GENERAL CRITERIA

1. DESIGN LOADS SHALL BE CONFIGURED USING INTERNATIONAL BUILDING CODE (IBC) 2018 EDITION, AND ASCE CHAPTER 7 MINIMUM DESIGN LOADS FOR BUILDING AND OTHER STRUCTURES.

   A. SNOW LOAD DESIGN DATA
   GROUND SNOW LOAD, *Pg*_____110 psf (Below 5300')
   FLAT-ROOF LOAD, *Pf* = 0.7*Ce*Ct*I*PG___110 psf
   SNOW EXPOSURE FACTOR (Ce)_____1.0
   SNOW IMPORTANCE FACTOR (Is)_____1.0
   THERMAL FACTOR (Ct)_____1.0

   B. WIND LOAD DATA
   BASIC WIND SPEED, *VULT*_____115 MPH 3SEC-GUST
   WIND IMPORTANCE FACTOR_____1.0
   WIND EXPOSURE_____C
   BUILDING CLASSIFICATION_____II
   TOPOGRAPHIC FACTOR, KZT_____1.0
   WIND BASE SHEAR. V_____TOTAL: 270, TRANS: 155 KIPS, LONG: 118 KIPS

   C. EARTHQUAKE DESIGN DATA
   RISK CATEGORY_____II
   IMPORTANCE FACTOR (Ie)_____1.0
   Ss_____0.3240
   S1_____0.0830
   SITE CLASS_____C
   SDs_____0.333
   SD1_____0.133
   SEISMIC DESIGN CATEGORY_____B
   SEISMIC FORCE-RESISTING SYSTEM____LIGHT FRAMED WALLS W/ WOOD STRUCT. PANELS
   SEISMIC COEFFICIENT, CS _____0.0512
   SEISMIC COEFFICIENT, R_____6.5
   SEISMIC BASE SHEAR. V_____78.60 KIPS 10-26-23

   D. DESIGN LOADS FLOOR & ROOF
   ROOF
   DEAD LOAD_____15 psf
   LIVE LOAD_____110 psf (Snow)

   FLOOR
   DEAD LOAD_____20 psf
   LIVE LOAD_____40 psf (OFFICE & RESIDENTIAL AREA)
   LIVE LOAD_____100 psf (CORRIDOR,STAIRS & STORAGE)

2. CAST-IN-PLACE CONCRETE:
   A. F'c = 4000psi @ 28 DAYS (AIR ENTRAINED) - ALL EXTERIOR BUILDING
   B. F'c = 3000psi @ 28 DAYS (NONE AIR ENTRAINED) - ALL BUILDING

3. REINFORCING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A615 OR A706, GRADE 60.

4. UNLESS NOTED OTHERWISE, LAP SPLICED OR EMBEDMENT LENGTHS SHALL CONFORM TO TABLE A, CLASS B SPLICE. SEE THIS SHEET, TABLE A.

5. UNLESS NOTED OTHERWISE, CONCRETE COVER OVER STEEL REINFORCEMENT SHALL CONFORM TO THE MINIMUMS REQUIRED BY CURRENT ADDITION OF ACI 318.

6. REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315, EXCEPT WHERE OTHERWISE INDICATED.

7. COVER: UNLESS OTHERWISE NOTED OR DETAILED, THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT

EXPOSURE	MINIMUM COVER (IN.)
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3
B. CONCRETE EXPOSED TO EARTH OR WEATHER:	
#6 - #18 BARS	2
#5 - AND SMALLER	1 1/2
C. CONCRETE NOT EXPOSED TO WEATHER OR NOT IN CONTACT WITH GROUND:	
SLABS, WALLS, JOISTS:	
#11 AND SMALLER	3/4
BEAMS, COLUMNS:	
TIES, STIRRUPS, PRIMARY REINFORCEMENT	1 1/2

FOUNDATION NOTES - GEO-TECHNICAL REPORT GOVERNS ALL REQUIREMENTS FOR GEO-TECHNICAL /SOIL AND FOUNDATION PIERS REQUIREMENTS.

1. FOR COMPACTED FILL SOIL AND EXCAVATION REQUIREMENTS, SEE GEO-TECHNICAL REPORT BY NWCC JOB # 22-12552, DATED 3/18/22 AND 2018 IBC CHAPTER 18. CONTRACTOR SHALL BE RESPONSIBLE TO FOLLOW THE RECOMMENDATIONS SPECIFIED THEREIN. 10-26-23

2. DESIGN FOUNDATION BEARING PRESSURE (NET) 3000 PSF DEAD + LIVE LOAD INCREASED BY 1/3 FOR COMBINED VERTICAL AND WIND/SEISMIC LOADS.

3. REINFORCEMENT SHALL BE PLACED MID-DEPTH OF SLAB, U.N.O.

4. CHAMFER EXPOSED EDGES OF CONCRETE 3/4", AT EXPOSED EDGES UNLESS OTHERWISE NOTED.

5. SUB-GRADE PREPARATION:
   A. EXISTING FOUNDATIONS AND UTILITIES AT ANY POINT BENEATH OR WITHIN 3'-0" OF THE NEW STRUCTURES SHALL BE REMOVED ENTIRELY. ANY FILL MATERIAL FROM PREVIOUS CONSTRUCTION ACTIVITIES WHICH IS ENCOUNTERED WITHIN THE BUILDING FOOTPRINT SHOULD ALSO BE REMOVED ENTIRELY. EXPOSED SUB-GRADE AT THE BASE OF REQUIRED EXCAVATION WHICH IS TO RECEIVE FILL SHALL BE COMPACTED TO NOT LESS THAN 90% MAXIMUM LAB DENSITY FOR COHESIVE MATERIAL, AND 95% MAXIMUM LAB DENSITY FOR COHESION-LESS MATERIAL, TO A MINIMUM DEPTH OF 8". SEE EXCAVATION/FILL DETAIL THIS SHEET.

5. FILL:
   A. ALL FILL PLACED UNDER BUILDING SLABS SHALL BE NON-EXPANSIVE AND SHALL BE COMPACTED TO NOT LESS THAN 95% MAXIMUM DENSITY ACCORDING TO ASTM D-1557.

STRUCTURAL STEEL

1. ALL STRUCTURAL STEEL MEMBERS SUCH AS COLUMNS, BEAMS, GIRTS AND BRACES SHALL BE PER SCHEDULE OF CONSTRUCTION MATERIALS THIS SHEET. MISCELLANEOUS STEEL ITEMS SHALL BE ASTM A36. MISCELLANEOUS STEEL TUBES SHALL BE ASTM A500, GRADE B.

2. ALL WELDING SHALL BE DONE IN ACCORDANCE WITH AWS CODE.

3. ALL BOLTS FOR BEAM CONNECTIONS SHALL BE ASTM A325 WITH A MINIMUM DIAMETER OF 3/4", UNLESS NOTED OTHERWISE. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS. WASHERS SHALL BE INSTALLED UNDER NUTS OF FASTENERS WHEN REQUIRED BY THE SPECIFICATION FOR JOINTS.

4. ALL FIELD WELDS SHALL BE INSPECTED PER SHEET S0.2.

5. STEEL BEAMS SHALL BE CONCENTRIC WITH COLUMNS, UNLESS OTHERWISE NOTED.

6. ALL ANCHOR BOLTS SHALL BE ASTM A36 OR A307, UNLESS NOTED OTHERWISE.

7. NO OPENINGS SHALL BE CUT IN STRUCTURAL MEMBERS UNLESS SHOWN ON THE DRAWINGS.

8. BUILDING STEEL ROOF BEAMS AND SUPPORT POST ARE NON-SELF-SUPPORTING. THE ROOF METAL DECK, VERTICAL STEEL BRACING AND MASONRY WALLS ARE REQUIRED TO PROVIDE LATERAL STABILITY FOR THE STEEL BEAMS AND COLUMNS. CONTRACTOR SHALL PROVIDE ALL TEMPORARY BRACING REQUIRED TO MAINTAIN STABILITY OF THE POST AND BEAM SYSTEM UNTIL THESE BRACING ELEMENTS ARE IN PLACE.

