP	PERPENDICULAR PARTIAL JOINT	TJ	TOP OF JOIST
		GR	GRADE	PJP		TL	
CONT							
COORD	COORDINATE,	HAS	HEADED ANCHOR STUD	PLF	POUND PER LINEAR FOOT	TW	TOP OF WALL
20		HDC		DD		TVD	ΤΥΡΙΟΛΙ
		טעוד חחם				111 ⁷	
				P3	POUNDO DED COMPETENT		
UY		HURIZ	HUKIZUNTAL	PSF	POUNDS PER SQUARE FOOT	UNU	UNLESS NOTED OFHERWIS
DAB DFT	DEFORMED ANCHOR BAR	НР НТ	HIGH POINT HEIGHT	PSI PSI	POUNDS PER SQUARE INCH PARALLEL STRAND LUMBER		VERTICAL VERIEV IN FIELD
DEV	DEVELOP	ID		PT	(GENERIC TERM) POST TENSIONED,	WP	WORK POINT
DIAG	DIAGONAL	INT		PTN	PRESSURE TREATED PARTITION	WT	WEIGHT
DIM	DIMENSION	IT		PWD		WWF	WEI DED WIRE FARRIC
		JB				XS	
						YOEAT	
אוע		JJ] IT					
אט		JI	JUINT	KE	KEFERENCE, REFER 10	XXS	DOUBLE EXTRA STRONG
0 T			KIP (1 000 RS)				

			SYM	BOLS KEY			
	DIRECTION OF DECK SPAN	Γ	XXX'-X	TOP OF CONCRETE			WOOD BEARING WALL
		1.		OR MASONRY ELEVATION			WOOD SHEAR WALL
GRID	GRID DESIGNATION		[XXX'-X]	TOP OF BEAM ELEVATION		A	
Â	REVISION		JB XXX'-X	JOIST BEARING ELEVATION		Ó	COLUMN <u>ABOVE</u>
SWx	SHEAR WALL]-′	/				COLUMN OR OTHER ELEMENT
 公	SHORING	.	BL XXX'-X	BRICK LEDGE ELEVATION		∕ XXx	<u>BELOW</u> SEE SCHEDULES & NOTES Cx = COLUMN
7777	STEP IN FLOOR ELEVATION		(XXX'-X)	TOP OF FOOTING ELEVATION	SNO		BPx = BASE PLATE
			$e^{XXX'-X}$	TOP OF FLOOR ELEVATION	GNAT		ABx = ANCHOR BOLT
	CMU (CONCRETE MASONRY UNIT)			COLUMN CONTINUOUS FROM LEVEL BELOW	N DESI	∠CONT ∠C	HDX = HOLDOWN
	BRICK	TIONS			OLUM	66	COLUMN CONTINUOUS FROM LEVEL BELOW
		ESIGNA			DING C		"X" NUMBER OF KING STUDS BELOW "Y" NUMBER OF TRIMMER STUDS
		MND	В	SEE FRAMING PLAN AT NEXT LOWER	BUIL	-	"X" NUMBER OF BUILT-UP
	EXISTING STONE	COLUI		LEVEL			2x6 STUDS IN COLUMN BELOW
		DING	G STUB	THIS LEVEL OF FRAMING		X	"X" NUMBER OF BUILT-UP
9 - 4 - 4 - 4	EXISTING CONCRETE	BUI	CXX	COLUMN CONNECTING A LOWER			BELOW
- <u></u>				LEVEL OF FRAMING		•	HOLDOWN
	EARTH		/ XXX'-X	TOP OF CONCRETE			WOOD HEADER
FX.X	ISOLATED SPREAD FOOTING MARK	_	/	MASONRY ELEVATION			
FXX	SPREAD FOOTING MARK			STEP TOP OF WALL		ſ	SUPPORTED BY METAL
STEP	STEP IN BOTTOM OF WALL/GRADE BEAM		BL XXX'-X				
XX:12	ROOF SLOPE	┥┙	/	BRICK LEDGE ELEVATION			OVER
SLOPE	DIRECTION OF SLOPE (DOWN)	1	(XXX'-X)	TOP OF FOOTING ELEVATION		<u> </u>	INTERMEDIATE SUPPORT
	STAIR OR RAMP DIRECTION	<u> </u>	$\mathbf{e}^{\mathbf{XXX}^{-\mathbf{X}}}$	TOP OF FLOOR ELEVATION			WOOD JOIST BEARING ON TOP OF SUPPORT
		_				K	

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DRAWING TITLE

ABBREVIATIONS, SYMBOLS KEY & 3D VIEW

JVA, Inc. 213 Linden Street, Suite 200 Fort Collins, CO 80524 970.225.9099 www.jvajva.com Boulder ● Fort Collins ● Winter Park Glenwood Springs ● Denver JVA #19872

ARCHITECTURE PLANNING LANDSCAPE INTERIORS

10/15/19

Vista CO 80487 RESIDENCE \mathbf{O} - Eagle's Springs, 1907 AMPBELI Steamboat S Lot S

