FAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Valley Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: CBS0306SA-7-HIESCO COMPLETE - 1218972

CG1, CG2, CG3, GE1, GE10, GE11, GE12, GE13, GE14, GE15, GE2, GE3, GE4, GE5, GE6, GE7, GE8, GR1, GR10, GR11, GR2, GR3, GR4, GR6, GR7, GR8, GR9, J1, J10, J11, J12, J13, J14, J2, J3, J4, J5, J6, J7, J8, J9, T1, T11, T12, T12A, T13, T14, T15, T16, T17, T18, T19, T2, T20, T21, T22, T23, T24, T25, T26, T27, T28, T29, T3, T30, T31, T32, T33, T34, T35, T36, T37, T38, T39, T4, T40, T41, T42, T43, T44, T45, T46, T47, T48, T49, T5, T50, T51, T52, T53, T54, T55, T56, T57, T58, T6, T7, T7A, T7G, T8, T9, T9A, V10, V11, V12, V13, V14, V15, V2, V3, V4, V5, V6, V7, V8, V9

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (CO, PE-39632)

My license expiration date for the state of CO is 10/31/2025.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Empowering great component manufacturers.

FAGLE METAL

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Valley Truss**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: CBS0306SA-8-HIESCO COMPLETE - 1219268 T12, T12A, T14, T19, T21, T22, T23, T24, T37, T4, T5, T57, T58

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



These are the ones that were replaced.

Arturo A. Hernandez (CO, PE-39632)

My license expiration date for the state of CO is 10/31/2025.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Empowering great component manufacturers.

DESIGN NOTES

- have been prepared under and are subject to ANSI / TPI 1 published by the Truss Plate Institute, www.tpinst.org. The Truss Design Drawing(s) provided with these Design Notes Capitalized terms have the meanings provided in ANSI / TPI 1.
- ده installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses Copies of each Truss Design Drawing shall be furnished to the

DESIGN LIMITATIONS

- ļu of specifications provided by the Building Designer. (if any) assumes any responsibility for the adequacy or accuracy the Truss Designer, Eagle, nor an engineer who seals this design by the Building Designer in accordance with ANS1 / TPI 1. Neither The Truss Design Drawing is based upon specifications provided
- general conformance with the design of the Building for reviewing and verifying that the information shown is in based upon the Truss Design Drawing and shall be responsible The Building Designer is solely responsible for the suitability
- ţ'n. solely for the individual truss indicates acceptance of professional engineering responsibility component (a truss). A seal on the Truss Design Drawing Each Truss Design Drawing is for the individual building
- 9 protected from the environment. Each Truss Design Drawing assumes trusses will be suitably

HANDLING, INSTALLING, & BRACING

- N Association, www.sbcindustry.com be obtained from the Structural Building Components handling, installing, restraining and bracing trusses. Copies can Refer to Building Component Safety Information (BCSI) for
- 90 respectively, of the erector and Building Designer. load and diagonal or cross bracing, are the responsibility, lengths. All temporary and permanent bracing, including lateral support of individual truss components only to reduce buckling Bracing shown on each Truss Design Drawing is for lateral
- عب Eagle is not responsible for improper truss fabrication, handling,
- 10, Compression chords shall be laterally braced by the roof or floor shown, unless noted otherwise sheathing, directly attached, or have purlins provided at spacing

- 11. Bottom chord required bracing shall be at 10ft spacing or less, if no structural rated ceiling is installed, unless noted otherwise.
- 12. including flooring systems, to limit deflection and reduce Strongbacking shall be installed on all parallel chord trusses, vibration. Refer to BCSI-B7
- 13. other materials on inadequately braced truss; refer to BCSI Never exceed the design loading shown. Never stack building or
- 14. shall not be applied to the trusses at any time; refer to BCSI Concentration of construction loads greater than the design loads
- 15 Refer to BCSI for recommended truss handling and erection Trusses shall be handled with care prior to erection to avoid damage.

MATERIALS & FABRICATION

- 16. Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise
- 17. Lumber used shall be of the species and size, and in all respects equal to or better than that specified.
- 18. Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber
- 19. regulated in accordance with ANSI / TPI 1. embedded fully. Knots and wane at joint locations shall be Plates shall be applied on both faces of truss at each joint and
- 20. For a specified plate gauge and grade, the specified size is a
- 21. Connections not shown are the responsibility of others
- 22 Adequate support shall be provided to resist gravity, lateral and uplift loads
- 23 For 4X2 truss orientation, locate plates 0 - 1/16" from outside the edge of the truss
- 24. Fabrication of truss shall be in accordance with ANSI / TPI 1.

OTHER NOTES

- 25 Camber is a non-structural consideration and is the responsibility of truss fabricator.
- 26. approval from a professional engineer. Do not cut or alter any truss member or plate without prior
- 27 design values are by others. Lumber design values are in accordance with ANSI / TPI 1; lumber
- 28 Install specified hangers per manufacturer recommendations,

SYMBOLS

PLATE SIZE

the length parallel to slots perpendicular to slots. Second dimension is 3X4 - The first dimension is the width

-, /, l, Indicates required direction of information slots; Reference "Joint Details" for more

3X10-20HS - 20 Ga Gr60 connectors required 8X10-18HS - 18 Ga Gr60 connectors required 20 Ga Gr40 connectors required

LATERAL BRACING

required on the member of the truss. When this symbol shown, continuous lateral bracing is

Indicates location where bearings

(supports) occur.



PLATE LOCATION & ORIENTATION

with the design drawing/QC full joint and/or placed in accordance scale details The plate shall be centered on



REFERENCES

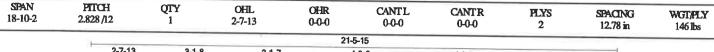
- Metal Plate Connected Wood Trusses •ANSI / TPI 1: National Design Standard for
- Metal Plate Connected Wood Trusses Handling, Installing, Restraining, & Bracing of Information - Guide to Good Practice for •BCSI: Building Component & Safety
- Construction •NDS: National Design Specification for Wood
- Code Council. www.icc-es.org •ESR: 1082 published by the International

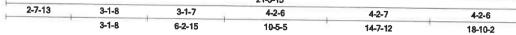
Truss: CG1

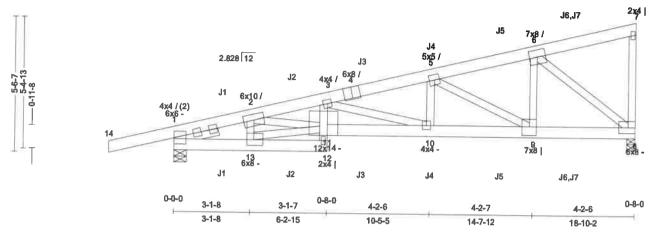
CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:08

Page:







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) Carried Loads (psf) TCLL: 110 TCDL: 10 BCLL: 0		CSI TC: 0.59 (3-5) BC: 0.82 (9-10) Web: 0.85 (3-11)	Deflection VertTL: 0.37 in VertLL: 0.29 in Horz TL: 0.12 in	L/ L/577 L/750	(loc) 11 11 8	Allowed L/240 L/360
BCDL: 10	Lumba D.O.L., 100 %					09/12/2024

Reaction

Л	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift N	Aax C&C Ublift	Max Unlift	Max Horiz
1	1	6.278 in	1.56 in	3,589 lbs		-304 lbs	(4)	-304 lbs	54 lbs
8	1	4.157 in	2.58 in	5,915 lbs		-470 lbs	(%))	-470 lbs	54 103

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG 2400/1.8 2 x 4: 2-11 **Bracing**

TC: Sheathed or Purlins at 6-0-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Pactored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) Non-concurrent minimum storage aftic loading has been applied in accordance with IBC 1607.1

Load Case Lrl: Std Live Load

Picture of Facilities

DISTRIBUTED LORES							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-6-0	Down	Proj	0 plf	24.31 plf	1130
Тор	-1-6-0	2-1-0	Down	Proj	24.31 plf	0 plf	
Тор	-2-7-13	-1-6-0	Down	Proj	0 plf	24.31 plf	
Тор	-1-6-0	2-1-0	Down	Proj	24.31 plf	0 plf	
Тор	1 7- 1-4	18-10-2	Down	Proj	21.3 plf	3.9 plf	

Load Case D1: Std Dead Load

Distributed Loa	ds						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-6-0	Down	Proj	0 plf	12.15 plf	17207
Тор	-1-6-0	2-1-0	Down	Proj	12.15 plf	0 plf	
Тор	-2-7-13	-1-6-0	Down	Proj	0 plf	12.15 plf	
Тор	-1-6-0	2-1-0	Down	Proj	12.15 plf	0 plf	
Тор	17-1-4	18-10-2	Down	Proj	10.65 plf	1.95 plf	
Bot	17-1-4	18-10-2	Down	Proj	10.65 plf	1.95 plf	

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300 lbs are shown in this table. TC 1-2 0.490 -3,371 lbs 3-5 0.590 -5,668 lbs -6.823 lbs

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss

133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: CG1

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 07:59:08

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	SPAN 18-10-		PITCE 2.828 /		TY 1		HTL '-13	OHR 0-0-0	(CANTL 0-0-0	CANTR 0-0-0	PLYS 2	SPACING 12.78 in	WGT/PLY 146 lbs
ВС	8-9 9-10	0.715 0.816	3,188 lbs 5,437 lbs	(-255 lbs) 10-11 (-435 lbs) 12-13	0.608 0.247	6,827 lbs 378 lbs	(-546 lbs) 13-1 (-30 lbs)	0.564	3,177 lbs	(-361 lbs)				
Web	2-13 2-11 13-11	0.130 0.434 0.828	-1,053 lbs 3,512 lbs 2,869 lbs	(-289 lbs) 5-10 (-221 lbs) 5-9	0.214 0.295 0.387	-1,447 lbs 1,021 lbs -2,561 lbs	(-58 lbs) 6-8	0.712 0.656	2,468 lbs -4,101 lbs	(-152 lbs)				

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
n	TC	1-11-8
u	TC	1-11-8
J2	TC	4-9-7
J2	TC	497
J2	BC	497
J2	BC	497
13	TC	7-7-6
J3	TC	7-7-6
J3	BC	7-7-6
J3	BC	7-7-6
J4	TC	10-5-5
J4	TC	10-5-5
J4	BC	10-5-5
J4	BC	10-5-5
J5	TC	13-3-4
J5	TC	13-3-4
J5	BC	13-3-4
J5	BC	13-3-4
J7	TC	16-1-4
37	BC	16-1-4
J6	TC	16-1-4
J6	BC	16-1-4
л	BC	1-11-8
Л	BC	1-11-8

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

4) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.

5) A creep factor of 1.50 has been applied for this truss analysis.

Truss: CG1

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
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SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 18-10-2 2.828 /12 2-7-13 0-0-0 θ ഹ 12.78 in 146 lbs

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135'x3"] TC-2 staggered rows @ 0-11-4 cc, BC-2 staggered rows @ 1-0-0 oc, Webs-1 row 10d Nails or Gun Nails [min 0.120'x2.875"] @ 1-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 12d Nails or Gun Nails [min 0.135"x3"] as follows within 24" of the location shown:

TC: 1-11-8,(2)Connectors

TC: 4-9-7,(4)Connectors

TC: 7-7-6,(7)Connectors

TC: 10-5-5,(4)Connectors

TC: 13-3-4,(7)Connectors

TC:16-1-4,(7)Connectors

BC: 1-11-8,(1)Connectors

BC: 4-9-7,(1)Connectors

BC:7-7-6,(1)Connectors

BC: 10-5-5,(7)Connectors BC: 13-3-4,(8)Connectors

BC: 16-1-4,(11)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 13) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

Truss: CG2

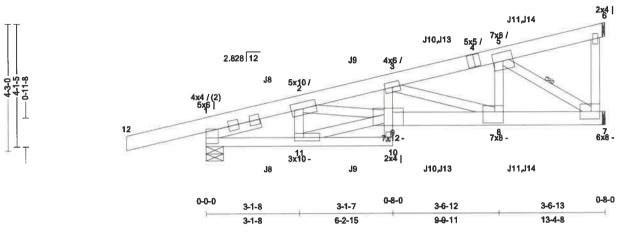
CBS0306SA-7-HIESCO COMPLETE Job:

0-8-0

Designer: Shane Allen Date: 09/12/24 07:59:09 Page: 1 of 2

				16	-0-5					
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGI/PLY	
13-4-8	2.828/12	1	2-7-13	0-0-0	0-0-0	0-0-0	1	11.35 in	99 lbs	





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)		IBC2018/	TC: 1.00(12-1)	VertTL:	0.23 in	L/651	10	L/240
TCLL: 110	•	TPI 1-2014	BC: 0.98 (7-8)	VertLL: (0.18 in	L/855	9	L/360
TCDL: 10	Rep Mbr:	No	Web: 0.88 (2-9)	HorzTL: (0.07 in		7	
DOTI . O	Tumber DOI	. 100 %						00/17/20

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rod Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
1	1	7.028 in	1.65 in	1,895 lbs	•	-169 lbs	-84 lbs	-169 lbs	92 lbs /
7	1	1.5 in		2,596 lbs		-202 lbs		-202 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 4 except:

SP-FT/PG-2400/1.8 2 x 6: 7-9

Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 3-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 5-7

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

DISTRIBUTIONS				0.920 - 321			
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	24.31 plf	
Тор	-1-5-10	2-1-12	Down	Proj	24.31 plf	0 plf	
Top	-2-7-13	-1-5-10	Down	Proj	0 plf	24.31 plf	
Тор	-1-5-10	2-1-12	Down	Proj	24.31 plf	0 plf	
Тор	11-6-13	13-1-12	Down	Proj	18.91 plf	3.14 plf	
Тор	13-1-12	13-4-4	Down	Proj	3.14 plf	3.14 plf	

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	12.15 plf	
Top	-1-5-10	2-1-12	Down	Proj	12.15 plf	0 plf	
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	12.15 plf	
Top	-1-5-10	2-1-12	Down	Proj	12.15 plf	0 plf	
Тор	11-6-13	13-1-12	Down	Proj	9.45 plf	1.57 plf	
Тор	13-1-12	13-4-4	Down	Proj	1.57 plf	1.57 plf	
Bot	11-6-13	13-1-12	Down	Proj	9.45 plf	1.57 plf	
Bot	13-1-12	13-4-4	Down	Proj	1.57 plf	1.57 plf	

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Truss: CG2

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen

Date: 09/12/24 07:59:10 Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
13-4-8	2.828/12	1	2-7-13	0-0-0	0-0-0	0-0-0	1	11,35 in	99 lbs

Mer	nber l	Forces	Table	indicates: M	anbar i	ID, max CSI	max axial for	ce, (max comp	r. force	if different f	iom max axial :	force). Only fo	rces greater than 300 lbs are shown in this table.
TC	1-2	0.987	-2,510 lbs		3-5	0.526	-3,624 lbs		1			- 1	ř
	2-3	0.544	-5,445 lbs										
BC	7-8	0.981	3,430 lbs	(-267 lbs)	8-9	0.647	5,431 lbs	(431 lbs)	11-1	0.564	2,289 lbs	(-882 lbs)	
Web	2-11	0.082	-648 lbs		3-8	0.537	-2,115 lbs						
	2-9	0.878	3,044 Ibs	(-257 lbs)	5-8	0.539	1,869 lbs	(-92 lbs)					
	11-9	0.605	2,097 Ibs	(-276 lbs)	5-7	0.563	-4,106 lbs		l .			- 1	J.

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
18	TC	2-1-0
18	TC	2-1-0
18	BC	2-1-0
18	BC	2-1-0
19	TC	4-10-15
19	TC	4-10-15
19	BC	4-10-15
19	BC	4-10-15
J13	TC	7-8-14
J13	BC	7-8-14
110	TC	7-8-14
110	BC	7-8-14
J14	TC	10-6-13
J14	BC	10-6-13
J11	TC	10-6-13
л1	BC	10-6-13

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 13) Incising is not permitted.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: CG3

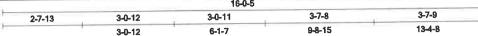
Job: CBS0306SA-7-HIESCO COMPLETE

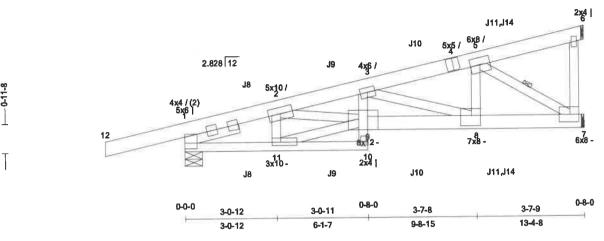
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Designer: Shane Allen 09/12/24 07:59:11 Date:

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
13-4-8	2.828/12	2	2-7-13	0-0-0	0-0-0	0-0-0	1	11.35 in	99 lbs
				16	L-0-5				





All plates shown to be Eagle 20 unless otherwise noted.

•					- 7	·	A 700 B
Loading (psf)	Ceneral		CSI	Deflection	L'	(loc)	Allowed
				VertTL: 0.23 in	L/655	10	L/240
Carried Loads (psf)	Bldg Code:	IBC2018/	TC: 1.00 (12-1)		-,	10	•
TCLL: 110		TPI 1-2014	BC: 0.93 (7-8)	VertLL: 0.18 in	L/866	9	L/360
				1	•	7	
TCDL: 10	Reo Mbr:	No	Web: 0.62 (9-10)	Horz TL: 0.07 in		,	4 4

Lumber D.O.L.: 100 % BCLL: 0 BCDL: 10

Brg Width

7.028 in

1.5 in

Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift Max Horiz Max React -167 lbs -86 lbs -167 lbs 81 lbs

Material

Reaction

JT Brg Combo

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 6: 7-9

Web: SP-FT/PG#2 2x 4 except SP-FT/PG 2400/1.8 2 x 4: 2-9

TC: Sheathed or Purlins at 3-4-0, Purlin design by Others.

-200 lbs

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 5-7

-200 lbs

- 1) This truss has been designed for the effects of balanced (110 pst) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.

Rqd Brg Width

1.63 in

- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL = 1.60

1,871 lbs

2,562 lbs

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	24.31 plf	
Тор	-1-5-10	2-1-12	Down	Proj	24.31 plf	0 plf	
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	24.31 plf	
Тор	-1-5-10	2-1-12	Down	Proj	24.31 plf	0 plf	
Тор	11-6-13	13-1-12	Down	Proj	18.91 plf	3.14 plf	
Тор	13-1-12	13-4-8	Down	Proj	3.14 plf	3.14 plf	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss

133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: CG3

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen 09/12/24 07:59:11

	02/12/27
Page:	2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
13-4-8	2.828/12	2	2-7-13	0-0-0	0-0-0	0-0-0	1	11.35 in	99 lbs

Load Case D1; Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	12.15 plf	22277722
Тор	-1-5-10	2-1-12	Down	Proj	12.15 plf	O plf	
Тор	-2-7-13	-1-5-10	Down	Proj	0 plf	12.15 plf	
Тор	-1-5-10	2-1-12	Down	Proj	12.15 plf	Oplf	
Тор	11-6-13	13-1-12	Down	Proj	9.45 plf	1.57 plf	
Тор	13-1-12	13-4-8	Down	Proj	1.57 plf	1.57 plf	
Bot	11-6-13	13-1-12	Down	Proj	9.45 plf	1.57 plf	
Bot	13-1-12	13-4-4	Down	Proi	1.57 nlf	1.57 plf	

Me	nber	Forces	Table	indicates; M	lember I	D, mex CS	, max axial fon	ce, (max com	r. force	if different f	iom max axial	force). Only fo	owes greater than 300lbs are shown in this table.
TC	1-2	0.987	-2,400 lbs		13-5	0.682	-3.648 lbs	•	1				Barre Paris Doorge Molinowiti at title English
	2-3	0.479	-5,451 lbs				-,						
BC	7-8	0.929	3,422 lbs	(-268 lbs)	8.9	0.538	5,448 lbs	(431 lbs)	11-1	0.540	2,183 lbs	(-883 lbs)	
Web	2-11	0.080	-636 lbs		3-8	0.555	-2.133 lbs				-,	(000 200)	
	2-9	0.392	3,171 lbs	(-265 lbs)	5-8	0.463	1,603 lbs	(-78 lbs)				- 1	
	11-9	0.577	2,000 lbs	(-297 lbs)	5-7	0.560	4.064 The	(/ 0 100 /				- 1	

Truss to Truss Connection Summary

Carried Truss	Carving Chord	Carrying Offset
J8	TC	2-1-0
J8	TC	2-1-0
J8	BC	2-1-0
18	BC	2-1-0
J9	TC	4-10-15
J 9	TC	4-10-15
19	BC	4-10-15
19	BC	4-10-15
J10	TC	7-8-14
110	TC	7-8-14
J10	BC	7-8-14
110	BC	7-8-14
J14	TC	10-6-13
Л14	BC	10-6-13
J11	TC	10-6-13
J11	BC	10-6-13

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 13) Incising is not permitted.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

OHR

Truss: GE1

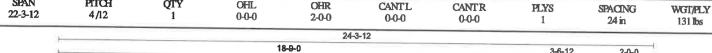
Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date 09/12/24 07:59:12

SPACING

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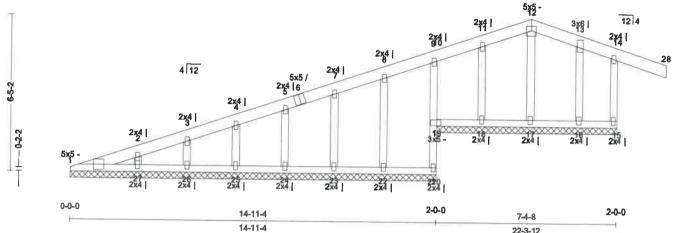
09/12/202





CANTL

CANTR



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	T/	(loc)	Allowed
TCLL: 110	Bldg Code: IBC 2018/	TC: 0.91 (14-28)	VertTL: 0 in UP	L/999	15	L/240
TCDL: 10	TPI 1-2014	BC: 0.04 (20-22)	VertLL: 0 in	L/999	15	L/360
BCLL: 0	Rep Mbr: No	Web: 0.63 (14-15)	Horz TL: 0 in	L(333	13	12/300
BCDL: 10	Lumber D.O.L.: 100 %	1 100 (1 , 15)	I I I I I I I I I I I I I I I I I I I			

Reaction

SPAN

PITCH

QTY

OHL

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplif	Max C&CUplift	Max Unlift	Max Horiz
1	20	1,509 lbs	358 plf	-463 lbs	-120 lbs	-298 lbs	-463 lbs	_149 lbs

Material

TC: SP-FT/PG2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following use defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

BC	T -												
	2-27 3-26 4-25	0.050 0.062 0.083	-406 lbs -492 lbs -612 lbs	5-24 7-23 8-22	0.106 0.136 0.177	-673 lbs -672 lbs	10-19 11-18	0.125 0.136	-689 lbs -671 lbs	13-16 14-15	0.145 0.634	501 lbs -1,496 lbs	(-222 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x420ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 7) At leastone web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.

8) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE1

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 07:59:12

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SPAN PITCH QTY OHL OHR CANTL CANTR 22-3-12 PLYS **SPACING** WGT/PLY 4/12 0-0-0 2-0-0 0-0-0 0-0-0 24 in 9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 16 may need to be considered. 131 lbs

10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated. 11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) All connectors, i.e. nails, screws, bolts, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

14) Incising is not permitted.

Valley Truss 133 Range View Loop

Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GE2

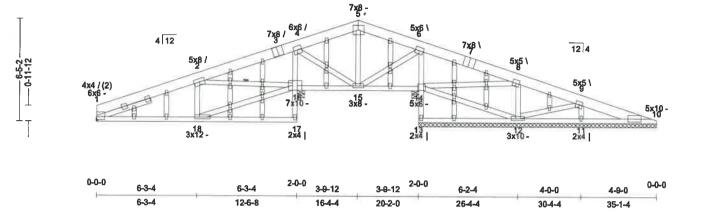
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CBS0306SA-7-HIESCO COMPLETE Job:

Designer. Shane Allen Date: 09/12/24 07:59:20 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
35-1-4	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	278 lbs	

T-				35-1-4			
	6-3-4	6-3-4	3 -9 -12	3-9-12	6-2-4	4-0-0	4-9-0
	6-3-4	12-6-8	16-4-4	20-2-0	26-4-4	30-4-4	35-1-4



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10	Bldg Code: IBC 2018/ TPI 1-2014	CSI TC: 0.82 (2-4) BC: 0.40 (18-1)	VertTL: 0.12 in VertLL: 0.07 in	L/ L/999 L/999	(loc) (17-18) (18-1)	Allowed L/240 L/360
BCLL: 0	Rep Mbr: No	Web: 0.72 (18-16)	Horz TL: 0.02 in		16	
BCDL: 10	Lumber D.O.L.: 100 %	` '				

Rea	action								
JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	Max C&C Uplift	Max Uplift	Max Ho
1	1	1.5 in		1,724 lbs		-106 lbs	-221 lbs	-221 lbs	-30 lb
16	1	5.5 in	3.17 in	3,632 lbs		-151 lbs	-348 lbs	-348 lbs	
14	1	5.5 in	1.90 in	2,178 lbs		-112 lbs	-223 lbs	-223 lbs	
11	1	179.25 in	N/A	861 lbs		-56 lbs	-115 lbs	-115 lbs	
12	1	179.25 in	N/A	1,845 lbs	7.	-92 lbs	-200 lbs	-200 lbs	
13	1	179.25 in	N/A	98 lbs	*	•			
10	1	179.25 in	N/A	129 lbs	92	-12 lbs	-21 lbs	-21 lbs	
10	1	179.25 in	N/A	719 lbs	5	-56 lbs	-111 lbs	-111 lbs	• 170
10	1	179.25 in	N/A	31 lbs		:5	•	800	. /

TC: SP-FT/PG 2400/1.8 2 x 8

BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-16

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user

defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber I	Forces	Table	indicates: M	ember I	D, max CS	, max axial force,	(max compr	force if	different f	iom max axial force). Only f	orces greater than 300lbs are shown in this table.
TC	1-2	0.637	-2,537 lbs		4-5	0.695	-503 lbs		6-8	0.764	-433 lbs	
	2-4	0.822	691 lbs		5-6	0.645	-620 lbs					
BC	15-16	0.160	-324 Ibs									
	18-1	0.397	2,358 lbs	(-235 lbs)				7.				
Web	2-18	0.095	-636 lbs		4-15	0.184	636 lbs		8-12	0.268	-1.769 lbs	
	2-16	0.530	-2,697 lbs		5-15	0.112	-443 lbs		9-11	0.101	-805 lbs	
	18-16	0.715	2,478 lbs	(-247 lbs)	6-15	0.095	329 lbs	- 1				
	4-16	0.443	-2,513 lbs		6-14	0.370	-2,099 Ibs	- 1				8

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GE2

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:21

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S SPACING	WGT/PLY
24 in	278 lbs
Y	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 6) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.
- 7) Gable must be sheathed on one side or lateral bracing applied appropriately.
- 8) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Laterall braces shall be installed within 6 "of each web panel point.
- 9) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 10) A creep factor of 1.50 has been applied for this truss analysis.
- 11) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

 12) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided
- on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 13) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 14) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 15) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 16) Incising is not permitted.
- 17) Listed wind uplift reactions based on MWFRS & C&C loading.

16-6-0

Truss: GE3

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
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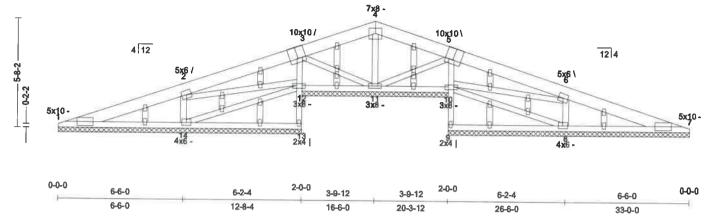
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09/12/2024

SPAN 33-0-0	PITCH 4/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 235 lbs
-				33-	0-0				
2	6-6-0		6-2-4	3-9-12	3 -9- 12	6-2-4		6-6-0	

20-3-12

26-6-0



All plates shown to be Eagle 20 unless otherwise noted.

6-6-0

Loading (psf) TCLL: 110 TCDL: 10	General Bidg Code:		CSI TC: 0.73 (2-3)	Deflection Vert TL: 0.07 i	-,,,,,	(loc) (8-9)	Allowed L/240
BCLL: 0	Rep Mbr:	TPI 1-2014 No	BC: 0.29 (8-9) Web: 0.24 (2-14)	VertLL: 0.04 in Horz TL: 0 in	n L/999	(8-9)	L/360

12-8-4

Th	44	
ж	eachnn	

Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplit	t Max MWPRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
1	396 in	N/A	1,922 lbs	200	-91 lbs	-202 lbs	-202 lbs	
1	396 in	N/A	97 lbs	(6)				
1	396 in	N/A	1,653 lbs	8.0	-116 lbs	-205 lbs	-205 lbs	246 lbs
1	396 in	N/A	1,066 lbs	3+3	-41 lbs	-110 lbs	-110 lbs	-127 lbs
1	396 in	N/A	1,653 lbs	200	-116 lbs	-205 lbs	-205 lbs	-246 lbs
1	396 in	N/A	97 lbs		1.71		-00 25	210 23
1	396 in	N/A	1,922 lbs		-91 lbs	-202 lbs	-202 lbs	
1	396 in	N/A	54 lbs	-14 lbs			-14 lbs	-442 lbs
1	396 in	N/A	54 lbs	-14 lbs			-14 lbs	442 lbs
1	396 in	N/A	1,109 lbs	•	-79 lbs	-156 lbs	-156 lbs	836 lbs
1	396 in	N/A	29 lbs	-18 lbs			-18 lbs	050 103
1	396 in	N/A	1,109 lbs		-79 lbs	-156 lbs	-156 lbs	-836 lbs
1	396 in	N/A	29 lbs	-18 lbs		-	-18 lbs	-050 108

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	mber	Forces	Table indicates: N	/lember II), max CSI	, max axial force, (max con	pr. force if different from max axial force). Only fo	onces greater than 300ths are shown in this table
TC	1-2	0.629	-591 lbs	5-6	0.732	-631 lbs	1	
	2-3	0.732	-631 lbs	6-7	0.629	-591 lbs		
BC				1				
-	_							
Web	2-14	0.235	-1,789 lbs	6-8	0.235	-1,789 Ibs		(i)
	3-12	0.229	-1,632 lbs	1			1	
	4-11	0.155	-847 Ibs	1			1	
	5-10	0.229	-1,632 lbs	1			1	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST, DESIGN VALID ONLY WHEN FAGUE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE3

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:22

Page: 2 of 2

SPAN PITCH QTY OHL OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
33-0-0 4/12 1 0-0-0 0-0-0	0-0-0	0-0-0	1	24 in	235 lbs

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 6) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 1, 7, 1, 7 may need to be considered.
- 9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details,
- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 13) Incising is not permitted.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

OHR

CANTL

CANTR

Truss: GF4

PLYS

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

91 lbs

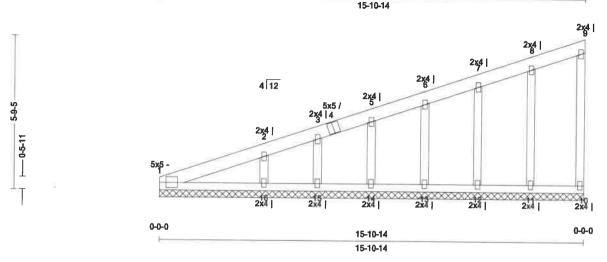
Designer: Shane Allen Date: 09/12/24 07:59:23 1 of 2 Page:

SPACING

24 in

15-10-14	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	
				15-1	0-14			
				15-1	0-14			

OHL



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General TCLL: 110 Bidg Code: IBC2018/ TCDL: 10 TPI 1-2014 BCLL: 0 Rep Mbr: No BCDL: 10 Lumber D.O.L.: 100 %	CSI TC: 0.56 (1-2) BC: 0.08 (16-1) Web: 0.28 (9-10)	Deflection VertTL: 0 in VertLL: 0 in UP Horz TL: 0 in	L/ L/999 L/999	(loc) (16-1) 10	Allowed L/240 L/360
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Reaction

SPAN

PICH

QTY

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWPRS Upli	ft Max C&C Uplift	Max Unlift	Max Horiz
1	+1	1,178 lbs	332 plf	-342 lbs	-81 lbs	-163 lbs	-342 lbs	711 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

	mber!	Forces	Table	indicates: M	arber II	O, max CSI,	max axial force,	(max compr. force	if different fi	om max axial force)	Only forces greater than 300lbs are shown in this tal	nle
TC	1-2	0.559	869 lbs	(-251 lbs)						,		J
BC									_			_
Web	2-16 3-15	0.114 0.086	-899 lbs -630 lbs		5-14 6-13	0.077 0.100	-483 lbs -489 lbs	7-12 8-11	0.133 0.174	-489 lbs -486 lbs		_

Notes 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) Gable requires continuous bottom chord bearing.

4) Attach gable webs with 2x420g a plates, U.N.O.

3) Gable webs placed at 24 "OC, U.N.O.

5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.

6) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

7) A creep factor of 1.50 has been applied for this truss analysis.

09/12/2024

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252		Job	igner: Shane Allen :: 09/12/24 07:5	7-HIESCO COMPLETE 1923
SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
15-10-14	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	91 lbs

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

⁸⁾ Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

¹⁰⁾ Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

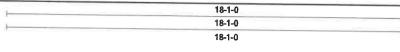
¹⁴⁾ Listed wind uplift reactions based on MWFRS & C&C loading.

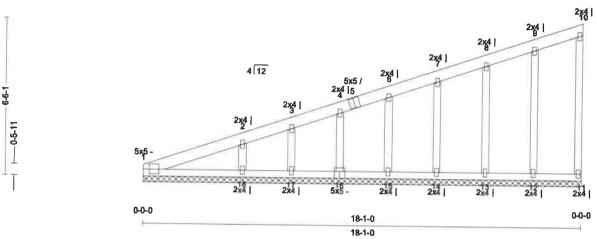
Truss: GE5

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:24 Page: 1 of 2

SPAN	PITCH	QTY	OHIL	OHR	CANT L	CANTR	PLYS	SPACING	WGT/PLY	
18-1-0	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	108 lbs	
		2		18	-1-0					_





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCL: 110 TCDL: 10 BCLL: 0 BCDL: 10 BCDL: 10 BCDL: 10	TPI 1-2014 No	CSI TC: 0.59 (1-2) BC: 0.08 (18-1) Web: 0.35 (10-11)	Deflection VertTL: VertLL: Horz TL:	Oin Oin Oin Oin	L/ L/999 L/999	(loc) (18-1) 11	Allowed L/240 L/360
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Reaction

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUblift	Max Unlift	Max Horiz
1		1,268 lbs	334 plf	-398 lbs	-88 lbs	-160 lbs	-398 lbs	800 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following uses defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL=20 psf.

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. 971 lbs (-261 lbs)

BC													_
Web	2-18 3-17	0.119 0.084	-935 lbs -612 lbs	4-16 6-15	0.098 0.100	-607 lbs -480 lbs	7-14 8-13	0.135 0.180	486 lbs 489 lbs	9-12	0.229	-484 lbs	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x420g a plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 7) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE5

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:24

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 18-1-0 0-0-0 0-0-0 0-0-0 0-0-0 24 in 108 lbs

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) All connectors, i.e. nails, screws, boils, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

14) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTHIN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GE6

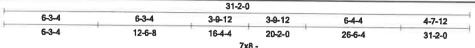
Job: CBS0306SA-7-HIESCO COMPLETE

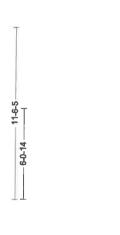
Designer: Shane Allen Date: 09/12/24 07:59:25

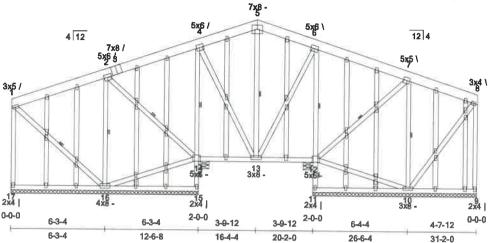
Page: 1 of 2

09/12/202

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 31-2-0 0-0-0 0-0-0 0-0-0 0-0-0 24 in 409 lbs 31-2-0







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCL: 110 TCDL: 10 BCLL: 0 BCDL: 10	Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No	CSI TC: 0.83 (1-2) BC: 0.38 (16-17) Web: 0.62 (6-12)	Vert TL: 0.03 in Vert LL: 0.02 in Horz TL: 0.01 in	L/ L/999 L/999	(loc) (12-13) 13 9	Allowed L/240 L/360
		` '/		Lijiji	9	•

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L'ann	TILONY .
	LINUALI.

Л	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
14	1	8 in	2.12 in	2,437 lbs		-189 lbs	-290 lbs	-290 lbs	
12	1	8 in	2.07 in	2,378 lbs	120	-180 lbs	-278 lbs	-278 lbs	
15	1	150.5 in	N/A	92 lbs	*			270 200	-2.lbs
16	1	150.5 in	N/A	1,647 lbs		-60 lbs	-184 lbs	-184 lbs	145 lbs
17	1	150.5 in	N/A	988 lbs		-46 lbs	-112 lbs	-112 lbs	93 lbs
9	1	132 in	N/A	751 lbs	8	-26 lbs	-89 lbs	-89 lbs	25 203
10	1	132 in	N/A	1.526 lbs	©	-53 lbs	-166 lbs	-166 lbs	. /
11	1	132 in	N/A	100 lbs	#	*	(*.)	-100,03	. (

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-16, 2-16, 2-14, 4-15, 5-13, 6-11, 7-12, 7-10

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 3) This truss has been designed for the effects of TCLL = 20 psf.
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

		Forces		Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown.										
TC	1-2 2-4	0.828	-453 lbs 386 lbs	(-297 lbs)	4-5	0.636	-331 lbs -329 lbs	6-7	0.773	382 lbs	(-292 lbs)			
BC	1	0.000	300 108	(-237 106)	5-0	0.033	-529 108	7-8	0.678	-306 lbs				
Web	1-17	0.467	-939 lbs	_	4-13	0.151	524 lbs	18-9	0.427	-720 lbs				
	1-16	0.100	345 lbs	(-51 lbs)	5-13	0.230	-652 lbs		0.12)	720 103				
	2-16	0.480	-1,811 lbs		6-13	0.144	501 lbs							
	2-14 4-14	0.123	-404 lbs -2,044 lbs		6-12 7-10	0.620 0.444	-2,098 lbs -1,684 lbs				Į.			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).

REFER TrueBuild®Truss Software v5.7.12
Eagle Metal Products

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE BAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE6

CBS0306SA-7-HIESCOCOMPLETE Job:

Designer: Shane Allen 09/12/24 07:59:26 Date:

	/	
Page:	2 of 2	

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 31-2-0 4/12 0-0-0 0-0-0 0-0-0 0-0-024 in 409 lbs

6) Gable must be sheathed on one side or lateral bracing applied appropriately.

7) At leastone web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.

8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

9) A creep factor of 1.50 has been applied for this truss analysis.

10) Mindicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

11) Due to the inherent corrosion of treated himber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

Truss: GE7

09/12/2024

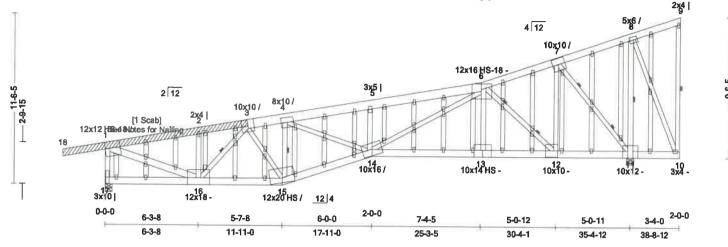
CBS0306SA-7-HIESCO COMPLETE Job:

2-0-0

Designer: Shane Allen Date: 09/12/24 07:59:27 Page: 1 of 2

38-8-12	2/12	QIY 1	OHL 2-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 466 lbs	
-				41	-7-4					_

2-10-8 6-3-8 6-0-0 7-4-5 5-0-12 5-0-11 3-4-0 6-3-8 17-11-0 25-3-5 30-4-1 35-4-12 38-8-12



All plates shown to be Eagle 20 unless otherwise noted.