TABLE A - REINFORCEMENT TENSION LAPS, EMBEDMENT AND HOOK LENGTHS (1)
fy = 60000 psi f'c = 3000 psi (2) (3)

BAR SIZE (d)	CLEAR SPACING (IN) (4)			EMBEDMENT AND CLASS A LAP (IN) (5)(6)(7)						CLASS B LAP (IN) (6)(8)						HOOK (IN) (9)
				TOP BAR (10)			OTHER BARS			TOP BAR (10)			OTHER BARS			
	2d	3d	5d	2d<S<3d	S≥3d (11)	S≥5D (12)	2d<S<3d	S≥3d (11)	S≥5D (12)	2d<S<3d	S≥3d (11)	S≥5D (12)	2d<S<3d	S≥3d (11)	S≥5D (12)	
3	3/4	1 1/8	1 7/8	16	16	16	13	13	13	21	21	21	16	16	16	9
4	1	1 1/2	2 1/2	22	22	22	17	17	17	28	28	28	22	22	22	11
5	1 1/4	1 7/8	3 1/8	27	27	27	21	21	21	35	35	35	27	27	27	14
6	1 1/2	2 1/4	3 3/4	35	32	32	27	25	25	46	42	42	35	32	32	17
7	1 3/4	2 5/8	4 3/8	48	38	38	37	29	29	63	49	49	48	38	38	20
8	2	3	5	63	45	43	49	35	33	82	59	56	63	45	43	22
9	2.256	3 3/8	5 5/8	80	57	48	62	44	37	104	74	63	80	57	48	25
10	2.54	3.81	6.35	102	73	58	78	56	45	132	94	76	102	73	58	28
11	2.82	4.23	7.05	125	89	71	96	69	55	162	116	93	125	89	71	31

◯ NOTES FOR TABLE A

1. LENGTHS SHOWN CONFORM WITH NON SEISMIC PROVISIONS OF ACI 318-95 FOR UNCOATED BARS NOT ENCLOSED BY CLOSELY SPACED SPIRALS OR TIES. DEVELOPMENT OF REINFORCEMENT NOT COVERED BY THE TABLE SHALL CONFORM WITH ACI 318-95.

2. MULTIPLY LENGTHS SHOWN BY 0.87 FOR 4000 PSI. CONCRETE, BUT LENGTH OF LAP SHALL NOT BE LESS THAN 12 INCH.

3. MULTIPLY LENGTHS SHOWN BY 1.3 FOR LIGHTWEIGHT AGGREGATE CONCRETE.

4. BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS WHEN ALL BARS ARE LAPPED AT THE SAME LOCATION. WHEN BAR LAPS ARE STAGGERED, AND LAP HALF THE BARS ARE LAPPED AT THE SAME LOCATION, THE BAR CLEAR SPACING IS TWICE THE CENTER TO CENTER BAR SPACING MINUS TWO BAR DIAMETERS. WHEN ALL BARS ARE EMBEDDED AT THE SAME LOCATION, THE BAR CLEAR SPACING IS THE CENTER TO CENTER BAR SPACING MINUS ONE BAR DIAMETER.

5. CLASS A LAP LENGTHS APPLY ONLY WHERE NOTED ON THE DRAWINGS.

6. LAP AND EMBEDMENT LENGTHS SHOWN APPLY WHEN MINIMUM CONCRETE COVER OVER BARS CONFORMS WITH VALUES GIVEN IN THE TABLE FOR "CONCRETE COVER". THESE COVER VALUES CONFIRM WITH ACI 318-95.

7. CLASS A LAP AND EMBEDMENT LENGTH HAVE SAME VALUE.

8. CLASS B LAP LENGTHS APPLY FOR ALL SPLICES UNLESS NOTED OTHERWISE.

9. HOOK LENGTH GIVEN IS THE STRAIGHT LINE DISTANCE FROM THE LOCATION OF MAXIMUM STRESS IN THE BAR TO THE OUTSIDE END OF THE HOOK. MULTIPLY LENGTHS GIVEN BY 0.7 FOR HOOKS WITH SIDE COVER NORMAL TO THE HOOK NOT LESS THAN 2-1/2 INCH AND FOR 90 DEGREE HOOKS COVER ON BAR EXTENSION BEYOND HOOK NOT LESS THAN 2 INCH.

10. TOP BARS ARE HORIZONTAL REINFORCEMENT PLACED SO THAT MORE THAN 12 INCHES OF CONCRETE IS CAST BELOW THE REINFORCEMENT.

11. MULTIPLY LAP AND EMBEDMENT LENGTHS GIVEN BY 2.0 FOR BARS WITH CLEAR SPACING OF TWO BAR DIAMETERS OR LESS OR CONCRETE COVER OF ONE BAR DIAMETER OR LESS.

12. MINIMUM CONCRETE COVER FROM FACE OF MEMBER TO EDGE BAR SHALL NOT BE LESS THAN TWO AND ONE HALF BAR DIAMETERS.

1 REBAR TYPICAL LAPS & NOTES

Scale: N.T.S.

SCHEDULE OF CONSTRUCTION MATERIALS

	LOCATION	28-DAY COMPRESSIVE STRENGTH
CONCRETE	EXTERIOR CONCRETE (EXPOSED TO FREEZING AND/OR DE-ICERS)	4,000 P.S.I. MIX TYPE D
	EXTERIOR CONCRETE (NOT EXPOSED TO FREEZING)	3,000 P.S.I. MIX TYPE A
	FOOTINGS	3,000 P.S.I. MIX TYPE A
	FOUNDATION WALLS	3,000 P.S.I. MIX TYPE D
	INTERIOR SLABS ON GRADE	3,000 P.S.I. MIX TYPE E
	NOTE: CONCRETE STRENGTH USED IN DESIGN IS 3,000 P.S.I.	
STRUCTURAL STEEL	APPLICATION	MATERIAL
	SQUARE OR RECTANGULAR HSS	ASTM A500 (46ksi) GRADE B
	WIDE FLANGES SECTIONS	ASTM A992 (50ksi)
	OTHER SHAPES AND PLATES	ASTM A36 (36ksi)
REINFORCING STEEL	BARS SHOWN ON DRAWING TO BE FIELD BENT	ALL OTHER BARS
	ASTM A615, GRADE 40 OR GRADE 60 (SEE LAP SPLICE SCHEDULE D/S003 FOR LAP LENGTHS	ASTM A615, GRADE 60 (SEE LAP SPLICE SCHEDULE D/S003 FOR LAP LENGTHS)

Type A - Water reducing admixture. This is used to reduce the quantity of mixing water at a given workability or increase workability at a given water content.
Type B - Retarding admixture used for increasing setting time of concrete.
Type C - Accelerating admixture used for decreasing setting time and to develop early strength gain.
Type D - Water reducing and retarding admixture has the effects of both A and B.
Type E - Water reducing and accelerating admixtures has the effects of both A and C.
Type F - Water reducing, high range admixture used to reduce the quantity of mixing water required to produce concrete of a given consistency by 12% or more, and can be used to produce high slump or flowing concrete.
Type G - Water reducing, high range, and retarding admixtures are used to reduce the quantity of mixing water required to produce concrete of given consistency by 12% or more and retard setting times of concrete.
Type S - Specific performance admixtures used for shrinkage reduction, ASR mitigation, viscosity modification or any other specific requirement.

	APPLICATION	SPECIES AND MINIMUM GRADE
WOOD DIMENSION LUMBER	TOP PLATES STRUTS ROOF JOISTS FLOOR JOISTS MISC. FRAMING HEADERS BEAMS LEDGERS	DOUGLAS FIR-LARCH #2 OR BETTER HEM FIR #2 OR BETTER SPRUCE-PINE-FIR #2 OR BETTER MSR 1650F 1.5E OR BETTER
	BLOCKING	DOUGLAS FIR-LARCH #2 OR BETTER HEM FIR #2 OR BETTER SPRUCE-PINE-FIR #2 OR BETTER MSR 1650F 1.5E OR BETTER
	POSTS AND TIMBERS 5"x5" AND LARGER	DOUGLAS FIR-LARCH #2 OR BETTER SOUTHERN PINE #2 OR BETTER
	SILL PLATES	2x4's: DOUGLAS FIR-LARCH STANDARD OR BETTER; HEM FIR STANDARD OR BETTER; SPRUCE-PINE-FIR STANDARD OR BETTER; SCL 1.3E 2x6's OR GREATER: DOUGLAS FIR-LARCH #2 OR BETTER; HEM FIR #2 OR BETTER; SPRUCE-PINE-FIR #2 OR BETTER; SCL 1.5E
	TRUSSED RAFTERS (CHORDS AND WEBS)	DOUGLAS FIR-LARCH #2 OR BETTER HEM FIR #2 OR BETTER SPRUCE-PINE-FIR #2 OR BETTER MSR 1650F 1.5E OR BETTER
	EXTERIOR WALL STUDS & INTERIOR STRUCTURAL WALL STUDS	DOUGLAS FIR-LARCH STUD GRADE OR BETTER. SEE NOTE 3 HEM FIR STUD GRADE OR BETTER. SEE NOTE 3 SPRUCE-PINE-FIR STUD GRADE OR BETTER. SEE NOTE 3
	INTERIOR NON-STRUCTURAL WALL STUDS	DOUGLAS FIR-LARCH STANDARD, UTILITY, CONSTRUCTION OR BETTER. SEE NOTE 3 HEM FIR STANDARD, UTILITY, CONSTRUCTION OR BETTER. SEE NOTE 3 SPRUCE-PINE-FIR STANDARD, UTILITY, CONSTRUCTION OR BETTER. SEE NOTE 3

APPLICATION		DESIGN VALUES (SEE NOTE 1) - P.S.I.					
		Fb	Fv	Fc ⊥	Fc ∥	E x 10 6	
STRUCTURAL COMPOSITE LUMBER (SCL)	1-1/2" x=< 5-1/2"	1700	285	680	1400	1.3	
	1-1/2" x ALL DEPTHS	2,250	285	750	2,175	1.5	
	1-3/4" x ALL DEPTHS	2,600	285	750	2,510	1.9	
	3-1/2" x ALL DEPTHS	1,700	285	680	1,400	1.3	
GLUED LAMINATED BEAMS		ALLOWABLE STRESSES - P.S.I.					
		Fb TENSION ZONE	Fb COMPRESSION ZONE STRESSED IN TENSION		Fv	Fc	E x 10 6
	ALL BEAMS 24F-V4 DF/DF OR 24F-V3 SP/SP (W/ STRESS CLASS) OR 24F-1.8E	2,400 (MIN.)	1450 (MIN.) SEE NOTE 2		265 (MIN.)	1,600 (MIN.)	1.8 (MIN.)