ISSUE NAME DATE

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HOLDOWNS: • HOLDOWNS ARE INDICATED ON PLAN THUS: • HOLDOWNS INDICATED ARE LOCATED AT THE FLOOR LEVEL • SEE HOLDOWN SCHEDULE ON SHEET S5.3

TYPICAL WOOD FRAMED WALLS & WALL SHEATHING: • ALL EXTERIOR WALLS HATCHED THUS: ARE SW6 2x6 @ 16" STUD SHEAR WALLS SHEATHED WITH ZIP SYSTEM R-6 SHEATHING OR EQUIVALENT (7/16" APA LAMINATED TO 1" RIGID INSULATION) NAILED WITH 10d SHANK (0.131"Øx3") @ 3" AT PANEL EDGES AND 12" IN FIELD OF PANEL; BLOCK AND NAIL ALL ÉDGES BETWEEN STUDS ALL INTERIOR WALLS HATCHED THUS:
 ALL INTERIOR WALLS HATCHED THUS:
 STUD SHEAR WALLS SHEATHED WITH 15/32" APA NAILED WITH 8d COM (0.131"Øx2 1/2") @ 6" AT PANEL EDGES AND 12" IN FIELD OF PANEL; BLOCK AND NAIL ALL EDGES BETWEEN STUDS; EXPOSED SURFACE TO BE NAILED CAREFULLY FOR FINISHED APPEARANCE CAREFULLY FOR FINISHED APPEARANCE • ALL INTERIOR WALLS HATCHED THUS: W4 ARE 2x4 @ 16" STUD BEARING WALLS. W6 ARE 2x6 @ 16" STUD BEARING WALLS

TYPICAL FLOOR SHEATHING:

LOWER LEVEL FRAMING PLAN 1/4" = 1'-(

3/4" STURD-I-FLOOR, APA RATED 24" O.C. TONGUE & GROOVE SHEATHING GLUED AND NAILED WITH 8d NAILS (0.113"Ø x 2 3/8") @ 6" ALONG PANEL EDGES AND @ 12" ALONG INTERMEDIATE RAMING MEMBERS. LAY PANELS PERPENDICULAR TO FRAMING MEMBERS AND STAGGER PANEL JOINTS.

• USGS ELEVATION 7059.50' = 100'-0" DRAWING ELEVATION
 • TOP OF FLOOR SHEATHING = 87'-10 1/4" UNLESS NOTED THUS:
 ⊕ XXX'-X"

• ALL BEAMS ARE FLUSH, UNLESS NOTED OTHERWISE ON PLAN

• ALL HEADERS ARE FLUSH, UNLESS NOTED OTHERWISE ON PLAN ALL COLUMNS ARE BELOW

0 1' 2' 4'

NORTH

• ALL EXTERIOR HEADERS ARE (3) 2x10 HEADERS UNLESS NOTED OTHERWISE ON PLAN

- 8'

• ALL INTERIOR HEADERS ARE (3) 2x8 UNLESS NOTED OTHERWISE ON PLAN

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MAIN LEVEL FLOOR FRAMING PLAN

STUD SHEAR WALLS SHEATHED WITH 15/32" APA NAILED WITH 8d COM (0.131"Øx2 1/2") @ 6" AT PANEL EDGES AND 12" IN FIELD OF PANEL; BLOCK AND NAIL ALL EDGES BETWEEN STUDS; EXPOSED SURFACE TO BE NAILED

• ALL INTERIOR WALLS HATCHED THUS:
 W4 ARE 2x4 @ 16"
 STUD BEARING WALLS.
 W6 ARE 2x6 @ 16" STUD BEARING WALLS

CAREFULLY FOR FINISHED APPEARANCE

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^{(0.131&}quot;Øx2 1/2") @ 6" AT PANEL EDGES AND 12" IN FIELD OF PANEL; BLOCK AND NAIL ALL EDGES BETWEEN STUDS; EXPOSED SURFACE TO BE NAILED • ALL INTERIOR WALLS HATCHED THUS: W4 ARE 2x4 @ 16" STUD BEARING WALLS. W6 ARE 2x6 @ 16" STUD BEARING WALLS

TYPICAL VERTICAL CONSTRUCTION JOINT IN WALL

WALL CONSTRUCTION JOINT S5.0 3/4" = 1'-0"