Reaction

_1	T	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift!	Max C&CUplift	Max Uplift	Max Horiz
1	17	1	5.5 in	4.99 in	5,726 lbs		-288 lbs	-727 lbs	-727 lbs	428 lbs
1	11	1	5.5 in	2.88 in	5,459 lbs		-389 lbs	-855 lbs	-855 lbs	774

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 8-11-0, Purlin design by Others.

Web: One Midpoint Row: 3-16, 4-15, 8-11, 9-10

Two Third Point Rows: 6-12, 7-11

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 3-6

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 4-14, 6-14, 6-12, 7-12, 7-11

SP-FT/PG 2400/1.8 2 x 6: 1-16 Scabs

18-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B-L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater the												ater than 30	Offbs are shown	in this table.			
ı	TC	1-2	0.589	-7,545 lbs		4-5	0.676	-13,639 lbs		7-8	0.838	385 lbs	(-152 lbs)				1
L		2-3	0.222	-7,463 lbs		5-6	0.931	-13,610 lbs					(1			'
ı	_	3-4	0.422	-9,381 lbs		6-7	0.894	-3,404 lbs									
ı	BC	11-12	0.389	3,023 lbs	(-302 lbs)	13-14	0.759	7,623 lbs	(-937 lbs)	15-16	0.698	8,759 lbs	(-1,123 lbs)				
L		12-13	0.624	7,647 lbs	(-9 30 lbs)	14-15	0.912	9,900 Ibs	(-1,235 lbs)	16-17	0.096	-402 lbs	(-, , ,				1
П	Web	1-17	0.808	-5,652 lbs		3-15	0.268	928 lbs	(43 lbs)	6-14	0.824	6,661 lbs	(-775 lbs)	8-11	0.565	-1.836 lbs	
1		1-16	0.617	7,834 lbs	(-728 lbs)	4-15	0.523	-3,723 lbs	` _ ′	6-12	0.839	-6,323 lbs	(110 100)	8-10	0.105	365 lbs	(49 Ibs)
ı		2-16	0.200	-1,002 lbs	1	4-14	0.522	4,219 lbs	(471 lbs)	7-12	0.496	4,007 lbs	(438 lbs)		0.649	-373 lbs	(45 100)
ı		3-16	0.362	-2,284 lbs		5-14	0.360	-1.967 lbs	,	7-11	0.781	4.951 lbs	(.55 -55)	1 20	01015	373 200	1

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.

5) The fabrication tolerance for this roof truss is 10% (Cq =0.90).

			V	Valley Tru 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252		J I I	Chuss: GE7 ob: CBS0306SA-7 Designer: Shane Allen Date: 09/12/24 07:5 dage: 2 of 2	-HIESCO COMPLET
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
38-8-12	2/12	1	2-10-8	0-0-0	0-0-0	0-0-0	1	24 in	466 lbs

6) Gable must be sheathed on one side or lateral bracing applied appropriately.

7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

8) A creep factor of 1.50 has been applied for this truss analysis.

9) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information. 10) Due to the inherent corrosion of treated himber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Scab 3 - 18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

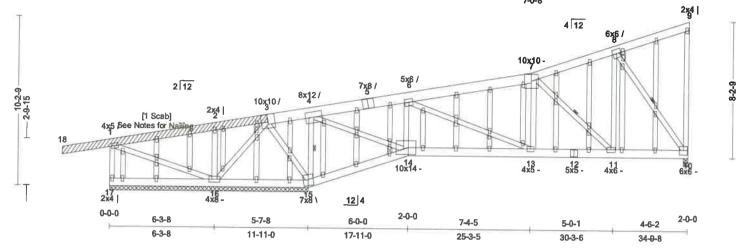
Truss: GE8

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date 09/12/24 07:59:28

Page: 1 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 34-9-8 2/12 2-10-8 0-0-0 0.000.0024 in 377 lbs 37-8-0 2-10-8 6-3-8 3-1-2 2-6-6 6-0-0 7-4-5 5-0-1 4-6-2 6-3-8 9-4-10 11-11-0 17-11-0 25-3-5 34-0-8 7-0-8



TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.

Web: One Midpoint Row: 4-15, 7-11, 8-10

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCL: 110 TCDL: 10 BCLL: 0 BCDL: 10	Bldg Code: IE	PI 1-2014 To	CSI TC: 0.89 (7-8) BC: 0.27 (11-13) Web: 0.80 (1-16)	Deflection VertTL: VertLL: Horz TL:	0.19 in 0.13 in 0.06 in	L/ L/999 L/999	(loc) (13-14) (13-14) 10	Allowed L/240 L/360

Reaction

	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	Max C&CUplift	Max Uplift	Max Horiz
10	1	3.5 in	2.36 in	2,712 lbs		-197 lbs	-364 lbs	-364 lbs	
15	1	143 in	N/A	5,996 lbs	90	-360 lbs	-736 lbs	-736 lbs	1,721 lbs
16	1	143 in	N/A	829 lbs	-270 lbs	-15 lbs	-48 lbs	-270 lbs	-1,721 lbs
17	1	143 in	N/A	1,707 lbs	8	-133 lbs	-211 lbs	-211 lbs	42 lbs
Material					Bra	acing			Ę

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 3-5,5-7

SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except: BC. Sheathed or Purlins at 5-9-0, Purlin design by Others.

SP-FT/PG 2400/1.8 2 x 4: 4-14

18-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 6

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

		orces	Table	indicates: M	ember ID	, max CSI	, max axial for	z, (max com	or. force	if different f	iom max axial :	force). Only fi	orces pres	terthan 30	Olbs are shown in this	table :
TC	1-2	0.874	1,204 lbs	(-137 lbs)	3-4	0.473	2,622 lbs	(-339 lbs)	6-7	0.681	-3.078 lbs	,			COLOR CONTROL	BURLONGA
-	2-3	0.242	1,162 lbs	(-94 lbs)	4-6	0.741	-2,180 lbs	`[7-8	0.888	-1,777 Ibs					
BC	10-11	0.163	1,468 lbs	(-113 lbs)	13-14	0.260	1,941 lbs	(-268 lbs)								_
	11-13	0.270	2,854 lbs	(-338 lbs)	14-15	0.169	-2,506 lbs	` ′								
Web	1-17	0.236	-1,654 lbs		3-15	0.490	-1,485 lbs		6-13	0.413	1,431 lbs	(-90 lbs)	8-10	0.660	-2.560 lbs	
	1-16	0.800	-1,164 lbs	1	4-15	0.533	-3,789 lbs		7-13	0.144	-435 lbs	(30,200)	9-10	0.524	-563 lbs	- 1
	2-16	0.218	-1,096 lbs		4-14	0.582	4,702 lbs	(-544 lbs)	7-11	0.410	-1.935 lbs		/ 10	COLIT	500 108	
	3-16	0.293	1,014 lbs	(-197 lbs)	6-14	0.442	-2,416 lbs	1	8-11	0.381	1,319 lbs	(-131 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

Page: 2 of 2				Valley Tr 133 Range View Westcliffe, CO Phone (719) 37	w Loop 81252	Joh De Da	signer: Shane Allen ne: 09/12/24 07:5	-HIESCO COMPLETA 9:28
1 44-54-8 7/17 1 7/100 000 000 000	34-9-8	2/12	2-10-8	0-0-0		 PLYS 1		WGT/PLY 377 lbs

be sheathed on one side or lateral bracing applied appropriately.

7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

8) A creep factor of 1.50 has been applied for this truss analysis.

9) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information. 10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 16 may need to be considered.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, totreated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Scab 3-18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

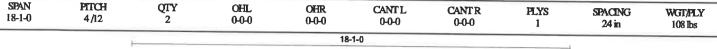
Truss: GE10

CBS0306SA-7-HIESCO COMPLETE Job:

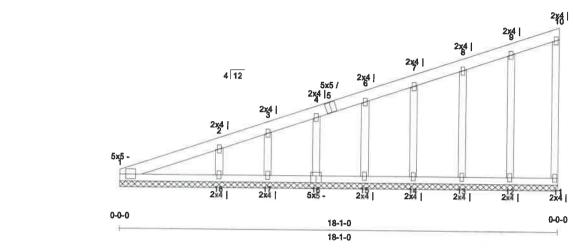
Designer: Shane Allen 09/12/24 07:59:13 Date:

Page: 1 of 2

09/12/202







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	I/	(loc)	Allowed
TCLL: 110 TCDL: 10	Bkdg Code: IBC 2018/ TPI 1-2014	TC: 0.59 (1-2) BC: 0.08 (18-1)	Vert'IL: 0 in Vert'LL: 0 in	L/999 L/999	(18-1) 11	L/240 L/360
BCLL: 0 BCDL: 10	Rep Mbr: No Lumber D.O.L.: 100 %	Web: 0.35 (10-11)	Horz TL: 0 in	2,,,,,	**	L/300

Reaction

66-1

Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
1		1,268 lbs	334 plf	-398 lbs	-88 lbs	-160 lbs	-398 lbs	800 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

Member Forces			Table	indicates: Member II	D, max CSL	max axial force, (r	nax comor, force i	f different fi	on max axial force	Only forces one	sterthon 201	Who am aboum i	n thin table
TC	1-2	0.592	971 lbs	(-261 lbs)						, only made git	MM 11441 JOH	was are allowed in	II WIIS LAIDIE,
BC													
Web	2-18 3-17	0.119	-935 lbs -612 lbs	4-16 6-15	0.098	-607 lbs -480 lbs	7-14 8-13	0.135	-486 lbs	9-12	0.229	-484 lbs	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 2x420g a plates, U.N.O.
 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 7) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE10

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:13

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 18-1-0 0-0-0 0.0000-0-0 0-0-0 24 in 108 lbs

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 1 may need to be considered.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

⁹⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

¹¹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

Valley Truss 133 Range View Loop Westcliffe, CO 81252

Phone (719) 371-8508

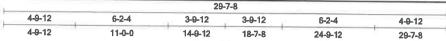
Truss: GE11

09/12/2024

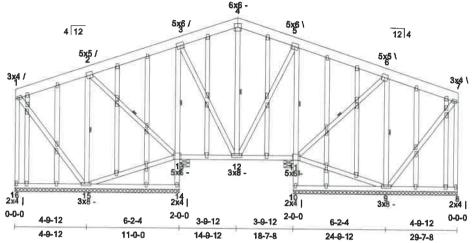
Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:14
Page: 1 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS 29-7-8 4/12 1 0-0-0 0-0-0 0-0-0 0-0-0 1	SPACING 24 in	WGT/PLY 397 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

T . 12 . / 6	0 1		I ave=	T-			
Loading (psf)	General		CSI	Deflection	I/	(loc)	Allowed
TCLL: 110	Bldg Code:	IBC 2018/	TC: 0.75 (2-3)		3 in L/999	(11-12)	L/240
TCDL: 10	-	TPI 1-2014	BC: 0.30 (8-9)		2 in L/999	12	L/360
BCLL: 0	Rep Mbr:	No	Web: 0.61 (5-11)		l in	8	11/300
BCDI 10	Limber DOI	· 100 %				U	

D	~		щ.	
Ų	2	ш	Щ	Ш

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
13	1	8 in	2.07 in	2,372 lbs		-179 lbs	-275 lbs	-275 lbs	
11	1	8 in	2.07 in	2,372 lbs		-179 lbs	-275 lbs	-275 lbs	. 1
14	1	132 in	N/A	96 lbs	•	*			-2 lbs
15	1	132 in	N/A	1,504 lbs		-52 lbs	-167 lbs	-167 lbs	-148 lbs
16	1	132 in	N/A	790 lbs	•	-53 lbs	-92 lbs	-92 lbs	103 lbs
8	1	132 in	N/A	789 lbs	•	-26 lbs	-92 lbs	-92 lbs	100 100
9	1	132 in	N/A	1,504 lbs		-52 lbs	-167 lbs	-167 lbs	. /
10	1	132 in	N/A	96 lbs		•		10, 133	. /

Material

TC: SP-FT/PG2400/1.8 2 x 8 BC: SP-FT/PG2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Puriins at 6-3-0, Puriin design by Others.
BC: Sheathed or Puriins at 10-0-0, Puriin design by Others.
Web: One Midpoint Row: 2-15, 2-13, 3-14, 4-12, 5-10, 6-11, 6-9

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1,60
- 3) This truss has been designed for the effects of TCLL=20 psf.
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber i	Forces	Table	indicates; M	lember II), max CS	, max axial force, (max compr. force	if different fr	om max axial '	force). Only fo	orces greater than 300lbs are shown in this table.
TC	1-2 2-3	0.693	-323 lbs 375 lbs	(-283 lbs)	34	0.644 0.644	-333 lbs	5-6	0.748 0.693	375 lbs -323 lbs	(-283 lbs)	
BC				(MOU MA)	,,,	0,011	933 108	10-7	0.093	-525 IUS		
Web	1-16	0.448	-756 lbs		4-12	0.227	-644 lbs	7-8	0.448	-756 lbs		
	2-15	0.444	-1,665 lbs		5-12	0.146	508 lbs	1				
	2-13	0.090	-300 lbs		5-11	0.613	-2,074 Ibs	- 1				
	3-13	0.613	-2,074 lbs		6-11	0.090	-300 lbs				- 1	
	3-12	0.146	508 lbs		69	0.444	-1,665 lbs	- 1				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq =0.90).

Truss: GE11

ob: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:15

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 29-7-8 4/12 0-0-0 0-0-0 0-0-0 0-0-0 24 in 397 lbs

Gable must be sheathed on one side or lateral bracing applied appropriately.

7) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.

8) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

9) A creep factor of 1.50 has been applied for this truss analysis.

10) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, BERCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAIL ABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GE12

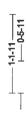
Job: CBS0306SA-7-HIESCO COMPLETE

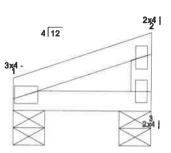
Designer: Shane Allen
Date: 09/12/24 07:59:15

200000	03/12/27
Page:	1 of 2

2-0-0 4/12 2 0-0-0 0-0-0 0-0-0 1 24 in 7 lbs	SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
	2-0-0	4/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	7 lbs

2-0-0 2-0-0 2-0-0





0-0-0 2-0-0 0-0-0 2-0-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General	CSI TC: 0.10 (1-2) BC: 0.09 (3-1) Web: 0.15 (2-3)	Deflection VertTL: 0 in VertLL: 0 in HozzTL: 0 in	L/ L/999 L/999	(loc) (3-1) (3-1) 3	Allowed L/240 L/360
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Reaction

·	Л	Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplif	t Max MWFRS Uplift N	Aax C&C Uplift	Max Uplift	Max Horiz
	1	1	5.5 in	240 lbs			-7 lbs	-69 lbs	-69 lbs	40 lbs
	3	1	5.5 in	280 lbs			-24 lbs	-102 lbs	-102 lbs	·

Material

TC:	SP-FT/PG 2400/1.8 2 x 4
BC:	SP-FT/PG 2400/1.8 2 x 4
Web:	SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads, in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 4) This truss has been designed for the effects of TCLL = 20 psf.

Men	nber Forces	Table indicates: Me	ember ID, max CSI, max axial force, (max comp	or, force if different from max axial force). Only f	broes greater than 300lbs are shown in this table.
TC				1	
BC					
Web					

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach gable webs with 2x420ga plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 6) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE12

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:16

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
2-0-0	4/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	7 lbs

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, BRECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

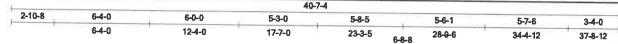
⁹⁾ All connectors, i.e. nails, sciews, bolls, trus s plates, etc. to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

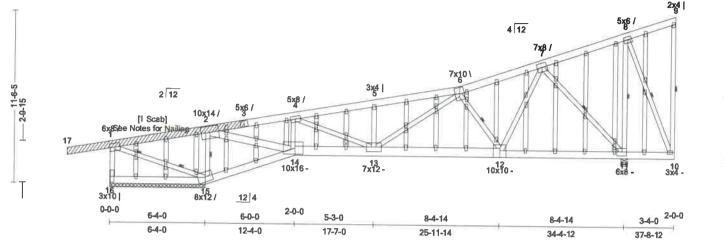
Truss: GE13

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:17
Page: 1 of 2

SPAN	PTTCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
37-8-12	2/12	2	2-10-8	0-0-0	0-0-0	0-0-0	1	24 in	412 lbs





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.94 (5-6) BC: 0.35 (12-13) Web: 0.78 (1-15)	Deflection L/ Vert TL: 0.38 in L/886 Vert LL: 0.29 in L/999 Cant/OH TL: 0.07 in UP 2L/999 Cant/OH LL: 0.05 in UP 2L/999	(loc) (12-13) (12-13) 10 10	Allowed L/240 L/360 2L/120 ^{2L} /09212/20
			Horz TI: 011 in	11	4 09712/20

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWPRS Uplift N	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	3.87 in	4,438 lbs		-307 lbs	-677 lbs	-677 lbs	
15	1	76 in	N/A	7,696 lbs	*	-426 lbs	-910 lbs	-910 lbs	400 lbs
16	1	76 in	N/A	1,428 lbs	-720 lbs	-127 lbs	17	-720 lbs	41 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 12-14
Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 7-11 SP-FT/PG 2400/1.8 2 x 6: 2-14 **Bracing**

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others.
 BC: Sheathed or Purlins at 4-7-0, Purlin design by Others.

Web: One Midpoint Row: 2-15, 6-12, 7-11, 8-11, 8-10, 9-10 Two Third Point Rows: 1-15

Scabs

17-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Table	indicates: M	ember ID	, max CS	, max axial for	e, (max comp	r, force	if different f	forn max axial	force). Only f	nmes one	ater than 30	Othe see chosen	in this table
TC	1-2	0.776	4,171 lbs	(-560 lbs)	4-5	0.871	-5,417 lbs	1	6-7	0.926	-3,539 lbs		l gra	and district DO	OLOS SEC SELVATI	IN MINS FORMS
-	2-4	0.791	-3,650 lbs		5-6	0.942	-5,436 lbs		7-8	0.921	401 lbs	(-166 lbs)				
BC	11-12	0.288	2,191 lbs	(-210 lbs)	13-14	0.268	3,048 lbs	(473 lbs)								
	12-13	0.352	4,418 lbs	(-553 lbs)	14-15	0.244	-3,974 lbs	` f								
Web	1-16	0.213	-1,382 lbs		4-14	0.407	-2,829 lbs		6-12	0.379	-2,439 lbs		8-10	0.095	328 Ibs	(-59 lbs)
	1-15	0.775	-4,270 lbs		4-13	0.735	2,547 lbs	(-226 lbs)	7-12	0.715	2,476 lbs	(-276 lbs)		0.655	-331 lbs	(-25)
	2-15	0.618	-4,677 lbs		5-13	0.317	-1,767 lbs		7-11	0.677	-3,327 lbs	(~.0100)	1 10	Group.	-001 108	
	2-14	0.579	7,357 Ibs	(-866 lbs)	6-13	0.452	1,567 lbs	(-349 Ibs)		0.538	-1,928 lbs					1

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach structural gable blocks with 2x4 20g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.

5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

Truss: GE13

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
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SPAN	PTICH 2/12	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
37-8-12		2	2-10-8	0-0-0	0-0-0	0-0-0	1	24 in	412 lbs

6) Gable must be sheathed on one side or lateral bracing applied appropriately.

7) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

8) A creep factor of 1.50 has been applied for this truss analysis.

9) Mindicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 16 may need to be considered.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Scab 3 - 17 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

Truss: GE14

CBS0306SA-7-HIESCO COMPLETE Job:

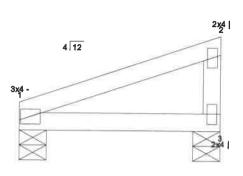
Designer: Shane Allen 09/12/24 07:59:18 Date: Page: 1 of 2

PTTCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	12 lbs

3-4-8



SPAN 3-4-8



0-0-0	3-4-8	0-0-0
	3-4-8	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	Conoral		CSI	D-G-4'	¥ /	a >	477 7
			1.0	Deflection	L/	(loc)	Allowed
TCLL: 110	Bldg Code:	IBC2018/	TC: 0.61 (1-2)	VertTL: 0.00	2 in L/999	(3-1)	L/240
TCDL: 10	-	TPI 1-2014	BC: 0.24 (3-1)	VertLL: 0.0		(3-1)	L/360
BCLL: 0	Reo Mbr:	No	Web: 0.55 (2-3)	Horz TL: Oin	-,,,,	(3-1)	L/300
BCDL: 10	Lumber DOL		(Z-5)	11012112 011	I.	3	00/12/

Reaction

T	Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWHRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
1	1	5.5 in	419 lbs	•		-15 lbs	-124 lbs	-124 lbs	69 lbs
3	1	5.5 in	459 lbs	•		-38 lbs	-166 lbs	-166 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 4 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 4) This truss has been designed for the effects of TCLL = 20 psf.

	mber	Forces	Table indicate	s: Member ID, max CSI, max axial force, (max comp	r. force if different from max axial force). Only f	orces greater than 300ths are shown in this table
TC	1-2	0.614	-448 lbs			
BC						
Web	2-3	0.552	-387 lbs			

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach gable webs with 2x420g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 6) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE14

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:18

Page:

SPAN 3-4-8	PTICH 4/12	QTY 1	OHL 0-0-0				1 - 0	1 - 0		
				OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 12 lbs	
7) Due to the inheren	nt corrosion of treated	lumber and steel cor	mectors and the num	ber of variables that	affect the rate of com	sion the G60 coating	special on			
metal plates shall be	reviewed and appropri	red by the Building D	ecioner consideries			Dibit, the Goo coming	provided our			

metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWPRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, BRECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

⁹⁾ All connectors, i.e nails, sciews, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

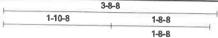
Truss: GE15

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:19

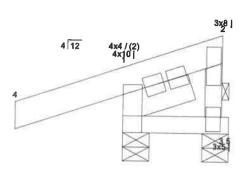
Page: 1 of 2

3	4/12	1	1-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 18 lbs	
				3-	8-8					_





SPAN 1-8-8



1-10-8	0-0-0	1-8-2-0	0-0-0 0-1-8
		1-8-8	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.85 (4-1) BC: 0.67 (5-3) Web: 0.36 (2-3)	Deflection L/ VertTL: 0.01 in UP L/ VertLL: 0.01 in UP L/ Cant/OHTL: 0 in UP 2L/ Cant/OHTL: 0 in UP 2L/ HorzTL: 0 in	999 (3-1) 1999 (1-1)	Allowed L/240 L/360 2L/120 2L/120
---	--	--	--	-------------------------	---

Reaction

JT	Brg Combo	Brg Width	Max React	Ave React	Max Grav Uplift	Max MWFRS Unit	ift Max C&C Uplift	May Unlift	Max Horiz
1	1	5.5 in	1,340 lbs	(*)		-90 lbs	-384 lbs	-384 lbs	
5	1	5.5 in	47 lbs		-407 lbs	-10 lbs	-204 ID2		126 lbs
					-TO1 103	-10 TD2		-407 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others. Web: One Midpoint Row: 2-3

1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 - 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed to account for the effects of ice dams forming at the eaves.

- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

5) This truss has been designed for the effects of TCLL=20 psf.

Me	mber	Forces	Table	indicates: Me	mber ID, max CSL max axial force, (max come	er from it different from more anial forms (Called	orces greater than 300ths are shown in this table.
TC	1-2	0.726	501 lbs	(334 Ibs)	, and the state of	I made it difficult from their axial forces. Only i	orces greater than 300 libs are shown in this table.
BC	3-1	0.179	-590 lbs				
Web	2-3	0.364	446 lbs	(41 lbs)			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable webs placed at 24 "OC, U.N.O.
- 3) Attach gable webs with 4x420g a plates, U.N.O.
- 4) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 5) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 6) Gable must be sheathed on one side or lateral bracing applied appropriately.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 8) A creep factor of 1.50 has been applied for this truss analysis.
- 9) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: GE15

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 07:59:19

Page: 2 of 2

SPAN PITCH QTY OHI. OHR CANTL CANTR PLYS SPACING WGT/PLY 1-8-8 4/12 1-10-8 0-0-0 0-0-0 0-0-0 1 24 in 18 lbs

10) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 5 may need to be considered.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

Truss: GR1

Job: CBS0306SA-7-HIESCO COMPLETE

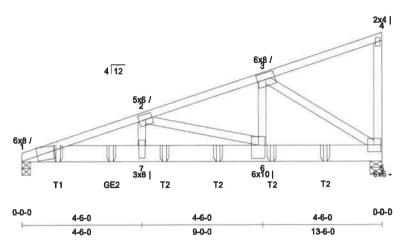
Designer: Shane Allen Date: 09/12/24 07:59:29

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SPAN	PTTCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
13-6-0	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	3	12.75 in	86 lbs
				12	60				







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	l	L	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC2018/	TC:	0.47 (1-2)	Vert TL:	0.21 in	L/711	(6-7)	L/240
TCLL: 110	_	TPI 1-2014	BC:	0.79 (7-1)	VertLL:	$0.17 \mathrm{m}$	L/8 7 9	(6-7)	L/360
TCDL: 10	Rep Mbr:	Yes	Web:	0.63 (3-6)	Horz TL:	0.05 in		5	
BCLL: 0	Lumber D.O.L	.: 100 %							09/17

BCDL: 10
Reaction

JT	Brg Combo	Brg Width	Rod Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift M	fax C&CUplift	Max Uplift	Max Horiz
1	1	4 in	1.87 in	6,437 lbs		-304 lbs		-304 lbs	150 lbs
5	1	5.5 in	1.58 in	5,433 lbs	*	-316 lbs	0.40	-316 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 4 BC: SP-FT/PG 2400/1.8 2 x 8 Web: SP-FT/PG#2 2 x 4 **Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	11-5-4	Down	Proj	21.25 plf	21.25 plf	
Top	11-5-4	13-6-0	Down	Proj	21.25 plf	0.62 plf	

Load Case D1: Std Dead Load

Distributed Loads

Distributed Load	S						
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	11-5-4	Down	Proj	10.63 plf	10.63 plf	
Top	11-5-4	13-6-0	Down	Proj	10.63 plf	0.31 plf	
Bot	0-0-0	11-5-4	Down	Proj	10.63 plf	10.63 plf	
Dot	11.5.4	12.6.0	Down	Deni	10.63 nH	031 ก็โร	

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only force greater than 300lbs are shown in this table.

TC	1-2	0.473	-4,961 lbs		2-3	0.246	-2,472 lbs		<u></u>				
BC	5-6	0.551	2,314 lbs	(-133 lbs)	6-7	0.496	4,691 lbs	(-260 lbs)	7-1	0.789	4,691 lbs	(-260 lbs)	
Web	2-7	0.369	1,278 lbs	(-58 lbs)	3-6	0.634	2,198 lbs	(-104 lbs)					
	2-6	0.349	-2,486 lbs		3-5	0.406	-2,774 lbs		1				

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss 133 Range View Loop

Westcliffe, CO 81252 Phone (719) 371-8508

OHR

0-0-0

CANTL

0-0-0

CANTR

0-0-0

Truss: GR1

3

Job: CBS0306SA-7-HIESCO COMPLETE

86 lbs

Designer: Shane Allen Date: 09/12/24 07:59:29

1 rago	2012		
PLYS	SPACING	WITHV	

12.75 in

Truss to Truss Connection Summary

PITCH

4/12

Carried Truss	Carrying Chord	Carrying Offset
T1	BC	144
GE2	BC	344
T2	BC	544
T2	BC	7-4-4
T2	BC	944
T2	BC	1144

Notes

SPAN

13-6-0

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

QTY

- The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Three identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ412, 3 - ply) Screws TC-1 row @ 1-10-8 oc, BC-2 staggered rows @ 0-9-4 oc, Webs-1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 2-0-0

OHL

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- 6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.
- 7) Lateral bracing shall be attached to each ply
- 8) Install screws per manufacturer recommendations.
- 9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 13) Incising is not permitted.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: GR2

Job: CBS0306SA-7-HIESCO COMPLETE Designer: Shane Allen

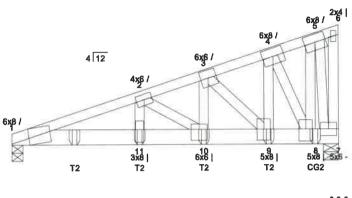
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SPAN	PTTCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
10-1-0	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	12.38 in	73 lbs

1.		10-1-0			
	3-11-4	2-0-0	2-0-0	1-5-4	0-8-8
	3-11-4	5-11-4	7-11-4	9-4-8	10-1-0





0-0-0	3-11-4	2-0-0	2-0-0	1-5-4	0-8-8	
1	3-11-4	5-11-4	7-11-4	9-4-8	10-1-0	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI	Deflection		L/	(loc)	Allowed
Carried Loads (psf)		IBC2018/	TC: 0.57 (1-2)		0.16 in	L/717	(11-1)	L/240
TCLL: 110		TPI 1-2014	BC: 0.86(11-1)		0.12 in	L/900	(11-1)	L/360
TCDL: 10	Rep Mbr:	No	Web: 0.70 (5-8)	Horz TL:	0.03 in		7	
BCLL: 0	Lumber D.O.L.	.: 100 %						09/12/2

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplift N	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.58 in	3,636 lbs	*	-207 lbs		-207 lbs	117 lbs
7	1	5.5 in	237 in	5.430 lbs		-383 lbs		-383 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 4 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 5-9-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terzain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-5-4	Down	Proj	20.63 plf	20.63 plf	
Top	7-5-4	8-8-4	Down	Proj	20.63 plf	8.12 plf	

Load Case D1: Std Dead Load

Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-5-4	Down	Proj	10.31 plf	10,31 plf	
Тор	7-5-4	8-8-4	Down	Proj	10.31 plf	4.06 plf	
Bot	0-0-0	7-5-4	Down	Proj	10.31 plf	10.31 plf	
Det	751	001	Dorrm	Doni	10.31 nH	4.06 plf	

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300ths are shown in this table.

	A PATE	MO-CA I		1111/11			,	,	.,					
1	TC	1-2	0.571	-4,368 lbs		3-4	0.152	-1,475 lbs						- 1
		2-3	0.301	-2,916 lbs		4-5	0.057	-553 Ibs						
i	BC	7-8	0.490	440 lbs	(-31 lbs)	9-10	0.393	2,757 lbs	(-170 lbs)	11-1	0.860	4,153 lbs	(-254 Ibs)	
		8-9	0.382	1,380 lbs	(-91 lbs)	10-11	0.448	4,153 lbs	(-254 lbs)					_ļ
1	Web	2-11	0.351	1,218 lbs	(-56 lbs)	3.9	0.245	-1,912 lbs		5-8	0.697	2,415 lbs	(-169 lbs)	- 1
		2-10	0.209	-1,651 lbs		4-9	0.606	2,101 lbs	(-119 lbs)	5-7	0.295	-2,250 lbs		1
		3-10	0.484	1.677 lbs	(-86 lbs)	4-8	0.270	-2,087 Ibs					- 1	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EACLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GR2

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:33

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SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
10-1-0	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	12.38 in	73 lbs	

Truss to Truss Connection Summary

Carying Chord	Carrying Offset
BC	1-11-4
BC	3-11-4
BC	5-11-4
BC	7-11-4
BC	948
	BC BC BC BC

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

4) A creep factor of 1.50 has been applied for this truss analysis.

5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ278, 2-ply) Screws TC-1 row @ 2-0-0 oc, BC-2 staggered rows @ 0-10-8 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120 x2.875] @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2-ply) Sciews as follows within 24 "of the location shown:

BC: 9-4-8,(8)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

7) Lateral bracing shall be attached to each ply

1) Lateral bracing shall be allached to each ply.
 8) Install screws per manufacturer recommendations,

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 11) All connectors, i.e. nails, screws, boils, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

Truss: GR3

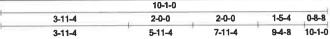
Job: CBS0306SA-7-HIESCO COMPLETE
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Designer: Shane Allen
Date: 09/12/24 07:59:34

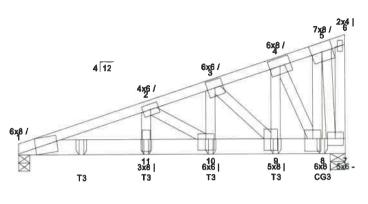
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09/12/2024

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
10-1-0	4/12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	12.7 in	73 lbs







0-0-0	3-11-4	2-0-0	2-0-0	1-5-4	0-8-8
	3-11-4	5-11-4	7-11-4	9-4-8	10-1-0

All plates shown to be Eagle 20 unless otherwise noted.

•	_							
Loading (psf)	General		CSI	Deflection	n	L	(loc)	Allowed
Carried Loads (psf)		IBC2018/	TC: 0.56 (1-2)	Vert TL:	0.15 in	L/725	(11-1)	L/240
TCLL: 110		TPI 1-2014	BC: 0.85 (11-1)	VertLL:	0.12 in	L/912	(11-1)	L/360
TCDL: 10	Rep Mbr:	No	Web: 0.70 (5-8)	Horz TL:	0.03 in		7	
BCLL: 0	Lumber D.O.I	⊥:100%						

BCDL: 10 Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift N	Max C&C Uplift	Max Uplift	Max Horiz
1	1	4 in	1.57 in	3,596 lbs		-232 lbs	16	-232 lbs	115 lbs
7	1	5.5 in	2.36 in	5,426 lbs		-405 lbs	3	-405 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 4 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 5-10-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	7-4-3	Down	Proj	20.63 plf	20.63 plf	
Тор	7-4-10	8-5-4	Down	Proj	21.17 plf	10.63 plf	

Load Case D1: Std Dead Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	7-4-3	Down	Proj	10.31 plf	10.31 plf	
Тор	7-4-10	8-5-4	Down	Proj	10.59 plf	5.31 plf	
Bot	0-0-0	7-4-3	Down	Proj	10.31 plf	10.31 plf	
Dot	7.4.10	8.5.4	Down	Pmi	10.59 nH	5.31 plf	

Member Forces Table indicates; Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.562 0.298	-4,319 lbs -2,883 lbs		3.4 4.5	0.150 0.057	-1,458 lbs -552 lbs						
BC	7-8	0.487	440 lbs	(-33 lbs)		0.389	2,725 lbs	(-188 lbs)	11-1	0.850	4,106 lbs	(-282 lbs)	
	8-9	0.381	1,363 lbs	(-100 lbs)	10-11	0.443	4,106 lbs	(-282 lbs)					
Web	2-11	0.348	1,204 lbs	(-65 lbs)	3-9	0.243	-1,892 lbs		5-8	0.698	2,418 lbs	(-180 lbs)	
	2-10	0.207	-1,634 lbs		4-9	0.600	2,079 lbs	(-134 lbs)	5-7	0.295	-2,249 lbs		,
	3-10	0.479	1.658 lbs	(-99 lbs)	4-8	0.266	-2,060 lbs		l .				1

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REPER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss

133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR3

Job: CBS0306SA-7-HIESCO COMPLETE

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SIPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
10-1-0	4/12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	12.7 in	73 lbs	

Truss to Truss Connection Summary

Canied Truss	Carrying Chord	Carrying Offset
T3	BC	1-11-4
T3	BC	3-11-4
T3	BC	5-11-4
T3	BC	7-11-4
CG3	BC	948

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ278, 2 ply) Screws TC-1 row @ 2-0-0 oc, BC-2 staggered rows @ 0-10-8 oc, Webs 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2-ply) Sciews as follows within 24" of the location shown:

BC: 9-4-8 (8) Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

7) Lateral bracing shall be attached to each ply.
8) Install screws per manufacturer recommendations.

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 11) All connectors, i.e. nails, screws, bolis, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

Valley Truss 133 Range View Loop

Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR4

CBS0306SA-7-HIESCO COMPLETE Job:

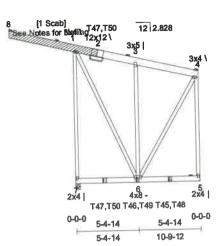
Designer: Shane Allen 09/12/24 07:59:36 Date:

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SPAN	PTICH -2.828/12	QTY	OHL 5-6-6	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 4	SPACING 24.63 in	WGT/PLY 149 lbs
10-9-12	-2.020/12	*							







All plates shown to be Eagle 20 unless otherwise noted.

All plates shown to be	Eagle 20 unless otherwise noted.					A 70 T
Loading (psf) Carried Loads (psf) TCL: 110 TCDL: 10	General	CSI TC: 0.92 (1-3) BC: 0.13 (5-6) Web: 0.45 (1-7)	VertTL: 0.04 in VertLL: 0.02 in U Horz TL: 0 in	L/ L/999 P L/999	(loc) (5-6) (5-6) 5	Allowed L/240 L/360

Reaction

BCDL: 10

Kez	ection		04000000000000000000000000000000000000	Mr. Denet	Most Come Unlift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
JT	Brg Combo	Brg Width	Rod Brg Width		Iviax Grav Opint	-3.233 lbs	-1.004 lbs	-3.233 lbs	-2601bs
7	1	4.778 in	1.50 in	4,315 lbs			-1,00-100	-1.754 lbs	
5	í	1.5 in	_	2,128 lbs	-757 lbs	-1,754 lbs		-11104 IDS	

Material

TC SP-FT/PG 2400/1.8 2 x 8

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4 Scabs

Bracing

TC. Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

8-2 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user Loads defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lrl: Std Live Load

Distributed Loads Member Top	Location 1 -5-6-6 -3-6-1 -5-6-6 -3-6-1 9-2-5 9-2-3	Location 2 -3-6-1 2-9-7 -3-6-1 2-9-5 10-9-12 10-9-12	Direction Down Down Down Down Down Down Down	Spread Proj Proj Proj Proj Proj Proj	0 plf 42.61 plf 0 plf 42.54 plf 20.49 plf 20.56 plf	End Load 42.61 pif 0 pif 42.54 pif 0 pif 42.7 pif 4.27 pif	Trib Width
--	--	---	---	--------------------------------------	--	---	------------

Load Case D1: Std Dead Load

Distributed Loads		T	Direction	Spread	Start Load	End Load	Trib Width
Member	Location 1	Location 2				21.31 plf	
Тор	-5-6-6	-3-6-1	Down	Proj	0 plf	-	
	-3-6-1	2-9-7	Down	Proj	21.31 plf	0 plf	
Тор			Down	Proj	0 plf	21.27 plf	
Тор	-5-6-6	-3-6-1	=		21.27 plf	0 plf	
Тор	-3-6-1	2-9-5	Down	Proj		2.13 plf	
	9-2-5	10-9-12	Down	Proj	10.24 plf		
Тор	9-2-3	10-9-12	Down	Proi	10.28 plf	2.13 plf	
Тор				Proj	10.24 plf	2.13 plf	
Bot	9-2-5	10-9-12	Down			2.13 plf	
Bot	9-2-3	10-9-12	Down	Proj	10.28 plf	2.15 pii	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTHIN THE FAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GR4

Job: CBS0306SA-7-HIESCO COMPLETE

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CDANT									
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
10-9-12	-2.828/12	1					ILIS		
10-7-12	-2.020/12	1	5-6-6	0-0-0	0-0-0	0-0-0	4	24.63 in	149 lbs

Men	nber	Forces	Table	indicates: M	ember 1	D, max CSI,	max axial for	e, ímax como	r. force	if different fo	om max axial for	e) Only forces o	greater than 300lbs are shown in this tab	la
TC	1-3	0.921	-372 lbs										Same and South areas with the fire fall	-
BC														-
	1-7	0.454	-1,078 lbs		3-6	0.203	704 Ibs	(-551 lbs)	4-5	0.114	-446 lbs			٦.
	1-6	0.222	533 lbs	(460 lbs)	4-6	0.135	516 lbs	(406 lbs)						ı

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T50	BC	2-6-5
T47	BC	2-6-7
T49	BC	544
T46	BC	54-6
T48	BC	8-2-3
T45	BC	8-2-5
T50	TC	2-6-5
T47	TC	2-6-7

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 5 mayneed to be considered.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Four identical trusses shall be built and attached as follows: SDS Simpson 0.250 'x6" (4-ply) Screws TC-2 staggered rows @ 0-11-8 oc, BC-2 staggered rows @ 20-0 oc, Webs 1 row @ 2-0-0 oc.
- 8) When applied loads are on one side of girder, do not fip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connectors spacing). Connectors on opposite sides of the girder shall be offset.
- 9) Lateral bracing shall be attached to each ply
- 10) Install screws per manufacturer recommendations.
- 11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 13) All connectors, i.e. nails, sorews, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 15) Incising is not permitted.
- 16) Scab 2 8 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Nails or Gun Nails [min 0.135 % 3"] @ 6 oc.
- 17) Listed wind uplift reactions based on MWFRS & C&Cloading.