NOTES:
1. DESIGN VALUES ARE FOR NORMAL DURATION. REPETITIVE FRAMING FACTORS AND SIZE FACTORS HAVE NOT BEEN APPLIED.
2. 1850 FOR DF/DF COMBINATION AND 1950 FOR SP/SP COMBINATION.
3. FOR 2x6 AND LARGER, USE #2 GRADE OR BETTER FOR ANY OF THE THREE WOOD TYPES.

TYPICAL ANCHOR BOLT SCHEDULE

INSTALLATION TYPE	CAST-IN-PLACE (PRE AUTHORIZED) [2]				DRILL-IN-OPTIONS (SUBMITTAL REQUIRED) [3]
BOLT TYPE	STANDARD J-BOLT	HEADED ANCHOR	THREADED ROD ANCHOR	SIMPSON "SSTB" ANCHOR BOLT	ADHESIVE ANCHOR
PROJECTION NOTE (1) / MIN. EMBED UNO PER PLAN — EMBEDMENT REQUIREMENTS	7 1/2"	12x DIA.	TACK; 1/4" MIN.	PER MFR	[4]; NOT ALLOWED AT P-T SLAB
LIMITS	5/8"Ø MAX.	5/8"Ø THRU 2 1/2"Ø		FOR WOOD FRAME ONLY	5/8"Ø THRU 1"Ø
ANCHOR BOLT MATERIAL - A325 OR F1554 GRADE 36 MIN.					
DIA = ANCHOR BOLT DIAMETER (NOMINAL)					

NOTES:

1. CONTRACTOR SHALL DETERMINE THE REQUIRED THREAD PROJECTION SUITABLE FOR THE THICKNESS OF MATERIAL BEING FASTENED PLUS GROUT ALLOWANCE, IF ANY, AND CONSTRUCTION TOLERANCES, UNO.
2. CONTRACTOR MAY SELECT APPROPRIATE CAST-IN-PLACE ANCHOR BOLT OPTION WITHOUT SUBMITTAL.
3. DRILL-IN OPTIONS ARE NOT APPROPRIATE AT ALL CONDITIONS. IF DRILL-IN METHOD IS PREFERRED, SUBMIT MANUFACTURER'S INFORMATION, ALLOWABLE LOAD VS. EMBEDMENT DATA AND LOCATIONS OF WHERE SUBSTITUTIONS ARE REQUESTED. ENGINEER WILL DETERMINE IF SUBSTITUTION IS APPROPRIATE FOR LOCATION AND LOADING.
4. EMBEDMENT OF DRILL-IN ANCHORS SHALL BE PER ENGINEER'S SUBMITTAL REVIEW COMMENTS. EMBEDMENT SHALL BE (9) NINE TIMES THE NOMINAL ANCHOR DIAMETER, UNO.
5. AT PRESSURE TREATED SILLS, PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL ANCHORS.

2 ANCHOR DETAILS

Scale: N.T.S.

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NAP

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STRUCTURAL GENERAL NOTES

S0.1

SHEET: 7 OF 140

DEFINITIONS

THE FOLLOWING DEFINITIONS COVER THE MEANINGS OF CERTAIN TERMS USED IN THESE NOTES:

'ARCHITECT/ENGINEER' - THE ARCHITECT OF RECORD AND THE STRUCTURAL ENGINEER OF RECORD.

- 'STRUCTURAL ENGINEER OF RECORD' (SER) - THE STRUCTURAL ENGINEER WHO IS LICENSED TO STAMP AND SIGN THE STRUCTURAL DOCUMENTS FOR THE PROJECT. THE SER IS RESPONSIBLE FOR THE DESIGN OF THE PRIMARY STRUCTURAL SYSTEM.
- 'SUBMIT FOR REVIEW' - SUBMIT TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION OR CONSTRUCTION.
- 'PER PLAN' - INDICATES REFERENCES TO THE STRUCTURAL PLANS, ELEVATIONS AND STRUCTURAL GENERAL NOTES.
- 'SPECIALTY STRUCTURAL ENGINEER' (SSE) - A PROFESSIONAL ENGINEER (PE OR SE), LICENSED IN THE STATE WHERE THE PROJECT IS LOCATED, (TYPICALLY NOT THE SER), WHO PERFORMS SPECIALTY STRUCTURAL ENGINEERING SERVICES FOR SELECTED SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS, AND WHO HAS EXPERIENCE AND TRAINING IN THE SPECIALTY. DOCUMENTS STAMPED AND SIGNED BY THE SSE SHALL BE COMPLETED BY OR UNDER THE DIRECT SUPERVISION OF THE SSE.
- 'BIDDER-DESIGNED' - COMPONENTS OF THE STRUCTURE THAT REQUIRE THE GENERAL CONTRACTOR, SUBCONTRACTOR, OR SUPPLIER WHO IS RESPONSIBLE FOR THE DESIGN, FABRICATION AND INSTALLATION OF SPECIALTY-ENGINEERED ELEMENTS IDENTIFIED IN THE CONTRACT DOCUMENTS TO RE-RETAIN THE SERVICES OF AN SSE. SUBMITTALS OF 'BIDDER-DESIGNED' ELEMENTS SHALL BE STAMPED AND SIGNED BY THE SSE.

OTHER DRAWINGS: REFER TO THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, CIVIL AND PLUMBING DRAWINGS FOR ADDITIONAL INFORMATION INCLUDING BUT NOT LIMITED TO: DIMENSIONS, ELEVATIONS, SLOPES, DOOR AND WINDOW OPENINGS, NON-BEARING WALLS, STAIRS, FINISHES, DRAINS, WATERPROOFING, RAILINGS, MECHANICAL UNIT LOCATIONS, AND OTHER NON-STRUCTURAL ITEMS.

STRUCTURAL DETAILS: THE STRUCTURAL DRAWINGS ARE INTENDED TO SHOW THE GENERAL CHARACTER AND EXTENT OF THE PROJECT AND ARE NOT INTENDED TO SHOW ALL DETAILS OF THE WORK.

STRUCTURAL RESPONSIBILITIES: THE STRUCTURAL ENGINEER (SER) IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE PRIMARY STRUCTURE IN ITS COMPLETED FORM.

COORDINATION: THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING DETAILS AND ACCURACY OF THE WORK; FOR CONFIRMING AND CORRELATING ALL QUANTITIES AND DIMENSIONS; FOR SELECTING FABRICATION PROCESSES; FOR TECHNIQUES OF ASSEMBLY; AND FOR PERFORMING WORK IN A SAFE AND SECURE MANNER.

MEANS, METHODS AND SAFETY REQUIREMENTS: THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND ALL JOB RELATED SAFETY STANDARDS SUCH AS OSHA AND DOSH (DEPARTMENT OF OCCUPATIONAL SAFETY AND HEALTH).

BRACING/SHORING DESIGN ENGINEER: THE CONTRACTOR SHALL, AT HIS DISCRETION EMPLOY AN SSE, A REGISTERED PROFESSIONAL ENGINEER FOR THE DESIGN OF ANY TEMPORARY BRACING AND SHORING.

TEMPORARY SHORING, BRACING: THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURE IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

CONSTRUCTION LOADS: LOADS ON THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE DESIGN LOADS AS NOTED IN DESIGN CRITERIA & LOADS BELOW OR THE CAPACITY OF PARTIALLY COMPLETED CONSTRUCTION AS DETERMINED BY THE CONTRACTOR'S SSE FOR BRACING/SHORING.