	ADHES	IVE ANCHO	R IN 2500 PSI	MIN & 21 DAY /	AGE MIN CON	CRETE	
	ANCHOR		PIL OT			MIN	MIN CONC
TYPE	THRD ROD	REBAR	HOLE	UNO H	DISTANCE	SPACING S	THICKNESS
	3/8"ø	#3	1/2"ø	3"	1 3/4"	3"	5"
SIMPSON	1/2"ø	#4	5/8"ø	4"	1 3/4"	3"	6 1/2"
SET-XP	5/8"ø	#5	3/4"ø	5"	1 3/4"	3"	8 1/4"
(ICC-ESR	3/4"ø	#6	7/8"ø	6"	1 3/4"	3"	9 1/4"
2508)	7/8"ø	#7	1"ø	7"	1 3/4"	3"	11 1/2"
	1"ø	#8	1 1/8"ø	8"	1 3/4"	3"	13"
	3/8"ø	#3	1/2"ø	3"	1 7/8"	1 7/8"	4 1/4"
HILTI HIT-	1/2"ø	#4	5/8"ø	4"	2 1/2"	2 1/2"	5 1/4"
RE 500-SD	5/8"ø	#5	3/4"ø	5"	3 1/8"	3 1/8"	6 1/4"
(ICC-ESR	3/4"ø	#6	7/8"ø	6"	3 3/4"	3 3/4"	7 1/2"
2322)	7/8"ø	#7	1"ø	7"	4 3/8"	4 3/8"	8"
	1"ø	#8	1 1/8"ø	8"	5"	5"	10"
TOP OF CONC TOP OF CONC TOP CONC TOP CONCO							

STEP TOP OF WALL,

SEE PLAN

NO	T	Έ	ŝ

11

- INSTALL ADHESIVE ANCHORS PER MANUFACTURER'S INFORMATION AND ICC REPORT CONTRACTOR TO VERIFY MINIMUM EDGE DISTANCES, SPACING AND THICKNESS ARE IN ACCORDANCE WITH SCHEDULE PRIOR TO INSTALLING ANCHOR.
- HOLES TO BE DRILLED WITH ROTARY DRILL ONLY. WHEN DRILLING HOLES IN EXISTING CONCRETE, USE CARE AND CAUTION TO AVOID CUTTING OR DAMAGING THE EXISTING REINFORCING BARS. MAINTAIN A REASONABLE CLEARANCE BETWEEN REINFORCEMENT AND THE DRILLED-IN ANCHOR. FILL
- ABANDONED HOLES WITH HIGH STRENGTH GROUT. 4. SPECIAL INSPECTION IS REQUIRED PER IBC SECTION 1705 AND THE REQUIREMENTS OF THE ICC REPORTS. THE SPECIAL INSPECTOR MUST BE ON THE JOB SITE PERIODICALLY DURING ANCHOR INSTALLATION TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, HOLE CLEANLINESS, EMBEDMENT DEPTH, CONCRETE TYPE, CONCRETE COMPRESSIVE STRENGTH, DRILL BIT DIAMETER, HOLE DEPTH, EDGE DISTANCE(S), ANCHOR SPACING(S), CONCRETE THICKNESS, AND ADHESIVE INJECTION.

$\overline{7}$	ADHESIVE ANCHORS
S5.0	3/4" = 1'-0"

MINIMUM LAP SPLICE LENGTH AND STANDARD HOOK						
BAR SIZE	MINIMUM LAP SPLICE LENGTH	90° DEGREE HOOK DIMENSION				
4	2'-8"	9"				
5	3'-4"	12"				
6	4'-0"	14"				
7	5'-10"	16"				
8	6'-8"	18"				
9	7'-6"	23"				
10	8'-6"	26"				

2 #5 VERTS W/ 10"

HOOK AT TOP AT

CONTINUOUS

STEP —

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S5.0

LAP SCHEDULE 3/4" = 1'-0"

FOOTING SCHEDULE				
MARK	WIDTH	LENGTH	THICKNESS	REINFORCING
F2.0	2'-0"	2'-0"	1'-0"	(3) #5 EACH WAY, BOTTOM
F2.5	2'-6"	2'-6"	1'-0"	(3) #5 EACH WAY, BOTTOM
F3.0	3'-0"	3'-0"	1'-0"	(4) #5 EACH WAY, BOTTOM
F4.0	4'-0"	4'-0"	1'-0"	(5) #5 EACH WAY, BOTTOM
F4.5	4'-6"	4'-6"	1'-0"	(5) #5 EACH WAY, BOTTOM
F6.0	6'-0"	6'-0"	1'-2"	(7) #5 EACH WAY, BOTTOM
F24	2'-0"	CONT	1'-0"	(3) #5 CONT, #5 @ 16" TRANSVERSE
F30	2'-6"	CONT	1'-0"	(3) #5 CONT, #5 @ 16" TRANSVERSE
F36	3'-0"	CONT	1'-0"	(4) #5 CONT, #5 @ 16" TRANSVERSE

FOOTING NOTES: • CENTER FOOTINGS UNDER STEMWALLS, PILASTERS, & COLUMNS, TYPICAL UNLESS NOTED OTHERWISE • FOOTING WIDTHS SHALL NOT VARY IN THE FIELD FROM SIZE NOTED; OVERSIZE FOOTINGS ARE NOT ALLOWED

• LAPPED BOARD FORMING NOT ALLOWED TRENCH FORMING NOT ALLOWED

0 6" 1'

FOOTING SCHEDULE 3/4" = 1'-0"

- SLOPING BARS, SIZE AND

QUANTITY TO MATCH

BOTTOM REINF BARS

STEP, SEE PLAN ------

WHERE COLD JOINTS ARE REQUIRED.

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