Truss: GR6

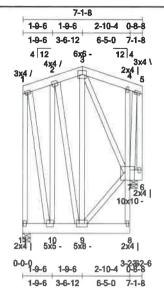
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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
7-1-8	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	11 in	138 lbs





5.2.6

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 110	Bildg Code:	IBC2018/	TC:	0.06 (2-3)	Vert TL:	0 in	L/999	(9-10)	L/240
TCDL: 10	_	TPI 1-2014	BC:	0.01 (9-10)	Vert LL:	0 in	L/999	(9-10)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.21 (1-11)	Cant/OHTL:	0 in UP	2L/999	6	2L/120
BCDL: 10	Lumber D.O.L.	: 100 %			Cant/OHLL:	0 in UP	2L/999	6	2L09202/201
					Howa TT +	Oin		7	_

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift N	fax C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.50 in	375 lbs	X.	-86 lbs	-93 lbs	-93 lbs	112 lbs
7	1	5.5 in	1.50 in	474 lbs	*	-89 lbs	-156 lbs	-156 lbs	8

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 8 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads, in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Pactored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60
- 4) This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. TC BC

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6" of each web panel point.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

Truss: GR6

CBS0306SA-7-HIESCO COMPLETE Job:

> WGT/PLY 138 lbs

Designer: Shane Allen

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING		
7-1-8	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	11 in		
The former charm for this multi-rilatines are not related to a fact of all alice. Two identical transact shall be built and attached as fallows. Two I are										

CANTE

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ278, 2 - ph) Screws TC - 2 staggered rows @ 2-0-0 oc, BC - 2 staggered rows @ 2-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"]

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply

9) Install screws per manufacturer recommendations.

10) Due to the infrarent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 12) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and
- 13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

14) Incising is not permitted.

CTDART

Valley Truss

133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR7

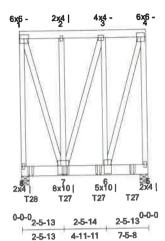
CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 07:59:38

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SPACING WGT/PLY PLYS CANTL CANTR OHR OHL QTY PITCH SPAN 116 lbs 7.375 in 0-0-0 θ 0-0-0 0-0-0 0/12 7-5-8

	7-5-8	
2-5-13	2-5-14	2-5-13
2-5-13	4-11-11	7-5-8



All plates shown to be Eagle 20 unless otherwise noted.

2 III PROGRAMME OF CO.				- /	~ \	A WI
Loading (psf) Carried Loads (psf) TCLL: 110 TCDL: 10	General Bldg Code: IBC2018/ TPI 1-2014 Rep Mbr: Yes Lumber DOL: 100 %	CSI TC: 0.06 (2-3) BC: 0.92 (7-8) Web: 0.47 (4-6)	VertTL: 0.06 in VertLL: 0.04 in Horz TL: 0 in	L/ L/999 L/999	(loc) (6-7) (6-7) 5	Allowed L/240 L/360

BCDL: 10

	action	D 337.14.	Rod Brg Width	Max React	Max Grav Unlift	Max MWIRS Uplift M	lax C&C Uplift	Max Uplift	Max Horiz
JT	Brg Combo	Brg Width 5.5 in	2.42 in	8.324 lbs	William Orac Opini	-642 lbs	::*!!	-642 lbs	90 lbs
8	1	5.5 in	2.18 in	7,509 lbs		-578 lbs	1950	-578 lbs	(i)

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 8 Web: SP-FT/PG#2 2 x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 pst) roof snow loads. in accordance with ASCE7 - 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60

Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Top	0-0-0	7-5-8	Down	Proj	12.29 plf	12.29 plf	

Load Case D1: Std Dead Load

Distributed to A

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	TribWidth
Тор	0-0-0	7-5-8	Down	Proj	6.15 plf	6.15 plf 6.15 plf	
Bot	0-0-0	7-5-8	Down	Proj	6.15 plf	0.12 bit	

Me	mber	Forces	Table	indicates: M	ember II	D, max CSI	, max axial forc	e, (max comp	n. force	if different for	om max axial force). Or	oly forces greater than 300fbs are shown in this table.
TC	1-2	0.046	-448 lbs		2-3	0.064	-448 lbs		34	0.045	-450 lbs	
BC	6-7	0.239	450 lbs	(-35 lbs)				44000 \	-			
Web	1-8	0.361	-1,611 lbs 1,609 lbs	(-124 Ibs)	4-6 4-5	0.467 0.363	1,617 lbs -1,618 lbs	(-125 lbs)				

Truss to Truss Connection Summary

Canied Truss	Carrying Chord	Carrying Offset
T28	BC	0-6-12
T27	BC	2-6-12
T27	BC	4-6-12
T27	BC	6-6-12

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GR7

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
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SPAN PITCH QTY OHL OHR CANT'L 7-5-8 0/12 1 0-0-0 0-0-0	CANTR	PLYS	SPACING	WGT/PLY
	0-0-0	3	7.375 in	116 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Three identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ412, 3 ply) Screws TC-2 staggered rows @ 2-0-0 oc, BC-2 staggered rows @ 2-0-0 oc, Webs 1 row @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ412, 3-ply) Screws as follows within 24" of the location shown:

BC: 0-6-12,(12)Connectors

BC: 2-6-12,(12)Connectors

BC: 4-6-12,(12)Connectors

BC: 6-6-12,(12)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

- 7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

 8) Lateral bracing shall be attached to each ply
- 9) Install screws per manufacturer recommendations.
- 10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 12) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 14) Incising is not permitted.
- 15) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: GR8

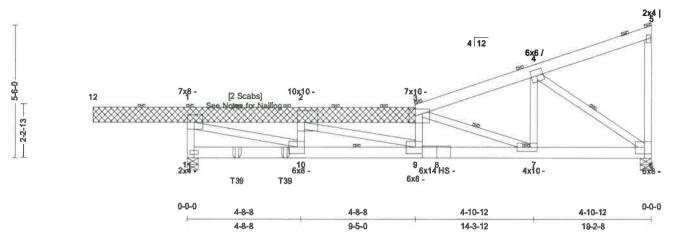
CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 07:59:40 Date

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1,20	0,12		5-10-0		-1-0			3011	133 103
SPAN 19-2-8	0/12	QIY 1	OHL 3-10-8	OHR 0-0-0	0-0-0	0-0-0	PLYS	SPACING 36 in	WGI/PLY 155 lbs

3-10-8 4-8-8 4-10-12 4-10-12 4-8-8 14-3-12 19-2-8 2-2-13



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC2018/	TC:	0.96 (3-4)	Vert TL:	0.3 in	L/743	9	L/240
TCLL: 110		TPI 1-2014	BC:	0.65 (7-9)	Vert LL:	0.23 in	L/969	9	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.83 (2-9)	Horz TL:	0.05 in		6	
BCLL: 0	Lumber D.O.J	L.: 100 %		, ,					
BCDL: 10									

Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Upl	ift Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
11	1	5.5 in	1.95 in	3,706 lbs		-254 lbs	-529 lbs	-529 lbs	182 lbs
6	1	5.5 in	1.45 in	2.748 lbs		-153 lbs	-311 lbs	-311 lbs	

Bracing

TC: Sheathed or Purlins at 4-7-0, Purlin design by Others.

BC: Sheathed or Purlins at 7-0-0, Purlin design by Others.

Web: One Midpoint Row: 3-7, 4-6

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 12-3

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-10

Scabs

12-3 [Qty: 2] SP-FT/PG 2400/1.8 2 x 8

Loads

- 1) This truss has been designed for the effects of balanced (110 pst) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-3-10-8	-3-1-11	Down	Proj	40 plf	40 plf	
Тор	-3-1-11	-0-11-4	Down	Proj	40 plf	40 plf	
Тор	5-0-12	19-2-8	Down	Proj	20 plf	20 plf	
Тор	-3-10-8	19-2-8	Down	Proi	20 plf	20 plf	

Load Case D1: Std Dead Load

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-3-10-8	-3-1-11	Down	Proj	20 plf	20 plf	
Тор	-3-1-11	-0-11-4	Down	Proj	20 plf	20 plf	
Тор	5-0-12	19-2-8	Down	Proj	10 plf	10 plf	
Тор	-3-10-8	19-2-8	Down	Proj	10 plf	10 plf	
Bot	5-0-12	19-2-8	Down	Proj	10 plf	10 plf	
Bot	0-0-0	19-2-8	Down	Proj	10 plf	10 plf	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss 133 Range View Loop

Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR8

CBS0306SA-7-HIESCO COMPLETE Joh:

Designer: Shane Allen Date: 09/12/24 07:59:40

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SPAN	PTTCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	36 in	155 lbs

Mer	nber .	Forces	Tabl	e indicates; M	ember	ID, max CSI	, max axial for	ce, (max comp	r, force	if different f	from max axial	force). Only fo	was greater than 300lbs are shown in this table.
TC	1-2	0.877	-2,983 lbs		34	0.964	-3,254 lbs		1				grand and sould so shown in this label.
	2-3	0.181	-5,706 lbs		4-5	0.962	-308 Ibs						
BC	6-7	0.261	2,864 lbs	(-271 lbs)	7-9	0.647	5,624 lbs	(-695 lbs)	9-10	0.439	2,983 lbs	(-1.790 lbs)	
Web	1-11	0.489	-3,679 lbs		2-9	0.825	2,859 lbs	(-369 lbs)	4-7	0.332	1,150 lbs	(90 lbs)	
	1-10	0.452	3,155 lbs	(-1,893 lbs)	3.9	0.118	-906 lbs		4-6	0.629	-3,522 lbs	,/	
	2-10	0.220	-975 lbs		3-7	0.485	-3,053 lbs		5-6	0.301	-657 lbs	- 1	j

Truss to Truss Connection Summary

Canied Trass	Carrying Chord	Carrying Offset
T39	BC	2-0-12
T39	BC	4-0-12

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information. 8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on
- metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 10) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 12) Incising is not permitted.
- 13) Scalo 3-12 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc Z(TSLZ278, 2-ply) Screws @ 6 oc, One Each Side, Stagger Rows on Opposing Sides.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: GR9

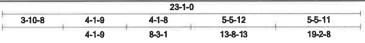
Job: CBS0306SA-7-HIESCO COMPLETE

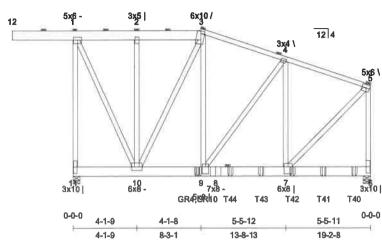
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Designer: Shane Allen
Date: 09/12/24 07:59:41

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SPACING WGT/PLY SPAN PITCH QTY OHL OHR CANTL CANTR PLYS 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 26.38 in 211 lbs





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	n	L/	(loc)	Allowed
Carried Loads (psf)		IBC2018/	TC:	0.89 (1-2)	Vert TL:	0.1 in	L/999	(7-8)	L/240
TCLL: 110		TPI 1-2014	BC:	0.50 (6-7)	VertI.L:	0.07 in	L/999	(7-8)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.69 (1-11)	Horz TL:	0.01 in		6	
BCLL: 0	Lumber D.O.L	: 100 %							00/12

BCDL: 10
Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
11	1	5.5 in	1.88 in	4,317 lbs		-1,504 lbs	-287 lbs	-1,504 lbs	-240 lbs
6	1	5.5 in	1.93 in	4.430 lbs		-1.479 lbs	190	-1.479 lbs	(4)

Material

TC: SP-FT/PG 2400/1.8 2 x 4 except: SP-FT/PG 2400/1.8 2 x 8: 12-3
BC: SP-FT/PG 2400/1.8 2 x 8

Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

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Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 4) Non-concurrent minimum storage affic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distribu	tedL	coads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-3-10-8	19-2-8	Down	Proj	20.21 plf	20.21 plf	
Top	-3-10-8	5-8-13	Down	Proj	19.79 plf	19.79 plf	
Top	5-9-1	7-1-9	Down	Proj	23.76 plf	10 plf	

Load Case D1: Std Dead Load

Distributed Loads

Distributed Loads							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	-3-10-8	19-2-8	Down	Proj	10.1 plf	10.1 plf	
Top	-3-10-8	5-8-13	Down	Proj	9.9 plf	9.9 plf	
Тор	5-9-1	7-1-9	Down	Proj	11,88 plf	5 plf	
Bot	0-0-0	19-2-8	Down	Proj	10.1 plf	10.1 plf	
Bot	0-0-0	6-1-13	Down	Proj	9.9 plf	9.9 plf	
Pot	6 1 12	710	Dorres	Desi	0.0 =16	5 -1F	

Mer	nber	Forces	Table	indicates: M	iember II), max CSI	l, max axial for	ce, (max comp	m. force if different from max axial force). Only forces greater than 300lbs are shown in this table.
TC	1-2	0.887	-516 lbs		34	0.404	-1,073 lbs		
	2-3	0.120	-522 lbs		4-5	0.422	-1,291 lbs		
BC	7-9	0.474	1,186 lbs	(499 lbs)	9-10	0.149	963 lbs	(-646 lbs)	

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: GR9

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
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SPA 19-2		PITCE 0/12	-	ТҮ 1		HL 10-8	OHR 0-0-0		NTL -0-0	CANTR 0-0-0	PLYS 2	SPACING 26.38 in	WGT/PLY 211 lbs	
Web 1-11 1-10 2-10	0.692 0.362 0.159	-2,116 lbs 1,253 lbs 553 lbs	(-785 lbs) 3-10 (-299 lbs) 4-9	0.420 0.264 0.123	-1,069 lbs 904 lbs 360 lbs	(-781 lbs) 5-7 (-353 lbs) 5-6	0.093 0.492 0.296	-383 lbs 1,704 lbs -1,757 lbs	(-700 lbs)					

Truss to Truss Connection Summary

Carrying Chord	Carrying Offset
BC	8-0-1
BC	8-1-9
BC	10-1-12
BC	12-1-12
BC	14-1-12
BC	16-1-12
BC	18-1-12
	BC BC BC BC BC

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows: TrussLoc-Z(TSLZ278, 2-ply) Screws TC-1 row @ 0-9-8 oc, BC-2 staggered rows @ 1-8-12 oc, Webs-1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 2-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach girder plies with supplemental TrussLoc - Z(TSLZ278, 2-ply) Sciews as follows within 24 "of the location shown:

BC 8-0-1.(7)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations,

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply

9) Install screws per manufacturer recommendations.

10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 12) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

14) Incising is not permitted.

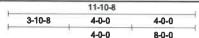
Truss: GR10

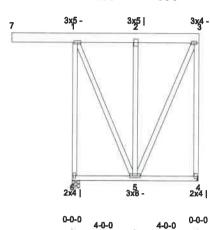
CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen 09/12/24 07:59:30 Date:

Page:	1 of 2
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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
8-0-0	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	2	24 in	107 lbs





4-0-0 8-0-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10	Bldg Code: IBC 2018/ TPI 1-2014		Deflection VertTL: 0.01 in VertLL: 0.01 in	L/ L/999 L/999	(loc) (4-5) 5	Allowed L/240 L/360
BCLL: 0	Rep Mbr: No	Web: 0.52 (3-4)	Horz TL: 0 in		4	
BCDL: 10	Lumber D.O.L.: 100 %	, ,				

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	2,424 lbs		-366 lbs	-567 lbs	-567 lbs	322 lbs
4	1	1.5 in		789 lbs	-322 lbs	-237 lbs	-160 lbs	-322 Ibs	

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 - 16 except as noted, with the following designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 - 16 except as noted, with the following designed for the effects of balanced (110 psf) roof snow loads. defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed to account for the effects of ice dams forming at the eaves.

- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

5) This truss has been designed for the effects of TCLL=20 psf.

6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces

TC	L				1	-, <u>-</u> ,		, quar our	A. AND II GIIILLUX ROSSI	HIN BALL KALLE CALL	y rorces greater treat	Source are allowed in this table.
BC												
Web	1-6	0.516	-1,195 lbs		34	0.520	-375 lbs				T	
	2-5	0.162	562 Ibs	(-199 lbs)					l.			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FACLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss 133 Range View Loop

Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR10

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:31

- 1	Page:	2 of 2	

3-10-8 0-00 0-00 0-00 2 24 in 10	SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PI
	8-0-0	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	2	24 in	107 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.

8) The forces shown for this multi-plytruss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply, 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 1 row @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 1-0-0 oc.

9) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 10) Lateral bracing shall be attached to each ply

11) All fasteners minimum 2-1/2" long, unless otherwise noted.

12) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

13) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

14) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 15) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 16) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

17) Incising is not permitted.

Valley Truss 133 Range View Loop

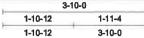
Westcliffe, CO 81252 Phone (719) 371-8508 Truss: GR11

CBS0306SA-7-HIESCO COMPLETE

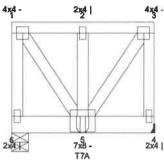
Designer: Shane Allen 09/12/24 07:59:31 Date:

Page: 1 of 2.

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
3-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	20.88 in	33 lbs







0-0-0 1-10-12 1-11-4 1-10-12 3-10-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CS	I	Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code: IBC	C2018/ TC:	0.07 (2-3)	Vert TL:	0.01 in	L/999	5	L/240
TCLL: 110	TP	I 1-2014 BC:	0.13 (5-6)	VertLL:	0.01 in	L/999	5	L/360
TCDL: 10	Rep Mbr: No) Web	: 0.27 (1-5)	Horz TL:	0 in		4	
BCLL: 0	Lumber D.O.L.: 100	0%						
BCDL: 10								09/12/2024

Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
6	1	5.5 in	1.50 in	1,966 lbs		-152 lbs		-152 lbs	89 lbs
4	1	1.5 in	_	1.930 lbs		-149 lbs	*	-149 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 4 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) roof snow loads, in accordance with ASCE7 - 16 except as noted, with the following under defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	3-4-15	Down	Proj	34.79 plf	34.79 plf	
Top	3-4-15	3-10-0	Down	Proj	34.79 plf	34.79 plf	

Load Case D1: Std Dead Load

TAISH IDNIED TONES							
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	3-4-15	Down	Proj	17.4 plf	17.4 plf	
Тор	3-4-15	3-10-0	Down	Proj	17.4 plf	17.4 plf	
Bot	0-0-0	3-10-0	Down	Proj	17.4 plf	17.4 plf	

Member Forces Table indicates: Member IID, max CSI, max axial force, (max compr. force if different from max axial force). Only force greater than 300lbs are shown in this table.

IC	11-2	0.065	-522 lbs	2-3	0.008	-522 lbs		
BC	di —							
Web	1-6	0.114	-888 lbs	3-5	0.267	926 lbs	(-73 lbs)	
	1-5	0.272	941 Ibs	(-75 lbs) 3-4	0.111	-872 Ibs	Į.	

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
T7A	BC	1-10-12

Truss: GR11

Job: CBS0306SA-7-HIESCO COMPLETE

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SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
3-10-0	0/12	1	0-0-0	0-0-0	0-0-0	0-0-0	2	20.88 in	33 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).

3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.

4) Provide adequate drainage to prevent ponding.

5) Brace bottom chord with approved sheathing or purins per Bracing Summary.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135'x3"] TC-1 row @ 0-10-0 oc, BC-2 staggered rows @ 1-0-0 oc, Webs-1 row 10d Nails or Gun Nails [min 0.120'x2.875"] @ 1-0-0 oc.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 12d Nails or Gun Nails [min 0.135"x3"] as follows within 24" of the location shown:

BC 1-10-12, Ensure hanger fasteners engage all plies

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

9) Lateral bracing shall be attached to each ply.

10) All fasteners minimum 2-1/2" long, unless otherwise noted.

11) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

12) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

13) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

14) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

15) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

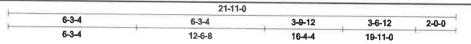
16) Incising is not permitted.

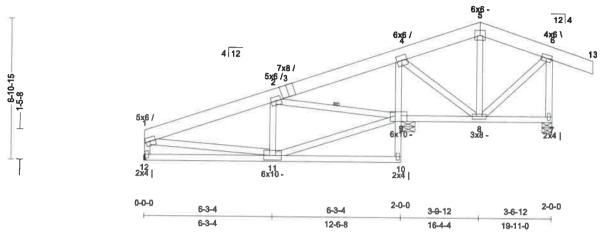
Truss: T1

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Date: 09/12/24 07:59:56
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PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGTPLY 4/12 0-0-0 2-0-0 0-0-0 0-0-0 1 24 in 160 lbs





All plates shown to be Eagle 20 unless otherwise noted.

TCDL: 10 BCLL: 0	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.80 (1-2) BC: 0.37 (11-12) Web: 0.61 (11-9)	VertTL: 0.1 in VertLL: 0.06 in HorzTL: 0 in	L/ L/999 L/999	(loc) (10-11) 11 7	Allowed L/240 L/360
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Reaction

SPAN

19-11-0

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	Max MWFRS Uplift!	Max C&CUplift	Max Uplift	Max Horiz
12	1	1.5 in	_	1,726 lbs		-57 lbs	-184 lbs	-184 lbs	142 lbs
9	1	8 in	3.20 in	3,670 lbs	*	-194 lbs	-382 lbs	-382 lbs	
7	1	5.5 in	1.52 in	1,738 lbs	300	-111 lbs	-234 lbs	-234 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.
 BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-9

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	nber	Forces	Table	indicates: M	amber I	D, max CSI	, max axial for	ce, (max comp	or. force	if different t	iom max axial i	force). Only fo	orces greater than 300lbs are shown in this table.
TC	1-2	0.796	-2,310 lbs		4-5	0.673	-423 lbs		1				grand and south the state state of
	2-4	0.776	728 Ibs	(-34 lbs)	5-6	0.582	-420 lbs						
BC	8-9	0.156	-378 lbs										
Web	1-12	0.207	-1,672 lbs		2-9	0.479	-2,420 lbs		4.8	0.189	657 lbs	(-8 Ibs)	
	1-11	0.587	2,032 lbs	(-132 lbs)	11-9	0.607	2,104 lbs	(-236 lbs)	5-8	0.147	-475 lbs	(0.00)	
	2-11	0.149	-852 lbs		4-9	0.538	-2,509 lbs	` [6-7	0.325	-1,708 lbs		

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST. DESIGN VALID ONLY WHEN FAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: T1

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 07:59:56

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SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-11-0	4/12	1	0-0-0	2-0-0	0-0-0	0-0-0	1	24 in	160 lbs
(1) Dun to 4 - 1-1-	A C								

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details,

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

14) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹¹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

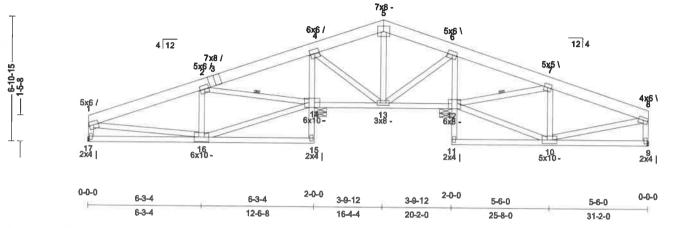
Truss: T2

09/12/2024

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:11 Date: Page: 1 of 2

)	4/12	QIY 4	OHL 0-0-0	OHR 0-0-0	CANTIL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 237 lbs
				31-2	-0				
	6-3-4	4	6-3-4	3-9-12	3-9-12	5-6-0	7	5-6-0	
	6-3-4	4	12-6-8	16-4-4	20-2-0	25-8-0	-	31-2-0	



All plates shown to be Eagle 20 unless otherwise noted,

T 35 (0			T const					
Loading (psf)	General		CSI	Deflection		I/	(loc)	Allowed
TCLL: 110	Bldg Code:	IBC2018/	TC: 0.80 (1-2)	Vert TL:	0.1 in	L/999	(15-16)	L/240
TCDL: 10		TPI 1-2014	BC: 0.32(16-17)	VertLL:	0.06 in	L/999	16	L/360
BCLL: 0	Rep Mbr:	Yes	Web: 0.60 (16-14)	Horz TL:	0.01 in	-,,,,,	9	2,500
BCDL: 10	Lumber DOI	· 100 %	(, ,				-	

SPAN

31-2-0

Л	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWPRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
17	1	1.5 in	_	1,706 lbs		-89 lbs	-209 lbs	-209 lbs	44 lbs
14	1	8 in	3.17 in	3,639 lbs	•	-167 lbs	-356 lbs	-356 lbs	(0)
12	1	8 in	2.82 in	3,240 lbs		-145 lbs	-306 lbs	-306 lbs	
9	1	1.5 in	_	1,595 lbs	•	-801bs	-190 lbs	-190 lbs	

Material

TC: SP-FT/PG2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-14, 7-12

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60
- 3) This truss has been designed for the effects of TCLL = 20 psf.
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

M	lembe	er F	orces	Table	indicates: M	enber II	D, max CSI	max axial foro	e, (max compr. fo	rce if d	ifferent fi	om max axial :	force). Only fo	proes greater than 300lbs are shown in this table.
TC	1-2	2	0.797	-2,270 lbs		4-5	0.673	-320 lbs	16-7		0.674	558 lbs	(-7 lbs)	
	2-4	1	0.777	808 Ibs		5-6	0.625	-403 lbs	7-8		0.742	-1,748 lbs	(, 25)	
BC	12-	-13	0.160	-313 lbs										
	13-	-14	0.160	-453 lbs										
We	b 1-1	17	0.205	-1,653 lbs		4-14	0.533	-2,486 lbs	7-1	2 (0.308	-1,795 lbs		
	1-1		0.575	1,993 lbs	(-184 lbs)	4-13	0.189	654 lbs	12-		0.456	1.580 lbs	(-109 Ibs)	
	2-1		0.146	-833 lbs		5-13	0.193	-622 lbs	7-1	0 (0.166	-859 Ibs	(
	2-1		0.486	-2,458 lbs		6-13	0.144	498 Ibs	8-1	0 (0.446	1.544 Ibs	(-138 lbs)	•
	16-	-14	0.596	2,065 lbs	(-182 lbs)	6-12	0.466	-2,173 lbs	8-9	1	0.197	-1,548 lbs	(/	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90),
- 3) Hangers are for graphical interpretation only Install hangers per manufacturer's recommendations.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At leastone web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 1252		Job	igner: Shane Allen :: 09/12/24 08:0	7-HIESCO COMPLET 10:11
SPAN	PTTCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
31-2-0	4/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	237 lbs

⁹⁾ Due to the inherent corrosion of treated lumber and sizel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) Incising is not permitted.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹¹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

¹²⁾ Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

¹⁴⁾ Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T3

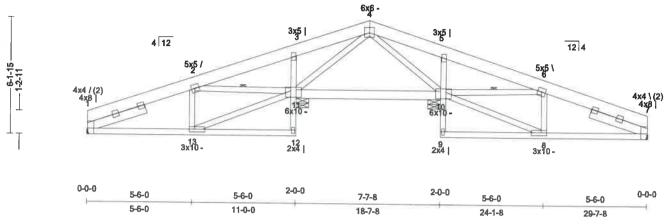
29-7-8

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:27 Page: 1 of 2

SPAN 29-7-8	PITCH 4/12	QTY 4	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 219 lbs
				29-	7-8				
	5-6-0	04	5-6-0	3 -9 -12	3-9-12	5-6-0		5-6-0	
	5-6-0		11-0-0	14-9-12	18-7-8	24-1-8	-	20.7.9	

24-1-8



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10		TC: 0.73 (2-3)	VertTL: 0.1 in VertLL: 0.05 in Horz TL: 0.02 in	L/ L/724 L/999	(loc) (10-11) (10-11) 7	Allowed L/240 L/360
---	--	----------------	---	----------------------	----------------------------------	---------------------------

Read	tion

SPAN

PITCH

Л	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift 1	Max C&C Uplift	Max Uplift	Max Horiz
1	1	1.5 in	_	1,577 lbs		-93 lbs	-194 lbs	-194 lbs	29 lbs
- 11	1	8 in	2.90 in	3,328 lbs		-136 lbs	-310 lbs	-310 lbs	:(*/)
10	1	8 in	2.90 in	3,328 lbs	•	-136 lbs	-310 lbs	-310 lbs	(4)
7	1	1.5 in	~	1,577 lbs		-93 lbs	-194 lbs	-194 lbs	267

Material

TC: SP-FT/PG 2400/1,8 2 x 8 SP-FT/PG 2400/1.8 2 x 4 except: SP-FT/PG 2400/1.8 2 x 6: 10-11 Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-11, 6-10

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 3) This truss has been designed for the effects of TCLL=20 psf.
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	mber	Forces	Table	indicates: M	lember II	D, max CS	L, max axial foro	e, (max com	or. force	if different t	from max axial	force). Only f	Ornes ene	aterthan 30	Who are shown	in this table
TC	1-2	0.528	-2,019 lbs		3-4	0.640	456 lbs	(-13 lbs)	56	0.732	618 lbs		I		ALCO BEACHT	III UIIO GELIEL
_	2-3	0.732	618 lbs		4-5	0.640	456 lbs	(-13 lbs)	6-7	0.528	-2.019 lbs					
BC	7-8	0.270	1,836 lbs	(-163 lbs)							4019 100					
	13-1	0.270	1,836 lbs	(-163 lbs)												
Web	2-13	0.090	-601 lbs		3-11	0.287	-1.833 lbs		5-10	0.287	-1.833 lbs		6-8	0.090	-601 lbs	
	2-11	0.362	-2,139 lbs		4-11	0.298	-670 lbs		6-10	0.362	-2,139 lbs		اس	0.050	-001 100	ı
	13-11	0.568	1,970 lbs	(-175 lbs)	4-10	0.298	-670 lbs		10-8	0.568	1 970 lbs	(175 lbc)	1			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hangers are for graphical interpretation only. Install hangers per manufacturer's recommendations,
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) At leastone web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALLD ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: T3

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:28

Page: 2 of 2

0) Day 4 - 1 - 1		*	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	219 lbs
SPAN 29-7-8	PTTCH 4/12	QTY	OHIL 0-0-0	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

¹¹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

Truss: T4

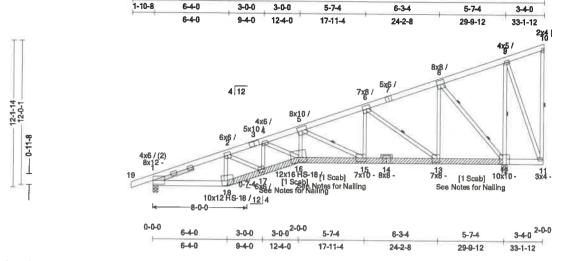
CBS0306SA-8-HIESCO COMPLETE Job:

5

Designer-Shane Allen Date: 09/16/24 08:09:09 Page: 1 of 2

09/16/2024

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
33-1-12	4/12	70	1-10-8	0-0-0	000	000	1	24 in	283 lbs
				35	5-0-4				



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110	General Bldg Code:				Deflection Vert TL:	0.68 in	L/ L/510	(loc) (15-16)	Allowed L/240
TCDL: 10				0.73 (18-1)	Vert LL:	0.54 in	L/652	16	L/360
BCLL: 0 BCDL: 10	Rep Mbr:	Yes	Web:	0.79 (5-16)	Cant / OHTL			11	2L/120
BCDL: 10	Lumber D.O.L.:	100 %			Cant/OHLL	0.09 in UP 0.19 in	2L/853	11	2L/120

Reaction Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift JT Brg Combo Brg Width Rqd Brg Width Max React Max Horiz 5.5 in 4,597 lbs 4.01 in -193 lbs -517 lbs -517 lbs 480 lbs 5.5 in 12 2.52 in 4,777 lbs -371 lbs -734 lbs -734 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FI/PG2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 5-15, 6-13, 8-12 Scabs

Bracing

TC: Sheathed or Parlins at 2-4-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 5-15, 6-13, 9-12, 10-11

Two Third Point Rows: 8-12

12-14 [Qty:1] SP-FT/PG 2400/1.8 21/466 [Qty:1] SP-FT/PG 2400/1.8 21/668 [Qty:1] SP-FT/PG 2400/1.8

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the caves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TC LL = 20 psf.

Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	gber 1	Force:	S Tabl	e indicates: M	ember ID	mex CSI,	max axial force	(max compr.	kweif d	iliaani from	max axial force	Only forces	greater than 300lbs are shown in this table.
TC	1-2	0.726	-8,285 lbs		4-5	0.657	-11,011 lbs		6-8	0.769	-2.978 lbs	,	
	2-4	0.623	-10,369 lbs		5-6	0.743	-6,649 lbs				-,		1
BC	12-13	0.189	2,601 lbs	(-209 lbs)	15-16	0.573	10,167 lbs	(-1,195 lbs)	17-18	0.376	7.972 lbs	(-985 lbs)	
-	13-15	0.264	6,141 lbs	(-658 lbs)	16-17	0.575	10,189 lbs	(-1,223 lbs)	18-1	0.731	7,659 lbs	(-943 lbs)	
Web	2-18	0.376	-2,543 lbs		5-16	0.793	2,746 lbs	(-277 lbs)	8-13	0.737	2,555 lbs	(-227 lbs)	
	2-17	0.744	2,578 lbs	(-259 lbs)	5-15	0.757	-5,489 lbs	, ,	8-12	0.712	-4.124 lbs	(-,-,-,	
	4-17	0.254	-1,709 lbs		6-15	0.766	2,655 lbs	(-203 lbs)	9-12	0.481	-1,412 lbs		
	4-16	0.349	1,211 lbs	(-83 lbs)	6-13	0.788	-4.346 lhs	` ′			-,		

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 1.50 has been applied for this truss analysis.
- 6) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Indicates non-structural members.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: T4

Job: CBS0306SA-8-HIESCO COMPLETE

Designer Shane Allen Date: 09/16/24 08:09:09 Page: 2 of 2

SPAN 33-1-12	PITCH 4/12	QTY 70	OHL 1-10-8	OHR 0-0-0	CANTL 000	CANT R 0-0-0	PLYS	SPACING	WGT/PLY
			1 10 0	V-0-0	0-0-0	000	1	24 in	283 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolks, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 12 - 14 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 14) Scab 14 - 16 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

15) Scab 16-18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

16) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

OHR

0-0-0

Truss: T4

Job: CBS0306SA-7-HIESCO COMPLETI

298 lbs

100

2-0-0

Designer: Shane Allen
Date: 09/12/24 08:00:42

24 in

	1 age.	1012	
CANTR	PLYS	SPACING	WGT/PLY

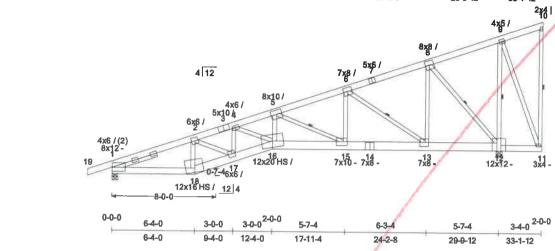
1

				33-0-4			
1-10-8	6-4-0	3-0-0	3-0-0	5-7-4	6-3-4	5-7-4	3-4-0
	6-4-0	9-4-0	12-4-0	17-11-4	24-2-8	29-9-12	33-1-12
							2 ₃

CANTL

0-0-0

0-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Horz TI: 0.24 in 12	Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: IBC2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.85 (19-1) BC: 0.88 (16-17) Web: 0.86 (8-12)	Deflection L/ Vert TL: 0.77 in L/451 Vert LL: 0.61 in L/576 Cant/OHTL: 0.14 in UP 2L/542 Cant/OHTL: 0.11 in UP 2L/685 Cant/OHTL: 0.11 in UP 2L	(loc) Allowed (15-16) L/240 16 L/360 11 2L/120 11 2L/120
---------------------	---	--	--	--	--

Reaction

SPAN

33-1-12

12-1-14

PITCH

4/12

QTY

OHL

1-10-8

Л	Brg Combo	Brg Width	Rod Brg Width	Max React	Max Grav Uplift Max	MWFRS Up	ift Max C&CUplift	Max Uplift	Max Horiz
1	1	5.5 in	4.00 in	4,583 lbs	3	-191 lbs	-514 lbs	-514 lbs	480 lbs
12	1	5.5 in	2.53 in	4,800 lbs		-372 lbs	-738 lbs	-738 lbs	. 6

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 8 except:

SP-FT/PG 2400/1.8 2 x 6: 11-12, 18-1

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 5-15,6-13,8-12

Bracing

TC: Sheathed or Purlins at 2-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 5-15, 6-13, 8-12, 9-12, 10-11

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber i	Forces	Table	indicates: M	anbar II), max CS	L max axial for	ce, (max com	or, florce i	f different i	iom max axial	force), Only fi	orces greater than 300lbs are shown in this	e toble
TC	1-2	0.726	-8,228 lbs		45	0.667	-11,181 lbs		6-8	0.769	-2,914 lbs		Bonne and pooled and any in in any	BEEL/MEA
7	2-4		-10,577 lbs		5-6	0.749	-6,648 lbs		8-9	0.714	314 lbs	(-158 lbs)		
BC	12-13	0.250	2,541 lbs	(-200 lbs)		0.869	10,267 lbs	(-1,200 lbs)	17-18	0.575	8,156 lbs	(-1,005 lbs)		_
	13-15	0.391	6,142 lbs	(-651 lbs)	16-17	0.876	10,387 lbs	(-1,236 lbs)	18-1	0.741	7,606 lbs	(-930 lbs)		
Web	2-18	0.384	-2,596 lbs	8	5-16	0.806	2,792 lbs	(-282 lbs)	8-13	0.736	2,552 lbs	(-226 lbs)		_
	2-17	0.740	2,565 lbs	(-250 lbs)	5-15	0.765	-5,587 Ibs		8-12	0.865	-4,109 lbs	(
	4-17	0.237	-1,652 lbs		6-15	0.771	2,671 Ibs	(-204 lbs)	9-12	0.480	-1,427 lbs			-
	4-16	0.335	1,162 lbs	(-80 lbs)	6-13	0.785	-4,397 lbs		L					

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T4

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:42

Page: 2 of 2

WGT/PLY

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING 33-1-12 4/12 70 1-10-8 0-0-0 0-0-0 0-0-0 24 in 298 lbs 7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on

metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

9) All connectors, i.e. nails, screws, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

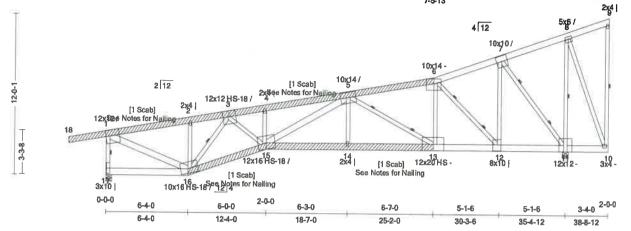
Truss: T5

CBS0306SA-8-HIESCO COMPLETE Job:

를

Designer:Shane Allen 09/16/24 08:09:14 Date: Page: 1 of 2

SPAN 38-8-12	PITCH 2/12	QTY 20			OHL OHR 2-10-8 0-0-0		CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 353 lbs
					41	-7-4				
	2-10-8	6-4-0	3-0-10	2-11-6	6-3-0	6-7-0	5-1-6	5-1-6	3-4-0	
		6-4-0	9-4-10	12-4-0	18-7-0	25-2-0	7-5-13 30-3-6	35-4-12	38-8-12	
Ť							-		2x4	т.