CHANGES IN LOADING: THE CONTRACTOR HAS THE RESPONSIBILITY TO NOTIFY THE SER OF ANY ARCHITECTURAL, MECHANICAL, ELECTRICAL, OR PLUMBING LOAD IMPOSED ONTO THE STRUCTURE THAT DIFFERS FROM, OR THAT IS NOT DOCUMENTED ON THE ORIGINAL CONTRACT DOCUMENTS (ARCHITECTURAL / STRUCTURAL / MECHANICAL / ELECTRICAL OR PLUMBING DRAWINGS). PROVIDE DOCUMENTATION OF LOCATION, LOAD, SIZE AND ANCHORAGE OF ALL UNDOCUMENTED LOADS IN EXCESS OF 400 POUNDS. PROVIDE MARKED-UP STRUCTURAL PLAN INDICATING LOCATIONS OF ANY NEW EQUIPMENT OR LOADS. SUBMIT PLANS TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO INSTALLATION.

NOTE PRIORITIES: PLAN AND DETAIL NOTES AND SPECIFIC LOADING DATA PROVIDED ON INDIVIDUAL PLANS AND DETAIL DRAWINGS SUPPLEMENTS INFORMATION IN THE STRUCTURAL GENERAL NOTES.

DISCREPANCIES: IN CASE OF DISCREPANCIES BETWEEN THE GENERAL NOTES, SPECIFICATIONS, PLAN/DETAILS OR REFERENCE STANDARDS, THE ARCHITECT/ENGINEER SHALL DETERMINE WHICH SHALL GOVERN. DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK. SHOULD ANY DISCREPANCY BE FOUND IN THE CONTRACT DOCUMENTS, THE CONTRACTOR WILL BE DEEMED TO HAVE INCLUDED IN THE PRICE THE MOST EXPENSIVE WAY OF COMPLETING THE WORK, UNLESS PRIOR TO THE SUBMISSION OF THE PRICE, THE CONTRACTOR ASKS FOR A DECISION FROM THE ARCHITECT AS TO WHICH SHALL GOVERN. ACCORDINGLY, ANY CONFLICT IN OR BETWEEN THE CONTRACT DOCUMENTS SHALL NOT BE A BASIS FOR ADJUSTMENT IN THE CONTRACT PRICE.

SITE VERIFICATION: THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE SITE. CONFLICTS BETWEEN THE DRAWINGS AND ACTUAL SITE CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT/ENGINEER BEFORE PROCEEDING WITH THE WORK.

ADJACENT UTILITIES: THE CONTRACTOR SHALL DETERMINE THE LOCATION OF ALL ADJACENT UNDERGROUND UTILITIES PRIOR TO EARTHWORK, FOUNDATIONS, SHORING, AND EXCAVATION. ANY UTILITY INFORMATION SHOWN ON THE DRAWINGS AND DETAILS IS APPROXIMATE AND NOT NECESSARILY COMPLETE.

ALTERNATES: ALTERNATE PRODUCTS OF SIMILAR STRENGTH, NATURE, AND FORM FOR SPECIFIED ITEMS MAY BE SUBMITTED WITH ADEQUATE TECHNICAL DOCUMENTATION TO THE ARCHITECT/ENGINEER FOR REVIEW. ALTERNATE MATERIALS THAT ARE SUBMITTED WITHOUT ADEQUATE TECHNICAL DOCUMENTATION OR THAT SIGNIFICANTLY DEVIATE FROM THE DESIGN INTENT OF MATERIALS SPECIFIED MAY BE RETURNED WITHOUT REVIEW. ALTERNATES THAT REQUIRE SUBSTANTIAL EFFORT TO REVIEW WILL NOT BE REVIEWED UNLESS AUTHORIZED BY THE OWNER.

SUBMITTALS

SUBMIT FOR REVIEW: SUBMITTALS OF SHOP DRAWINGS, AND PRODUCT DATA ARE REQUIRED FOR ITEMS NOTED IN THE INDIVIDUAL MATERIALS SECTIONS AND FOR BIDDER DESIGNED ELEMENTS.

SUBMITTAL REVIEW PERIOD: SUBMITTALS SHALL BE MADE IN TIME TO PROVIDE A MINIMUM OF TWO WEEKS FOR REVIEW BY THE ARCHTECT/ENGINEER PRIOR TO THE ONSET OF FABRICATION.

GENERAL CONTRACTOR'S PRIOR REVIEW: PRIOR TO SUBMISSION TO THE ARCHITECT/ENGINEER, THE CONTRACTOR SHALL REVIEW THE SUBMITTAL FOR COMPLETENESS. DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE 'SER', AND THEREFOR, MUST BE VERIFIED BY THE GENERAL CONTRACTOR. CONTRACTOR SHALL PROVIDE ANY NECESSARY DIMENSIONAL DETAILS REQUESTED BY THE DETAILER AND PROVIDE THE CONTRACTOR'S REVIEW STAMP AND SIGNATURE BEFORE FORWARDING TO THE ARCHITECT/ENGINEER.

SHOP DRAWING REVIEW: ONCE THE CONTRACTOR HAS COMPLETED THE REQUIRED PRIOR REVIEW, THE 'SER' WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN INTENT AND THE CONTRACT DOCUMENTS OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. MARKINGS OR COMMENTS SHALL NOT BE CONSTRUED AS RELIEVING THE CONTRACTOR FROM COMPLIANCE WITH THE PROJECT PLANS AND SPECIFICATIONS, NOR THE DEPARTURES THERE FROM.

SHOP DRAWING DEVIATIONS: WHEN SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) DIFFER FROM OR ADD TO THE REQUIREMENTS OF THE STRUCTURAL DRAWINGS THEY SHALL BE DESIGNED AND STAMPED BY THE RESPONSIBLE 'SSE'.

DEFERRED SUBMITTALS

BIDDER-DESIGNED ELEMENTS
SUBMIT "BIDDER-DESIGNED" DEFERRED SUBMITTALS TO THE ARCHITECT AND 'SER' FOR REVIEW PRIOR TO SUBMISSION TO THE APPROVING AGENCY.

DESIGN OF PREFABRICATED, "BIDDER-DESIGNED", MANUFACTURED, PRE-ENGINEERED, OR OTHER FABRICATED PRODUCTS SHALL COMPLY WITH THE FOLLOWING REQUIREMENTS:

1. DESIGN CONSIDERS TRIBUTARY DEAD, LIVE, WIND AND EARTHQUAKE LOADS IN COMBINATIONS REQUIRED BY IBC.
2. DESIGN WITHIN THE DEFLECTION LIMITS NOTED HEREIN AND AS SPECIFIED OR REFERENCED IN THE IBC.
3. DESIGN SHALL CONFORM TO THE SPECIFICATIONS AND REFERENCE STANDARDS OF THE GOVERNING CODE.
4. SUBMITTAL SHALL INCLUDE:
   a. CALCULATIONS PREPARED, STAMPED AND SIGNED BY THE 'SSE' DEMONSTRATING CODE CONFORMANCE.
   b. ENGINEERED COMPONENT DESIGN DRAWINGS ARE PREPARED, STAMPED AND SIGNED BY THE 'SSE'.
   c. PRODUCT DATA, TECHNICAL INFORMATION AND MANUFACTURER'S WRITTEN REQUIREMENTS AND AGENCY APPROVALS AS APPLICABLE.
   d. 'SSE' MAY SUBMIT TO THE ARCHITECT/ENGINEER, A REQUEST TO UTILIZE RELEVANT ALTERNATE DESIGN CRITERIA OF SIMILAR NATURE AND GENERALLY EQUIVALENCY WHICH IS RECOGNIZED BY THE CODE AND ACCEPTABLE TO THE AUTHORITY HAVING JURISDICTION. SUBMIT ADEQUATE DOCUMENTATION OF DESIGN.

DEFLECTION LIMITS:	VERTICAL	LIMIT
	ROOF MEMBERS, DEAD + LIVE OR SNOW OR WIND, TOTAL LOAD (TL) DEFLECTION	L / 240, WHERE (L IS SPAN LENGTH, INCHES)
	ROOF, LIVE OR SNOW OR WIND LOAD (RLL)	L / 360
	FLOOR MEMBERS, TOTAL LOAD (TL) uno	L / 240
	FLOOR LIVE LOAD (LL) uno	L / 360
	HORIZONTAL	LIMIT AND FOOTNOTE
	MEMBERS SUPPORTING BRITTLE FINISHES	L / 240 (1)
	MEMBERS SUPPORTING FLEXIBLE FINISHES	L / 180 (1)

(1) WIND LOAD IS REDUCIBLE TO 0.6 TIMES THE COMPONENT AND CLADDING LOADS PER TABLE 1604.3 FOOTNOTE f.

GENERAL CONTRACTOR'S PRIOR REVIEW: ONCE THE CONTRACTOR HAS COMPLETED THE REVIEW OF THE 'SSE' COMPONENT DRAWINGS, THE 'SER' WILL REVIEW THE SUBMITTAL FOR GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING AND WILL STAMP THE SUBMITTAL ACCORDINGLY. REVIEW OF THE SPECIALTY STRUCTURAL ENGINEER'S 'SSE' SHOP DRAWINGS (COMPONENT DESIGN DRAWINGS) IS FOR COMPLIANCE WITH DESIGN CRITERIA AND COMPATIBILITY WITH THE DESIGN OF THE PRIMARY STRUCTURE AND DOES NOT RELIEVE THE 'SSE' OF RESPONSIBILITY FOR THAT DESIGN. ALL NECESSARY BRACING, TIES, ANCHORAGE, PROPRIETARY PRODUCTS SHALL BE FURNISHED AND INSTALLED PER MANUFACTURER'S INSTRUCTIONS OR THE 'SSE'S' DESIGN DRAWINGS AND CALCULATIONS. THESE ELEMENTS INCLUDE BUT ARE NOT LIMITED TO THE FOLLOWING:

"BIDDER-DESIGNED" DEFERRED SUBMITTALS

- PRE-FABRICATED METAL PLATED ROOF & FLOOR TRUSSES
- TEMPORARY SHORING SYSTEMS
- STAIR HANDRAILS, GUARDRAILS, BALCONY RAIL ANCHORAGES

QUALITY ASSURANCE (TESTING AND INSPECTION)

QUALITY ASSURANCE (TESTING AND INSPECTION) AS REQUIRED BY THE OWNER AND SECTIONS 1704 THRU 1709 OF THE IBC, SHALL BE PROVIDED BY A QUALIFIED AGENCY SELECTED BY THE OWNER. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE REQUIRED INSPECTIONS. ALL TESTING AND INSPECTION REPORTS SHALL BE SENT WITHIN 24 HOURS OF THE TEST OR INSPECTION TO THE OWNER, ARCHITECT, APPLICABLE ENGINEER, BUILDING OFFICIAL AND GENERAL CONTRACTOR. ITEMS REQUIRING QUALITY ASSURANCE (TESTING AND INSPECTION) ARE:

1. SOILS/ENGINEERED FILL (IBC SECTION 1705.6 AND TABLE 1705.6):
   A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
   B. PRIOR TO PLACEMENT OF THE PREPARED FILL, THE INSPECTOR SHALL DETERMINE THAT THE SITE HAS BEEN PREPARED IN ACCORDANCE WITH THE SOILS REPORT.
   C. DURING PLACEMENT AND COMPACTION OF THE FILL MATERIAL, THE INSPECTOR SHALL DETERMINE THAT THE MATERIAL BEING USED AND THE MAXIMUM LIFT THICKNESS COMPLIES WITH THE GEO-TECHNICAL'S REPORT.
   D. THE INSPECTOR SHALL DETERMINE THAT THE IN-PLACE DRY DENSITY OF THE ENGINEERED FILL MATERIAL COMPLIES WITH THE GEO-TECHNICAL REPORT.
      1. CONTINUOUS FOOTING ENGINEERED FILL: SEE SPECIFICATIONS.
      2. SPOT FOOTING ENGINEERED FILL: SEE SPECIFICATIONS.
      3. SITE WORK ENGINEERED FILL (PAVED AREAS, SIDEWALKS, TRENCHES, ETC.): SEE SPECIFICATIONS.
      4. BUILDING PAD ENGINEERED FILL: SEE SPECIFICATIONS.

2. CONCRETE (IBC SECTION 1705.3 AND TABLE 1705.3):
   A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
   B. CYLINDERS, SLUMP, TEMPERATURE AND AIR-ENTRAINMENT SHALL BE DONE. PROVIDE SIX SAMPLES EACH FOR EXTERIOR SITE WORK CONCRETE, FOOTINGS, FOUNDATION WALLS AND INTERIOR SLABS ON GRADE.
   C. PROTECTION OF CONCRETE DURING COLD AND HOT WEATHER.

3. PREFABRICATED METAL PLATE WOOD TRUSSES (IBC SECTIONS 1705.5)
   A. REFER TO SPECIFICATION SECTION 06 1753 SHOP-FABRICATED WOOD TRUSSES: TRUSSED RAFTERS FOR CERTIFICATION REQUIREMENTS.
   B. THE INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS OF INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATORS ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS.

4. WOOD DIAPHRAGMS AND SHEAR WALLS (IBC SECTION 1705.5)
   A. FOR WALLS AND ROOF AREAS WHERE THE NAIL SPACING IS 4 INCHES AND LESS ON CENTER, THE INSPECTOR SHALL VERIFY WOOD PANEL SHEATHING GRADE, THICKNESS AND NOMINAL SIZE OF FRAMING MEMBERS, ADJOINING PANEL EDGES, NAIL SIZE AND SPACING, BOLTING AND OTHER FASTENING OF COMPONENTS.

5. EPOXY ANCHORS (IBC SECTION 1706.1):
   A. REFER TO SPECIFICATION SECTION 01 4523 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
   B. INSPECTION SHALL VERIFY ALL DRILLED HOLES SIZE AND DEPTH PRIOR TO INSTALLATION OF EPOXY AND ANCHOR ROD. SEE SPECIFICATIONS FOR QUANTITY OF TESTING.

6. STEEL (IBC SECTION 1705.2):
   A. INSPECTION DURING FABRICATION IS NOT REQUIRED IF THE FABRICATOR IS REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT INSPECTION. SPECIAL INSPECTION OF FIELD WELDS AS REQUIRED BY IBC SECTION 1704 .

7. MASONRY (IBC SECTION 1705.4):
   A. REFER TO SPECIFICATION SECTION 04 0000 FOR ADDITIONAL AND SPECIFIC TESTING AND INSPECTION REQUIREMENTS.
   B. SPECIAL INSPECTION OF MASONRY IS NOT REQUIRED.

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8/21/23

REVIEWED FOR CODE COMPLIANCE
01/12/2024

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REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
STRUCTURAL GENERAL NOTES

S0.2

SHEET: 8 OF 140

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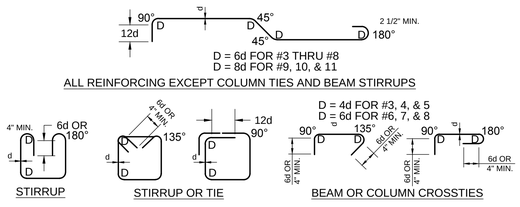

1 REBAR TYPICAL BEND DETAILS

Scale: N.T.S.

TYPICAL ANCHOR BOLT SCHEDULE

	INSTALLATION TYPE	CAST-IN-PLACE (PRE AUTHORIZED) [2]				DRILL-IN-OPTIONS (SUBMITTAL REQUIRED) [3]
PROJECTION NOTE (1)	BOLT TYPE	STANDARD J-BOLT	HEADED ANCHOR	THREADED ROD ANCHOR	SIMPSON "SSTB" ANCHOR BOLT	ADHESIVE ANCHOR
MIN. EMBED UNO PER PLAN	EMBEDMENT REQUIREMENTS	7 1/2"	12x DIA. TACK 1/4" MIN.		PER MFR	[4] NOT ALLOWED AT P-T SLAB
	LIMITS	5/8"Ø MAX.	5/8"Ø THRU 2 1/2"Ø		FOR WOOD FRAME ONLY	5/8"Ø THRU 1"Ø
	ANCHOR BOLT MATERIAL - A325 OR F1554 GRADE 36 MIN.					
	DIA = ANCHOR BOLT DIAMETER (NOMINAL)					

NOTES:

1. CONTRACTOR SHALL DETERMINE THE REQUIRED THREAD PROJECTION SUITABLE FOR THE THICKNESS OF MATERIAL BEING FASTENED PLUS GROUT ALLOWANCE, IF ANY, AND CONSTRUCTION TOLERANCES, UNO.
2. CONTRACTOR MAY SELECT APPROPRIATE CAST-IN-PLACE ANCHOR BOLT OPTION WITHOUT SUBMITTAL.
3. DRILL-IN OPTIONS ARE NOT APPROPRIATE AT ALL CONDITIONS. IF DRILL-IN METHOD IS PREFERRED, SUBMIT MANUFACTURER'S INFORMATION, ALLOWABLE LOAD VS. EMBEDMENT DATA AND LOCATIONS OF WHERE SUBSTITUTIONS ARE REQUESTED. ENGINEER WILL DETERMINE IF SUBSTITUTION IS APPROPRIATE FOR LOCATION AND LOADING.
4. EMBEDMENT OF DRILL-IN ANCHORS SHALL BE PER ENGINEER'S SUBMITTAL REVIEW COMMENTS. EMBEDMENT SHALL BE (9) NINE TIMES THE NOMINAL ANCHOR DIAMETER, UNO.
5. AT PRESSURE TREATED SILLS, PROVIDE HOT DIPPED GALVANIZED OR STAINLESS STEEL ANCHORS.

2 ANCHOR DETAILS

Scale: N.T.S.