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: Rep Mbr: Lumber D.O.L.:	IBC 2018/ TPI 1-2014 Yes 100 %	BC:	0.86 (6-7) 0.55 (14-15) 0.84 (6-13)	Deflection Vert TL: Vert LL: Cant / OHTL Cant / OHLL	0.77 in 0.62 in : 0.12 in UP		(loc) (14-15) (14-15) 10	Allowed L/240 L/360 2L/120
DCDE: 10	Dillion D.O.L.	100 76			Horz TL:	0.1 in UP 0.25 in	2L/768	10 11	2L/120

Reaction

TT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	4.99 in	5,725 lbs	41	-283 lbs	-723 lbs	-723 lbs	443 lbs
11	1	5.5 in	2.88 in	5,459 lbs	63	-394 lbs	-859 lbs	-859 lbs	(2)

Bracing

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG#2 2x 6: 5-15

SP-FT/PG 2400/1.8 2 x 4: 3-16, 3-15, 5-13, 6-12, 7-12, 7-11

SP-FT/PG 2400/1.8 2 x 6: 1-16 Scabs

15-16 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Two Third Point Rows: 5-13, 6-12, 7-11

TC: Sheathed or Purlins at 5-4-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-17, 3-16, 8-11, 9-10

18-3 [Qty:1] SP-FT/PG 2400/1.8 2x86 [Qty:1] SP-FT/PG 2400/1.8 218-65 [Qty:1] SP-FT/PG 2400/1.8 2x 6

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TC LL= 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Tab	le indicates: M	anber ID	, max CSI,	max axial force	, (max compr.	force if d	lifficient from	max axial force	e). Only forces	enester (han 300lbs	are shown in th	e table
TC	1-2	0.591	-6,728 lbs		4-5	0.578	-14,015 lbs		17-8	0.834	380 lbs	(-155 lbs)				D MADELLA
	2-3	0.317	-6,757 lbs		5-6	0.578	-6,776 lbs					(200 200)				
_	3-4	0.417	-13,976 lbs		6-7	0.858	-3,190 lbs									
BC	11-12	0.374	2,844 lbs	(-274 lbs)	13-14	0.451	11,191 lbs	(-1,355 lbs)	15-16	0.522	10.502 lbs	(-1,383 lbs)				_
	12-13	0.534	6,469 lbs	(-790 lbs)	14-15	0.547	11,191 lbs	(-1,355 lbs)	16-17	0.155	-413 lbs	(1,000 110)				
Web	1-17	0.730	-5,673 lbs		3-15	0.608	4.917 lbs	(543 lbs)	6-13	0.843	2,923 lbs	(-268 lbs)	8.11	0.630	-1.852 lbs	
	1-16	0.564	7,164 lbs	(-657 lbs)	4-15	0.230	-1,406 lbs	(0.000)	6-12	0.759	-5,486 lbs	·	8-10	0.102	354 lbs	(-50 lbs)
	2-16	0.317	-1,328 lbs		5-15	0.616	2,946 lbs	(421 lbs)		0.486	3,926 lbs	(439 lbs)		0.721	-365 lbs	(-301105)
	3-16	0.723	-5,886 lbs	l l	5-13	0.835	-5,461 lbs	,	7-11	0.811	-4.836 lbs	(102 100)	7 10	0.721	500 108	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			7	Valley Tru 33 Range View Westcliffe, CO 8 hone (719) 371	Loop 31252		Job:	mer:Shane Allen 09/16/24 08:0	8-HIESCO COMPLE 19:14
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
38-8-12	2/12	20	2-10-8	0-0-0	0-0-0	000		24 in	353 lbs

⁷⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

⁹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

¹²⁾ Scab 3 - 18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

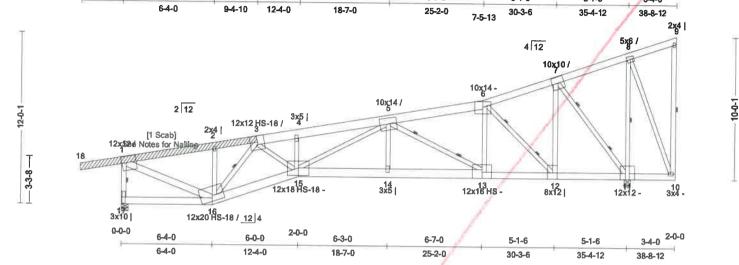
¹²⁾ Scab 3 - 6 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-pty) Screws @ 6 oc.
14) Scab 13 - 15 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-pty) Screws @ 6 oc.
15) Scab 15 - 16 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-pty) Screws @ 6 oc.
15) Scab 15 - 16 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-pty) Screws @ 6 oc.

Truss: T5

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:56 Page: 1 of 2

SPAN 38-8-12	PITCH 2/12	QTY 20	OHL 2-10-8		OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 373 lbs
					41	-7-4				
	2-10-8	6-4-0	3-0-10	2-11-6	6-3-0	6-7-0	5-1-6	5.1.6	3.4.0	fi.



All plates shown to be Eagle 20 unless otherwise noted.

CSI TCL: 110 Bldg Code: IBC2018/ TC: 0.92 (6-7) BCL: 0 BCDL: 10 Rep Mbr: Yes Lumber D.O.L.: 100% Web: 0.84 (6-13)	Deflection L/ (loc) Allowed VertTL: 1.03 in L/403 (14-15) L/240 VertLL: 0.83 in L/504 (14-15) L/360 Cant/OHTL: 0.17 in UP 2L/443 10 2L/120 Cant/OHLL: 0.14 in UP 2L/547 10 2L/120 Horz TI: 0.36 in 11 00.11
---	---

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift Max	MWFRS Uplif	Max C&CUblift	Max Unlift	Max Horiz
17	1	5.5 in	4.99 in	5,725 lbs		-283 lbs	-723 lbs	-723 lbs	444 lbs
- 11	1	5.5 in	2.88 in	5.459 lbs	. /	-304 lbs	-850 lbs	950 Iba	777 103

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 3-6

BC: SP-FT/PG 2400/1.8 2 x 6 except:

SP-FT/PG 2400/1.8 2 x 8: 13-15, 15-16 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 3-16, 3-15, 5-15, 5-13, 6-12, 7-12, 7-11 SP-FT/PG 2400/1.8 2 x 6: 1-16

Bracing TC: Sheathed

BC: Sheathed or Purlins at 9-6-0, Purlin design by Others.

Web: One Midpoint Row: 1-17, 3-16, 8-11, 9-10 Two Third Point Rows: 5-13, 6-12, 7-11

Scahs

18-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber l	Forces	Table	indicates: M	ember ID	, mex CS	I, max axial for	ce, (max com	or. force i	f different t	from max axial	force). Only f	OTCES OTE	esterthan 30	00lbs are shown	in this table
TC	1-2	0.592	-6,567 lbs		4-5	0.877	-14,559 lbs		7-8	0.852	377 lbs	(-155 lbs)			JOING ME INTO WIL	and the telescope
	2-3	0.367	-6,668 lbs		5-6	0.820	-6,969 lbs					(
	3-4	0.655	-14,496 lbs		6-7	0.916	-3,218 lbs									
BC	11-12	0.371	2,858 Ibs	(-276 lbs)	13-14	0.705	11,569 lbs	(-1,399 lbs)	15-16	0.872	10,763 lbs	(-1,417 lbs)				
	12-13	0.570	6,937 lbs	(-846 Ibs)	14-15	0.916	11,569 lbs	(-1,399 Ibs)	16-17	0.151	-414 lbs	(-,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,				
Web	1-17	0.731	-5,679 lbs	/	3-15	0.643	5,196 lbs	(-573 lbs)	6-13	0.845	2,928 lbs	(-263 lbs)	8-11	0.625	-1,836 lbs	
	1-16	0.554	7,040 lbs	(-643 lbs)	4-15	0.230	-1,537 lbs	• 1	6-12	0.804	-5.812 lbs	(-00 100)	8-10	0.104	362 lbs	(-50 lbs)
	2-16	0.300	-1,344 lbs		5-15	0.384	3,102 Ibs	(440 lbs)	7-12	0.481	3.885 lbs	(427 lbs)		0.721	-371 Ibs	(50,100)
	3-16	0.746	-6,141 lbs		5-13	0.844	-5.624 lbs		7-11	0.816	4 863 lbc	(,,	1	0,721	-511100	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

TrueBuild®Truss Software v5.7.12

Eagle Metal Products

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:57

Page: 2 of 2

PITCH QTY OHL CANTL CANTR 38-8-12 PLYS 2/12 20 SPACING WGT/PLY 2-10-8 0-0-0 0-0-0 0-0-0 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing byothers. See BCSI-B3 for additional information. 24 in

7) Due to the inherent corrosion of ireated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

9) All connectors, i.e. nails, screws, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

SPAN

12) Scab 3 - 18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

13) Listed wind uplift reactions based on MWFRS & C&Cloading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST DESIGN VALID ONLY WHEN FAGLE METAL CONNECTORS ARE USED.

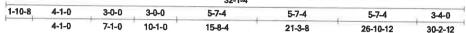
Truss: T6

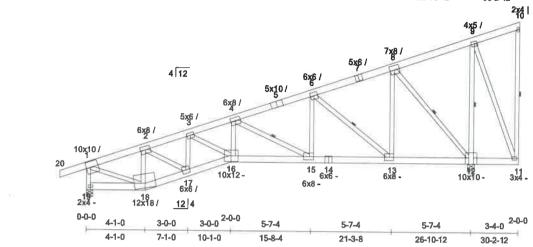
CBS0306SA-7-HIESCO COMPLETE Job:

20-0

Designer: Shane Allen Date: 09/12/24 08:01:13 Page: 1 of 2

SPAN	PITCH	QТҮ	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
30-2-12	4/12	13	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	266 lbs
	1			32	-1-4				





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	L/	(loc)	Allowed
TCLL: 110		TC: 0.85 (20-1)	VertTL: 0.49 in	L/645	(15-16)	L/240
TCDL: 10	TPI 1-2014	BC: 0.72 (15-16)	VertLL: 0.38 in	L/825	(15-16)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.79 (6-13)	Cant/OHTL: 0.08 in U		11	2L/120
BCDL: 10	Lumber D.O.L.: 100 %	l.	Cant/OHLL: 0.06 in U	P 2L/999	11	2L/09/012/202
			Horz TI · 0 18 in		12	

Reaction

12-0-1

<u>JT</u>	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
19	1	5.5 in	2.28 in	4,319 lbs		-162 lbs	-467 lbs	-467 lbs	422 lbs
12	1	5.5 in	2.31 in	4,381 lbs	•	-356 lbs	-692 lbs	-692 lbs	8

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-18, 8-12

Bracing

TC. Sheathed or Purlins at 3-1-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 4-15, 6-13, 8-12, 9-12, 10-11

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

		Forces	Table	indicates; M	ember ID	, max CS	l, max axial foro	e, (max comp	or. force i	f different f	iom max axial	force). Only f	DITUES PIRE	ater than 3	00ths are show	n in this table
TC	1-2	0.584	-4,855 lbs		34	0.490	-8,217 lbs		6-8	0.707	-2.588 lbs		1			
_	2-3	0.561	-7,115 lbs		4-6	0.711	-5,191 lbs									
BC	12-13	0.301	2,256 lbs	(-191 lbs)	15-16	0.720	7,577 lbs	(-907 lbs)	17-18	0.393	4,547 Ibs	(-638 lbs)				
	13-15	0.393	4,748 lbs	(497 lbs)	16-17	0.673	6,928 lbs	(-883 lbs)	18-19	0.050	-422 lbs	(400 400)				ı
Web	1-19	0.567	-4,266 lbs		3-17	0.304	-1,924 lbs		6-15	0.574	1,990 lbs	(-1.55 lbs)	9-12	0.495	-1.454 lbs	
	1-18	0.587	4,748 lbs	(-311 lbs)	3-16	0.448	1,553 lbs	(-94 lbs)	6-13	0.786	-3,248 lbs	(01133	1,12110	- 1
	2-18	0.485	-3,084 lbs		4-16	0.433	1,501 lbs	(-160 lbs)	8-13	0.619	2,144 lbs	(-195 lbs)				
	2-17	0.752	2,604 lbs	(-272 lbs)	4-15	0.746	-3,892 lbs	, ,	8-12	0.774	-3.603 lbs	(132 113)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Truss: T6 Job:

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:01:13 Date:

Page: 2 of 2

SPAN PITCH OTY OHL OHR CANTL CANTR 30-2-12 PLYS SPACING 4/12 WGT/PLY 13 1-10-8 0-0-0 0-0-0 0-0-0 24 in 266 lbs

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

9) All connectors, i.e nails, sciews, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 g alvanization including but not limited to repairs and add

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

OHR

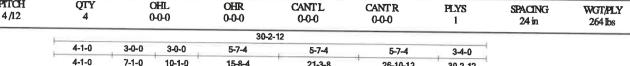
Truss: T7

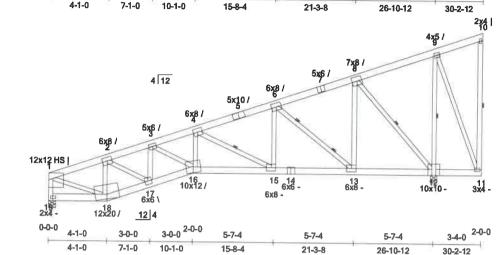
CBS0306SA-7-HIESCO COMPLETE Toh:

즐

Designer: Shane Allen 09/12/24 08:01:14 Date:

Page: 1 of 2





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.72 (6-8) BC: 0.76 (15-16) Web: 0.78 (8-12)	Deflection L/ VertTL: 0.51 in L/612 VertLL: 0.41 in L/774 Cant/OHTL: 0.08 in UP 2L/884 Cant/OHLL: 0.07 in UP 2L/999	(loc) (15-16) (15-16) 11 11	Allowed L/240 L/360 2L/120 2L/120
			Horz TL: 0.19 in	12	00/13

Reaction

12-0-1

1-11-3

SPAN

30-2-12

PITCH

OTY

OHL

JT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
19	1	5.5 in	2.04 in	3,876 lbs		-110 lbs	-369 lbs	-369 lbs	405 lbs
12	1	5.5 in	2.32 in	4,400 lbs	2	-357 lbs	-720 lbs	-720 lbs	•

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except

SP-FT/PG 2400/1.8 2 x 4: 1-18,8-12

Bracing

TC: Sheathed or Purlins at 2-11-0, Purlin design by Others.

BC: Sheathed or Purlins at 9-10-0, Purlin design by Others.

Web: One Midpoint Row: 4-15, 8-12, 9-12, 10-11

Two Third Point Rows: 6-13

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

4																		
Member Forces Table indicates: Member ID, max CSI, max axial force, (max con								ce, (max comp	or, florce id	different f	iom max axial :	force). Only fo	ICES PIE	terthan 30	Milhs are shown	in this table		
Т	C	1-2	0.324	-5,082 lbs		34	0.518	-8,686 lbs	i i	6-8	0.716	-2,605 lbs	1	i		JOHOD BED BELD WE	THE BUS GOOD	•
/ v=	_	2-3	0.595	-7,476 lbs		4-6	0.711	-5,237 lbs		"	01720	2,000 100						
В	C	12-13	0.302	2,272 lbs	(-192 lbs)	15-16	0.761	8,028 lbs	(-1,019 lbs)	17-18	0.415	4.825 lbs	(-715 lbs)					i
g=	_	13-15	0.396	4,791 lbs	(-567 lbs)	16-17	0.707	7,276 Ibs	(-968 lbs)		0.051	-397 lbs	(115100)					1
V	Veb	1-19	0.508	-3,822 lbs		3-17	0.313	-1,981 lbs		6-15	0.591	2.048 lbs	(-178 lbs)	9.12	0.495	-1,454 lbs		Ĺ
		1-18	0.623	5,039 lbs	(424 lbs)	3-16	0.444	1,540 lbs	(-132 lbs)	6-13	0.688	-3,502 lbs	(110 100)	712	UADO	-1,-10-100		
		2-18	0.515	-3,278 lbs		4-16	0.442	1,532 lbs	(-166 lbs)		0.671	2.325 lbs	(-234 lbs)					1
	- 1	2-17	0.774	2,681 lbs	(-282 Ibs)	4-15	0.769	4.014 lbs		8-12	0.779	-3.627 lbe	(207,100)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:01:15

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 30-2-12 4/12 0-0-0 4 0-0-0 0-0-0 0-0-0 24 in 264 lbs 7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on

metal bate shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE BAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

⁹⁾ All connectors, i.e nails, screws, bolts, trus s plates, etc, to treated wood shall have a minimum of C60 g alvanization including but not limited to repairs and add framing.

OHR

0.00

Truss: T7A

Job: CBS0306SA-7-HIESCO COMPLETE

WGT/PLY

197 lbs

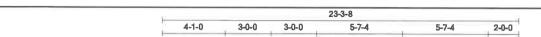
Designer: Shane Allen Date: 09/12/24 08:01:16

SPACING

24 in

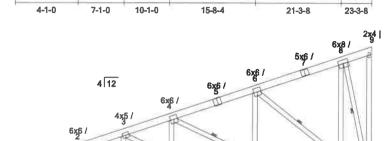
Page: 1 of 2

PLYS



OHL

0-0-0



CANTL

0-0-0

13 12 5x8 -5x5 - 6x8 -

7x8 -

CANTR

0-0-0

0-0-0 4-1-0 3-0-0 3-0-0 2-0-0 5-7-4 5-7-4 2-0-0 4-1-0 7-1-0 10-1-0 15-8-4 21-3-8 23-3-8

10x10

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflectio	ı.	L/	(loc)	Allowed
TCLL: 110	Bldg Code:	IBC2018/	TC:	0.76 (6-8)	Vert TL:	0.36 in	L/764	(13-14)	L/240
TCDL: 10	-	TPI 1-2014	BC:	0.62 (13-14)	VertLL:	0.28 in	L/983	(13-14)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.82 (6-11)	Horz TL:	0.13 in		10	_,
BCDL: 10	Lumber D.O.I	L.: 100 %		, ,					00/10/00
	1		11						09/12/20

15 5x5 /

12 4

16 10x14 -

Reaction

SPAN

23-3-8

PITCH

4/12

QTY

10x10 HS

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
17	1	5.5 in	1.79 in	3,403 lbs	*	-110 lbs	-344 lbs	-344 lbs	320 lbs /
10	1	1.5 in	_	3.029 lbs	92	-250 lbs	-428 lbs	-428 The	. 8

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4 except: SP-FT/PG 2400/1.8 2x 4: 1-16

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 4-13, 6-11, 8-10

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 3) This truss has been designed for the effects of TCLL = 20 psf.
- 4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber 1	Forces	Table	indicates: M	ember ID	, пех СЅ	, max axial forc	e, (max comp	or, force i	if different fi	iom max axial	force). Only fo	proes greater than 300fbs are shown in this table
TC	1-2	0.395	-4,210 lbs		34	0.421	-6,420 Ibs		6-8	0.756	-1,074 lbs	- 1	
-	2-3	0.585	-5,744 lbs		4-6	0.736	-3,751 lbs				•		
BC	10-11	0.167	670 lbs	(-65 Ibs)	13-14	0.621	5,922 Ibs	(-803 lbs)	15-16	0.382	3,935 lbs	(-602 lbs)	
-	11-13	0.387	3,389 lbs	(-398 Ibs)	14-15	0.590	5,554 lbs	(-799 lbs)	16-17	0.055	-306 Ibs		
Web	1-17	0.445	-3,352 lbs		3-15	0.226	-1,428 lbs		6-13	0.509	1,765 lbs	(-152 lbs)	
	1-16	0.508	4,107 lbs	(-386 lbs)	3-14	0.290	1,004 Ibs	(-66 lbs)	6-11	0.821	-3,391 lbs		
	2-16	0.418	-2,659 lbs		4-14	0.447	1,550 lbs	(-150 lbs)	8-11	0.618	2,141 lbs	(-266 lbs)	
H ()	2-15	0.563	1.952 lbs	(-220 lbs)	4-13	0.647	-3.377 lbs		R-10	0.671	-3 092 The	, ,	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: T7A

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:01:17

Date: 09/12/24 08:9 Page: 2 of 2

	GT/PLY 197 lbs
--	-------------------

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 10) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss

133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: T7G

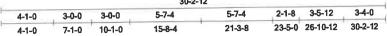
CBS0306SA-7-HIESCO COMPLETE Job:

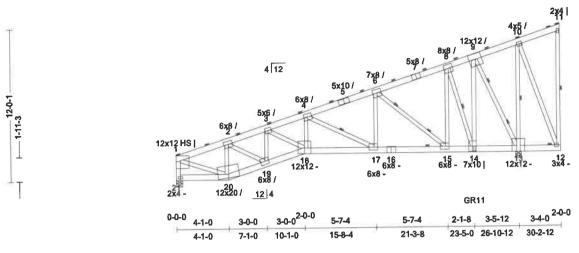
100-1

Designer: Shane Allen 09/12/24 08:01:18 Date:

Page: 1 of 2

SPAN 30-2-12	PITCH 4/12	QTY 1	OHL 0-0-0	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 36 in	WGT/PLY 289 lbs
				30	-2-12		9		
		1		F 7 4	674	2 1 9 2 5 12	3-4-0		





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) Carried Loads (psf) TCLL: 110		CSI TC: 0.75 (6-8) BC: 0.90 (17-18)	Deflection VertTL: 0.57 in VertLL: 0.44 in	L/ L/549 L/707	(loc) (17-18) (17-18)	Allowed L/240 L/360
TCDL: 10 BCLL: 0 BCDL: 10	Rep Mbr: No Lumber D.O.L.: 100 %	Web: 0.83 (9-13)	Cant/OHTL: 0.09 in UI Cant/OHLL: 0.07 in UI Horz TL: 0.22 in		12 12 13	2L/120 2L/12 09/12/

	Des Combo	Brg Width	Rod Brg Width	Max React	Max Gray Unlif	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
- 31	Brg Combo	5.5 in	2.12 in	4.020 lbs	William Crar Copus	-123 lbs	-337 lbs	-337 lbs	419 lbs
12	1	55 in	3.42 in	6.484 lbs		-519 lbs	-631 lbs	-631 lbs	

Material

TC: SP-FT/PG2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-20,9-14,9-13

Bracing

TC: Sheathed or Purlins at 2-7-0, Purlin design by Others.

BC: Sheathed or Purlins at 9-8-0, Purlin design by Others.

Web: One Midpoint Row: 4-17, 8-14, 10-13, 11-12

Two Third Point Rows: 6-15, 9-13

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user

defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated. 2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Pactored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Ltl: Std Live Load

Luan Cascia 1.0	MI LAYO LOUIS						
Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width_
Top Top Top Top Top	0-0-0 0-0-0 23-5-0 25-11-11 27-3-0	30-2-12 23-5-0 25-11-11 27-3-0 30-2-12	Down Down Down Down Down	Proj Proj Proj Proj Proj	20 plf 20 plf 36.67 plf 36.67 plf 20 plf	20 plf 20 plf 36.67 plf 40 plf 20 plf	

Load Case D1: St	id Dead Load						
Distributed Loads Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	0-0-0	30-2-12	Down	Proj	10 plf	10 plf	
Тор	0-0-0	23-5-0	Down	Proj	10 plf	10 plf	
Тор	23-5-0	25-11-11	Down	Proj	18.33 plf	18.33 plf	
Тор	25-11-11	27-3-0	Down	Proj	18.33 plf	20 plf	
Тор	27-3-0	30-2-12	Down	Proj	10 plf	10 plf	
Bot	0-0-0	30-2-12	Down	Proj	10 plf	10 plf	
Bot	0-0-0	23-5-0	Down	Proj	10 plf	10 plf	
Bot	23-5-0	25-11-11	Down	Proj	18.33 plf	18.33 plf	
Bot	25-11-11	27-3-0	Down	Proi	18.33 plf	20 plf	
Dot.	27.2.0	20.2.12	Down	Pmi	10 pÎf	10 plf	

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Truss: T7G

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:01:18
Page: 2 of 2

SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	_
30-2-12	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	36 in	289 lbs	

		orces	'Iable	indicates: Mi	anba ID	, max CS	, max axial forc	e, (max comp	or, force it	differnt i	iom max axial :	force). Only fi	OFFICEN GREE	ater than 30	Olbs are shown in this tab
TC	1-2	0.367	-5,290 lbs		3-4	0.648	-9,207 lbs		6-8	0.751	-3,488 lbs		Ι		
	2-3	0.512	-7,848 lbs		4-6	0.739	-6,216 lbs		8-9	0.333	-2,363 lbs				
BC	13-14	0.482	2,190 lbs	(-179 lbs)	15-17	0.552	5,728 Ibs	(-510 lbs)	18-19	0.831	7,647 lbs	(-932 lbs)	20-21	0.060	-419 lbs
	14-15	0.372	3,080 lbs	(-255 lbs)	17-18	0.900	8,518 lbs	(-961 lbs)	19-20	0.478	5,028 lbs	(-703 lbs)		0.000	417 MM
Web	1-21	0.527	-3,965 lbs		3-18	0.535	1,855 lbs	(-107 lbs)	8-15	0.677	2,345 lbs	(-262 lbs)			
	1-20	0.649	5,251 lbs	(-377 lbs)	4-18	0.449	1,554 lbs	(-170 lbs)	8-14	0.640	-2,889 Ibs	(====)			
	2-20	0.537	-3,419 lbs		4-17	0.769	-4,011 lbs	, , , , , ,	9-14	0.565	4,572 lbs	(-336 lbs)			
	2-19	0.827	2,865 lbs	(-255 lbs)	6-17	0.597	2,070 lbs	(-175 lbs)	9-13	0.830	-5.250 lbs	(550 103)	ľ		
	3-19	0.335	-2,119 lbs		6-15	0.709	-3,609 lbs	, ,	10-13	0.485	-1,424 lbs	1	li.		

Truss to Truss Connection Summary Carried Truss Carrying Chosed Carrying Offset GR11 BC 23-5-0

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

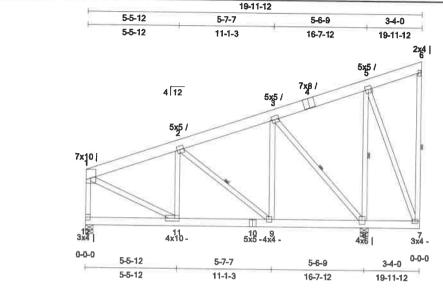
 The trust fine inherent correction of treated humber and close connection and the number of control of the connection and the number of the
- 7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 9) All connectors, i.e. nails, screws, bolts, trus s plates, etc, to treated wood shall have a minimum of C60 g alvanization including but not limited to repair and add framing.
- 10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 11) Incising is not permitted.
- 12) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T8

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:01:19
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-11-12	4/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	190 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.79 (1-2) BC: 0.20 (9-11) Web: 0.86 (6-7)	Deflection Vert TL: 0.08 in Vert LL: 0.06 in Cant/OH TL: 0.02 in Cant/OH LL: 0.02 in	L/ L/999 L/999 2L/999 2L/999	(loc) (10-11) (10-11) 7 7	Allowed L/240 L/360 2L/120 2L/120
			Horz TL: 0.02 in		8	00/13/3

Reaction

1000

34.3

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplit	tMax C&CUnlift	Max Unlift	Max Horiz
12	1	5.5 in	2.27 in	2,600 lbs		-27 lbs	-205 lbs	-205 lbs	359 lbs
8	1	5.5 in	2.70 in	3.099 lbs	*	-282 Ibe	571 lbs	-200 IDS	223 108

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-9, 3-8, 5-8, 6-7

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following used defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL=20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber l	Forces	Table	Table indicates: Member ID, max CSI, max axial force, (max compt. force if different from max axial force). Only forces greater than								was constant them 2008ha on shown in this sale.	
TC	1-2	0.789	-2,381 lbs		2-3	0.735	-1,502 lbs	, ,	3-5	0.511	311 lbs	(-157 lbs)	
BC	8-9	0.189	1,164 lbs	(-128 lbs)	9-11	0.199	2,042 lbs	(-240 lbs)	11-12	0.132	-329 lbs	(107 100)	
Web	1-12	0.400	-2,543 lbs		2-9	0,281	-1,180 lbs		5-8	0.451	-1.506 lbs		
	1-11	0.668	2,315 lbs	(-117 lbs)	3.9	0.244	846 lbs	(-53 Ibs)	_		-,500 200		
	2-11	0.317	-983 Ibs		3.8	0.601	-1 931 The	` ' '				1	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

	SS Loop 1252 -8508				-HIESCO COMPLETE				
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-11-12	4/12	4	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	190 lbs

⁷⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

⁹⁾ All connectors, i.e. nails, screws, bolts, trus s plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

¹¹⁾ Incising is not permitted.
12) Listed wind uplift reactions based on MWFRS & C&C loading.

OHR

0-0-0

Truss: T9

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

186 lbs

Designer: Shane Allen 09/12/24 08:01:21

SPACING

24 in

PLYS

1

		19-11-12		
4-3-0	4-3-0	4-3-12	4-3-12	2-10-4
4-3-0	9.6.0	120.10	474.0	40.44.40

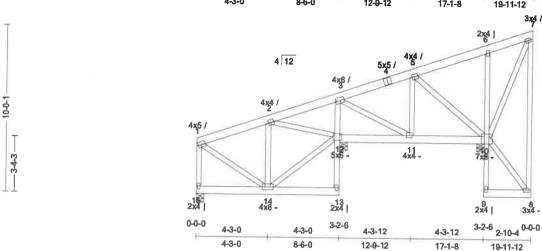
CANTL

0-0-0

CANTR

0-0-0





OHL

0-0-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.80 (1-2) BC: 0.11 (10-11) Web: 0.70 (7-8)	Deflection Vert TL: 0.02 in Vert LL: 0.01 in Cant/OHTL: 0.01 in I Cant/OHLL: 0.01 in I Horz TL: 0 in		(loc) (10-11) 11 8 8	Allowed L/240 L/360 2L/120 2L/109/12/2
---	---	--	--	--	----------------------------------	--

Reaction

SPAN

19-11-12

PITCH

4/12

QTY

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift N	Max C&CUplift	Max Uplift	Max Horiz
15	1	5.5 in	1.50 in	1,399 lbs		-18 lbs	-89 lbs	-89 lbs	359 lbs
12	1	5.5 in	2.13 in	2.445 lbs		-217 lbs	-260 lbs		223 NOS
10	1	5.5 in	1.68 in	1.923 lbs				-260 lbs	12.0
	-	227 MI	1.00 11	משו בגילו	•	-168 lbs	-418 lbs	-418 lbs	(4)

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG 2400/1.8 2 x 4: 7-8 **Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

					_	_		T. E.					
N	/Iem	ber I	Forces	Table	indicates: M	ember ID	, max CSI	max axial for	e. (max comi	or, force	if different fi	iom mar arial force) Only	forces greater than 300lbs are shown in this table.
T	c	1-2	0.803	-931 lbs		3-5	0.575	-759 Ibs		1		Only	I DONE THE TOTAL STORY OF SHOWIT IN THIS TANK
, <u>, , , , , , , , , , , , , , , , , , </u>		2-3	0.703	392 Ibs	(-118 lbs)		0.075	757 100					1
В	c	10-11	0.112	581 lbs	(-112 lbs)								
-		14-15	0.104	-329 lbs									
W	Æb	1-15	0.223	-1,353 lbs		14-12	0.262	908 Ibs	(-194 lbs)	6.10	0.511	-944 lbs	
	- 1	1-14	0.254	881 lbs	(-19 lbs)	3-12	0.265	-1.601 lbs	(15 + 100)	7-10	0.217	-315 lbs	
	- 1	2-14	0.287	-991 Ibs	(3-11	0.212	735 lbs		17-10	0.21/	-313 108	
	- 1	2-12	0.274	-924 Ibs		5-10	0.510	-935 lbs					Į.

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) At leastone web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."

5) A creep factor of 1.50 has been applied for this truss analysis.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252			Designer: Shar	0306SA-7 ne Allen 2/24 08:0	HIESCOCOMPLET
SPAN 19-11-12	PITCH 4/12	QTY 8	OHIL 0-0-0	OHR 0-0-0	CANT L 0-0-0	CANT R 0-0-0	PLYS 1		CING 4 in	WGT/PLY 186 lbs

⁶⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

7) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Incising is not permitted.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

⁸⁾ All connectors, i.e nails, sciews, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

⁹⁾ Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

¹¹⁾ Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T9A

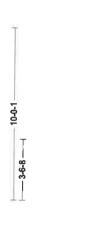
Job: CBS0306SA-7-HIESCO COMPLETE

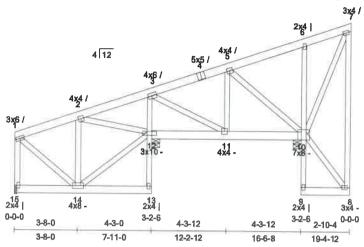
Designer: Shane Allen Date: 09/12/24 08:01:22

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SPAN	PTICH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
19-4-12	4/12	1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	183 lbs	







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.70 (1-2) BC: 0.11 (10-11) Web: 0.70 (7-8)	Deflection VertTL: 0.02 in VertLL: 0.01 in Cant/OHTL: 0.01 in I Cant/OHTL: 0.01 in I HorzTL: 0 in		(loc) (10-11) 11 8 (8-8)	Allowed L/240 L/360 2L/120 2L/120
---	---	--	---	--	--------------------------------------	---

Reaction JT Brg Co

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
15	1	1.5 in	1.50 in	1,294 lbs		-22 lbs	-801bs	-80 lbs	359 lbs
12	1	5.5 in	2.09 in	2,394 lbs		-218 lbs	-250 lbs	-250 lbs	0.00
10	1	5.5 in	1.67 in	1,918 lbs	•	-169 lbs	-419 lbs	-419 lbs	(0)

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG #2 2 x 4 except: Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

SP-FT/PG 2400/1.8 2 x 4: 7-8

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

3) This truss has been designed for the effects of TCLL=20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

N	/Iem	ber l	Forces	Table	indicates: M	icmber ID	, max CSI	, max axial for	ce, (max com	or, force	if diff acut fi	om max axial fi	orce). Only	orces greater than 300lbs are shown in this table.
T(C	1-2	0.700	-788 lbs		3-5	0.624	-766 Ibs		1				1
l =		2-3	0.682	359 lbs	(-116 lbs)									
B	C	10-11	0.113	571 lbs	(-113 lbs)									
7 <u>=</u>		14-15	0.092	-326 lbs										
W	leb	1-15	0.222	-1,255 lbs		14-12	0.222	771 lbs	(-208 lbs)	6-10	0.513	-949 lbs		
	- 1	1-14	0.235	813 lbs	(-23 lbs)	3-12	0.271	-1,639 lbs	,	7-10	0.217	-314 lbs		
	- 1	2-14	0.273	-942 Ibs		3-11	0.206	714 lbs						
	- 1	2-12	0.228	-768 lbs		5-10	0.502	-920 lbs						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Laterall braces shall be installed within 6 "of each web panel point.

5) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Truss: T9A

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

183 lbs

Designer: Shane Allen Date: 09/12/24 08:01:23

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR 19-4-12 PLYS 4/12 SPACING 0-0-0 0-0-0 0-0-0 0-0-0 6) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on 24 in metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond

7) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

8) All connectors, i.e. nails, sciews, bolts, trus s plates, etc., to treated wood shall have a minimum of G60 galvarization including but not limited to repairs and add

9) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

10) Incising is not permitted.

11) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, BRECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

OHR

Truss: T11

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

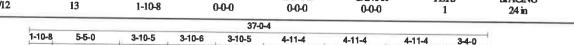
302 lbs

Designer: Shane Allen Date 09/12/24 07:59:57

SPACING

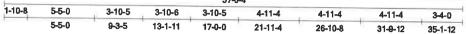
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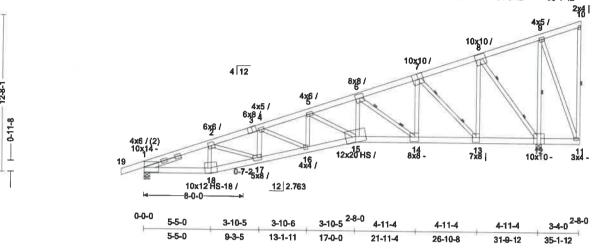
PLYS



CANTL

CANTR





All plates shown to be Eagle 20 unless otherwise noted.

Horz TI: 0.20 in 12

Reaction

SPAN

35-1-12

PITCH

4/12

QTY

OHL

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplif	Max C&CUplift	Max Unlift	Max Horiz
1	1	5.5 in	4.19 in	4,806 lbs		-204 lbs	-545 lbs	-545 lbs	506 lbs
12	1	5.5 in	2.65 in	5,034 lbs		-390 lbs	-770 lbs	-770 lbs	200 105

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 6-15,7-14,8-13,8-12

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 8-7-0, Purlin design by Others.

Web: One Midpoint Row: 9-12, 10-11

Two Third Point Rows: 6-14, 7-13, 8-12

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Table	indicates; M	ember II), max CS	, max axial for	ce, (max comp	or, florce in	f different	fiom max axia	force) Only fi	neroes com	sterthan 3/	Vilhe see choss	n in this table.
TC	1-2	0.878	-8,810 lbs		5-6	0.584	-9.758 lbs		1				1	THE PART OF	OIG at all w	TI TII UTIO KACIONY
	24	0.872	-11,078 lbs		6-7	0.622	-5,813 lbs									
	4-5	0.644	-10,776 lbs		7-8	0.603	2,738 lbs									
BC	12-13	0.334	2,425 lbs	(-190 lbs)	14-15	0.780	8,980 lbs	(-1,023 lbs)	16-17	0.879	10,637 lbs	(-1,321 lbs)	19.1	0.915	8.135 lbs	(-1,051 lbs)
-	13-14	0.441	5,355 lbs	(-577 lbs)	15-16	0.882	10,342 lbs	(-1,267 lbs)		0.834	8,259 lbs	(-1.068 lbs)		0.515	0,133 118	(-1,001 100)]
Web	2-18	0.280	-2,017 lbs		5-16	0.102	352 lbs	(-237 lbs)	7-14	0.364	2,946 lbs	(-276 lbs)	_	0.471	-1,384 lbs	
	2-17	0.737	2,555 lbs	(-261 lbs)	5-15	0.696	-2,124 lbs	` '	7-13	0.803	-4,227 lbs	(210100)		0.471	1,507 IUS	i
	4-17	0.192	-1,252 lbs		6-15	0.420	3,394 Ibs	(-334 lbs)	8-13	0.380	3.076 lbs	(-329 Ibs)				- 1
	4-16	0.153	529 Ibs	(416 lbs)	6-14	0.750	-4,710 lbs		8-12	0.710	-4,272 lbs	(-20 100)				

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 07:59:57
Page: 2 of 2

SPAN	PTICH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
35-1-12	4/12	13	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	302 lbs

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&Cloading.

⁹⁾ All connectors, i.e nails, screws, bolts, trus s plates, etc, to treated wood shall have a minimum of C60 g alvanization including but not limited to repairs and add framing.

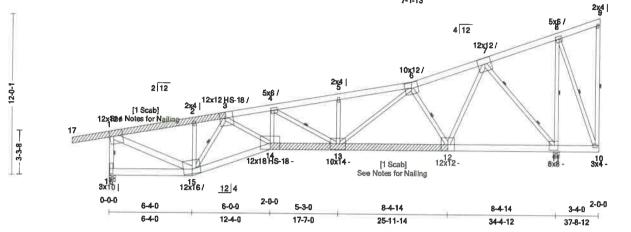
Truss: T12

09/16/2024

CBS0306SA-8-HIESCO COMPLETE Job:

Designer:Shane Allen Date: 09/16/24 08:08:28 Page: 1 of 2

SPAN 37-8-12	PITCH 2/12	QTY 32		OHL 10-8	OHR 0-0-0	CANTL 000	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 329 lbs
					40	-7-4				
	2-10-8	6-4-0	2-6-6	3-5-10	5-3-0	5-7-0	5-7-6	5-7-6	3-4-0	
		6-4-0	8-10-6	12-4-0	17-7-0	23-2-0 7-	1-13 28-9-6	34-4-12	37-8-12	



All plates shown to be Eagle 20 unless otherwise noted,

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0	General Bklg Code: IBC 2018/ TH 1-2014 Rep Mbr: Yes	CSI TC: 0.94 (6-7) BC: 0.91 (14-15) Web: 0.85 (7-11)	Deflection L/ Vert TL:	(loc) (13-14) (13-14) 10	Allowed L/240 L/360 2L/120
BCDL: 10	Lumber D.O.L.: 100 %	Web: 0.85 (7-11)	Cant / OHTL: 0.15 in UP 2L / 500 Cant / OHLL: 0.12 in UP 2L / 622	10 10	2L/120 2L/120
			Horz TL: 0.26 in	11	

Reaction JT Brg Combo Brg Width Rod Brg Width Max React

Max Grav Uplift Max MWHRS Uplift Max C&C Uplift Max Uplift Max Horiz 16 5.5 in 4.88 in 5.592 lbs -273 lbs -705 lbs -705 lbs 443 lbs 5.5 in 4.65 in 5.329 lbs -388 lbs -844 lbs -844 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 3-14, 6-13, 6-12, 7-12, 7-11 SP-FT/PG 2400/1.8 2 x 6: 1-15

Bracing TC: Sheathed

BC: Sheathed or Purlins at 8-9-0, Purlin design by Others. Web: One Midpoint Row: 1-16, 3-15, 4-13, 6-12, 8-11, 8-10, 9-10

Two Third Point Rows: 7-11

17-3 [Qty:1] SP-FT/PG 2400/1.8 21/26/4 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Scabs Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TC LL = 20 psf.

Non-concurrent minimum storage artic loading has been applied in accordance with IBC 1607.1

Mei	mber l	Forces	Tab	le indicates: M	ember II.), max CSI,	max axial force	, (max compr.	force if d	ifficient from	ı max axial force	. Only forces	orester ti	han 300lihs	are shown in this	e table
TC	1-2	0.591	-6,512 lbs		4-5	0.871	-10,876 lbs		7-8	0.922	391 lbs	(-177 lbs)				, emones
	2-3	0.306	-6,541 lbs		5-6	0.850	-10,905 lbs					(
_	3-4	0.869	-13,168 lbs		6-7	0.937	-5,199 lbs									
BC	11-12	0.371	3,110 lbs	(-326 lbs)	13-14	0.583	12,900 lbs	(-1,637 lbs)	15-16	0.086	-412 lbs					
-	12-13	0.334	6,936 lbs	(-882 lbs)	14-15	0.906	9,446 lbs	(-1,264 lbs)								
Web	1-16	0.713	-5,543 lbs		3-14	0.592	4,786 lbs	(-542 lbs)	6-13	0.614	4,962 lbs	(-565 lbs)	8-11	0.594	-1.919 lbs	
	1-15	0.545	6,929 lbs	(-625 lbs)	4-14	0.090	312 lbs	(-20 lbs)	6-12	0.596	-4.656 lbs	(8-10	0.092	319 lbs	(-59 lbs)
	2-15	0.327	-1,370 lbs		4-13	0.475	-2,633 lbs		7-12	0.563	4,551 lbs	(-528 lbs)		0.728	-329 lbs	(57 200)
	3-15	0.766	-5,060 lbs		5-13	0.402	-1,740 lbs		7-11	0.846	-4,855 lbs	,	J			

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-8-HIESCO COMPLETE

Designer-Shane Allen Date: 09/16/24 08:08:29 Page: 2 of 2

SPAN	PITCH	OTV	OTH	0.77					
		211	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
37-8-12	2/12	32	2-10-8	0-0-0	0-0-0	000	1		
				000	000	0-0-0		24 in	320 Iba

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

14) Listed wind uplift reactions based on MWFRS & C&C loading.

⁹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

¹²⁾ Scab 3 - 17 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 13) Scab 12 - 14 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

Valley Truss Truss: T12 133 Range View Loop Job: CBS0306SA-7-HIESCO COMPLETE Westcliffe, CO 81252 Designer, Shane Allen Date: 09/12/24 07:59:58 Phone (719) 371-8508 Page: 1 of 2 SPAN PITCH QTY OHI OHR CANTL CANTR PLYS SPACING WGT/PLY 37-8-12 2/12 32 2-10-8 0-0-0 0-0-0 0-0-0 1 24 in 339 lbs 40-7-4 2-10-8 6-4-0 2-6-6 3-5-10 5-3-0 5-7-0 5-7-6 5-7-6 3-4-0 6-4-0 8-10-6 12-4-0 17-7-0 23-2-0 28-0-6 34-4-12 37-8-12 7-1-13 2x4 | x6 / 4 12 12x12/ 10x12/ 3x4 | 1001 2 12 12-0-1 2x4 | 12x12 HS-18 / 6x6 / [1 Scab] 12x\$86 Notes for N 200 33.38 12x24 HS / 10x16 12x12 3x4 -3x10 15 12x16/ 12 4 0-0-0 3-4-0 2-0-0 2-0-0 6-4-0 6-0-0 5-3-0 8-4-14 8-4-14 6-4-0 12-4-0 17-7-0 25-11-14 34-4-12 37-8-12 All plates shown to be Eagle 20 unless otherwise noted. Loading (psf) General CSI Deflection L Allowed (loc) TOLL: 110 Bldg Code: IBC2018/ TC: 0.94 (6-7) Vert TL: 0.96 in L/419 (13-14)L/240 TCDL: 10 TFI 1-2014 BC: 0.96 (14-15) VertI.L: 0.77 inL/524 (13-14)L/360 BCLL: 0 Rep Mbr: Yes Web: 0.85 (7-11) Cant/OHTL: 0.17 in UP 2L/444 10 2L/120 BCDL: 10 Lumber D.O.L.: 100 % Cant/OHLL: 0.13 in UP 2L/553 10 2L/120Horz TL: 0.31 in11 09/12/2024 Reaction JT Brg Combo Brg Width Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift Rqd Brg Width Max React Max Horiz 16 5.5 in 4.88 in 5.592 lbs -273 lbs -705 lbs -705 lbs 443 lbs 11 5.5 in 4.65 in 5,329 lbs -388 lbs 844 lbs -844 Ths **Material Bracing** TC: SP-FT/PG 2400/1.8 2 x 6 TC: Sheathed SP-FT/PG 2400/1.8 2 x 6 except BC: Sheathed or Purlins at 8-8-0, Purlin design by Others. SP-FT/PG 2400/1.8 2 x 8: 12-14 Web: SP-FT/PG#2 2 x 4 except: Web: One Midpoint Row: 1-16, 3-15, 4-13, 6-12, 8-11, 8-10, 9-10 SP-FT/PG 2400/1.8 2 x 4: 3-14,6-13,6-12,7-12,7-11 Two Third Point Rows: 7-11 SP-FT/PG2400/1.8 2 x 6: 1-15 Scabs 17-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 6 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated. 2) This truss has been designed to account for the effects of ice dams forming at the eaves. 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60 4) This truss has been designed for the effects of TCLL = 20 psf. 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1 **Member Forces** Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. 1-2 0.591 -6.518 lbs -11,098 lbs 0.923 7-8 390 lbs (-177 lbs) 2-3 0.309 -6.547 lbs 5-6 0.932 -11,126 lbs 0.865 13,503 lbs 0.941 -5,225 lbs BC 11-12 0.376 3,122 lbs (-327 lbs) 13-14 0.88713,294 lbs (-1,683 lbs) 15-16 0.087 -412 Ibs 12-13 0.494 6,997 lbs (-890 lbs) 14-15 0.957 9,434 lbs (-1,262 lbs) Web 1-16 0.714 -5.547 lbs 0.626 5.060 lbs (-580 lbs) (6-13 0.635 5.132 lbs (-587 lbs) 8-11 0.593 -1.917 lbs 1-15 0.546 6.935 lbs (-625 lbs 4-14 0.092 (-16 lbs) 6-12 0.593 4.697 lbs 8-10 0.092 318 lbs (-59 lbs) 0.328 2-15 -1.375 lbs 4-13 0.519 -2.777 Ibs 7-12 4,567 lbs (-530 lbs) 9-10 0.728 -329 lbs 3-15 5-13 0.394 -1.819 lbs 7-11 0.849 -4,872 lbs

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).

3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCOCOMPLETE

Designer: Shane Allen Date: 09/12/24 07:59:58

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
37-8-12	2/12	32	2-10-8	0-0-0	0-0-0	0-0-0	1	24 in	339 lbs
7) Due to the inherent metal plates shall be a	t corrosion of treated	humber and steel co	nnectors and the num	ber of variables that	affect the rate of corro	sion, the G60 coating	provided on	-	

metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.
12) Scab 3 - 17 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

13) Listed wind uplift reactions based on MWFRS & C&C loading.

⁹⁾ All connectors, i.e nails, screws, bolts, truss plates, etc., to treated wood shall have a minimum of C60 galvanization including but not limited to repairs and add framing.

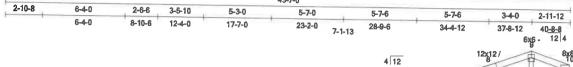
Truss: T12A

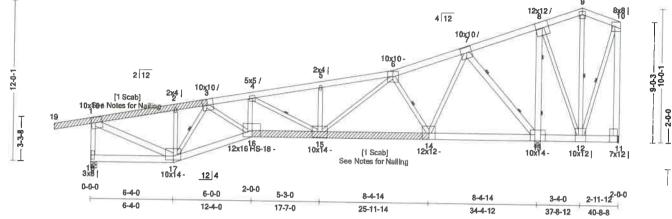
09/16/2024

Joh: CBS0306SA-8-HIESCO COMPLETE

Designer-Shane Allen 09/16/24 08:08:33 Date: Page: 1of2

SPAN 40-8-8	PITCH 2/12	QTY 1	OHL 2-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 389 lbs
	1			43	-7-0				
	2 10 0	10 00							





All plates shown to be Eagle 20 unless otherwise noted,

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10 BCDL: 10 BCDL: 10	TPI 1-2014 BC: Web:	0.88 (5-6) 0.70 (16-17) 0.82 (7-13)	Deflection Vert TL: Vert LL: Horz TL:	0.69 in 0.55 in 0.19 in	L/588 L/737	(loc) (15-16) (15-16) 13	Allowed L/240 L/360
---	---------------------	---	--	-------------------------------	----------------	-----------------------------------	---------------------------

Reaction Brg Combo Brg Width Rod Brg Width Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max React Max Horiz 18 5.5 in 4.15 in 4,758 lbs -253 lbs -643 lbs -643 lbs 401 lbs 13 5.5 in 3.72 in 10,346 lbs -599 lbs -1,256 lbs -1,256 lbs 11 15 in 1.50 in 339 lbs -3,125 lbs -3,125 lbs

Material

TC: SP-FT/PG2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FI/PG#2 2 x 4 except:

SP-FT/PG#2 2x 6: 7-13, 10-11

SP-FT/PG 2400/1.8 2 x 4: 3-16, 6-15, 6-14, 7-14, 8-12, 10-12 SP-FT/PG 2400/1.8 2 x 6: 1-17, 8-13

Bracing

TC: Sheathed or Purlins at 2-6-0, Purlin design by Others.

BC: Sheathed or Purlins at 5-10-0, Purlin design by Others.

Web: One Midpoint Row: 3-17, 4-15, 6-14, 9-12, 10-12

Two Third Point Rows: 7-13, 8-13

Scahs 19-3 [Qty:1] SP-FT/PG 2400/1.8 214466 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL = 157 psf), Terrain C, Exposure (Ce = 1.0), Thermal (Ct = 1.00), DOL = 1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TC LL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	ember		-	le indicates: M	ember II), max CSI ,	max axial force	, (max compr.	force if d	lifferent from	n max axial force	c). Only forces	ovester if	an 3000hs	are shown in th	ie toble
TC	1-2	0.590	-2,107 106		4-0	0.717	-8,213 lbs		7-8	0.852	2,921 lbs	(-266 lbs)	ľ			is distant
	2-3	0.276	-5,172 lbs		5-6	0.879	-8,231 lbs		8-9	0.751	1,095 lbs	(-75 lbs)				- 1
	3-4	0,609	-10,172 lbs		6-7	0.850	-3,021 lbs		9-10	0.226	1,048 lbs	(-78 lbs)				I
BC	12-13	0.219	-2,512 lbs		14-15	0.252	4,873 lbs	(-544 lbs)	16-17	0.702	7,333 lbs	(-1,065 lbs)				
-	13-14	0.325	969 lbs	(-101 lbs)	15-16	0.452	9,999 lbs	(-1,349 lbs)		0.086	-371 lbs	(-1,000 115)	l			1
Web		0.761	-4,708 lbs		4-16	0.093	323 lbs	(-25 lbs)		0.532	4.298 lbs	(488 lbs)	10.12	0.739	21/08	
	1-17	0.430	5,466 lbs	(-517 lbs)	4-15	0.443	-2.342 lbs		7-13	0.815	-5,299 lbs	(-100 ILB)	10-12		-3,162 lbs	
	2-17	0,290	-1,214 lbs		5-15	0.324	-1,401 lbs		8-13	0.672	-6,060 lbs		10-11	0.666	3,186 lbs	(-327 lbs)
	3-17	0.569	-3,755 lbs		6-15	0.504	4,077 lbs	(-587 lbs)		0.521	4,214 lbs	(-495 lbs)				- 1
	3-16	0.454	3,671 lbs	(421 lbs)	6-14	0.568	-4,439 lbs		9-12	0.432	-1,112 lbs	(-493 IOS)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REPER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM FACLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLEMETAL CONNECTORS ARE USED.

			7	Valley Tru 33 Range View Westcliffe, CO 8 hone (719) 371	Loop 31252		Job:	igner:Shane Allen e: 09/16/24 08:	8-HIESCO COMPLI 08:34
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
40-8-8	2/12	1	2-10-8	0-0-0	000	0-0-0	1	24 in	389 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 11 may need to be considered.

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
10) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3 - 19 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSL Z278, 2 - ply) Screws @ 6 oc.
14) Scab 14 - 16 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSL Z278, 2 - ply) Screws @ 6 oc.

15) Listed wind uplift reactions based on MWFRS & C&C loading.

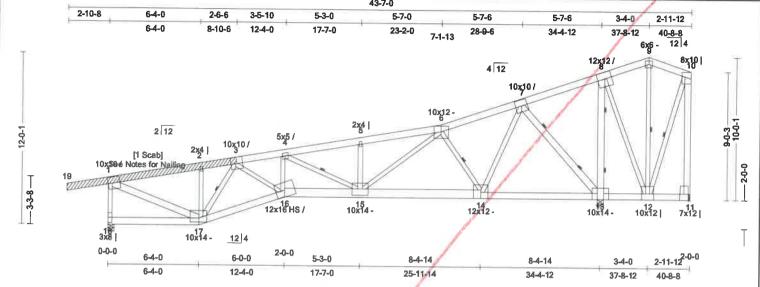
Truss: T12A

09/12/2024

CBS0306SA-7-HIESCO COMPLETI Job:

Designer: Shane Allen Date: 09/12/24 07:59:59 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
40-8-8	2/12	1	2-10-8	0-0-0	0-0-0	0-0-0	1	24 in	397 lbs
				10	2.4				



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General Bidg Code: IBC2018/	CSI TC: 0.88 (5-6)	Deflection Vert TL:	0.73 in	L/ L/551	(loc)	Allowed
TCDL: 10	TPI 1-2014	BC: 0.72 (16-17)		0.75 in	L/690	(15-16) (15-16)	L/240 L/360
BCLL: 0 BCDL: 10	Rep Mbr: Yes Lumber D.O.L.: 100 %	Web: 0.81 (7-13)	Horz TL:	0.22 in		13	,

Bracing

TC: Sheathed or Purlins at 2-6-0, Purlin design by Others.

BC: Sheathed or Purlins at 5-7-0, Purlin design by Others.

Web: One Midpoint Row: 3-17, 4-15, 6-14, 9-12, 10-12

Two Third Point Rows: 7-13, 8-13

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
18	1	5.5 in	4.10 in	4,705 lbs		-249 lbs	-637 lbs	-637 lbs	401 lbs
13	1	5.5 in	3.84 in	10,699 lbs	. /	-625 lbs	-1.300 lbs	-1.300 lbs	401 103
11	1	1.5 in	1.50 in	377 lbs	-3,425 lbs		*	-3.425 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 14-16

Web: SP-FT/PG#2 2 x 4 except

SP-FT/PG#2 2x 6: 7-13, 10-11

SP-FT/PG 2400/1.8 2 x 4: 3-16, 6-15, 6-14, 7-14, 8-12, 10-12

SP-FT/PG 2400/1.8 2 x 6: 1-17, 8-13

19-3 [Qty. 1] SP-FT/PG 2400/1.8 2 x 6

Scabs Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber l	Forces	Tabl	e indicates: M	ember ID	, max CS	I, max axial for	ce, (max come	n. force i	f different 1	from max axial	force). Only f	ntes ones	terthan 3/	Ollhe am etwas	n in this table.
TC	1-2	0.590	-5,084 lbs		4-5	0.729	-8,111 lbs		7-8	0.853	3.146 lbs	(-294 lbs)			AZILIS ENG CHECHY.	II III UIII (ADIC,
	2-3	0.269	-5,086 lbs	1	5-6	0.875	-8,131 lbs		8-9	0.755	1,186 lbs	(-79 lbs)				- 1
	34	0.607	-10,145 lbs	/	6-7	0.853	-2,776 lbs		9-10	0.229	1,139 lbs	(-81 lbs)				
BC	12-13	0.232	-2,725 lbs		14-15	0.353	4,654 lbs	(-517 lbs)	16-17	0.724	7,195 lbs	(-1,048 lbs)				
	13-14	0.320	739 lbs	(-92 lbs)	15-16	0.666	10,000 lbs	(-1,348 lbs)	17-18	0.087	-371 lbs	()				
Web	1-18	0.753	-4,657 lbs		4-16	0.113	391 lbs	(-32 lbs)	7-14	0.528	4,273 lbs	(484 lbs)	10-12	0.806	-3,449 lbs	
	1-17	0.423	5,376 lbs	(-505 lbs)	4-15	0.455	-2,438 lbs	. 1	7-13	0.811	-5,274 lbs	(10.22)	10-11	0.729	3,486 Ibs	(-364 lbs)
	2-17	0.286	-1,198 lbs		5-15	0.307	-1,416 lbs		8-13	0.713	-6.426 lbs				51.00100	(557 100)
	3-17	0.555	3,665 lbs		6-15	0.520	4,207 lbs	(-604 Ibs)	8-12	0.564	4,557 lbs	(-537 lbs)				
	3-16	0.456	3,685 lbs	(423 lbs)	6-14	0.560	-4,438 lbs	1	9-12	0.450	-1,159 lbs	,==, ==,				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

Valley Truss 133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508

Truss: T12A

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:00
Page: 2 of 2

SPAN	THIVET	A 200 T							/
	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING /	WGT/PLY
40-8-8	2/12	1	2-10-8	0-0-0	0-0-0	0-0-0	1		
				000	000	0-0-0		24 in	397 lbs

3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) A creep factor of 1.50 has been applied for this truss analysis.

6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 11 may need to be considered.

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bols, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3 - 19 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

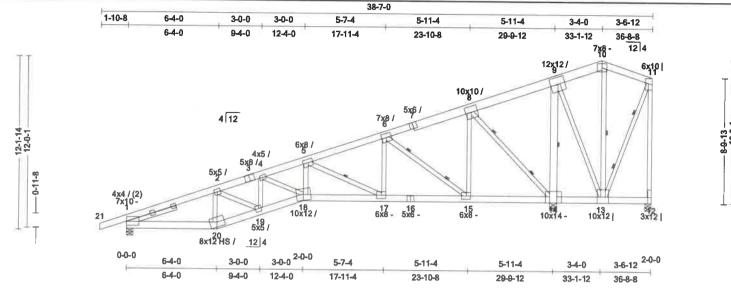
14) Listed wind uplift reactions based on MWFRS & C&Cloading.

Truss: T13

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:00 Page: 1 of 2

SPAN	PTICH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
36-8-8	4/12	4	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	337 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	Bldg Code: I	BC2018/ IPI 1-2014 Yes 00 %	CSI TC: 0.85 (21-1) BC: 0.73 (18-19) Web: 0.85 (9-14)	Deflection VertTL: 0.65 ir VertLL: 0.51 ir HorzTL: 0.2 in	-,	(loc) (17-18) (17-18) 14	Allowed L/240 L/360
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Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
1	1	5.5 in	3.23 in	3,710 lbs		-167 lbs	-475 lbs	-475 lbs	433 lbs
14	1	5.5 in	3.48 in	9,677 lbs	:÷:	-600 lbs	-1.194 lbs	-1.194 lbs	755 105
12	1	5.5 in	1.50 in	310 lbs	-2,779 lbs	-29 lbs	2,127,200	-2,779 lbs	80

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 7-10

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 8-14, 9-13 SP-FT/PG 2400/1.8 2 x 6: 9-14

Bracing

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.

BC: Sheathed or Purlins at 5-9-0, Purlin design by Others.

Web: One Midpoint Row: 5-17, 9-14, 10-13 Two Third Point Rows: 6-15, 8-14, 11-13

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- 4) This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber l	Forces	Table	indicates; M	ember ID), max CS	I, max axial for	oe, (max comp	n. force i	f different i	from max axial	force). Only fo	nties one	aterthan 3	Miles are school	n in this table.
TC	1-2	0.721	-6,183 lbs		5-6	0.725	-4,182 lbs		9-10	0.674	1,155 lbs	(44 lbs)				
	24	0.460	-7,706 lbs		6-8	0.749	-852 lbs		10-11	0.311	1,132 lbs	(-50 lbs)				- 1
-	4-5	0.490	-8,204 lbs		8-9	0.750	2,990 lbs	(-279 lbs)			-,	()				
BC	13-14	0.228	-2,537 lbs		15-17	0.336	3,794 lbs	(447 lbs)	18-19	0.731	7,585 lbs	(-1,067 Ibs)	20-1	0.525	5,725 lbs	(-849 lbs)
_	14-15	0.310	600 lbs	(-92 lbs)	17-18	0.727	7,541 lbs	(-1,023 lbs)	19-20	0.523	5,966 lbs	(-887 Ibs)			0,742 200	(OI)
Web	2-20	0.277	-1,873 lbs		5-18	0.610	2,112 lbs	(-250 lbs)	8-15	0.700	2,427 lbs	(-274 lbs)	10-13	0.475	-1.234 lbs	
	2-19	0.501	1,735 lbs	(-197 lbs)	5-17	0.784	-4,164 lbs		8-14	0.790	4,600 lbs	(=, , , , ,	11-13	0.765	-2,845 lbs	1
	4-19	0.170	-1,146 lbs		6-17	0.582	2,016 lbs	(-219 lbs)	9-14	0.846	-6,021 lbs	- 1	11-12	0.818	2,834 lbs	(-295 lbs)
	4-18	0.158	548 lbs	(-37 lbs)	6-15	0.804	-3,997 lbs		9-13	0.485	3,918 lbs	(473 lbs)			-,	(200 120)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALLD ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:01

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SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 36-8-8 1-10-8 4 0-0-0 0-0-0 0-0-0 24 in 337 lbs

7) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 12 may need to be considered.

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T14

09/16/2024

Job: CBS0306SA-8-HIESCO COMPLETE

Designer:Shane Allen
Date: 09/16/24 08:08:39
Page: 1 of 2

4/12	3	OHL 1-10-8	OHR 000	CANTL 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 347 lbs
			38	-7-0				
			4/12 3 1-10-8	4/12 3 1-10-8 0-0-0	4/12 3 1-10-8 000 000	4/12 3 1-10-8 0-0-0 0-0-0 0-0-0	4/12 3 1-10-8 0-0-0 0-0-0 1	4/12 3 1-10-8 0-00 0-00 0-00 1 24 in 38-7-0

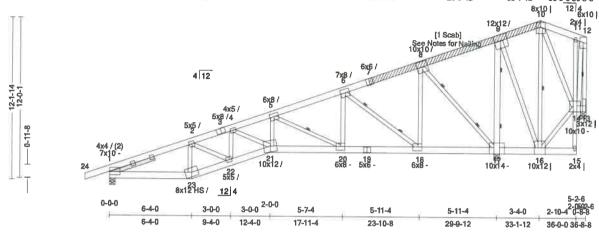
38-7-0
1-10-8 6-4-0 3-0-0 3-0-0 5-7-4 5-11-4 5-11-4 3-4-0 2-10-4 0-8-8
6-4-0 9-4-0 12-4-0 17-11-4 23-10-8 29-8-12 33-1-12 36-0-38-8-8

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.

BC: Sheathed or Purlins at 5-9-0, Purlin design by Others.

Web: One Midpoint Row: 5-20, 10-16, 12-14

Two Third Point Rows: 6-18, 8-17, 9-17



All plates shown to be Eagle 20 unless otherwise noted.

Color	TC: 0.85 (24-1)	Vert TL: 0.65 in Vert LL: 0.51 in Horz TL: 0.2 in	L/ L/540 L/681	(loc) (20-21) (20-21) 17	Allowed L/240 L/360
-------	-----------------	---	----------------------	-----------------------------------	---------------------------

Reaction
JT Brg Combo Brg Width Rqd Brg Width Max React Max Grav Uplift Max MWFRS Uplift Max

JT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
- 1	1	5.5 in	3.23 in	3,702 lbs		-167 lbs	-458 lbs	-458 lbs	480 lbs
17	1	5.5 in	3.49 in	9,721 lbs	6	-599 lbs	-1.241 lbs	-1.241 lbs	.00 25
13	1	5.5 in	1.50 in	340 lbs	-2,815 lbs	-47 lbs	¥	-2,815 lbs	- 2

Material

TC: SP-FT/PG2400/1.8 2 x 6

BC: SP-FT/PG2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 8-17, 9-16, 10-16

SP-FT/PG 2400/1.8 2 x 6: 9-17

Scales

7-10 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Loode

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated,

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TC LL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

ı	Men	nber l	Forces	Tabl	e indicates: M	ember ID	, max CSI,	max axial force	(max compr.	force if d	ifferent from	max axial force	. Only forces	greater th	an 300lbs:	are shown in th	is table
L	TC	1-2	0.720	-6,163 lbs		5-6	0.726	-4,149 lbs	•	9-10	0.479	1,224 lbs	(43 lbs)				
1		24	0.458	-7,675 lbs		6-8	0.720	-820 lbs		10-11	0.184	364 lbs	(-106 lbs)				- 1
ı		4-5	0.487	-8,164 lbs		8-9	0.525	2,990 lbs	(-313 lbs)	11-12	0.374	469 lbs	(-34 lbs)				
	BC	16-17	0.240	-2,532 lbs		20-21	0.724	7,502 lbs	(-1,104 lbs)	23-1	0.523	5,707 lbs	(-902 lbs)				
ı		17-18	0.323	559 lbs	(-63 lbs)	21-22	0.728	7,554 lbs	(-1,138 lbs)				,,				- 1
ı	_	18-20	0.336	3,764 lbs	(-513 lbs)	22-23	0.522	5,946 lbs	(943 lbs)								
1	₩ ∆	2-23	0.276	-1,866 lbs		5-20	0.782	-4,156 lbs		9-17	0.669	-6,030 lbs		11-14	0.448	-829 lbs	
		2-22	0.497	1,723 lbs	(-213 lbs)	6-20	0.581	2,013 lbs	(-227 lbs)	9-16	0.475	3,842 lbs	(493 lbs)		0.396	-2,434 lbs	- 1
		4-22	0.169	-1,137 lbs		6-18	0.807	-4,010 lbs		10-16	0.631	-2,455 lbs	(,	12-13	0.815	2,825 lbs	(-335 lbs)
ı		4-21	0.155	537 lbs	(49 lbs)	8-18	0.701	2,429 lbs	(-280 lbs)	10-14	0.574	1.988 lbs	(-228 lbs)			40-0 100	(500 200)
П		5-21	0.608	2,108 lbs	(-263 lbs)	8-17	0.798	-4.563 lbs		16-14	0.507	-1.744 lbs	(

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane

of the truss. Lateral braces shall be installed within 6 "of each web panel point.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM FAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-8-HIESCO COMPLETE

Designer/Shane Allen Date: 09/16/24 08:08:39

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36-8-8	4/12	3	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	347 lbs	
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

8) Indicates non-structural members.

9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.

10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

14) Incising is not permitted.

15) Scab 7 - 10 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 16) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

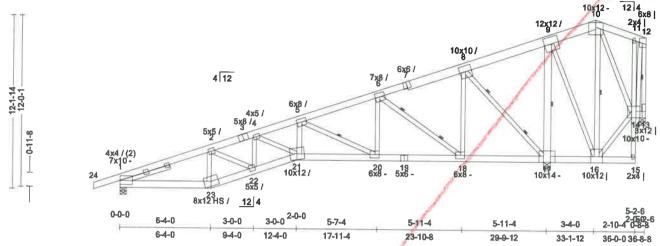
Truss: T14

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:02
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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
36-8-8	4/12	3	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	357 lbs
				38	-7-0				





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	Conceol	CCT	T 0 4		- /		
		CSI	Deflection	1	L/	(loc)	Allowed
TCLL: 110	Bldg Code: IBC 2018/	TC: 0.85 (24-1)	Vert TL:	0.65 in	L/538	(20-21)	L/240
TCDL: 10	TPI 1-2014	BC: 0.73 (21-22)	VertII:	0.51 in	L/680	(20-21)	L/360
BCLL: 0	Rep Mbr: Yes	Web: 0.85 (9-17)	Horz TL:	0.2 in	_,	17	27500
BCDL: 10	Lumber D.O.L.: 100 %		1	0.2.11		17	

Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
1	1	5.5 in	3.23 in	3,708 lbs		-168 lbs	-459 lbs	-459 lbs	480 lbs
17	1	5.5 in	3.48 in	9.690 lbs	1	-595 lbs	-1.237 lbs	-1.237 lbs	-100 IUS
13	1	5.5 in	1.50 in	337 lbs	-2.789 lbs	-49 lbs	-1,227,103	-1,237 lbs	. 8

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 7-10

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 8-17,9-16, 10-16

SP-FT/PG 2400/1.8 2 x 6: 9-17

Bracing

TC: Sheathed or Purlins at 3-1-0, Purlin design by Others.

BC: Sheathed or Puriins at 5-9-0, Puriin design by Others. Web: One Midpoint Row: 5-20, 9-17, 10-16, 12-14

Two Third Point Rows: 6-18, 8-17



1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber l	Forces	Tabl	e indicates: M	lember III.), max CS	I, max axial for	ce, (max comp	n force it	f different i	forn max axial	force). Only f	MUSS PRE	terthan 3	OOThs are show	m in this table
TC	1-2	0.721	-6,178 Ibs		5-6	0.725	-4,173 lbs	Î	9-10	0.731	1.164 lbs	(41 lbs)			OCIDS (40 BLIOW	عدما مان بندند
	24	0.460	-7,697 lbs		6-8	0.747	-840 lbs		10-11	0.267	359 lbs	(-98 lbs)				
	4-5	0.489	-8,193 lbs		8-9	0.762	2,993 lbs	(-317 lbs)	11-12	0.358	463 lbs	(-33 lbs)				
BC	16-17	0.234	-2,533 lbs		20-21	0.726	7,530 lbs	(-1,108 lbs)	23-1	0.525	5,720 lbs	(-904 lbs)				
	17-18	0.317	588 lbs	(-64 lbs)	21-22	0.730	7,576 lbs	(-1,140 lbs)	l		-,	(50.50)				'
	18-20	0.337	3,786 lbs	(-516 lbs)	22-23	0.523	5,960 lbs	(-944 lbs)								
Web	2-23	0.277	-1,871 lbs	15	5-20	0.783	-4,161 lbs		9-17	0.846	-6.020 lbs		11-14	0.430	-795 lbs	
	2-22	0.500	1,732 lbs	(-214 lbs)	6-20	0.582	2,015 lbs	(-227 lbs)	9-16	0.474	3,829 Ibs	(493 lbs)		0.394	-2.420 lbs	- 1
	4-22	0.170	-1,143 lbs	9	6-18	0.805	-4,002 lbs		10-16	0.609	-2,393 lbs	(.50)	12-13	0.808	2,799 lbs	(-332 lbs)
	4-21	0.157	544 lbs	(-50 lbs)	8-18	0.700	2,427 lbs	(-280 lbs)	10-14	0.555	1,923 lbs	(-215 lbs)		0.000	29122110	(552 108)
	5-21	0.609	2,111 lbs	(-263 lbs)	8-17	0.787	-4,578 lbs		16-14	0.481	-1,721 lbs	(===)				1

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REPER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE MEDAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508
 0

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:02

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SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 36-8-8 4/12 1-10-8 0-0-0 0-0-0 0-0-0 24 in 357 lbs

Brace bottom chord with approved sheathing or purlins per Bracing Summary.

4) At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member. 6) A creep factor of 1.50 has been applied for this truss analysis.

7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

14) Listed wind uplift reactions based on MWFRS & C&C loading.

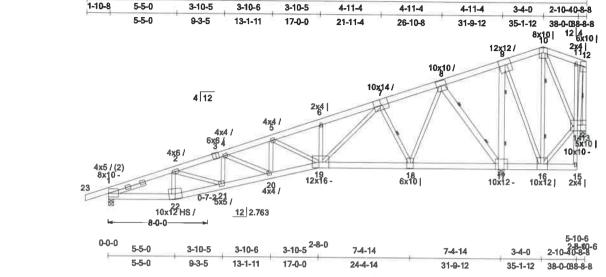
ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FACLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T15

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:03
Page: 1 of 2

SPAN	PTICH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	_
38-8-8	4/12	2	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	370 lbs	
	4			40	-7-0					-



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General	CSI	Deflection	I/	(loc)	Allowed
TCLL: 110	Bldg Code: IBC 2018/	TC: 0.93 (8-9)	VertTL: 0.68 in	L/547	(19-20)	L/240
TCDL: 10	TPI 1-2014	BC: 0.75 (22-1)	VertLL: 0.54 in	L/688	(19-20)	L/360
BCLL: 0	Rep Mbr: No	Web: 0.82 (10-16)	HorzTL: 0.18 in	_,	17	27,000
BCDL: 10	Lumber D.O.L.: 100 %					

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift:	Max C&CUplift	Max Uplift	Max Horiz
1	1	5.5 in	3.44 in	3,941 lbs		-177 lbs	-483 lbs	-483 lbs	521 lbs
17	1	5.5 in	3.69 in	10,286 lbs	*	-635 lbs	-1.322 lbs	-1.322 lbs	221 200
13	1	5.5 in	1.50 in	380 lbs	-3,098 lbs	-56 lbs	. 10	-3,098 lbs	. 8

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG #2 2 x 4 except:

SP-FT/PG#2 2 x 6: 8-17 SP-FT/PG2400/1.8 2 x 4: 7-19, 8-18, 9-16, 12-13

SP-FT/PG 2400/1.8 2 x 6; 9-17

Bracing

TC Sheathed or Purlins at 2-6-0, Purlin design by Others.
 BC Sheathed or Purlins at 5-2-0, Purlin design by Others.

Web: One Midpoint Row: 7-18, 12-14

Two Third Point Rows: 8-17, 9-17, 10-16



Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber l	Forces	Tabl	e indicates: M	ember II	O, max CSI	, max axial for	ce, (max comp	or. force i	f different i	from max axial	force). Only fi	OTDES ETER	tërthan 30	Olhs are shown	in this table.
TC	1-2	0.723	-6,744 lbs		5-6	0,435	-6,637 lbs	Ī	8-9	0.926	3.227 lbs	(-346 lbs)		0.398	511 lbs	(-32 lbs)
	2-4	0.647	-8,333 lbs		6-7	0.653	-6,685 lbs		9-10	0.830	1,361 lbs	(-50 lbs)		0100	211 100	(32100)
	4-5	0.529	-8,057 lbs		7-8	0.828	-1,456 lbs		10-11	0.435	403 lbs	(-98 lbs)				
BC	16-17	0.255	-2,806 lbs		20-21	0.737	8,005 lbs	(-1,223 lbs)								
	18-19	0.349	2,741 lbs	(-366 lbs)	21-22	0.688	6,322 lbs	(-1,020 lbs)	1				l))			
-	19-20	0.710	7,709 lbs	(-1,149 lbs)	22-1	0.750	6,225 lbs	(-1,004 lbs)								
Web	2-22	0.208	-1,502 lbs		5-19	0.529	-1,614 lbs		8-17	0.683	-4,647 lbs		16-14	0.548	-1.887 lbs	
	2-21	0.502	1,739 lbs	(-208 lbs)	6-19	0.209	-999 lbs		9-17	0.694	-6,259 Ibs		11-14	0.455	-842 lbs	- 1
	4-21	0.126	-821 lbs		7-19	0.615	4,969 lbs	(-746 lbs)	9-16	0.537	4,340 lbs	(-567 lbs)	12-14	0.438	-2,689 lbs	- 1
	4-20	0.115	-394 Ibs		7-18	0.639	-3,630 lbs	1	10-16	0.816	-2,741 lbs	,,	12-13	0.384	3,108 lbs	(-375 lbs)
	5-20	0.093	322 Ibs	(-14 lbs)	8-18	0.446	3,605 lbs	(-468 lbs)	10-14	0.614	2,128 lbs	(-250 lbs)		0.001	5,100 200	(313 203)

Note

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252		Job:	gner: Shane Allen 09/12/24 08:0	V-HIESCO COMPLETE 10:03
SPAN	PTICH 4/12	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
38-8-8		2	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	370 lbs

 Brace bottom chord with approved sheathing or purlins per Bracing Summary.
 At least one web of this truss has been designed with a panel point in the web. All panel points on such webs shall be braced laterally perpendicular to the plane of the truss. Lateral braces shall be installed within 6 "of each web panel point."

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 13 may need to be considered.

9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

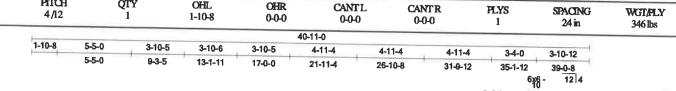
14) Listed wind uplift reactions based on MWFRS & C&C loading.

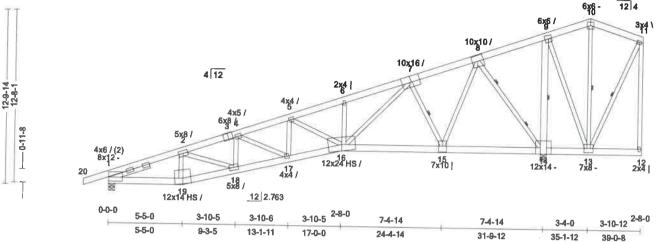
Truss: T16

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:04

Page: 1 of 2





All plates shown to be Eagle 20 unless otherwise noted.