FOOTING SCHEDULE

MARK	SIZE L x W x H	REINFORCEMENT	REMARKS
F25	2'-6" x 2'-6" x 1'-0"	3-#5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1
F30	3'-0" x 3'-0" x 1'-0"	4- #5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1
F40	4'-0" x 4'-0" x 1'-0"	5- #5 BARS T&B E.W.	PIER PER DETAIL 9/S3.1
F50	5'-0" x 5'-0" x 1'-0"	6- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1
F60	6'-0" x 6'-0" x 1'-0"	7- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1
F65	6'-6" x 6'-6" x 1'-0"	8- #5 BARS T&B E.W	PIER SIM. DETAIL 2/S3.1

1. TOP OF INTERIOR SPOT FOOTINGS TO BE FF = -6", TYP. UNLESS OTHERWISE NOTED ON FOUNDATION PLAN.
2. TOP OF PERIMETER FOOTINGS TO BE FF = -24", TYP. UNLESS OTHERWISE NOTED ON FOUNDATION PLAN
3. SPOT PERIMETER FOOTINGS MAY BE COMBINED WITH PERIMETER WALL FOOTINGS, BUT SPECIFIED REINFORCING IS IN ADDITION TO PERIMETER WALL FOOTINGS.

COLUMN FOOTING PIER SCHEDULE

MARK	SIZE L x W x H	REINFORCEMENT	T/F ELEV.	REMARKS
P1	16" x 16" x 8"	4 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT PERIMETER FOUNDATION WALLS
P2	16" x 16" x 18"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT FOOTINGS NEAR POOL WALLS
P3	18" x 18" x 54"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT CANOPY FOOTINGS
P4	16" x 16" x 54"	8 - #5 BARS VERTICAL #3 STIRRUPS AT 6" O.C. HORZ.	SEE PLAN	PIERS AT CANOPY FOOTINGS

1. PIER HEIGHT SHALL BE FROM TOP OF FOOTING UP TO FINISH FLOOR UNLESS NOTED OTHERWISE.
2. PERIMETER PIERS MAY BE COMBINED WITH PERIMETER FOUNDATION WALLS, BUT SPECIFIED REINFORCING IS IN ADDITION TO THE PERIMETER FOUNDATION WALL REINFORCING.
3. ALL HORIZONTAL TIES SHALL HAVE 4" x 4" LAP LOCK AT CORNER, DETAIL 1/S0.3.
4. ALL PIER REINFORCING SHALL HAVE THE MINIMUM COVER SPECIFIED ON GENERAL CRITERIA "GENERAL CRITERIA" NOTE 7, S0.1.

BEAM SCHEDULE

MARK	TYPE	SIZE	NOTES
B1	STL. A-50	W 6 x 12	
B2	STL. A-50	W 8 x 18	
B3	STL. A-50	W 10 x 30	
B4	STL. A-50	W 18 x 50	
B5	STL. A-50	W 21 x 83	
B6	STL. A-50	W 16 x 26	
B7	STL. A-46	TS 6 x 12	
B8	STL. A-36	MC 8 x 8.5	
B9	STL. A-50	W 8 x 58	

ELEVATOR BEAM SCHEDULE

MARK	TYPE	SIZE	NOTES
EB1	STL. A-50	W 6x4X5/16"	BRACE BEAM SIZED FOR VERT & HORZ. LOADS FROM CAR RAIL PER MANUFACTURER
EB2	STL. A-50	W 8 x 31	HOIST BEAM SIZED FOR 15,000LBS LOCATED AT THE CENTER OF THE BEAM

JOIST SCHEDULE

MARK	SIZE	SPACING	NOTES
J1	2 X 8	16" O.C. MAX.	SEE DETAILS
J2	11 7/8 TJI360	16" O.C. MAX.	SEE DETAILS
J3	2 x 6	16" O.C. MAX.	SEE DETAILS
J4	2 x 10	12" O.C. MAX.	SEE DETAILS
J5	4X8	32" O.C. MAX.	SEE DETAILS
J6	4X10	32" O.C. MAX.	SEE DETAILS

10-26-23

COLUMN SCHEDULE

MARK	TYPE	SIZE	NOTES
C1	STL. A-46	TS 4 x 4 x 5/16	SEE S3.1
C2	STL. A-46	TS 3 x 3 x 5/16	SEE S3.1
C3	STL. A-46	TS 6 x 6 x 5/16	SEE S3.1
C4	STL. A-46	TS 10 x 10 x 5/16	SEE S3.1

1. SEE FOUNDATION PLAN AND DETAIL 10/S5.3 FOR BASE PLATES.

WOOD BEARING WALL SCHEDULE

WALLS ON FLOOR LEVEL	TYPICAL EXTERIOR WALL - WW1			TYPICAL CORRIDOR WALL - WW2			TYPICAL INTERIOR BEARING WALLS, SUPPORTING DEMISING WALLS- WW3			DEMISING WALL SUPPORTING 2'-0" MAXIMUM TRIBUTARY WIDTH OF FLOOR/ROOF & DOUBLE 2X STAIRWAY WALLS FRAMING PER LEVEL - WW4			WW5		
	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE	STUD SIZE AND SPACING	STUD GRADE	PLATE GRADE
3&4	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2
1&2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@16"OC	SPF NO. 2	SPF NO. 2	2x6@12"OC	SPF NO. 2	SPF NO. 2

HEADER SCHEDULE 1,2

MARK	HEADER	FIRST & SECOND FLOOR		THIRD FLOOR		ROOF BEARING	
		JACK STUDS	KING STUDS	JACK STUDS	KING STUDS	JACK STUDS	KING STUDS
H1	2-2x8	2	2	2	2	2	1
H2	3-2x8	2	1	2	1	2	1
H3	3-2x10	3	1	2	1		
H4	3-2x8	2	1	2	1		
H5	3-2x12					2	1
H6	3-2x12	2	2	2	1		
H7	4-2x8	2	1	2	1	2	1
H8	1-6X12 SCL	2	2	2	1	2	1
H9	6X10	3	1				
H10	1-6X14 SCL	3	1				
H11	5.25x9.5 SCL					2	1
H12	3-1.75X14 SCL					2	1
H13	3-1.75X9.5 SCL					2	1

1. SEE SHEETS S1.1 THRU S1.4 FOR BUILDING LEVEL KEY
2. SEE SHEET S0.1 FOR SPECIES AND MINIMUM GRADE

5 SCHEDULES

Scale: N.T.S.

SHEAR WALL SCHEDULE

MARK	SHEATHING			ATTACHMENT AT PANEL EDGES			SOLE PLATE CONNECTION			HOLD DOWN			REMARKS
	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	LEVEL 1	LEVEL 2&3	LEVEL 4	
S1	5/8" GYP. SHEATHING, BLOCKED, ONE FACE	5/8" GYP. SHEATHING, UNBLOCKED, ONE FACE	5/8" GYP. SHEATHING, UNBLOCKED, ONE FACE	6d COOLER NAILS @ 7"OC	6d COOLER NAILS @ 7"OC	6d COOLER NAILS @ 7"OC	1/2"Ø ANCHORS @72"OC	10d NAILS AT 8"OC	10d NAILS AT 8"OC	1/2"Ø ANCHOR EACH END			SDPWS TABLE 4.3C
S2	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	1/2"Ø ANCHOR EACH END W/SIMPSON BPS BEARING PLATE			SDPWS TABLE 4.3C
S3	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	1/2" WOOD SHEATHING, UNBLOCKED, ONE FACE	6d COOLER NAILS @ 6"OC	6d COOLER NAILS @ 6"OC	6d COOLER NAILS @ 6"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	HDU5-SDS2.5 SEE NOTE 10	CS18	CS22	SDPWS TABLE 4.3A
S4	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	2 PLYS 5/8" GYP. SHEATHING, BLOCKED, ONE FACE	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	BASE PLY 6d COOLER NAILS @ 9"OC FACE PLY 8d COOLER NAILS @ 7"OC	1/2"Ø ANCHORS @72"OC	4-10d NAILS AT 16"OC	4-10d NAILS AT 16"OC	HDU11-SDS2.5	HDU8-SDS2.5	HDU4-SDS2.5	SDPWS TABLE 4.3C