Continue	CSI TC: 0.95 (2-4) BC: 0.97 (16-17) Web: 0.81 (8-14)	Deflection L/409 Vert TL: 0.91 in L/409 Vert LL: 0.73 in L/510 Cant/OHTL: 0.28 in UP 2L/482 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 How TL: 0.27 in L/510 Cant/OHLL: 0.28 in UP 2L/590 Cant/OHLL: 0.28 in UP 2L/590	(16-17) L (16-17) L 12 2L	llowed /240 /360 ./120 ./120
----------	---	---	---------------------------------	--

Reaction

SPAN

39-0-8

PITCH

QTY

OHL

JT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Unlift	May Links	Max Horiz
1	1	5.5 in	4.00 in	4,591 lbs		-212 lbs	-553 lbs	-553 lbs	455 lbs
14	I	5.5 in	2.36 in	6,574 lbs		-442 lbs	-1.091 lbs	-1.091 lbs	755 65

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 7-16, 8-15, 8-14 SP-FT/PG 2400/1.8 2 x 6: 9-14

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 8-1-0, Purlin design by Others.

Web: One Midpoint Row: 7-15, 9-14, 10-13, 11-13

Two Third Point Rows: 8-14

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607,1

				~	-		-TF		ATTENDED	C 1001,1	L					
	ember i	Forces	Tabl	e indicates; M	lember III), max CS	I, max axial fo	rce, (max com	or. force i	f different	firm may aviol	frees) Only 6		ntoughou 21	Villa and d	vn in this table.
TC	1-2	0.938	-8,296 lbs		5-6	0.675	-9.940 lbs	, ,	189	0.819	1,088 lbs	(142 flax)	uius gie	ater unan 30	JUIDS are show	m in this table.
	2-4	0.953	-10,674 lbs		6-7	0.760	-9.980 lbs		9-10	0.720	354 lbs	(-143 lbs)				
-	4-5	0.721	-10,954 lbs		7-8	0.820	-4,263 lbs		10-11	0.461	543 lbs	(-145 lbs) (-68 lbs)				- 1
BC	13-14	0.140	-967 lbs		15-16	0.600	5,514 lbs	(-516 lbs)		0.940	10,293 lbs			0.050		
	14-15	0.368	2,528 lbs	(-163 lbs)	16-17	0.973	10,524 lbs	(-1,304 lbs)		0.863	7,775 lbs	(-1,344 lbs)	19-1	0.952	7,658 lbs	(-1,067 lbs)
Web	2-19	0.261	-1.884 lbs		6-16	0.205	-983 lbs	(1,00 V 105)	8-14			(-1,084 lbs)				
	2-18	0.750	2,600 lbs	(-268 lbs)		0.678	5,484 lbs	(-782 lbs)		0.813	-4,894 lbs		11-13	0.249	-704 lbs	
	4-18	0.194	-1.271 lbs	(200 223)	7-15	0.684	-3,889 lbs			0.376	-2,602 lbs					
	5-16	0.411	-1.253 lbs		8-15	0.481			9-13	0.531	1,838 lbs	(-324 lbs)				
		01114	Theory 100		0-12	U.401	3,888 lbs	(-485 lbs)	10-13	0.425	-1.095 lbs					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

0-0-0

Truss: T16

24 in

CBS0306SA-7-HIESCO COMPLETE Job: Designer: Shane Allen

346 lbs

Date: 09/12/24 08:00:05

Page: 2of2 OHL OHR CANTL CANTR **PLYS** SPACING WGT/PLY 0-0-0

0-0-0

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

1-10-8

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

QTY

1

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

SPAN

39-0-8

12) Listed wind uplift reactions based on MWFRS & C&C loading.

PITCH

4/12

⁹⁾ All connectors, i.e. nails, sciews, bolts, trus s plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

Truss: T17

CBS0306SA-7-HIESCO COMPLETE Job:

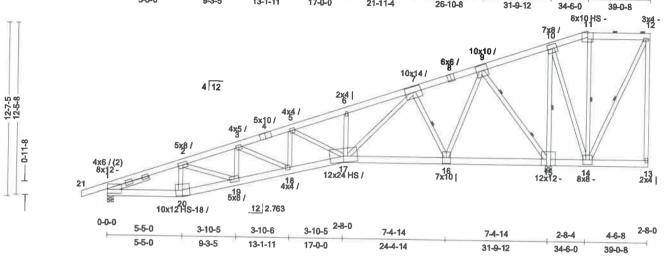
Designer: Shane Allen Date: 09/12/24 08:00:06

Page: 1 of 2

09/12/202

QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 39-0-8 4/12 1-10-8 0-0-0 0.0000-0-0 24 in 342 lbs 40-11-0

1-10-8 5-5-0 3-10-5 3-10-6 3-10-5 4-11-4 4-11-4 4-11-4 2-8-4 4-6-8 5-5-0 9-3-5 13-1-11 17-0-0 21-11-4 26-10-8 31-9-12 34-6-0 39-0-8



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCL: 110 TCDL: 10 BCL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.85 (21-1) BC: 0.89 (17-18) Web: 0.86 (10-15)	Deflection VertTL: 0.92 in VertLL: 0.74 in Cant/OHTL: 0.34 in U Cant/OHTL: 0.28 in U Horz TL: 0.28 in		(loc) (17-18) (17-18) 13 13	Allowed L/240 L/360 2L/120 2L/120
---	---	---	---	--	---	---

Reaction

SPAN

PITCH

Л	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Unlift	May Links	Max Horiz
1	1	5.5 in	4.03 in	4.624 lbs		-199 lbs	-552 lbs	-552 lbs	
15	1	5.5 in	3.39 in	6.441 lbs	ш				493 lbs
				0,777 100		-455 lbs	-1.039 lbs	-1.039 lbs	1.0

Material

TC SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except

SP-FT/PG 2400/1.8 2 x 4: 7-17, 9-16, 9-15

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 8-5-0, Purlin design by Others.

Web: One Midpoint Row: 7-16, 10-15, 11-14, 12-14

Two Third Point Rows: 9-15

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following use defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer TC	1-2	Forces 0.811	-8,374 lbs	indicates: M	ember II 5-6	0, max CS 0.605	T, max axial for -10.108 lbs	rce, (max com	pr. force i 19-10	f different	fiom max axial 1,144 lbs	force), Only f	oroes grea	ater than 3	00fbs are show	n in this table.
	2-3 3-5	0.819	-10,793 lbs -11,101 lbs		6-7 7-9	0.663	-10,141 lbs		10-11	0.601	433 lbs	(-124 lbs)				- 1
BC	14-15	0.178	-1,020 lbs		16-17	0.844	-4,396 lbs 5,702 lbs	(-556 lbs)	11-12	0.603	413 lbs 10,409 lbs	(-110 lbs) (-1,374 lbs)		0.861	7 721 11.	/1 000 W 3
Web	15-16 2-20	0.364	2,626 lbs	(-163 lbs)	_	0.889	10,668 lbs	(-1,339 lbs)	19-20	0.782	7,849 lbs	(-1,107 lbs)		0.001	7,731 lbs	(-1,089 lbs)
1112	2-19	0.763	2,644 lbs	(-275 lbs)	6-17 7-17	0.203 0.676	-970 lbs 5,467 lbs	(-783 lbs)	9-15 10-15	0.832	-4,967 lbs -2,540 lbs		12-14	0.424	-986 Ibs	
	3-19 5-17	0.198 0.411	-1,293 lbs -1,254 lbs		7-16 9-16	0.697 0.485	-3,961 lbs 3,923 lbs	(-484 lbs)	10-14 11-14	0.608	2,108 lbs -1,202 lbs	(-291 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALLD ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:06

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS 39-0-8 SPACING WGT/PLY 4/12 1-10-8 0-0-0 0-0-0 0-0-0 24 in 342 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolls, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Listed wind uplift reactions based on MWPRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALLOF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE MEDAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T18

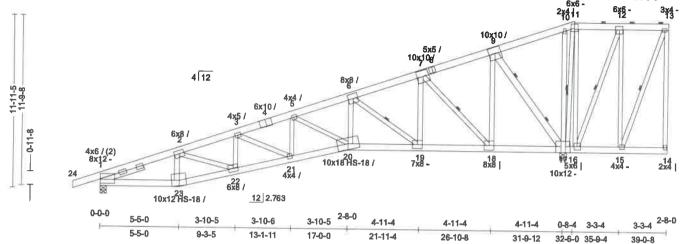
CBS0306SA-7-HIESCO COMPLETE

28-0

Designer: Shane Allen Date: 09/12/24 08:00:08 Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
39-0-8	4/12	1	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	372 lbs





All plates shown to be Eagle 20 unless otherwise noted.

			Allowed L/240 L/360 2L/120 2L/120
--	--	--	---

Reaction

JT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUblift	Max Uplift	Max Horiz
1	1	5.5 in	4.11 in	4,713 lbs		-208 lbs	-560 lbs	-560 lbs	466 lbs
17	1	5.5 in	3.28 in	6,216 lbs		-446 lbs	-1 008 lbs	-1 009 lbs	700108

Material

TC: SP-FT/PG2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG#2 2x 6: 9-17

SP-FT/PG 2400/1.8 2 x 4: 7-18,9-18

Bracing

TC: Sheathed

BC: Sheathed or Purlins at 8-5-0, Purlin design by Others.

Web: One Midpoint Row: 6-19, 7-18, 10-17, 11-17, 12-16, 13-15

Two Third Point Rows: 9-17

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

- 4) This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

			Forces	Table	indicates: M	ember ID	, max CS	I, max axial for	ce, (max comp	or, florce in	f different i	from max axial	force). Only f	nerces ones	terthan 30	Olhe om ohors	n in this table,
1	TC	1-2	0.842	-8,588 lbs		5-6	0.635	-10,571 lbs	i i	9-10	0.877	1,215 lbs	(-202 lbs)		ace when Do	OTOS SEC SELOW	THI HUS LADSE,
1		2-3	0.855	-11,118 lbs		6-7	0.778	-6.527 lbs		10-11	0.546	965 lbs	(-154 lbs)				- 1
l .		3-5	0.687	-11,487 lbs		7-9	0.855	-3,170 lbs		11-12	0.719	949 lbs	(-150 lbs)				
1	BC	16-17	0.926	-1,016 lbs		19-20	0.849	9,748 lbs	(-1,084 lbs)		0.803	8,048 lbs	(-1,105 lbs)				
1		17-18	0.423	2,773 lbs	(-190 lbs)	20-21	0.944	11,033 lbs	(-1,340 lbs)		0.885	7,928 lbs	(-1,088 lbs)				1
L.		18-19	0.493	6,030 lbs	(-547 lbs)	21-22	0.882	10,730 lbs	(-1,377 lbs)		UNDO	7,020 103	(-11000 IDD)				
1	Web	2-23	0.271	-1,955 lbs		6-20	0.862	2,986 lbs	(-352 lbs)		0.755	-5,134 lbs		12-15	0.195	677 lbs	(-146 lbs)
ı		2-22	0.799	2,768 lbs	(-280 lbs)	6-19	0.844	4,620 lbs	(10-17	0.329	-968 lbs		13-15	0.202	-575 lbs	(-140 106)
1		3-22	0.209	-1,364 Ibs		7-19	0.838	2,904 lbs	(-346 lbs)		0.479	-1,488 lbs		1545	0.202	-575 IOS	
		3-21	0.096	332 lbs	(-177 lbs)	7-18	0.762	-4,698 lbs	(= / 0 200)	11-16	0.327	1,133 lbs	(-171 lbs)				- 1
		5-20	0.400	-1,220 lbs		9-18	0.421	3,403 lbs	(-375 lbs)		0.767	-2,186 lbs	(-171108)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🗵 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:08

Page: 2 of 2

SPAN	PITCH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
39-0-8	4/12	1	1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	372 lbs
The second second									V 1 = 100

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY IRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹⁰⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

OHR

OHL

Truss: T19

09/16/2024

PLYS

Job: CBS0306SA-8-HIESCO COMPLETE

> WGT/PLY 288 lbs

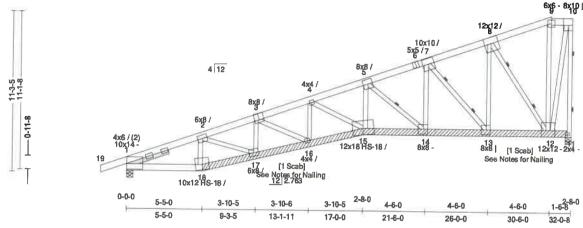
Designer:Shane Allen Date: 09/16/24 08:08:44 Page: 1 of 2

SPACING

32-0-0	4/12	1	1-10-8	0-0-0)	0-0-0	0-0-0	1	24 in	
					33-11-0					
	1-10-8	5-5-0	3-10-5	3-10-6	3-10-5	4-6-0	4-6-0	4-6-0	1-6-8	
		5-5-0	9-3-5	13-1-11	17-0-0	21-6-0	26-0-0	30-6-0	32-0-8	
TT									66 - 8x10	
								5	the III	

CANTL

CANTR



All plates shown to be Eagle 20 unless otherwise noted.

PITCH

QTY

LUC	BC LECHT								
JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Ublift	Max Unlift	Max Horiz
- 1	1	5.5 in	4.26 in	4,885 lbs					
				4,000 108	•	-230 lbs	-589 lbs	-589 lbs	440 lbs
- 11	1	5.5 in	2.20 in	4.822 lbs		216 11-	EEO II	ee0 2	11020
	-	III	anad III	حريد الله	•	-316 lbs	-559 lbs	-559 lbs	- 00

Bracing

TC: Sheathed

Material

SPAN

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FI/PG#2 2x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 2-17, 5-15, 7-14, 8-13, 8-12, 10-12, 10-11 Scales

Web: One Midpoint Row: 5-14 Two Third Point Rows: 7-13, 8-12, 10-11 11-15 [Qty: 1] SP-FT/PG 2400/1.8 216:68 [Qty: 1] SP-FT/PG 2400/1.8 2 x 6

BC: Sheathed or Puriins at 9-5-0, Puriin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and umbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL= 157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL= 1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TC LL= 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

					-	-		4.4									
		ber	Forces	Tab	le indicates: M	ember II), max CSI	max axial force	(max compr.	force if d	lifferent from	n max axial fi a c	e) Only forces	owater fl	on 2000be	om phonon in ti	rio talala
I	C	1-2	0.901	-8.978 lbs		4-5	0.685	-11.401 lbs		18-9	0.799	-1.156 lbs	mp camy source	Eranz a	BELL D'OULDS	TITE STIPOWIT III II	TRE CHOICE
		2-3		-11,675 lbs		5-7	0.759	-7,511 lbs		9-10	0.264	-1,150 lbs					- 1
		3-4	0.731	-12,225 lbs		7-8	0.768	-4.238 lbs				01020		ı			ı
Ē	C	12-13	0.407	3,804 lbs	(-321 lbs)	14-15	0.517	10.534 lbs	(-1,172 lbs)	16.17	0.504	11.358 lbs	(-1,416 lhs)	10.1	0.050	0.005.0	
		13-14	0.282	6,946 lbs	(-710 lbs)	15-16	0.561	11,747 lbs	(-1,380 lbs)		0.452	8,417 lbs	(-1,120 lbs)		0.953	8,295 lbs	(-1,103 lbs)
V	Veb	2-18	0.287	-2,068 lbs		4-15	0.358	-1,094 lbs	, , ,	7-13	0.832	-4.689 lbs		10-12	0.540	4 220 8	(4000)
	- 1	2-17	0.363	2,938 lbs	(-292 lbs)	5-15	0.377	3,047 lbs	(-333 lhs)		0.469	3,791 lbs	(401 lbs)			4,370 lbs	(487 lbs)
		3-17	0.229	-1,498 lbs		5-14	0.791	4,623 lbs		8-12	0.807		(-HUI IDS)	10-11	0.717	-4,318 lbs	- 1
	- 1	3-16	0.127	440 lbs	(4 The)		0.775	2 020 160	(224 %)		0.505	-5,294 lbs					1

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Indicates non-structural members.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			7	Valley Tru 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252		Job:	igner:Shane Allen e: 09/16/24 08:0	8-HIESCO COMPLI 08:44
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
32-0-8	4/12	1	1-10-8	0-0-0	000	000	1	24 in	288 lbs

⁹⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

16) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR IN INTHE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND ANIL ABILE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹¹⁾ All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

¹²⁾ Who the area paring retreated as the state of the sta

OHR

0-0-0

Truss: T19

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:09

Page:	1 of 2	
PLYS	SPACING	WGT/PLY
1	24 in	312 lbs

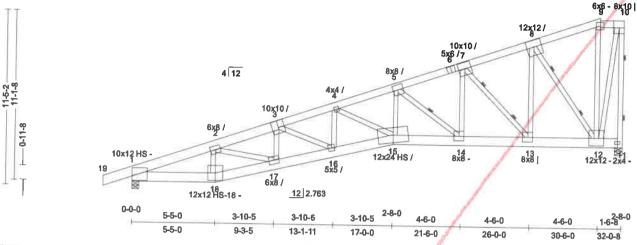
				33-11-0				
1-10-8	5-5-0	3-10-5	3-10-6	3-10-5	4-6-0	4-6-0	4-6-0	1-6-8
	5-5-0	9-3-5	13.1.11	17.0.0	24.0.0			32.0.8
	5-5-0	9-3-5	13-1-11	17-0-0	21-6-0	26-0-0	30-6-0	_

CANTL

0-0-0

CANTR

0-0-0



All plates shown to be Eagle 20 unless otherwise noted.

I anding (mat)	Comount		CICIT						
Loading (psf)	General		CSI		Deflection	E	T/	(loa)	A 10 1
TCLL: 110	Bldg Code:	IBC2018/		0.00 00 00		1		(loc)	Allowed
	Ding Code.	IDC 2018/	IC:	0.80 (8-9)	Vert TL;	0.97 in	L/386	(15-16)	L/240
TCDL: 10		TPI 1-2014	RC.	0.86 (15-16)				/	
BCLL: 0	D 30				Vert LL:	0.78 in	L/480	(15-16)	L/360
BULL: U	Rep Mbr:	Yes	Weh:	0.83 (2-17)	Horz TL:	0.28 in		11	-,,,,,,
RCDI · 10	Lumber DOI	. 100 07	1100	0.00 (2 17)	LIUIZ IL.	U.20 III		11	

Reaction

SPAN

32-0-8

PITCH

4/12

QTY

OHL

1-10-8

JI	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWIRS Uplift	Max C&CUblift	Max Unlift	Max Horiz
1	1	5.5 in	4.26 in	4.885 lbs					
11	4			-,	*	-231 lbs	-590 lbs	-590 lbs	437 lbs
11	I.	5.5 in	2.20 in	4.822 lbs	₩.	-315 lbs	-558 lbs	EE0 11	.01.200
				,		212102	-220 IDS	-558 lbs	10.00

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except SP-FT/PG 2400/1.8 2 x 8: 19-3 SP-FT/PG 2400/1.8 2 x 8

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 5-15, 7-14, 8-13, 8-12, 10-12, 10-11

Bracing TC: Sheathed

BC: Sheathed or Purlins at 9-4-0, Purlin design by Others.

Web: One Midpoint Row: 5-14

Two Third Point Rows: 7-13, 8-12, 10-11

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	ember l	Forces	i Table	e indicates: N	ferriber II), max CS	, max axial for	rce, fraex come	or. Nomoe i	f different	from mer erial	form) Only 6	-	ntanthan 21	0006	n in this table.
TC	1-2 2-3 3-4	0.706 0.554 0.751	-9,569 lbs -12,212 lbs -12,518 lbs		4-5 5-7 7-8	0.695 0.765 0.768	-11,612 lbs -7,610 lbs -4,278 lbs		8-9 9-10	0.797 0.266	-1,160 lbs -845 lbs	inace, Only is		ata (1281 5)	DUIDS ARE STIDY	vn m unis table.
BC	12-13 13-14	0.532 0.434	3,842 lbs 7,042 lbs	(-325 lbs) (-720 lbs)		0.787 0.857	10,718 lbs 12,009 lbs	(-1,191 lbs) (-1,408 lbs)		0.778	11,758 lbs 8,924 lbs	(-1,458 lbs) (-1,167 lbs)		0.805	8,795 lbs	(-1,149 lbs)
Wet	2-18 2-17 3-17 4-15	0.309 0.827 0.180 0.367	-2,335 lbs 2,867 lbs -1,265 lbs -1,150 lbs	(-294 lbs)	5-15 5-14 7-14 7-13	0.384 0.791 0.379 0.827	3,108 lbs -4,698 lbs 3,065 lbs -4,739 lbs	(-338 lbs) (-325 lbs)	8-13 8-12	0.470 0.796 0.488 0.539	3,803 lbs -5,311 lbs -512 lbs 4,354 lbs	(401 lbs)	10-11	0.706	4,299 lbs	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purtins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TOALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

	Valley Truss
13	33 Range View Loop
V	Vestcliffe, CO 81252
P	hone (719) 371-8508

OHL

1-10-8

Truss: T19

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

3121bs

Designer: Shane Allen Date: 09/12/24 08:00:10

SPACING

24 in

Page: 2 of 2

PLYS

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

OHR

0-0-0

CANTL

0-0-0

CANTR

0-0-0

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

QTY

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

SPAN

32-0-8

13) Listed wind uplift reactions based on MWFRS & C&C loading.

PITCH

4/12

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

OHR

0-0-0

Truss: T20

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:12

I rago.	1012	
PLYS	SPACING	WGT/PLY
1	24 in	276 lbs

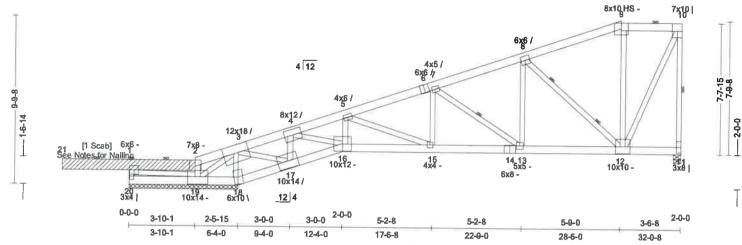
					35-11-0			
3-10-8	3-10-1	2-5-15	3-0-0	3-0-0	5-2-8	5-2-8	5-9-0	3-6-8
	3-10-1	-6-14 ⁶⁻⁴⁻⁰	9-4-0	12-4-0	17-6-8	22-9-0	28-6-0	32-0-8

CANTL

0-0-0

CANTR

0-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10	Bldg Code: IBC2018/ TPI 1-2014	CSI TC: 0.95 (8-9) BC: 0.49 (15-16)	Deflection Vert TL: 0.35 in Vert LL: 0.28 in	L/ L/877 L/999	(loc) (15-16) 15	Allowed L/240 L/360
BCLL: 0 BCDL: 10	Rep Mbr: No	Web: 0.80 (3-18)	Horz TL: 0.28 in	L/999	15	L/360

Reaction

SPAN

32-0-8

PITCH

0/12

QTY

OHL

3-10-8

11 1 5.5 in 2.71 in 3,111 lbs222 lbs -395 lbs -395 lbs 18 1 76 in N/A 8,510 lbs -562 lbs -1,070 lbs 5								
18 1 76 in N/A 8,510 lbs -562 lbs -1,070 lbs -1,070 lbs 5	JT	Max React		d Brg Width Max React Max Grav	Jplift Max MWFRS Uplif	ft Max C&CUplift	Max Uplift	Max Horiz
-1,070 IDS -1,070 IDS 3	11	3,111 lbs		2.71 in 3,111 lbs	-222 lbs	-395 lbs	-395 lbs	
	18	8,510 lbs	n N/A	N/A 8,510 lbs .	-562 lbs	-1.070 lbs		5.639 lbs
19 1 76 in N/A 439 lbs -2,990 lbs · -2,990 lbs -3	19	439 lbs	ı N/A	N/A 439 lbs -2,990	os ·	•	-,	-5,639 lbs
20 1 76 in N/A 2,817 lbs -187 lbs -187 lbs -187 lbs	20	2,817 lbs	n N/A	N/A 2,817 lbs	-187 lbs	-138 lbs		24 lhs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 21-2

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 3-19, 3-17, 4-16, 10-12

Bracing

TC. Sheathed or Purlins at 3-5-0, Purlin design by Others.

BC: Sheathed or Purlins at 3-1-0, Purlin design by Others.

Web: One Midpoint Row: 7-13, 10-11 Two Third Point Rows: 8-12

21-2 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Pactored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

ı					_											
		Forces	Table	indicates: M	ember II), max CSI	, max axial for	ce, (max comp	pr. force i	f different i	from max axial	force), Only fi	oroes grea	ter than 36	00lbs are shown in t	his table
TC	1-2	0.896	2,720 lbs	(-381 lbs)	4-5	0.313	-4,785 lbs		8.9	0.950	-1.795 lbs		1			1
	2-3	0.652	2,934 lbs	(-380 lbs)	5-7	0.785	-5,698 lbs		9-10	0.597	-1,362 lbs					
	3-4	0.510	348 Ibs	(-150 lbs)	7-8	0.904	-4,283 lbs		1	0.0077	1,500,100					- 1
BC	12-13	0.384	3,817 lbs	(-349 Ibs)	15-16	0.487	4,583 lbs	(-603 lbs)								_
	13-15	0.480	5,238 lbs	(-569 lbs)	17-18	0.477	-6,380 lbs	(1							
Web	1-20	0.343	-2,780 lbs		3-17	0.735	5,944 lbs	(-713 lbs)	7-13	0.345	-1,737 lbs		10-11	0.731	-3.065 lbs	
	1-19	0.720	-2,796 lbs		4-17	0.476	-3,606 lbs	. 1	8-13	0.325	1,126 lbs	(-88 lbs)	.011	0.751	D,OOD HAS	
	2-19	0.371	1,284 lbs	(-80 lbs)	4-16	0.610	4,929 lbs	(-546 lbs)	8-12	0.726	-3,260 lbs	(00 20)				
	3-19	0.495	4,004 lbs	(-587 lbs)	5-16	0.202	-1,520 lbs	(0.10.100)	9-12	0.654	-803 Ibs	1				
	3-18	0.798	-6,066 lbs	, (5-15	0.203	703 lbs		10-12	0.410	3.311 lbs	(-384 The)				- 1

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.

TrueBuild®Truss Software v5.7.12 Eagle Metal Products

09/12/2024

Truss: T20

CBS0306SA-7-HIESCO COMPLETE Designer: Shane Allen

Date: 09/12/24 08:00:13

Page: 2 of 2

QTY OHL OHR CANTL CANTR 32-0-8 PLYS SPACING 0/12 WGT/PLY 3-10-8 0-0-0 0-0-0 0-0-0 1 24 in 276 lbs 4) Brace bottom chord with approved sheathing or purins per Bracing Summary.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 150 has been applied for this truss analysis.

PITCH

- 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information. 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 19 may need to be considered.
- 9) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

10) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 11) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and
- 12) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

13) Incising is not permitted.

SPAN

14) Scab 2 - 21 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - pty) Screws @ 6 oc.

12-4-0

2-2-14

Truss: T21

Job: CBS0306SA-8-HIESCO COMPLETE

Designer:Shane Allen 09/16/24 08:08:48 Date: Page: 1 of 2

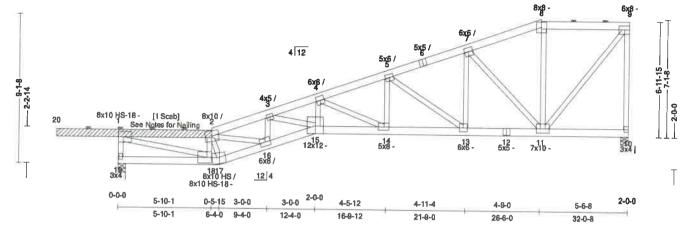
32-0-8

SPAN 32-0-8	PITCH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 259 lbs
4					35-11-0				
24	3-10-8	5-10-1	3-5-15	3-0-0	4-5-12	4-11-4	4-9-0	5-6-8	-1
19		5-10-1	9-4-0	12-4-0	16.0.12	21.0.0	20.00	2000	⊣

16-9-12

21-9-0

26-6-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General TCLL: 110 Bdg Code: IBC 2018/ TCDL: 10 TPI 1-2014 BCLL: 0 Rep Mbr: Yes BCDL: 10 Lumber D.O.L.: 100 %	BC: 0.85 (14-15) Vert LL: (0.89 in L/418 (14-15) L/240 0.72 in L/520 15 L/360 0.21 in 10
--	-----------------------------	---

Reaction 09/16/2024 Brg Width JT Brg Combo Rqd Brg Width Max React Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift Max Horiz 19 5.5 in 2.40 in 5.512 lbs -317 lbs -607 lbs -607 lbs 301 lbs 10 5.5 in 1.79 in 4,104 lbs -530 lbs -530 lbs

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4 except: SP-FT/PG 2400/1.8 2 x 4: 1-18 Bracing

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Scabs

20-2 [Qty: 1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TC LL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	mber :	Forces	Table	indicates: M	ember ID	, max CSI,	max axial force	, (max comor. :	force if di	General from	n max axial force)	Onlyfores	arrester th	on ANNIbe	— chooses in thi	o toble	
TC	1-2 2-3	0.903	-5,541 lbs -7,852 lbs		34 45	0.547 0.355	-9,146 lbs -5,893 lbs	`	5-7 7-8	0.258	-3,592 lbs -1,740 lbs	Omy made	8-9	0.361	-1,532 lbs	s table.	I
BC	11-13 13-14	0.266 0.430	3,285 lbs 5,531 lbs	(-340 lbs) (-622 lbs)		0.846 0.801	8,422 lbs 7,787 lbs	(-1,002 lbs) (-954 lbs)	16-17	0.502 0.583	5,004 lbs 5,262 lbs	(616 lbs)					-
Web	1-19 1-18 2-18 2-17	0.332 0.715 0.270 0.140	-2,675 lbs 5,780 lbs -2,177 lbs -1,125 lbs	(-597 lbs)	2-16 3-16 3-15 4-15	0.770 0.163 0.366 0.580	2,669 lbs -1,305 lbs 1,268 lbs 2,010 lbs	(-325 lbs) (-124 lbs) (-228 lbs)	5-14 5-13	0.481 0.418 0.505 0.461	-3,178 lbs 1,448 lbs -2,735 lbs 1,596 lbs	(-153 lbs)	7-11 8-11 9-11	0.575 0.090 0.706 0.418	-2,520 lbs -438 lbs 2,446 lbs -2,025 lbs	(-293 lhs))

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			7	Valley Trus 33 Range View Westcliffe, CO 8 hone (719) 371	Loop 31252		Job:	gner:Shane Allen : 09/16/24 08:0	8-HIESCO COMPLE 08:49
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANT R	PLYS	SPACING	WGT/PLY
32-0-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	2	24 in	259 lbs

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolts, truss plates, etc. to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Scab 2 - 20 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Nails or Gun Nails [min 0.135"x3"] @ 6 oc.

17) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWINGARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

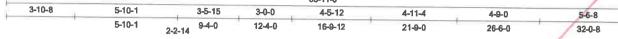
Truss: T21

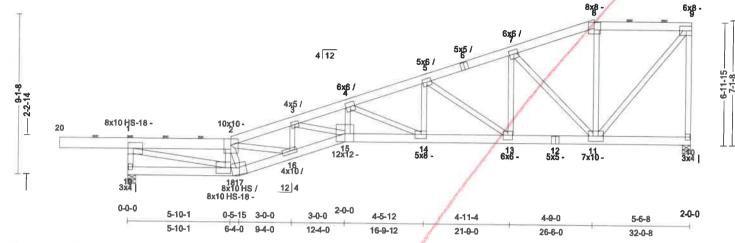
Job: CBS0306SA-7-HIESCO COMPLETE

2-0-0

Designer: Shane Allen
Date: 09/12/24 08:00:14
Page: 1 of 2

SPAN	9/12	QTY	OHL	OHR	CANTL	CANT'R	PLYS	SPACING	WGT/PLY
32-0-8		1	3-10-8	0-0-0	0-0-0	0-0-0	2	24 in	266 lbs
1				35-	11-0			/	





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCL: 110 TCDL: 10		CSI TC: 0.78 (1-2) BC: 0.85 (14-15)	 .89 in	L/ L/421 L/524	(loc) (14-15)	Allowed L/240 L/360
BCLL: 0 BCDL: 10	Rep Mbr: Yes Lumber D.O.L.: 100 %	Web: 0.71 (9-11)	 1.71 in 1.21 in	L/524	15 10	L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift M	fax MWFRS Uplift	Max C&CUplift	Max Uplift	Max Horiz
19	1	5.5 in	2.40 in	5.512 lbs	y	-317 lbs	-607 lbs	-607 lbs	302 lbs
10	1	5.5 in	1.79 in	4.104 lbs	. /	-284 lbc	-530 lbs	620 lbs	302 108

Material

TC SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 20-2

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

eb: SP-FT/PG#2 2 x 4 except: SP-FT/PG2400/1.8 2 x 4: 1-18

Bracing

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

3963

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

		Forces		e indicates: M	lember ID	, max CS	, max axial for	ce, (max compr. for	e if different	from max axial	force). Only f	orces gre	ater than 30	001bs are shown	n in this table
TC	1-2	0.777	-5,417 lbs		3-4	0.547	-9,144 lbs	5-7	0.258	-3,592 lbs		189	0.361	-1.532 lbs	
	2-3	0.488	-7,930 lbs		4-5	0.355	-5,894 lbs	7-8	0.382	-1,740 lbs		"	010-02	**************************************	'
BC	11-13	0.266	3,285 lbs	(-340 lbs)	14-15	0.849	8,420 lbs	(-1,002 lbs) 16-1	7 0.511	5,508 lbs	(-679 lbs)				
	13-14	0.430	5,532 lbs	(-623 lbs)	15-16	0.805	7,805 Ibs	(-956 lbs) 17-1	8 0.612	5,579 lbs	(-686 lbs)				
Web	1-19	0.331	-2,673 lbs	F.	2-16	0.642	2,224 lbs	(-269 lbs) 4-14	0.480	-3,175 lbs		7-11	0.575	-2.520 lbs	
	1-18	0.695	5,620 lbs	(-575 lbs)	3-16	0.136	-1,089 lbs	5-14	0.418	1.448 lbs	(-154 lbs)		0.090	-438 lbs	- 1
	2-18	0.246	-1,988 lbs	1	3-15	0.359	1,243 lbs	(-121 lbs) 5-13	0.505	-2.735 lbs		9-11	0.706	2,446 lbs	(-294 lbs)
	2-17	0.163	-1,310 lbs	/	4-15	0.580	2,009 lbs	(-228 lbs) 7-13		1,596 lbs	(-167 lbs)		0.418	-2,025 lbs	(*25+106)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:14
Page: 2 of 2

CIDANT	-								1
SPAN	PITCH	OTY	OHL	OHR	CANTL	CANTR	PLYS	CENA CENTO	/
32-0-8	0/12	`,					PLIS	SPACING	/ WGT/PLY
32-0-6	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	2.	24 in	266 lbs

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply

9) All fasteners minimum 2-12"long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

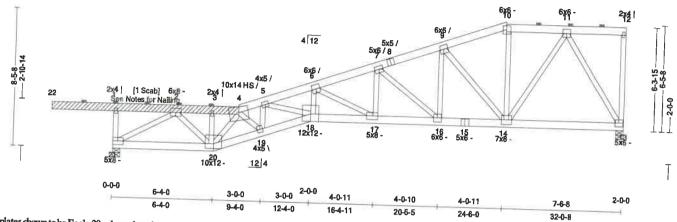
Truss: T22

09/16/207

Job: CBS0306SA-8-HIESCO COMPLETE

DesignerShane Allen Date: 09/16/24 08:08:53 Page: 1 of 2

	0/12	Qr 1	Y	OI 3-1		OHR 0-0-0		CANTI 000	_	CANT R 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 262.lbs
1	3 40 0						35-11-0						202108
1	3-10-8	3-11-1	2-4-15	1-6-1	1-5-15	3-0-0	4-0-11		4.4.40				
		3-11-1	6-4-0	7 10 1	0.40				4-0-10	4-0-11	3-9-4	3-9-4	
			0-4-0	2-1	0-14-0	12-4-0	16-4-11	r ~ =	20-5-5	24-6-0	20.2.4	20.00	



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	[C]		T					
TCLL: 110 TCDL: 10 BCLL: 0		Yes	CSI TC: 0.89 (1-2) BC: 0.86 (17-18) Web: 0.73 (2-20)	Deflection Vert TL: Vert LL: Horz TL:	0.76 in 0.61 in 0.23 in	L/ L/490 L/608	(loc) (17-18) (17-18) 13	Allowed L/240 L/360

Reaction

31	Brg Combo	Brg Width	Rod Brg Width	Max React	Max Grav Unité	Max MWFRS Uplift			
21	1	5.5 in	2.41 in	5,523 lbs	- Char Opan	Max MINTES OPER		Max Uplift	Max Horiz
13	1	5.5 in	1.79 in	4.104 lbs	8	-326 lbs	-600 lbs	-600 lbs	274 lbs
3.5				-y= 0 1 III3	-	-275 lbs	-511 lhs	-511 lbc	

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6

SPAN

32-0-8

PITCH

QTY

Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 5-6-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

22-4 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user definitions of the contract of the input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C. Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TC LL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me TC	2-3 3-4 4-5	0.182 0.223 0.464	-4,579 lbs -4,552 lbs -7,818 lbs		7-9	0, max CSI, 0.548 0.363 0.236	max axial force, -9,172 lbs -6,056 lbs -3,965 lbs		_		n max axial for -2,277 lbs -2,084 lbs	e). Only forces	greater (han 300libs	are shown in thi	is table.
Web	13-14 14-16	0.114 0.301 0.190	1,128 lbs 3,658 lbs	(-101 lbs (-390 lbs	17-18		5,673 lbs 8,457 lbs	(-629 lbs) (-972 lbs)		0.810	7,803 lbs 6,498 lbs	(919 lbs) (-766 lbs)		0.252	2,772 lbs	(422 lbs)
	2-21 2-20 3-20 4-20	0.476 0.728 0.074 0.456	-1,504 lbs -3,252 lbs 2,523 lbs -586 lbs -3,551 lbs	(-241 lbs)	4-19 5-19 5-18 6-18 6-17	0.440 0.153 0.369 0.571 0.450	1,526 lbs -1,226 lbs 1,279 lbs 1,979 lbs	(-183 lbs) (-125 lbs) (-214 lbs)	7-17 7-16 9-16 9-14	0.446 0.415 0.472 0.458	1,546 lbs -2,593 lbs 1,637 lbs -2,374 lbs	(-158 lbs) (-174 lbs)	11-13	0.478	-2,147 lbs	
Note	26				1017	u-30	-3,120 lbs		11-14	0.547	1,895 lbs	(-188 lbs)				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REPER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-8-HIESCO COMPLETE

Designer/Shane Allen Date: 09/16/24 08:08:53

Page:

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT R	PLYS	SPACING	WGT/PLY
32-0-8	0/12	1	3-10-8	0-0-0	000	000	2	24 in	2621bs
6) The forces shown	for this multi-nly to	iss are ner ply and the	reactions are for all of	ioc Thus identical to	neces shall be been been	L-1-7-1	1. 101		

Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120'x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

- 10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
- 11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 13) All connectors, i.e. nails, screws, bolts, truss plates, etc. to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Scab 4 - 22 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Nails or Gun Nails [min 0.135"x3"] @ 6 oc.