UNLESS NOTED OTHERWISE:
1. SHEATHING TO BE CONTINUOUS FOR LENGTH OF SHEAR WALL.
2. SEE 8/S5.1 FOR ANCHORAGE OF TIE DOWN CONNECTORS TO SLAB ON GRADE.
3. PROVIDE MIN. OF 2-2x STUDS AT EACH END OF SHEAR WALL PANEL. SEE MANUFACTURER AND SCHEDULE FOR MIN.IMUM STUD NUMBER AND SIZE AT TIE DOWN LOCATIONS.
4. PROVIDE RSP STRAP TIES ON EACH SIDE OF TOP/SILL PLATES FOR BORED HOLE/NOTCH GREATER THAN 1 1/2" FOR 2x4 OR 2 1/2" FOR 2x6.
5. PROVIDE 6d @ 6" OC BETWEEN STUDS AT LOCATIONS WHERE WALL STUD SIZE CHANGES.
6. AT GYPSUM SHEATHING, ATTACHMENT AT INTERMEDIATE FIELD STUDS EQUALS ATTACHMENT AT PANEL EDGES.
7. AT WOOD SHEATHING, ATTACHMENT AT INTERMEDIATE FIELD STUDS EQUALS 12" O.C.
8. ALTERNATE FASTENERS ARE PERMITTED TO BE USED IF THEIR DIMENSIONS ARE NOT LESS THAN THE SPECIFIED DIMENSIONS. DRYWALL SCREWS ARE PERMITTED TO SUBSTITUTE FOR THE 8d (2 1/2" X 0.131") , AND 6d (1 7/8" X 0.092") NAILS LISTED ABOVE. NO. 6 (1 1/ 4") SCREWS TYPE S OR W ARE PERMITTED TO SUBSTITUTE FOR 6d (1 7/8" X 0.092) NAILS.
9. ALL WOOD MEMBERS FASTENER FOR SHEAR WALLS, FLOORS, ROOF WILL BE PER IBC FASTENING SCHEDULE TABLE 2304.9.1
10. AT SHEAR WALL 3 AT LEVEL 1 ANCHORS AT EACH END OF SHEAR WALLS WILL BE 1/2" Ø ANCHORS UNLESS OTHERWISE SHOWN ON PLANS. SEE PLAN VIEWS FOR HDU5-SDS2.5 HOLD DOWN LOCATIONS.
11. HOLD DOWN ANCHORS BOLT DIAMETER WILL BE PER MANUFACTURE REQUIREMENTS.

5 SCHEDULES

Scale: N.T.S.

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DRAWING ISSUE DATES:
10/26/23 Review Comments

REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

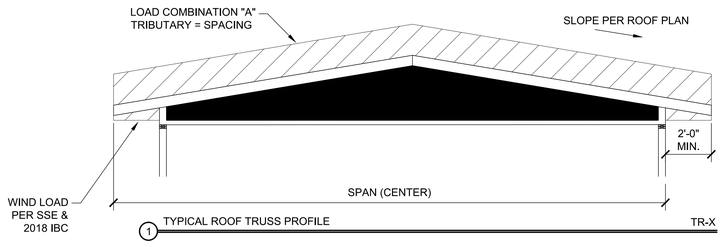
SHEET TITLE:
STRUCTURAL SCHEDULES

S0.3

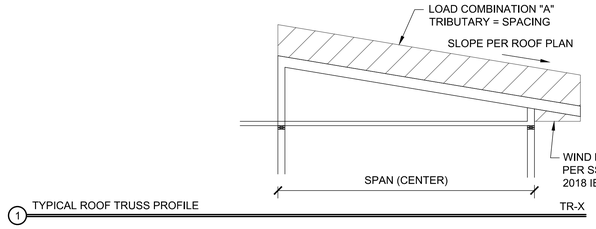
SHEET: 9 OF 140

ROOF / FLOOR TRUSS SCHEDULE		
TRUSS SPACING = 24" TYPICAL		
LOAD COMBINATION "A" (ROOF LOADS) TYPICAL		
MARK	SPAN	NOTES
TR1	7'-10 1/2"	
TF2	8'-4"	
TF3	8'-9"	
TF4	9'-9"	
TR5	10'-2 1/2"	
TR6	10'-9 1/2"	
TF7	13'-2"	
TR8	13'-7 1/2"	
TR9	13'-9 1/2"	
TF10	15'-0 1/2"	
TR11	16'-10 1/2"	
TR12	18'-7 1/2"	
TF13	18'-9"	
TR14	19-2 1/2"	
TR15	23'-2 1/2"	
TR16	27'-1 1/2"	
TR17	30'-0 1/2"	
TR18	32'-0 1/2"	
TR19	34'-7 1/2"	
TR20	35'-7 1/2"	

NOTES:
1. PARAPET AT EXTERIOR WALL ONLY SEE PLAN.
2. PARAPET AT BOTH ENDS SEE PLAN.
3. NO PARAPET AT ENDS.
4. SLOPED TOP CHORD SEE PLAN.
5. FLAT TOP CHORD.
6. ADJACENT TRUSS = SPAN +5" SEE PLAN



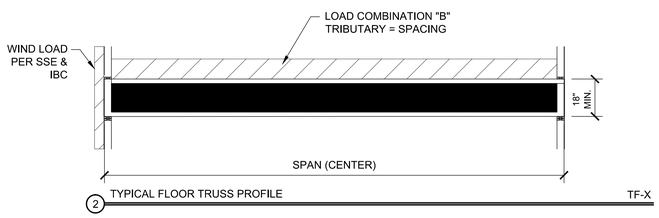
SPANS SHOWN ON ELEVATIONS AND SCHEDULES ARE FROM GRID TO GRID & CENTER OF STUD WALLS. MANUFACTURER TO ADJUST SPAN TO ACCOMMODATE RIM-BOARDS AND/OR WALL BLOCKING SEE DETAILS.



SEE ROOF FRAMING PLAN FOR MECHANICAL UNITS. COORDINATE WITH MECHANICAL SHEETS FOR UNIT SIZES AND WEIGHTS

LOAD COMBINATION "A"
Dead Load: 15 p.s.f.
Live Load: 110 p.s.f.
Total Load: 125 p.s.f.

10-26-23



LOAD COMBINATION "B"
Dead Load: 20 p.s.f.
Live Load: 40 p.s.f.
Total Load: 60 p.s.f.

LOAD COMBINATIONS

SPANS SHOWN ON ELEVATIONS AND SCHEDULES ARE FROM GRID TO GRID & CENTER OF STUD WALLS. MANUFACTURER TO ADJUST SPAN TO ACCOMMODATE RIM-BOARDS AND/OR WALL BLOCKING SEE DETAILS.

NOTE:
1. AT FIRST FLOOR THE TRUSS MANUFACTURER TO PROVIDE END WALL FLOOR TRUSSES BELOW SECOND FLOOR EXTERIOR WALLS PER DETAILS SHOWN ON S302.

PRE-FABRICATED METAL PLATED TRUSSES - ROOF & FLOOR

3

Scale: N.T.S.

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8/21/23

REVIEWED FOR CODE COMPLIANCE
01/12/2024

Holiday Inn Express

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DRAWING ISSUE DATES:
10/26/23 Review Comments

REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
STRUCTURAL SCHEDULES

S0.4

SHEET: 10 OF 140

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GENERAL NOTES

G1. FOOTING ELEVATIONS, IF SHOWN ON THE PLANS, ARE TO THE TOP OF THE FOOTING.

G2. REFER TO CIVIL DRAWINGS FOR FINISH GRADE ELEVATIONS UNLESS OTHERWISE NOTED.

G3. OVER-EXCAVATION OF SOIL REMOVED BELOW FOOTINGS SHALL BE REPLACED AND COMPACTED IN LAYERS TO 95% OF MODIFIED PROCTOR DENSITY. SEE DETAIL 13/S3.1

G4. INTERIOR CONCRETE SLABS ON GRADE, UNLESS OTHERWISE NOTED, SHALL BE REINFORCED WITH WELDED WIRE FABRIC MATS AS FOLLOWS:
4" SLAB - 6 x 6 W1.4 x W1.4
6" SLAB - 6 x 6 W2.1 x W2.1

G5. ELECTRIC CONDUIT AND OTHER PIPES EMBEDDED IN THE CONCRETE FLOOR SHALL BE PLACED IN ACCORDANCE WITH THE REQUIREMENTS OF ACI 318, PARAGRAPH 6.3.

G6. ANCHOR BOLTS ARE TO BE FURNISHED PER DETAILS AND AS SHOWN ON SHEET S0.3.

G7. LOCATE ALL SLEEVES, DRAINS, OPENINGS, EMBEDDED ITEMS, ETC. THAT ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ALL SUCH ITEMS ARE CORRECTLY POSITIONED & INSTALLED PRIOR TO PLACEMENT OF CONCRETE.

G8. G.C. SHALL COORDINATE ALL UNDER-SLAB PLUMBING AND ELECTRICAL CONDUIT LOCATIONS & INSTALLATION PER PLUMBING, MECHANICAL AND ELECTRICAL PLANS PRIOR TO POURING OF FLOOR SLAB.

G9. SEE SHEET S0.1 THRU S0.3 FOR ADDITIONAL NOTES AND LEGEND.