Truss: T22

CBS0306SA-7-HIESCO COMPLETE Tob:

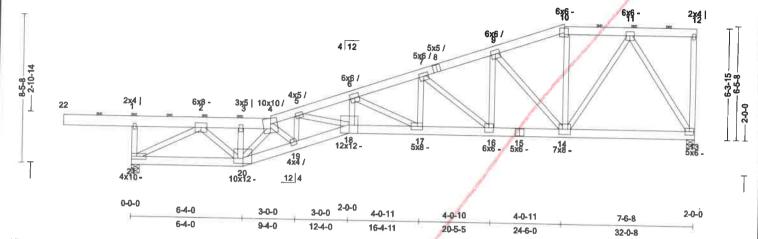
Designer. Shane Allen Date: 09/12/24 08:00:15

Page: 1 of 2

09/12/2024

OHR CANTL CANTR PLYS 32-0-8 **SPACING** WGT/PLY 0/12 3-10-8 0-0-0 0-0-0 0-0-0 268 lbs 35-11-0 3-10-8 3-11-1 2-4-15 1-6-1 1-5-15 3-0-0 4-0-11 4-0-10 4-0-11 394





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110	741 0 4	CSI TC: 088(22-1)	Deflection Verilli 077	I/	(loc)	Allowed
TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10		TC: 0.88 (22-1) BC: 0.95 (17-18) Web: 0.73 (2-20)	VertTL: 0.77 in VertLL: 0.62 in HorzTL: 0.23 in	L/484 L/600	(10c) (17-18) (17-18) 13	L/240 L/360

Reaction

SPAN

PITCH

OTY

OHT.

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift Ma	x MWFRS I Inli	ft Max C&C Listin	More I Latin	Max Horiz
21	1	5.5 in	2.41 in	5.523 lbs					MIX HOTZ
13	1	5.5 in				-325 lbs	-600 lbs	-600 lbs	275 lbs
10		J.J HI	1.79 in	4,104 lbs	* J	-276 lbs	-512 lbs	-512 lbs	147

Material

SP-FT/PG 2400/1.8 2 x 6 except SP-FT/PG 2400/1.8 2 x 8: 22-4 SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4

Bracing

TC: Sheathed or Purlins at 5-0-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60
- 4) This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

TC	2-3 3-4 4-5	0.233 0.239 0.533	Table -4,686 lbs -4,673 lbs -7,930 lbs	indicates; M	5-6 6-7 7-9	0,603 0.400 0.260	I, max axial force -9,176 lbs -6,055 lbs -3,965 lbs	ce, (max com	pr. force i 9-10 10-11	f difficient : 0.317 0.403	from max axial -2,277 lbs -2,084 lbs	force). Only fo	onces grea	eter than 30	00/lbs are show	n in this table.
BC	13-14 14-16	0.127 0.333	1,128 lbs 3,658 lbs	(-101 lbs) (-390 lbs)		0.495 0.950	5,673 lbs 8,458 lbs	(-630 lbs) (-973 lbs)		0.899	7,805 lbs 7,028 lbs	(-920 lbs) (-828 lbs)	20-21	0.287	2,853 lbs	(-392 lbs)
Web	1-21 2-21 2-20 3-20 4-20	0.186 0.482 0.727 0.101 0.471	-1,478 lbs -3,318 lbs 2,520 lbs -807 lbs -3,681 lbs	(-241 lbs)	4-19 5-19 5-18 6-18 6-17	0.287 0.106 0.370 0.574 0.450	994 lbs -853 lbs 1,282 lbs 1,988 lbs -3,122 lbs	(-122 lbs) (-125 lbs) (-215 lbs)	7-17 7-16 9-16	0.446 0.415 0.472 0.458 0.547	1,545 lbs -2,592 lbs 1,637 lbs -2,374 lbs 1,895 lbs	(-158 lbs) (-174 lbs)		0.478	-2,147 lbs	

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T22

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:16

Page:

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 32-0-8 0/12 3-10-8 0-0-0 θ 0-0-0 24 in 268 lbs

6) The forces shown for this multi-plytruss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply, 12d Nails or Gun Nails [min 0.135 x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120'x2.875"]@1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offiset. 8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

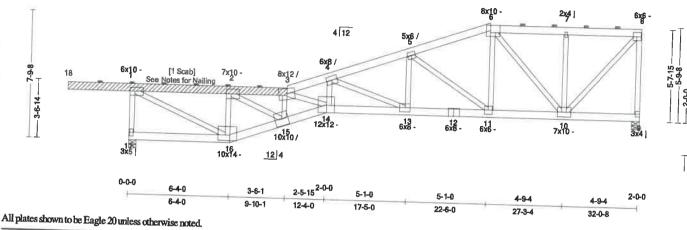
Truss: T23

09/16/2024

CBS0306SA-8-HIESCO COMPLETE Job:

Designer:Shane Allen Date: 09/16/24 08:08:57

SPAN	PITCH	OTTE					Pa	ge: 1 of 2	
32-0-8	0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANT R 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 257 lbs
	240.0				35-11-0				257 108
4	3-10-8	6-4-0	3-6-1	2-5-15	5-1-0				ly .
		6-4-0				5-1-0	4-9-4	4-9-4	
			3-10-1	-6-14 ¹²⁻⁴⁻⁰	17-5-0	22-6-0	27-3-4	32-0-8	<u>−</u> ‡



Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: Rep Mbr: Lumber D.O.L.;	IBC 2018/ TH 1-2014 Yes 100%	BC: 0	0.90 (1-2) 0.88 (13-14) 0.69 (8-10)	Deflection Vert TL: Vert LL: Horz TL:	0.83 in 0.67 in 0.22 in	L/ L/451 L/560	(loc) (13-14) (13-14) 9	Allowed L/240 L/360
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JT	Brg Combo	Brg Width	Rod Brg Width	Max React	Mor Constitute	M 1000000			
17	1	5.5 in	2.40 in		IVIAX CHAV Uplift	Max MWFRS Uplift N	Max C&C Unlift	Max Uplift	Max Horiz
Q	1			5,515 lbs		-334 lbs	-592 lbs		
,	1	5.5 in	1. 79 in	4,104 lbs		-267 lbs	-J72 IUS	-592 lbs	248 lbs

Material

Reaction

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-16, 2-15

Scale 18-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 6

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user define input: 110 psfRoof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated 2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOI_= 1.60

This truss has been designed for the effects of TC LL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

TC	mber 1-2 2-3	Forces 0.896 0.309	Tabl -3,564 lbs -7,752 lbs	le indicates: M			max axial force, -9,083 lbs -5,344 lbs		force if o	ifficent flor	-4,6/J IDS	e). Only forces	grateri 17-8	than 300ths 0.505	are shown in thi	s table.
BC Web	10-11 11-13	0.222 0.381 0.353	2,761 lbs 4,969 lbs -2,726 lbs	(-293 lbs) (-543 lbs)	13-14 14-15	0.878 0.830	8,398 lbs 7,728 lbs	(-936 lbs) (-867 lbs)		0.493	-1,586 lbs 3,636 lbs	(426 lbs)			1,500 106	
	1-16 2-16 2-15	0.490 0.359 0.600	3,961 lbs -2,772 lbs 4,848 lbs	(-391 lbs) (-529 lbs)	4-14	0.442 0.375 0.554 0.608	-3,547 lbs 1,300 lbs 1,920 lbs -3,694 lbs	(-137 lbs) (-188 lbs)		0.439 0.573 0.485 0.422	1,520 lbs -2,886 lbs 1,682 lbs -1,899 lbs	(-135 lbs) (-155 lbs)	8-10	0.154 0.689 0.325	-960 lbs 2,387 lbs -2,026 lbs	(-291 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer. 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T23

Job: CBS0306SA-8-HIESCO COMPLETE

Designer Shane Allen Date: 09/16/24 08:08:58

Page: 2 of 2

32-0		QIY 1	OHL 3-10-8	OHR 0-0-0	CANTL 000	CANTR 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 257 lbs	
6) The form	and a second of the second of				The state of the s					

own for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120'x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

- 10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
- 11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.
- 12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.
- 13) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

- 16) Scab 3 18 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Nails or Gun Nails [min 0.135"x3"] @ 6 oc.
- 17) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T23

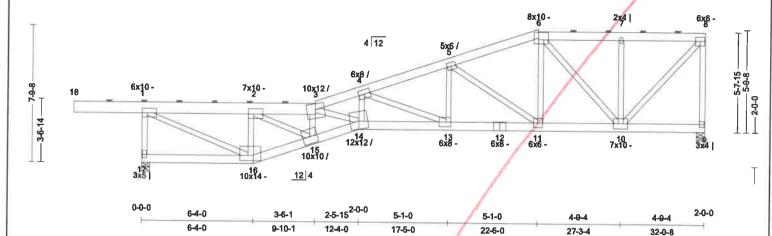
09/12/2024

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:17 Page:

32-0-8

OHL QTY OHR CANTL CANTR **PLYS** SPACING WGT/PLY 32-0-8 3-10-8 0-0-0 0-0-0 0-0-0 24 in 267 lbs 35-11-0 3-10-8 6-4-0 3-6-1 2-5-15 5-1-0 4-9-4 4-9-4 3-6-14 12-4-0 9-10-1 17-5-0 22-6-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General Bldg Code: IBC 2018/	CSI TC: 0.77 (1-2)	Deflection Ver II	L	(loc)	Allowed
TCDL: 10	TPI 1-2014	BC: 0.87 (13-14)	Vert TL: 0.84 in Vert LL: 0.67 in	L/446 L/554	(13-14) (13-14)	L/240 L/360
BCLL: 0 BCDL: 10	Rep Mbr: Yes	Web: 0.69 (8-10)	Horz TL: 0.22 in	,	9	2,500

Reaction

SPAN

PITCH

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	t Max MWFRS Uplift	Max C&CUplift	Max Unlift	Max Horiz
17	1	5.5 in	2.40 in	5,515 lbs		-334 lbs	-592 lbs	-592 lbs	249 lbs
9	1	5.5 in	1.79 in	4,104 lbs		-267 lbs	-493 lbs	493 lbs	247 103

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 18-3 SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-16, 2-15

Bracing

TC. Sheathed or Purlins at 5-1-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tennin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

		Forces	Table	indicates; M	lember ID	, max CS	C, max axial for	e, (max comp	or, aforce à	f different t	from max axial	force). Only f	oroes gre	ater than 30	Olbs are shown	in this table
TC	1-2	0.775	-3,624 lbs		34	0.582	-9,205 lbs	Ī	5-6	0.419	-2,873 lbs		7-8	0.505	-1.586 lbs	
	2-3	0.344	-7,602 lbs		4-5	0.345	-5,344 lbs		6-7	0.493	-1,586 lbs				-,2-00 100	
BC	10-11	0.222	2,761 lbs	(-293 lbs)		0.874	8,405 lbs	(-938 lbs)	15-16	0.426	3,695 lbs	(434 lbs)				
_	11-13	0.381	4,969 lbs	(-543 lbs)	14-15	0.865	8,273 Ibs	(-929 lbs)				()				
Web	1-17	0.350	-2,724 lbs		3-15	0.434	-3,495 lbs		5-13	0.439	1,522 lbs	(-135 lbs)	7-10	0.154	-960 lbs	
	1-16	0.496	4,008 lbs	(-396 lbs)	3-14	0.233	807 Ibs	(-82 lbs)	5-11	0.572	-2,885 lbs	(8-10	0.689	2.387 lbs	(-291 lbs)
	2-16	0.357	-2,777 lbs		4-14	0.642	2,223 Ibs	(-222 lbs)	6-11	0.485	1,681 lbs	(-155 lbs)		0.325	-2,026 lbs	(201100)
	2-15	0.564	4,563 lbs	(497 lbs)	4-13	0.610	-3,702 lbs	1	6-10	0.422	-1,899 lbs	,,			-,	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:17

Page: 2 of 2

SPAN PITCH QTY OHR CANTL CANTR PLYS 32-0-8 SPACING WGT/PLY 0/12 3-10-8 0.00 0-0-0 ഹ 24 in 267 lbs 6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d

Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 8) Lateral bracing shall be attached to each ply

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

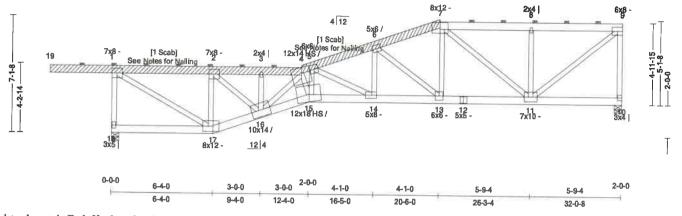
Truss: T24

CBS0306SA-8-HIESCO COMPLETE Job:

Designer:Shane Allen Date: 09/16/24 08:09:02 Page: 1 of 2

09/16/2024

SPAN 32-0-8	PITCH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANT L 000	CANTI 000	R PLYS	SPACING 24 in	WGT/PLY 261 lbs
					35-11-0				
78	3-10-8	6-4-0	3-0-0	2-6-1 0-5-15	4-1-0	4-1-0	5-9-4	5-9-4	1
		6-4-0	9-4-0	11-10-112-4-0	16-5-0	20-6-0	26-3-4	32-0-8	



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bkig Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.OL.: 100 %		Vert TL: 0.67 in Vert LL: 0.54 in Horz TL: 0.2 in	L/ L/555 L/695	(loc) (14-15) (14-15) 10	Allowed L/240 L/360
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Reaction

JT	Brg Combo		Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
18	1	5.5 in	2.39 in	5,487 lbs		-342 lbs	-585 lbs	-585 lbs	222 lbs
10	1	5.5 in	1.80 in	4,120 lbs	•	-259 lbs	-474 lbs	-474 lbs	(4)

Material

TC: SP-FT/PG 2400/1.8 2 x 6

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except: **Bracing**

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

SP-FT/PG 2400/1.8 2 x 4: 1-17, 2-16

Scales

19-4 [Qty:1] SP-FT/PG 2400/1.8 2 x46 [Qty:1] SP-FT/PG 2400/1.8 2 x 6

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TC LL= 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

TC	1-2 2-3 3-4	Forces 0.895 0.217 0.294	Tabl -2,871 lbs -5,441 lbs -5,441 lbs	e indicates: M	eniber ID 4-5 5-6 6-7	0.360 0.239 0.149	max axial force, -8,523 lbs -5,558 lbs -3,434 lbs	(тях сотрг.	force if d 7-8 8-9	0.579 0.607	1 max axial force -2,152 lbs -2,152 lbs). Only forces	greater th	an 300lbs	are shown in th	is table.	
BC	11-13 13-14	0.275 0.393	3,355 lbs 5,208 lbs	(-367 lbs) (-570 lbs)		0.868 0.882	8,023 lbs 8,219 lbs	(-871 lbs) (-890 lbs)		0.239	2,946 lbs	(-329 lbs)					-
₩b	1-18 1-17 2-17 2-16	0.365 0.413 0.347 0.427	-2,711 lbs 3,337 lbs -2,578 lbs 3,454 lbs	(324 lbs) (363 lbs)	4-15	0.105 0.509 0.228 0.495	-821 lbs -3,782 lbs 791 lbs 1,715 lbs		5-14 6-14 6-13	0.456 0.453 0.435 0.483	-3,152 lbs 1,571 lbs -2,656 lbs 1,675 lbs	(-139 lbs)	9-11	0.442 0.167 0.798 0.297	-1,868 lbs -1,141 lbs 2,764 lbs -2,026 lbs	(-324 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORIH IN THE FAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-8-HIESCO COMPLETE

Designer:Shane Allen Date: 09/16/24 08:09:02 Page: 2 of 2

SPAN 32-0-8	PITCH 0/12	QTY 1	OHIL 3-10-8	OHR 0-0-0	CANT L 000	CANT R 0-0-0	PLYS	SPACING	WGT/PLY
	0.12	-	2-10-0	0-0-0	0-0-0	0-0-0	2	24 in	261 lbs

6) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120'x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolks, truss plates, etc. to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

16) Scab 4-19 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Nails or Gun Nails [min 0.135" x3"] @ 6 oc.

17) Scab 4-7 to match size and grade of member to which it is attached. Attach with 2 staggered rows of 12d Naik or Gun Naik [min 0.135"x3"] @ 6 oc. 18) Listed wind uplift reactions based on MWFRS & C&C loading.

11-10-112-4-0

Truss: T24

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:19 Page: 1 of 2

32-0-8

5-9-4

32-0-8

SPAN 32-0-8	PITCH 0/12	QTY 1	OHIL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 2	SPACING 24 in	WGT/PLY 275 lbs
-				3	5-11-0				
Y	3-10-8	6-4-0	3-0-0	2-6-1 0-5-15	4-1-0	4.1.0	E 0.4	E 0.4	

16-5-0

20-6-0

4-1-0

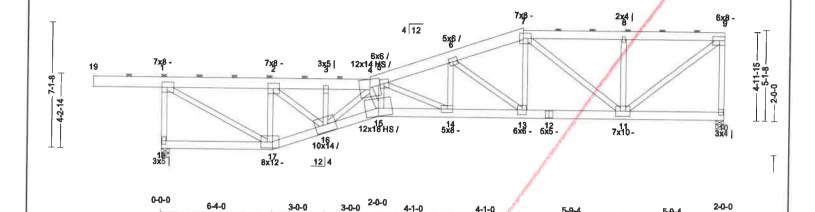
20-6-0

26-3-4

5-9-4

26-3-4

09/12/2024



16-5-0

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)		CSI	Deflection	L/	(loc)	Allowed
TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	TC: 0.77 (1-2) BC: 0.91 (15-16) Web: 0.80 (9-11)	Vert TL: 0.71 in Vert LL: 0.57 in Horz TL: 0.2 in	L/524 L/655	(14-15) (14-15) 10	L/240 L/360

9-4-0

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift !	Max C&CUblift	Max Unlift	Max Horiz
18	1	5.5 in	2.39 in	5,487 lbs		-342 lbs	-584 lbs	-584 lbs	223 lbs
10	1	5.5 in	1.80 in	4,120 lbs	36	-259 lbs	-474 lbs	-474 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 8 except: SP-FT/PG 2400/1.8 2 x 6: 7-9 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4 except: SP-FT/PG 2400/1.8 2 x 4: 1-17, 2-16 Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

6-4-0

		Forces	Table	indicates: M	ember ID	, max CSI	max axial for	ze, (max comp	or, force i	f different i	forn max axial	force). Only fi	mes one	sterthan 3/	Mile on chara	n in this table
TC	1-2 2-3 3-4	0.774 0.251 0.369	-2,916 lbs -5,557 lbs -5,557 lbs		4-5 5-6 6-7	0.459 0.310 0.251	-8,705 lbs -5,661 lbs -3,477 lbs		7-8 8-9	0.557 0.602	-2,147 lbs -2,147 lbs	ionally Only is	ACC GIO		JOSUS AIC SHOW!	II III IIIIS IZIME.
BC	11-13 13-14	0.273 0.401	3,326 lbs 5,326 lbs	(-364 lbs) (-585 lbs)	14-15	0.893	8,190 lbs 8,391 lbs	(-891 lbs) (-910 lbs)		0.249	2,992 lbs	(-335 lbs)				
₩ ₽	1-18 1-17 2-17 2-16	0.361 0.417 0.344 0.434	-2,711 lbs 3,373 lbs -2,583 lbs 3,507 lbs	(-327 lbs)	4-15	0.114 0.505 0.231 0.508	-897 lbs -3,782 lbs 802 lbs 1,759 lbs	11		0.457 0.445 0.443 0.490	-3,181 lbs 1,541 lbs -2,740 lbs 1,697 lbs	(-135 lbs) (-146 lbs)	9-11	0.434 0.163 0.796 0.297	-1,835 lbs -1,115 lbs 2,758 lbs -2,026 lbs	(-323 lbs)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 1252		Job:	mer: Shane Allen 09/12/24 08	
SPAN	PITCH	QTY	OHL	OHR	CANT L	CANTR	PLYS	SPACING	WGT/PLY
32-0-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0		24 in	275 lbs

b) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135'x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-12" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

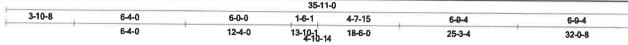
Valley Truss

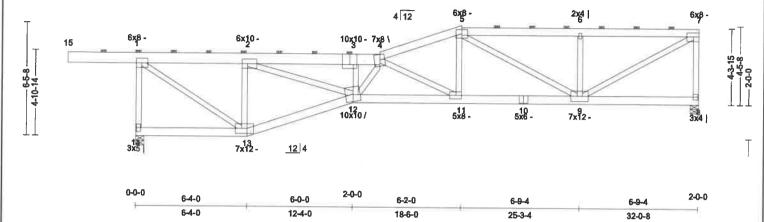
133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 Truss: T25

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:20
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
32-0-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	2	24 in	251 lbs
				25	11.0				





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) General	TC: 0.89 (1-2) BC: 0.75 (11-12)	Deflection VertTL: 0.63 in VertLL: 0.5 in Horz TL: 0.17 in	L/ L/593 L/753	(loc) (11-12) (11-12) 8	Allowed L/240 L/360
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Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	Max MWFRS Upli	ft Max C&CUplift	Max Uplift	Max Horiz
14	1	5.5 in	2.37 in	5,440 lbs		-350 lbs	-577 lbs	-577 lbs	197 lbs
8	1	5.5 in	1.90 in	4.353 lbs	9	-251 lbs	-455 lbs	-455 lbs	277 200

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 15-3, 3-4

BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG #2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-13, 2-12, 7-9

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loods

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B-L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Table	indicates: M	ember II), max CSI	, max axial forc	e, (max com	or. force	if different t	from max axial :	force). Only f	iorces gre	ater than 30	00lbs are show	n in this table.
TC	1-2	0.887	-2,546 lbs		34	0.336	-6,773 lbs	_	5-6	0.674	-2.889 lbs					
	2-3	0.411	-6,823 lbs		4-5	0.391	-4,379 lbs		6-7	0.707	-2.889 lbs					
BC	9-11	0.374	4,163 lbs	(444 Ibs)	12-13	0.421	2,609 lbs	(-272 lbs)								
	11-12	0.746	6,731 lbs	(-732 Ibs)	1		•	(,	l							
Web	1-14	0.380	-2,701 lbs		2-12	0.572	4,623 lbs	(494 lbs)	4-11	0.532	-3,291 lbs		69	0.182	-1.328 lbs	
	1-13	0.383	3,093 lbs	(-298 Ibs)	3-12	0.107	-851 lbs	,	5-11	0.469	1.625 lbs	(-120 lbs)	17.	0.414	3,345 lbs	(-373 lbs)
	2-13	0.345	-2,453 lbs		4-12	0.045	-355 lbs		5-9	0.533	-1,916 lbs	,	7-8	0.293	-2,137 lbs	(575108)

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis,

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T25

Joh: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:21 Date:

Page: 20f2

SPAN PITCH QTY OHL OHR CANTL 32-0-8 CANTR PLYS SPACING 3-10-8 WGT/PLY 0.000-0-0 0-0-0 6) The forces shown for this multi-ply truss are per ply and the reactions are for all piles. Two identical trusses shall be built and attached as follows, per ply: 12d 24 in 251 lbs

Nails or Gun Nails [min 0.135"x3"] TC-2 staggered rows @ 0-9-8 oc, BC-2 staggered rows @ 1-0-0 oc, Webs-1 row 10d Nails or Gun Nails [min

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and in stall half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset. 8) Lateral bracing shall be attached to each ply

9) All fasteners minimum 2-12" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

Truss: T26

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen 09/12/24 08:00:22 Date: Page: 1 of 2

32-0-8

32-0-8

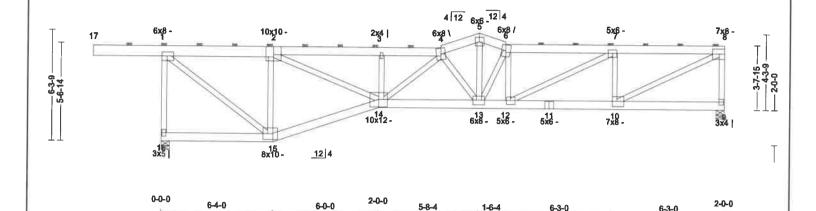
09/12/2024

SPAN 32-0-8	PTTCH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 2	SPACING- 24 in	WGT/PLY 255 lbs
V.				35-11-0					V
4	3-10-8	6-4-0	6-0-0	3-6-1	2-2-3	1-6-4 6	-3-0	6-3-0	

15-10-1 5-6-14-0-4 19-6-8 5-9-8

25-9-8

25-9-8



18-0-4

19-6-8

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCL: 0 BCDL: 10 BCDL: 10 BCDL: 10	lode: IBC 2018/ TPI 1-2014	CSI TC: 0.88 (1-2) BC: 0.53 (13-14) Web: 0.80 (1-15)	Deflection VertTL: 0.51 in VertLL: 0.4 in Horz TL: 0.14 in	L/ L/730 L/938	(loc) (13-14) (13-14) 9	Allowed L/240 L/360
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12-4-0

12-4-0

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplif	ît Max MWFRS Uplift I	Max C&CUplift	Max Uplift	Max Horiz
16	1	5.5 in	2.37 in	5,426 lbs		-359 lbs	-578 lbs	-578 lbs	168 lbs
9	1	5.5 in	1.90 in	4,350 lbs		-242 lbs	-448 lbs	-448 lbs	.00 25

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 17-2 SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 2-14, 8-10

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user

defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves. 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

6-4-0

6-4-0

Men	nber I	Forces	Table	indicates; M	enba ID	, max CSI	, max axial for	e, (max comp	n. force i	f different f	iom max axial	force). Only f	OKOEK ERES	terthan 30	Olhs are shown	in this table
TC	1-2	0.884	-2,170 lbs		3-4	0.380	-5,257 lbs	Ī	5-6	0.287	-4,388 lbs	,,	7-8	0.638	-3,323 lbs	
7	2-3	0.476	-5,209 lbs		4-5	0.287	-4,391 lbs		6-7	0.603	-4,717 lbs		, v		0,000	
BC	10-12	0.352	3,323 lbs	(-328 lbs)	13-14	0.527	5,231 lbs	(-561 lbs)								
	12-13	0.447	4,664 lbs	(489 lbs)	14-15	0.302	2,303 lbs	(-220 lbs)								
Web	1-16	0.409	-2,689 lbs		3-14	0.110	-851 lbs		6-13	0.186	-1.380 lbs		8-10	0.464	3.752 lbs	(403 lbs)
	1-15	0.800	2,773 lbs	(-267 lbs)	4-14	0.095	-645 lbs		6-12	0.117	-895 lbs		8.9	0.280	-2.138 lbs	(100,00)
	2-15	0.359	-2,358 lbs		4-13	0.275	-2,023 lbs		7-12	0.538	1.865 lbs	(-190 lbs)		0.200	2,100 100	- 1
	2-14	0.417	3,368 lbs	(-355 lbs)	5-13	0.637	2,206 lbs	(-245 lbs)	7-10	0.220	-1,678 lbs	, , , , , ,				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:22

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 32-0-8 0/12 3-10-8 0-0-0 θ 0-0-024 in 255 lbs

6) The forces shown for this multi-plytruss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 12d Nails or Gun Nails [min 0.135"x3"] TC - 2 staggered rows @ 0-9-8 oc, BC - 2 staggered rows @ 1-0-0 oc, Webs - 1 row 10d Nails or Gun Nails [min 0.120"x2.875"] @ 1-0-0 oc.

7) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

8) Lateral bracing shall be attached to each ply.

9) All fasteners minimum 2-1/2" long, unless otherwise noted.

10) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.

11) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 13) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 14) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

15) Incising is not permitted.

OHR

Truss: T27

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

239 lbs

88.5

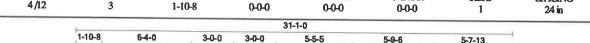
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Designer: Shane Allen Date 09/12/24 08:00:23

SPACING

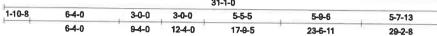
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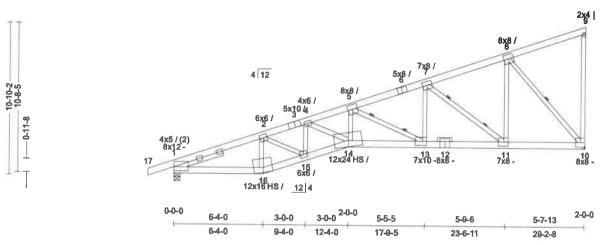
PLYS



CANTL

CANTR





OHL

QTY

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.85 (17-1) BC: 0.95 (13-14) Web: 0.87 (8-10)	Deflection VertTL: 0.8 in VertLL: 0.62 in HorzTL: 0.27 in	L/ L/431 L/552	(loc) (13-14) (13-14) 10	Allowed L/240 L/360 09/12/2024
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Reaction

SPAN

29-2-8

PITCH

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift 1	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.91 in	4,480 lbs		-207 lbs	-528 lbs	-528 lbs	428 lbs
10	1	1.5 in	Section 1	3.802 lbs		-296 lbs	-501 lbs	-501 lbc	

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2x 4 except SP-FT/PG 2400/1.8 2 x 4: 8-10 **Bracing**

TC: Sheathed or Purlins at 2-5-0, Purlin design by Others. BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.

Web: One Midpoint Row: 8-10

Two Third Point Rows: 5-13, 7-11

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Table	indicates; M	ember II), max CS	I, max axial for	ce, (max comp	n force i	f different f	iom max axial	force). Only fo	OFFICE ROOM	ater than 30	Offbs are shown	in this table
TC	1-2	0.726	-7,734 lbs		4-5	0,646	-10,818 lbs	1	7-8	0.719	-3.131 lbs		1		O COLOR CHE DESCRIPTION	AT GIRD GENER
	2-4	0.581	-9,707 lbs		5-7	0.715	-6,514 lbs			01125	5,151 100					
BC	10-11	0.249	2,772 lbs	(-257 lbs)	13-14	0.953	9,993 Ibs	(-1,176 lbs)	15-16	0.656	7.457 lbs	(964 lbs)				
	11-13	0.471	6,016 lbs	(-681 lbs)	14-15	0.924	9,592 lbs	(-1,200 lbs)	16-1	0.675	7,162 lbs	(-923 lbs)				
Web	2-16	0.345	-2,335 lbs		4-14	0.382	1,324 lbs	(-82 lbs)	7-13	0.686	2,378 lbs	(-188 lbs)		0.870	-4,228 lbs	
	2-15	0.730	2,530 lbs	(-257 lbs)	5-14	0.799	2,768 lbs	(-278 lbs)	7-11	0.798	-4.082 lbs		9-10	0.622	-554 lbs	
	4-15	0.253	-1,697 lbs		5-13	0.781	4,898 lbs	, ,	8-11	0.747	2,587 lbs	(-246 lbs)		0.000	33 T HAS	

Notes

1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.

2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation,

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:24

Page: 2 of 2

SPAN PICH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 29-2-8 1-10-8 0-0-0 0-0-0 0-0-0 24 in 239 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹⁰⁾ All connectors, i.e. nails, screws, boils, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

Truss: T28

PLYS

Tob: CBS0306SA-7-HIESCO COMPLETE

WGT/PLY

235 lbs

8-8-13

Designer: Shane Allen 09/12/24 08:00:25 Date: Page: 1 of 2.

SPACING

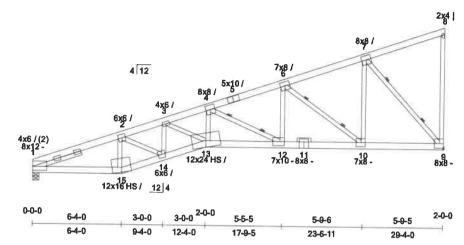
24 in

4/12	4/12	1	0-0-0	0-0-0	0-0		1)
		-			29-4-0			
		6-4-0	3-0-0	3-0-0	5-5-5	5-9-6	5-9-5	-
		6-4-0	9-4-0	12-4-0	17-0-5	22.6.11	20.4.0	\rightarrow

OHR

CANTL

CANTR



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.81 (7-8) BC: 0.97 (12-13) Web: 0.84 (4-12)	Deflection VertTL: 0.81 in VertLL: 0.63 in HorzTL: 0.28 in	L/ L/426 L/544	(loc) (12-13) (12-13) 9	Allowed L/240 L/360
		1	I.			09/12/202

Reaction

SPAN

29-4-0

10-8-13

0-11-8

1

PITCH

QTY

OHL

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplifi	Max MWFRS Uplift !	Max C&CUblift	Max Unlift	Max Horiz
1	1	5.5 in	3.54 in	4,065 lbs		-154 lbs	-434 lbs	-434 lbs	405 lbs
9	1	1.5 in		3,832 lbs	•	-299 lbs	-530 lbs	-530 lbs	

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG 2400/1.8 2 x 4: 7-9 Bracing

TC: Sheathed or Purlins at 2-4-0, Purlin design by Others. BC: Sheathed or Purlins at 8-6-0, Purlin design by Others.

Web: Two Third Point Rows: 4-12, 6-10, 7-9

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ci=1.00), DOL=1.00. Ventilated.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=1.5 ft, End Zone Truss, Both end webs considered. DCL=1.60

3) This truss has been designed for the effects of TCLL = 20 psf.

4) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	nber l	Forces	Table	e indicates; M	lember ID), max CS	I, max axial for	oe, (max comi	or, florce i	f different f	iom max axial	force) Only fr	ner ones	terthan 30	00lbs are shown in	a flain talala
TC	1-2	0.572	-8,171 lbs		3-4	0.657	-10.995 lbs		6-7	0.732	-3.211 lbs		l gam		COLOR DECORDANIA II	I UIIS IADRE.
-	2-3	0.598	-10,024 lbs		4-6	0.714	-6,621 lbs		"	0.752	Jul 1 103					- 1
BC	9-10	0.256	2,849 lbs	(-289 lbs)	12-13	0.968	10,158 lbs	(-1,341 lbs)	14-15	0.676	7,889 lbs	(-1,086 lbs)				
	10-12	0.477	6,117 lbs	(-757 lbs)	13-14	0.948	9,849 lbs	(-1,344 lbs)		0.644	7,578 lbs	(-1,040 lbs)				
Web	2-15	0.364	-2,464 lbs		3-13	0.349	1,208 lbs	(-99 lbs)	6-12	0.737	2,555 lbs	(-234 lbs)		0.724	-4,290 lbs	
	2-14	0.717	2,485 lbs	(-279 lbs)	4-13	0.819	2,836 lbs	(-304 lbs)		0.804	4.112 lbs	· - · /	89	0.628	-4,290 los -568 lbs	
	3-14	0.250	-1,679 lbs		4-12	0.842	-5,283 lbs	(7-10	0.752	2.607 lbs	(-279 lbs)		0.028	-200 IDS	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).
- 3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 1252		Jo	signer: Shane Allen te: 09/12/24 08:0	-HIESCO COMPLETI 025
SPAN	PITCH 4/12	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
29-4-0		1	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	235 lbs

⁸⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

¹³⁾ Listed wind uplift reactions based on MWFRS & C&C loading.

Valley Truss Truss: T29 133 Range View Loop CBS0306SA-7-HIESCO COMPLETE Job: Westcliffe, CO 81252 Designer: Shane Allen Phone (719) 371-8508 Date: 09/12/24 08:00:26 Page: 1 of 2 SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 24 in 157 lbs 23-1-0 3-10-8 5-8-8 5.8.8 3-10-12 3-10-12 5-8-8 11-5-0 19-2-8 2-10-13 2x4 4 12 7x10/ 8x10-5x5 -7хβ [1 Scab] 200 6x10 -5x8 10x12 0-0-0 0-0-0 5-8-8 5-8-8 7-9-8 5-8-8 11-5-0 19-2-8 All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC2018/ TPI 1-2014 Rep Mbr: Yes Lumber D.O.L.: 100 %	CSI TC: 0.79 (1-2) BC: 0.40 (7-9) Web: 0.62 (1-10)	VertTL: 0.23 in VertLL: 0.19 in Horz TL: 0.03 in	L/ L/934 L/999	(loc) 8 8 8 6	Allowed L/240 L/360
		The state of the s				09/12/202

Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Unlift	Max Unlift	Max Horiz
10	1	5.5 in	2.18 in	4,145 lbs	*	-245 lbs	-578 lbs	-578 lbs	201 lbs
6	1	5.5 in	1.50 in	2.527 lbs	- 4	-158 lbs	-343 lbc	-343 The	201105

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 11-3 BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-9,4-7

Bracing

TC: Sheathed or Purlins at 4-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others. Web: One Midpoint Row: 4-6

11-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 8

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DQL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber 1	Forces	Table	indicates: M	ember)	D, max CSI	max axial for	ce, (max come	nr. force	if different	from max axial :	force). Only f	nines ou	ester than 3/	Mile on chose	n in thic tobla
TC	1-2 2-3	0.787 0.247	-4,293 lbs -4,496 lbs		3-4	0.761	4,716 lbs					,, -,,,			SOUGH ALCOHOM	i iii uno tabic
BC	6-7	0.252	1,991 lbs	(-238 lbs)	7-9	0.404	4,293 lbs	(-684 lbs)	-				_			
Web	1-10 1-9	0.616 0.572	-4,072 lbs 4,628 lbs	(-737 lbs)	2-9 2-7	0.226 0.387	-1,646 lbs 1,341 lbs		3-7	0.368 0.456	-2,532 lbs 3,684 lbs	(452 lbs)	46 56	0.454 0.255	-2,774 lbs -540 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

Provide adequate drainage to prevent ponding.

4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:26

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 24 in 157 lbs 8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on

G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond

- 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 12) Incising is not permitted.
- 13) Scab 3-11 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc Z(TSLZ278, 2 ply) Screws @ 6 oc.
- 14) Listed wind uplift reactions based on MWFRS & C&C loading.

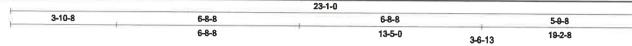
¹⁰⁾ All connectors, i.e. nails, screws, boils, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

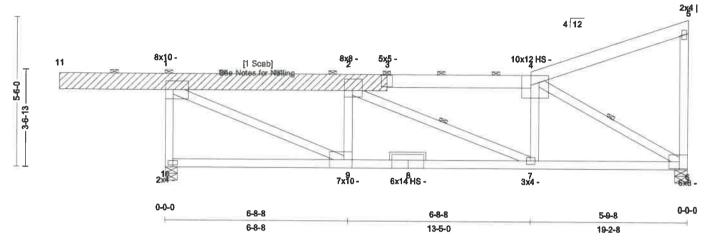
Truss: T30

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:29
Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANT'L	CANTR	PLYS	SPACING	WGT/PLY	
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	143 lbs	





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0	General Bidg Code: IBC2018/ TPI 1-2014 Rep Mbr: No	CSI TC: 0.89 (1-2) BC: 0.67 (7-9) Web: 0.82 (4-6)	Deflection Vert TL: 0.29 in Vert LL: 0.21 in Horz TL: 0.07 in	L/ L/761 L/999	(loc) (7-8) 8	Allowed L/240 L/360
BCDL: 10	Lumber D.O.L.: 100 %	Web: 0.82 (4-6)	H012 11.: 0.07 in		6	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplid	t Max MWFRS Uplift!	Max C&C Uplift	Max Uplift	Max Horiz
10	1	5.5 in	2.18 in	4,130 lbs	*	-248 lbs	-584 lbs	-584 lbs	201 lbs
6	1	5.5 in	1.50 in	2,395 lbs	2	-154 lbs	-351 lbs	-351 lbs	10

Bracing

TC: Sheathed

Web: One Midpoint Row: 2-7, 4-6

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 11-3

BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4 except SP-FT/PG 2400/1.8 2 x 4: 1-9 Scabs

11-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces
Table indicates: Member ID, max exial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

To 1-2 0.894 4,313 lbs 24 0.839 -3,562 lbs 4-5 0.851 -334 lbs

	1-2		-4,313 lbs		24		-3,562 lbs		4-5	0.851	-334 Ibs		1			
	6-7		3,625 lbs	(441 lbs)	7-9	0.672	4,313 lbs	(-462 lbs)								
Web		0.773	-4,072 lbs		2-9	0.309	-1,819 lbs		4-7	0.136	472 lbs	(-198 lbs)	5-6	0.415	-906 lbs	
	1-9	0.586	4,740 lbs	(-514 lbs)	2-7	0.232	-834 lbs		4-6	0.820	4,070 lbs	,				

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).

3) Provide adequate drainage to prevent ponding.

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

A creep factor of 1.50 has been applied for this truss analysis.

7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FOR THIN THE EAGLE METAL PRODUCTS DESIGN NOTIES ISSUED WITH THIS DESIGN AND AVAIL ABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:29 Date:

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 24 in 143 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3 - 11 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

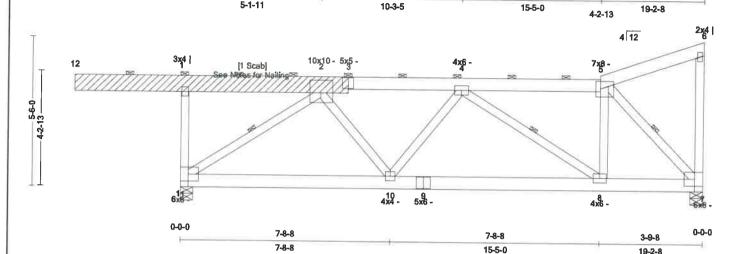
Truss: T31

09/12/2024

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:30 Page: 1 of 2

SPAN 19-2-8	PITCH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 156 lbs	
1				23	-1-0					-
	3-10-8	N.	5-1-11	5-	1-10	5-1-11		3.0.8		



All plates shown to be Eagle 20 unless otherwise noted.

Looding (not)	Commol	COT	155			
Loading (psf) TCLL: 110	Bldg Code: IBC2	CSI TC: 0.91 (2-4)	Deflection Vert TL: 0.17 in	L/ L/999	(loc) (8-9)	Allowed L/240
TCDL: 10 BCLL: 0	TPI 1 Rep Mbr: No	-2014 BC: 0.42 (8-10) Web: 0.70 (2-11)	VertLL: 0.12 in Horz TL: 0.06 in	L/999	(8-9)	L/360
BCDL: 10	Lumber D.O.L. 1009		HOIZ IL; U.UO IN		7	

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWPRS Uplift!	Max C&CUblift	Max Unlift	Max Horiz
11	1	5.5 in	2.15 in	4,083 lbs		-253 lbs	-590 lbs	-590 lbs	198 lbs
7	1	5.5 in	1.50 in	2.421 lbs	2	-150 lbs	-350 lbs	250 Iba	170103

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 12-3

BC: SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2x 4

Scabs

Bracing

TC: Sheathed or Purlins at 4-5-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 2-11, 4-8, 5-7

12-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

5-1-11

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user

defined input: 110 psf Roof (GSL=157 psf), Tennin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B-L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber]	Forces	Table	indicates: M	lember I	D, max CSI	, max axial for	ce, (max como	or, force i	f different i	from max axial:	force). Only f	nerve o	meater than 30	Olhe am ehony	m in this table
TC	2-4	0.906	-3,535 lbs		1				I				ه سدن ا	iciac paga 50	UIUS GIL BLIUWI	II III IIIIS KADAC
	4-5	0.892	-2,349 lbs										1			
BC	7-8	0.222	2,515 lbs	(-242 lbs)	8-10	0.416	3,934 lbs	(417 lbs)	10-11	0.334	2,980 lbs	(-462 lbs)				
Web	1-11 2-11	0.664 0.703	-2,636 lbs -3,664 lbs		2-10 4-10	0.268 0.233	929 Ibs -715 Ibs	(-29 lbs)	4-8 5-8	0.395 0.351	-1,927 lbs 1,216 lbs		5-7	0.540 0.286	-3,402 lbs -624 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq =0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen
Date: 09/12/24 08:00:30

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 19-2-8 0/12 3-10-8 0.000-0-0 0-0-0 24 in 156 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3-12 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

Truss: T32

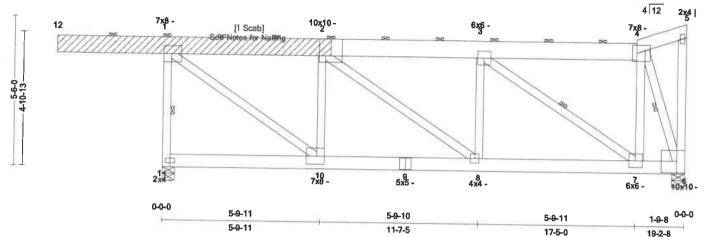
09/12/202

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen 09/12/24 08:00:31 Date: Page: 1 of 2

				22	l-1-0			Z4 II	178 lbs
SPAN 19-2-8	PTICH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANTR 0-0-0	PLYS	SPACING 24 in	WGT/PLY

3-10-8 5-9-11 5-9-10 5-9-11 1-9-8 5-9-11 4-10-13¹⁹⁻²⁻⁸ 11-7-5 17-5-0



All plates shown to be Eagle 20 unless otherwise noted.

TCLL: 110 TCDL: 10 BCLL: 0	Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No	CSI TC: 0.89 (1-2) BC: 0.37 (7-8) Web: 0.69 (3-7)	VertTL: 0.15 in VertLL: 0.12 in Horz TL: 0.04 in	L/ L/999 L/999	(loc) (8-9) (8-9) 6	Allowed L/240 L/360
BCDL: 10	Lumber D.O.L.: 100 %	` ′	0.0 / 11		U	

Reaction

Л	Brg Combo		Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Unlift	Max Unlift	Max Horiz
11	1	5.5 in	2.11 in	4,003 lbs		-259 lbs	-582 lbs	-582 lbs	195 lbs
0	1	5.5 in	1.48 in	2,809 lbs	•	-144 lbs	-362 lbs	-362 lbs	100 and

Material

TC: SP-FT/PG 2400/1.8 2 x 8 except: SP-FT/PG 2400/1.8 2 x 6: 4-5

SP-FT/PG 2400/1.8 2 x 6

Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG 2400/1.8 2 x 4: 1-10 Scales

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others. Web: One Midpoint Row: 1-11, 3-7, 4-6

12-2 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

	mber	Forces	Table	indicates: M	ember i	D, max CS	, max axial for	ce. (max como	or, force	if different t	form mer errial	forme) Only f	arces greater than 300lbs are shown in this table.
TC	1-2	0.892	-2,671 lbs		134	0.811	-1.085 lbs		I			MILLY CRITY IL	aces greater than 500/08 are shown in this table.
	2-3	0.780	-3,428 lbs				-,		1				
BC	6-7	0.131	1,126 lbs	(-76 lbs)	7-8	0.370	3,428 lbs	(-312 lbs)	8.10	0.291	2,812 lbs	(-334 Ibs)	
Web	1-11	0.584	-3,948 lbs		2-8	0.225	778 lbs	(95 lbs)		0.495	1,716 lbs		
	1-10	0.413	3,337 lbs	(-383 Ibs)	3-8	0.097	-353 lbs		46	0.474	-3.173 lbs	(-121 lbs)	
	2-10	0.520	-1.884 lbs	,	3-7	0.694	-2 897 lbs		170	U.+/+	-5,175 IDS	- 1	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen 09/12/24 08:00:32 Date:

Page: 2 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 19-2-8 0/12 3-10-8 0-0-0 0-0-0 θ 24 in 178 lbs 8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on

metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details,

- 10) All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 2 - 12 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

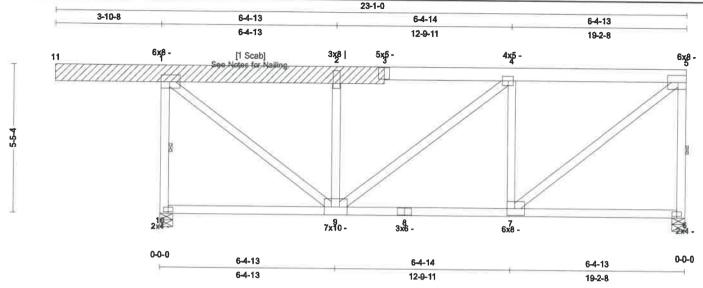
Truss: T33

09/12/202

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date 09/12/24 08:00:32 1 of 2

SPAN	PTICH	QTY	OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	148 lbs	



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) (TCLL: 110	Bldg Code: IBC 2018/	CSI TC: 0.94 (4-5)	Deflection Vert TL: 0,13 in	L/ L/999	(loc) (7-8)	Allowed L/240
	TPI 1-2014 Rep Mbr : No Lumber D.O.L.: 100 %	BC: 0.39 (7-9) Web: 0.78 (5-7)	VertLL: 0.1 in Horz TL: 0.02 in	L/999	(7-8) 6	L/360 L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upl	ift Max C&CUplift	Max Unlift	Max Horiz
10	1	5.5 in	1.86 in	3,530 lbs	%	-266 lbs	-578 lbs	-578 lbs	188 lbs
6	1	5.5 in	1.50 in	2.395 lbs	*	-137 lbs	-362 lbs	-362 lbs	100 E/2

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 11-3

BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4

Scabs

Bracing

TC: Sheathed or Purlins at 5-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-10, 5-6

11-3 [Qty. 1] SP-FT/PG 2400/1.8 2 x 8

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads, in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL = 1.60
- 5) This truss has been designed for the effects of TCLL=20 psf.
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Mer	nber i	Forces	Table	indicates: M	anbar I	D, max CSI	max axial force	, (max compr. forc	e if different :	fiom max axial	force). Only	ी जिल्हा ग	eater than 3	Miles are shown	in this table
TC	1-2	0.890	-1,951 lbs		24	0.846	-1,959 lbs	14-5	0.938	-2.109 lbs		1		50.00 tag gato 111	I MI MIMI MONA
BC	7-9	0.394	2,109 lbs	(-215 lbs)						4-05 105		_			
Web	1-10 1-9	0.566 0.720	-3,476 lbs 2,495 lbs	(-337 lbs)	2.9 4.9	0.503 0.484	-1,292 lbs -460 lbs	4-7 5-7	0.646 0.783	-1,567 lbs 2,713 lbs	(-398 lbs)	5-6	0.391	-2,342 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 none (719) 371	Loop 31252		Job	signer: Shane Allen e: 09/12/24 08:0	-HIESCO COMPLETE
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	148 lbs

⁸⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

¹⁰⁾ All connectors, i.e. nails, screws, bolis, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

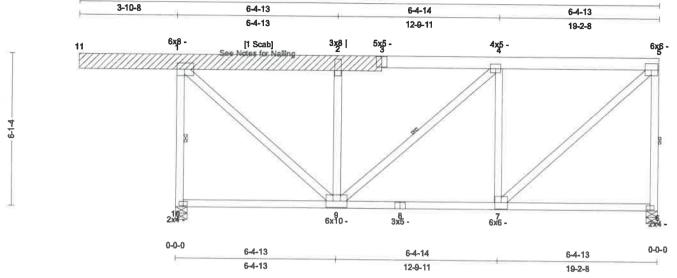
¹³⁾ Scab 3 - 11 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T34

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date 09/12/24 08:00:34 Page: 1 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 24 in 152 lbs 23-1-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014	CSI TC: 0.94 (4-5) BC: 0.38 (7-9)	Deflection VertTL: 0.13 in	L/ L/999	(loc) (7-8)	Allowed
BCLL: 0 BCDL: 10	Rep Mbr: No Lumber D.O.L.: 100 %	Web: 0.82 (4-7)	VertLL: 0.09 in Horz TL: 0.02 in	L/999	(7-8) 6	L/360

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift I	Max C&CUplift	Max Uplift	Max Horiz
10	1	5.5 in	1.86 in	3,530 lbs		-265 lbs	-578 lbs	-578 lbs	210 lbs
6	1	5.5 in	1.50 in	2,395 lbs	*	-137 lbs	-362 lbs	-362 The	210 203

Bracing

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8; 11-3

BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4

Scales

TC: Sheathed or Purlins at 5-7-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-10, 4-9, 5-6

11-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 8

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00,
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60
- 5) This truss has been designed for the effects of TCLL = 20 psf.
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber]	Forces	Table	indicates: M	ember I	D, max CSI,	, max axial force,	(max compr. fo	rce if different	from max axial	force). Only	forces on	eater than 30	Milhe are choss	m in thic toble
	1-2	0.890	-1,722 lbs		2-4	0.846	-1,730 lbs	14-5	0.939	-1.865 lbs		I		70103 tt 2 12 17	II III bibs weren
BC	7-9	0.380	1,865 lbs	(-167 lbs)								_			
Web	1-10 1-9	0.638 0.670	-3,476 lbs 2,321 lbs	(-313 lbs)	2-9 4-9	0.644 0.137	-1,290 lbs -428 lbs	4-7 5-7		-1,568 lbs 2,529 lbs	(-372 lbs)	5-6	0.443	-2,342 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FACLE UPON REQUEST. DESIGN VALID ONLY WHEN FACLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:34

Page: 2 of 2

	SPAN	PITCH	QTY	OHL	OHR	CANT'L	CANTR	PLYS	SPACING	WGT/PLY
	19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	152 lbs
- 01									27 11	132 108

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3-11 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc, totreated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

Valley Truss 133 Range View Loop Westcliffe, CO 81252 Phone (719) 371-8508 SPAN PITCH QTY OHL OHR CANTL CANTR PLYS 19-2-8 0/12 3-10-8 0-0-0 0-0-0 0-0-0 23-1-0 3-10-8 5-4-14 5-4-13 5-4-14 5-4-14 10-9-11 16-2-9 5x5

Truss: T35

Job: CBS0306SA-7-HIESCO COMPLETE

WGT/PLY

172 lbs

Designer: Shane Allen Date 09/12/24 08:00:35

SPACING

Page: 1 of 2

09/12/2024

24 in 2-11-15 19-2-8 3x8 | [1 Scab] 4x4 -5x8 / 12 4 6x8 | 2.JE 7x8 -3x5 · 415-7x8 -0-0-0 0-0-0 5-4-14 5-4-13 5-4-14 2-11-15 5-4-14 10-9-11 16-2-9 19-2-8

All plates shown to be Eagle 20 unless otherwise noted.

T 11 (6 6 1	Teles				
	osf) General	CSI	Deflection	L	(loc)	Allowed
TCLL: 110	Bldg Code: IBC 2018/	TC: 0.89 (1-2)	Vert TL: 0.12 in	L/999	(10-11)	L/240
TCDL: 10	TPI 1-2014	BC: 0.30 (9-11)	VertLL: 0.08 in	L/999	io	L/360
BCLL: 0	Rep Mbr: No	Web: 0.73 (1-12)	Horz TL: 0.02 in		7	-
BCDL: 10	Lumber D.O.L.: 100 %		1			

Reaction

Л	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Unlift	Max Uplift	Max Horiz
12	1	5.5 in	1.86 in	3,530 lbs		-276 lbs	-598 lbs	-598 lbs	-234 lbs
7	1	5.5 in	1.50 in	2,395 lbs		-127 lbs	-349 lbs	-349 lbs	-234 108

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 13-3

SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2x 4

Scabs

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-12, 4-11, 5-8, 6-7

13-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 8

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads, in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60
- 5) This truss has been designed for the effects of TCLL = 20 psf.
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber	Forces	Table	indicates; M	lember II	D, max CSI	, max axial for	e. (max com	or, force	if different t	form max axial	forme) Only fr	proes greater than 300ths are shown in this table.
TC	1-2	0.890	-1,329 lbs		4-5	0.761	-1.769 lbs		1			Long Conty II	Acce great than 30000s are shown in this large
_	2-4	0.723	-1,337 lbs		5-6	0.466	-1,017 lbs						'
BC	8-9	0.231	867 lbs	(-93 lbs)	9-11	0.305	1,769 lbs	(-147 lbs)					
Web	1-12	0.733	-3,484 lbs		4-11	0.209	-682 lbs		5-8	0.375	-1.817 lbs		
	1-11	0.600	2,078 lbs	(-359 lbs)	4.9	0.582	-888 Ibs		6-8	0.612	2.122 lbs	(-277 lbs)	
	2-11	0.601	-961 lbs		5-9	0.389	1,348 lbs	(-201 lbs)		0.434	-2,378 lbs	(2// 100)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SELFORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM FACLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:36

Page:	2 of 2	
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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	172 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated,

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3 - 13 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

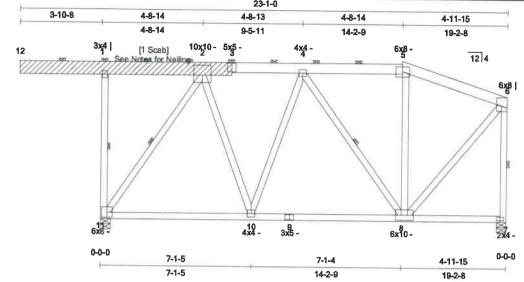
Truss: T36

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen 09/12/24 08:00:36

Page:	1 of 2	
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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANT'R	PLYS	SPACING	WGT/PLY
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	169 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.93 (4-5) BC: 0.54 (8-10) Web: 0.79 (2-11)	VertTL: 0.13 in VertLL: 0.07 in Horz TL: 0.03 in	L/ L/999 L/999	(loc) (10-11) (8-9) 7	Allowed L/240 L/360
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Reaction

T	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUblift	Max Unlift	Max Horiz
11	1	5.5 in	2.17 in	4,115 lbs		-286 lbs	-606 lbs	-606 lbs	-259 lbs
7	1	5.5 in	1.50 in	2.395 lbs		-117 lbs	-335 lbc	. 225 Iba	-2.77 108

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 12-3

BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4

Scabs

Bracing

TC: Sheathed or Parlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-11, 2-11, 4-8, 6-7

12-3 [Qty.1] SP-FT/PG 2400/1.8 2 x 8

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user

defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL=20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	nber !	Forces	Table	indicates: M	lamber II), max CSI	, max axial for	ce, (max come	or, force	if different t	iom max axial t	iome) Only	faces on	esterthan 30	Vilhe on chara	n in this table
TC	2-4	0.853	-1,737 lbs		5-6	0.646	-1.462 lbs		T)				1		JOIGN SEC BLICAN	ii iii uus tadas.
	4-5	0.930	-1,342 lbs				-,									
BC	8-10	0.544	1,982 lbs	(-152 lbs)	10-11	0.532	1,411 lbs	(-255 lbs)					-			
Web	1-11 2-11	0.656 0.788	-2,697 lbs -2,570 lbs		2-10 4-10	0.283 0.723	979 lbs -805 lbs	(-76 lbs)	4-8 5-8	0.366 0.763	-1,124 lbs -936 lbs		6-8 6-7	0.608 0.431	2,105 lbs -2,359 lbs	(-225 lbs)

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

			V	Valley Trus 33 Range View Vestcliffe, CO 8 hone (719) 371	Loop 31252			Designer: St Date: 09	BS0306SA-7	7-HIESCOCOMPLETE 00:37
SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS		PACING	WGT/PLY
19-2-8	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1		24 in	169 lbs

⁸⁾ Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

¹⁰⁾ All connectors, i.e. nails, screws, bolls, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and

¹³⁾ Scab 3 - 12 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T37

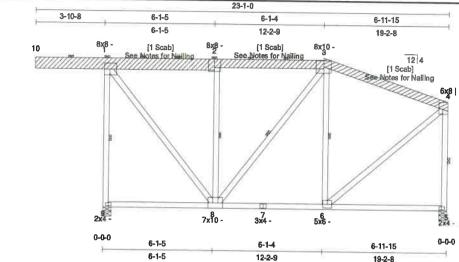
09/16/2024

CBS0306SA-8-HIESCO COMPLETE Job:

5-10-14

Designer.Shane Allen Date: 09/16/24 08:09:04 Page: 1of2

SPAN PITCH QTY OHL OHR CANTI 19-2-8 0/12 1 3-10-8 0-00 0-00	CANTR	PLYS	SPACING	WGT/PLY
	0-0-0	1	24 in	167 lbs



All plates shown to be Eagle 20 unless otherwise noted.

TCDL: 10	CSI BC 2018/ TC: 0.89 (1-2) TPI 1-2014 BC: 0.35 (6-8) No Web: 0.78 (1-9)	Deflection Ven TL:	L/ L/999 L/999	(loc) (5-6) (7-8) 5	Allowed L/240 L/360
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Reaction

9 1 55 in 1.89 in 4,143 lbs -296 lbs -607 lbs -607 lbs	JI	Brg Width Rqd Brg Wid	h Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Unlift	Max Horiz
5 1 65: 150: 5.000	9	-102 111	4,143 lbs					-283 lbs
-319 hs	5	5.5 in 1.50 in	2,405 lbs		-107 lbs	-319 lbs	-319 lbs	205 208

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except: SP-FT/PG 2400/1.8 2 x 8: 10-2 SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2 x 4 except:

Bracing

TC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-9, 2-8, 3-8, 3-6, 4-5

SP-FT/PG 2400/1.8 2 x 4: 1-9

10-2 [Qty:1] SP-FT/PG 2400/1.8 2 x28 [Qty:1] SP-FT/PG 2400/1.8 2 x84 [Qty:1] SP-FT/PG 2400//

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads. in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Terrain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure

C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60

This truss has been designed for the effects of TC LL = 20 psf.

Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	Member Forces Table indic					ID, max CSI,	max axial force,	(max comor. force if	different from	n max axial five	Onlyfore	meder	than 2000ba	ئىلە سەمەلەت	
TC	1-2	0.888	-1,529 lbs		2-3	0.455	-1.492 lbs	134	0.501	-1.725 lbs	o, comy maco	grau.	Men Some	are shown in the	S GROCE
BC	6-8	0.346	1,424 lbs	(-119 lbs)					0.002	1,720 133		_			
Web	1-9 1-8	0.781 0.703	-4,089 lbs 2,437 lbs	(-302 lbs)	2-8 3-8	0.517 0.286	-1,899 lbs -695 lbs	3-6 4-6	0.292 0.545	-1,061 lbs 1,888 lbs	(-198 lbs)	4-5	0.429	-2,349 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

Brace bottom chord with approved sheathing or purlins per Bracing Summary.

5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

5) Acreep factor of 1.50 has been applied for this truss analysis.
7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE FAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Truss: T37

Job: CBS0306SA-8-HIESCO COMPLETE

DesignerShane Allen Date: 09/16/24 08:09:05 2 of 2

Page: QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 3-10-8 0-0-0 000 0-0-0 24 in 167 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add

- 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.
- 12) Incising is not permitted.

SPAN

19-2-8

- 13) Scab 2 10 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc Z(TSLZ278, 2 ply) Screws @ 6 oc.
- 14) Scab 2 3 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

 15) Scab 3 4 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.

 15) Scab 3 4 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc-Z(TSLZ278, 2-ply) Screws @ 6 oc.
- 16) Listed wind uplift reactions based on MWFRS & C&C loading.

PITCH

0/12

ALL PERSONS FABRICATING, HANDLING, ERBCTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

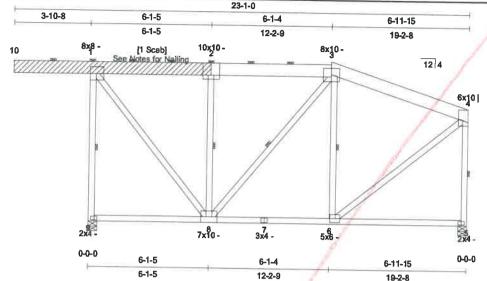
Truss: T37

CBS0306SA-7-HIESCO COMPLETI Job:

Designer: Shane Allen 09/12/24 08:00:38

Page: 1 of 2

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 19-2-8 0/12 3-10-8 0.0-0 0-0-0 0-0-0 24 in 177 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0	Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No	CSI TC: 0.89 (1-2) BC: 0.35 (6-8) Web: 0.78 (1-9)	Deflection Vert TL: 0.12 in Vert LL: 0.08 in Horz TL: 0.01 in	L/ L/999 L/999	(loc) (5-6) (7-8) 5	Allowed L/240 L/360
BCDL: 10	Lumber D.O.L.: 100 %				_	00 /2 0 /000

Reaction

JT	Brg Combo	Brg Width	Rod Brg Width	Max React	Max Grav Uplift 1	Max MWFRS Uplift	Max C&CUblift	Max Unlift	Max Horiz
9	1	5.5 in	1.89 in	4,143 lbs	9	-297 lbs	-608 lbs	-608 lbs	-285 lb
5	1	5.5 in	1.50 in	2,405 lbs	©	-106 lbs	-318 lbs	-318 lbs	
						100 40	310103	-210 102	- 8

Bracing

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4 except: SP-FT/PG 2400/1.8 2 x 4: 1-9

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-9, 2-8, 3-8, 3-6, 4-5

10-2 [Qty.1] SP-FT/PG 2400/1.8 2 x 8

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenzin C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.
- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B-L=15 ft, End Zone Truss, Both end webs considered. DOL=1.60
- This truss has been designed for the effects of TCLL = 20 psf.
- 5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber	Forces	Table	indicates: M	lember	ID, max CSI	max axial force	e, (max compr. force	if different t	inm mor orial	from Outro			2001	
TC	1-2	0.888	-1,484 lbs		2-3	0.690	-1,489 lbs	134	0.706	-1.738 lbs	ance, Only i	Dittes git	auer (nan 3)	JUDS are shown	m this table.
BC	6-8	0.352	1,436 lbs	(-118 lbs)		1				2,100 100					
Web	1-9 1-8	0.782 0.697	4,091 lbs 2,416 lbs	(-301 lbs)	2-8 3-8	0.497	-1,884 lbs -705 lbs	36 46	0.280	-1,051 lbs	(10¢ %)	4-5	0.416	-2,349 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) 🗵 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

			V	Valley Trus 33 Range View Vesteliffe, CO 8 hone (719) 371	Loop 31252		Job:		/
SPAN 19-2-8 8) Due to the inhere	PTICH 0/12	QTY 1	OHL 3-10-8	OHR 0-0-0	CANTL 0-0-0	CANT'R 0-0-0	PLYS 1	SPACING 24 in	WGT/PLY 177 lbs

and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

12) Incising is not permitted.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

⁹⁾ Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and 11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

¹³⁾ Scale 2 - 10 to match size and grade of member to which it is attached. A ttach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc. 14) Listed wind uplift reactions based on MWFRS & C&C loading.

Truss: T38

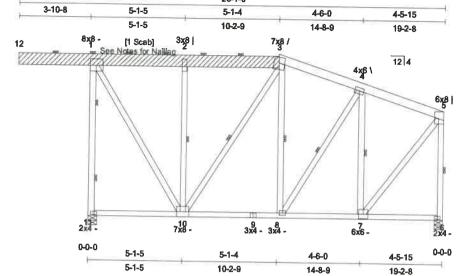
CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:39

Page: 1 of 2

09/12/2024





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.89 (1-2) BC: 0.25 (8-10) Web: 0.71 (1-11)	Deflection VertTL: 0.1 in VertLL: 0.07 in Horz TL: 0.02 in	L/ L/999 L/999	(loc) (9-10) (9-10) 6	Allowed L/240 L/360
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Reaction

T	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Upli	ft.Max C&CUblift	Max Uplift	Max Horiz
11	1	5.5 in	1.89 in	4,138 lbs	•	-307 lbs	-607 lbs	-607 lbs	-308 lbs
6	1	55 in	1.50 in	2,695 lbs	100	-96 lbs	-305 lbs	-305 lbs	200 100

Material

TC: SP-FT/PG 2400/1.8 2 x 6 except:

SP-FT/PG 2400/1.8 2 x 8: 12-3

SP-FT/PG 2400/1.8 2 x 4

Web: SP-FT/PG#2 2 x 4 except:

SP-FT/PG 2400/1.8 2 x 4: 1-11

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others. Web: One Midpoint Row: 2-10, 3-10, 3-8, 4-8, 4-7, 5-6

Two Third Point Rows: 1-11

12-3 [Qty: 1] SP-FT/PG 2400/1.8 2 x 8

Loads

1) This truss has been designed for the effects of balanced (110 psf) and unbalanced roof snow loads, in accordance with ASCE7 - 16 with the following user defined input: 110 psf Roof (GSL=157 psf), Tenain C, Exposure (Ce=1.0), Thermal (Ct=1.00), DOL=1.00. Ventilated.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 16 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60

4) This truss has been designed for the effects of TCLL = 20 psf.

5) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me	mber	Forces	Table	indicates: M	lember II	D, max CS	l, max axial fon	ar (max com)	or. force	if different	from mor avial	form) Only 6	ixues greater than 300lbs are shown in this table.
TC	1-2	0.890	-993 lbs		3-4	0.780	-1,543 lbs	, ,	1		WANTED STATE	mac, Only a	Aces greater digit 500 tips are shown in this table.
_	2-3	0.176	-1,009 lbs		4-5	0.855	-1,508 lbs						1
BC	7-8	0.210	1,255 lbs	(-94 lbs)	8-10	0.252	1,323 lbs	(-116 lbs)					
Web	1-11	0.712	-4,094 Ths		3-10	0.398	-977 lbs		4-7	0.351	-1.510 lbs		
	1-10	0.631	1,939 Ibs	(427 lbs)	3-8	0.137	-430 lbs		5.7	0.588	2,038 lbs	(-192 lbs)	i I
	2-10	0.382	-1,256 lbs		4-8	0.179	621 lbs	(-113 lbs)	56	0.485	-2.657 lbs	(122 105)	1

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).

3) Provide adequate drainage to prevent ponding.

- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.

6) A creep factor of 1.50 has been applied for this truss analysis.

7) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST, DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:39

Page: 2of2

SPAN PITCH QTY OHL OHR CANTL CANTR 19-2-8 PLYS SPACING 0/12 WGT/PLY 3-10-8 0-0-0 0.000-0-0 24 in 189 lbs

8) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated. 9) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

10) All connectors, i.e. nails, screws, bolls, truss plates, etc., to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.

11) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

12) Incising is not permitted.

13) Scab 3 - 12 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - pty) Screws @ 6 oc.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:40

Page: 1 of 2

OHIL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY	_
1-10-8	0-0-0	0-0-0	0-0-0	1	24 in	20 lbs	
	3-	A-R					_

1-10-8 1-8-8 1-8-8



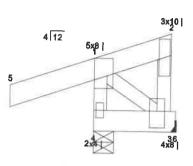
SPAN

1-8-8

PITCH

4/12

QTY



1-10-8	0-0-0	1-8080	9-0-0 0-1-8
		1-8-8	171

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bidg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.85 (5-1) BC: 0.55 (6-3) Web: 0.19 (1-4)	Deflection VertTL: 0 in UP VertLL: 0 in UP Cant/OHTL: 0 in Cant/OHTL: 0 in HorzTL: 0 in	L/ L/999 L/999 2L/999 2L/999	(loc) (3-4) (3-4) 4 4	Allowed L/240 L/360 2L/120 2L/120
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Reaction

Л	Brg Combo		Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift!	Max C&C Unlift	Max Unlift	Max Horiz
4	1	5.5 in	1.50 in	1,456 lbs		-97 lbs			
6	1	1.5 in					-361 lbs	-361 lbs	108 lbs
·		тэш	_	39 lbs	-523 lbs	-35 lbs	200.5	-523 lbs	2.0
_									

Material

TC: SP-FT/PG 2400/1.8 2 x 6 BC: SP-FT/PG 2400/1.8 2 x 6 Web: SP-FT/PG#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.00.
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60
- 5) This truss has been designed for the effects of TCLL = 20 psf.
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Me TC	mber	Forces	Table indica	tes: Member II	D, max CSI,	max axial for	ce, (max comp	or. force if different from max axial force). Only forces greater than 300lbs are shown in this table,
BC								
Web	1-4	0.192	-1,539 lbs	2-3	0.185	642 lbs	(-117 lbs)	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

CBS0306SA-7-HIESCO COMPLETE

Designer: Shane Allen Date: 09/12/24 08:00:41

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SPAN PITCH QTY OHL OHR CANTL CANTR **PLYS** SPACING WGT/PLY 1-8-8 4/12 2 1-10-8 0-0-0 0-0-0 0-0-0 24 in 20 lbs

6) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 6 may need to be considered.

7) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

8) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

9) All connectors, i.e. nails, screws, bolts, trus s plates, etc., to treated wood shall have a minimum of G60 g alvanization including but not limited to repairs and add framing.

10) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

11) Incising is not permitted.

12) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERBCTINGOR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

OHR

0.00

Truss: T40

CBS0306SA-7-HIESCO COMPLETE Job:

WGT/PLY

80 lbs

Designer: Shane Allen Date; 09/12/24 08:00:43

SPACING

24 in

Page: 1 of 2

PLYS

1

09/12/2024

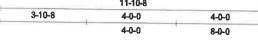
11-10-8 3-10-8 4.0.0 4-0-0

CANTI

0-0-0

CANTR

0-0-0





SPAN

8-0-0

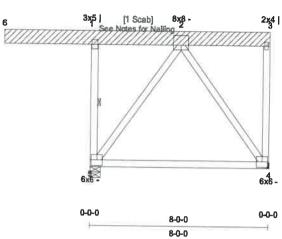
PITCH

0/12

QTY

OHL

3-10-8



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10	General Bldg Code: IBC 2018/ TPI 1-2014	CSI TC: 0.88 (1-2) BC: 0.58 (4-5)	Deflection VertTL: 0.32 in VertLL: 0.15 in	L/ L/274 L/605	(loc) (4-5)	Allowed L/240
BCLL: 0 BCDL: 10	TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	BC: 0.58 (4-5) Web: 0.50 (1-5)	VertLL: 0.15 in Horz TL: 0 in	L/605	(4-5) 4	L/360

Reaction

71	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWRS Uplift	Max C&CUblift	Max Unlift	Max Horiz
5	1	5.5 in	2.11 in	2,424 lbs		-255 lbs	-567 lbs	-567 lbs	-209 ibs
4	1	1.5 in	_	789 lbs	-322 lbs	-126 lbs	-160 lbs	-322 lbs	20, 203

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4

TC: Sheathed or Purlins at 10-0-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-5

[Qty: 1] SP-FT/PG 2400/1.8 2 x 8

Scabs I nads

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL = 1.00.
- This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Pactored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B=L=15 ft, End Zone Truss, Both end webs considered. DCL = 1.60
- 5) This truss has been designed for the effects of TCLL = 20 psf.
- 6) Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

M	ember Fo	rces
	•	

Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

10				- 1											
BC	4-5	0.585	-353 lbs				-								
Web	1-5	0.498	-2,863 lbs	2-5	0.220	628 lbs	(-299 lbs) 2	2-4	0.220	628 lbs	(-299 lbs)	3-4	0.405	-462 lbs	_

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10% (Cq = 0.90).
- 3) Hanger is for graphical interpretation only Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

Valley Truss
133 Range View Loop
Westcliffe, CO 81252
Phone (719) 371-8508

Job: CBS0306SA-7-HIESCO COMPLETE

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SPAN PITCH QTY OHL OHR CANTIL CANTR PLYS SPACING WGT/PLY 8-0-0 0/12 3-10-8 ഹ 0-0-0 0-0-0 24 in 80 lbs

9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.

10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

- 12) All connectors, i.e. nails, screws, bolts, truss plates, etc, to treated wood shall have a minimum of G60 galvanization including but not limited to repairs and add framing.
- 13) Wood and plating reductions for treated lumber are unique to the supplier. Only supplier shown shall be used for this design.

14) Incising is not permitted.

15) Scab 3 - 6 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

Truss: T41

CBS0306SA-7-HIESCO COMPLETE

Designer, Shane Allen Date 09/12/24 08:00:44

Page: 1 of 2

OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 3-10-8 0-0-0 0-0-0 0-0-0 24 in 83 lbs 11-10-8

3-10-8 4-0-0 4-0-0 8-0-0



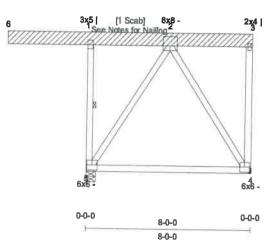
SPAN

8-0-0

PITCH

0/12

QTY



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 110 TCDL: 10 BCLL: 0 BCDL: 10	General Bldg Code: IBC 2018/ TPI 1-2014 Rep Mbr: No Lumber D.O.L.: 100 %	CSI TC: 0.88 (1-2) BC: 0.65 (4-5) Web: 0.57 (1-5)	Deflection VertTL: 0.36 in VertLL: 0.18 in Horz TL: 0 in	L/ L/246 L/482	(loc) (4-5) (4-5) 4	Allowed L/240 L/360
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Reaction

JT	Brg Combo	Brg Width	Rad Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&CUnlift	Max Unlift	Mov Horiz
- 5	1	5.5 in	2.11 in	2,424 lbs		-273 lbs			-
4	1	1.5 in		.,			-567 lbs	-567 lbs	231 lbs
_	*	ILL	_	789 lbs	-322 lbs	-144 lbs	-160 lbs	-322 lbs	S 8

Material

TC: SP-FT/PG 2400/1.8 2 x 8 BC: SP-FT/PG 2400/1.8 2 x 4 Web: SP-FT/PG#2 2x 4 Scabs

Bracing

TC: Sheathed or Purlins at 10-0-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Web: One Midpoint Row: 1-5

[Qty: 1] SP-FT/PG 2400/1.8 2 x 8

Loads

- 1) This truss has been designed for the effects of balanced (110 psf) roof snow loads. in accordance with ASCE7 16 except as noted, with the following user defined input: 157 psf ground snow load. NOTE: Conservatively, all flat/sloped roof factors have been ignored and the ground snow load has been used for the roof snow load, DOL=1.00.
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- 4) This truss has been designed for the effects of wind loads in accordance with ASCE7 16 with the following user defined input: 115 mph (Factored), Exposure
- C, Enclosed, Gable, Risk Category II, h=B-L=15 ft, End Zone Truss, Both end webs considered. DCL=1.60
- 5) This truss has been designed for the effects of TCLL=20 psf.
- Non-concurrent minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only for

TC										TOTAL CITY IO	into gita	act man 20	OTTIC SEC SCHOME	. III UUS EADE
BC	4-5	0.648	-313 lbs								_			
Web	1-5	0.569	-2,858 lbs	2-5	0.255	601 lbs	(-295 lbs) 2-4	0.255	601 lbs	(-295 lbs)	3-4	0.496	457 lbs	

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 10 % (Cq = 0.90).
- 3) Hanger is for graphical interpretation only. Install hanger per manufacturer's recommendation.
- 4) Provide adequate drainage to prevent ponding.
- 5) Brace bottom chord with approved sheathing or purins per Bracing Summary.
- 6) Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- 7) A creep factor of 1.50 has been applied for this truss analysis.
- 8) 🖾 Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.

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133 Range View Loop
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CBS0306SA-7-HIESCO COMPLETE Job:

Designer: Shane Allen Date: 09/12/24 08:00:45

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
8-0-0	0/12	1	3-10-8	0-0-0	0-0-0	0-0-0	1	24 in	83 lbs

9) Due to negative reactions in gravity load cases, special connections to the bearing surface at joint 4 may need to be considered.

10) Due to the inherent corrosion of treated lumber and steel connectors and the number of variables that affect the rate of corrosion, the G60 coating provided on metal plates shall be reviewed and approved by the Building Designer considering service expectancy life of the structure. If further protection is required beyond G60, the use of G185 can be investigated.

11) Field cuts of treated lumber shall be treated with approved end coating. See supplier for details.

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14) Incising is not permitted.

15) Scab 3 - 6 to match size and grade of member to which it is attached. Attach with 2 staggered rows of TrussLoc - Z(TSLZ278, 2 - ply) Screws @ 6 oc.

16) Listed wind uplift reactions based on MWFRS & C&C loading.

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANYTRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.