G10. DATUM FINISH FLOOR ELEVATION = F.F.= 0'-0"

KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

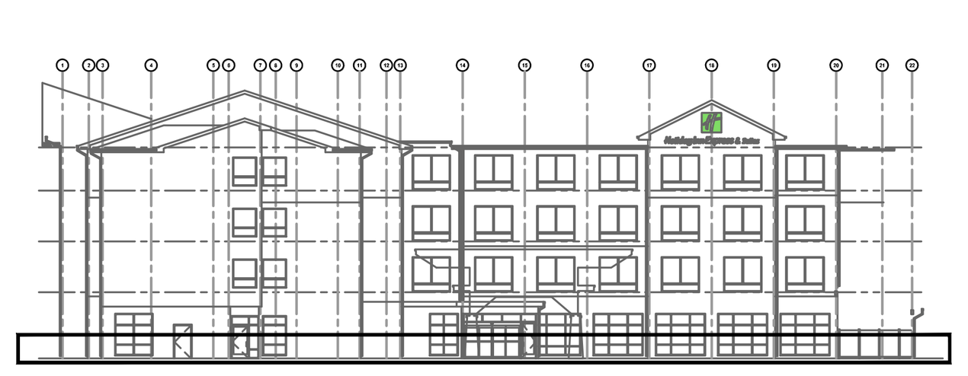
1. 4" CONCRETE SLAB WITH 4x4~10/10 WWF - F.F.=0'-0".
2. OUTLINE OF CONCRETE STEM WALL AND SPREAD FOOTING OR TURN-DOWN.
3. OUTLINE OF CONCRETE ISOLATED CONCRETE PIER AND FOOTING BELOW GRADE FOR COLUMN. SEE FOOTING SCHEDULE S0.3.
4. OUTLINE OF BOTTOM OF INTERIOR THICKENED SLAB.
5. 12" THICK CONCRETE ISOLATED FOOTING, AT LAUNDRY AREA. SEE LAUNDRY SUPPLIER DRAWINGS FOR DIMENSIONS & DETAIL 7/S3.1
6. ADA-COMPLIANT SHOWER, SEE ARCH. DRAWINGS FOR FLOOR ELEVATION.
7. CONTROL JOINT @ 12'-0" O.C. MAX. EACH WAY. TYPICAL FOR ALL SLAB ON GRADE.
8. VERIFY FINAL ELEVATOR PIT DIMENSIONS WITH SUPPLIER DRAWINGS PRIOR TO CONSTRUCTION.
9. SUMP PUMP DETAIL 8/S3.1. LOCATE / COORDINATE SUMP PLACEMENT WITH EQUIPMENT AND WITH CONCRETE CONTRACTORS.
10. TOP OF FOOTING AT INTERIOR SPOT FOOTINGS TO BE -6" BELOW FF, TYP.
11. TOP OF FOOTING AT PERIMETER STEM WALLS TO BE -4'-0" BELOW FF, TYP.
12. RETAINING STEM WALL. SEE DETAIL 4A/S3.1.
13. SITE RETAINING WALL. SEE DETAIL 6/S3.1.
14. 4" CONCRETE EQUIPMENT PAD ON TOP OF SLAB.
15. SIMPSON STRONG-WALL SB SWSB24X14 SEE SHEET S3.4 FOR DETAILS.

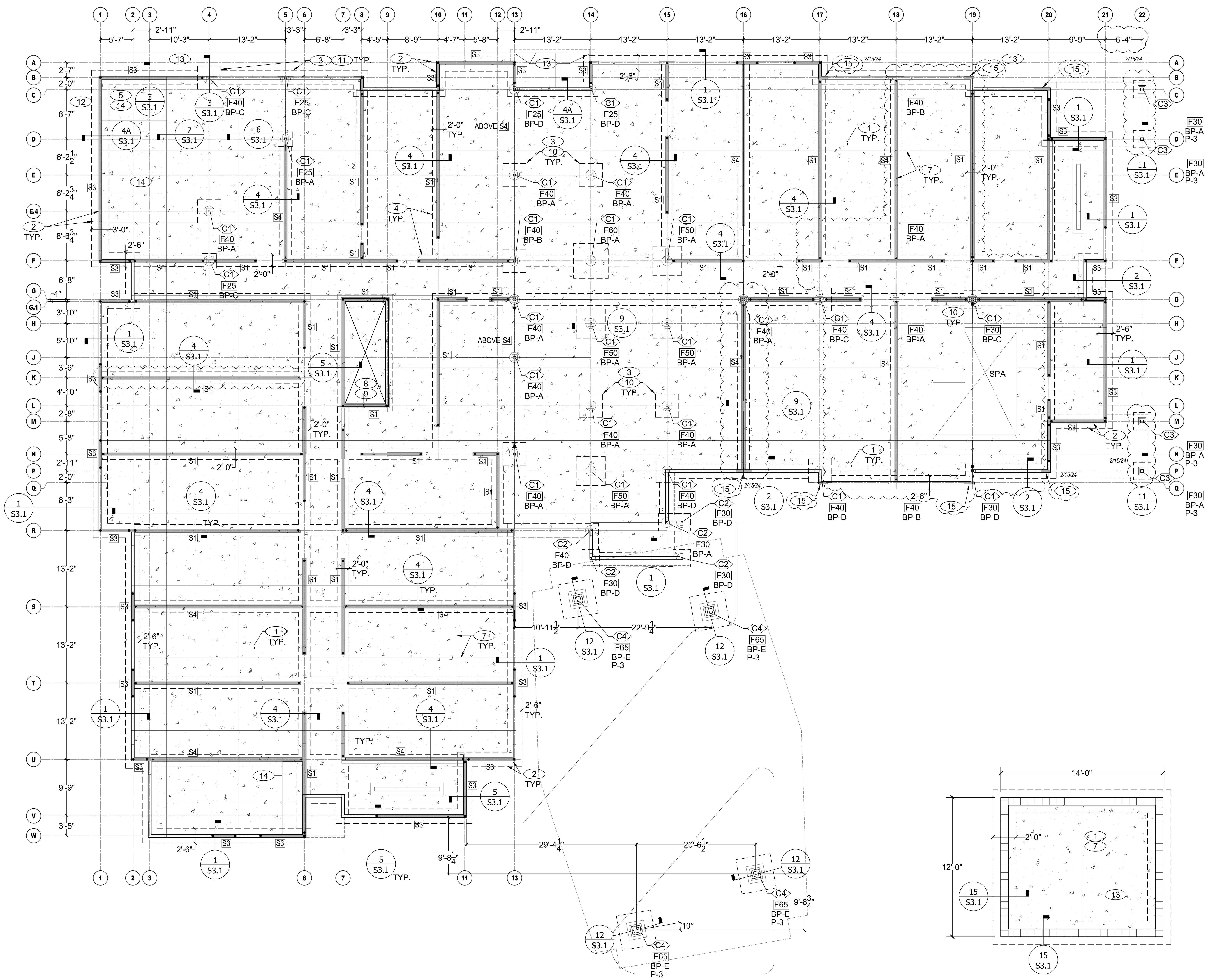
LEGEND

- EDGE OF FOOTING
- COLUMN AND BASE PLATE, SEE S3.1
- PIER AND ISOLATED SPOT FOOTING BELOW FLOOR SLAB, SEE S3.1 & 10/S5.3
- CONCRETE SLAB ON GRADE
- CJ — CONTROL JOINT
- C1 — DENOTES COLUMN TYPE, PER SCHEDULE ON S0.3
- BP-* — DENOTES BASE PLATE TYPE, SEE 10/S5.3
- ANCHOR BOLT AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3 & DETAIL 4A/S5.3
- HOLD DOWN ANCHOR AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3 & DETAIL 4B/S5.3
- S# — SHEAR WALL SEE SCHEDULE SHEET S0.3

0 2 4 8 16 24
1/8" = 1'-0"

BUILDING LEVEL KEY





1 FIRST FLOOR FOUNDATION PLAN

Scale: 1/8" = 1'-0"

2 STORAGE BLDG FOUNDATION PLAN

Scale: 1/4" = 1'-0"

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DRAWING ISSUE DATES:

REVISION DATES:
2/15/24 Pool Removal, Guest Room Addition

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
FIRST FLOOR FOUNDATION PLAN

S1.1

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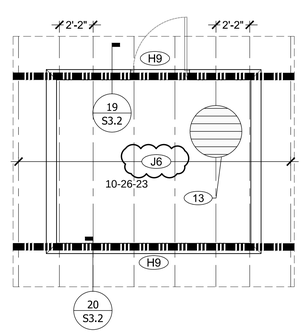
1 SECOND FLOOR FRAMING PLAN

Scale: 1/8" = 1'-0"



2 STORAGE BLDG FRAMING PLAN

Scale: 1/4" = 1'-0"

GENERAL NOTES

G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.

G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FRAMING.

G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

G4. FOR JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.

G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.

G6. SEE HEADER SCHEDULE S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.

G7. SEE S5 SHEETS FOR TYPICAL WOOD, AND STEEL FRAMING DETAILS.

KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

1. 3/4" A.P.A RATED FLOOR SHEATHING PANELS (EXPOSURE 1, SPAN RATING 0/24). NAIL @ ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8D @ 6" O.C. AND ALL INTERMEDIATE FRAMING @ 12" O.C. SEE 1, 2/S5.2 SEE DETAILS AND ARCHITECTURAL DRAWINGS FOR GYPCRETE TOPPING THICKNESS.
2. 2X WOOD STUDS. SEE S0.3 FOR SHEAR WALL SCHEDULE FOR SIZE AND SPACING. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
3. STAIRS, SEE S5.3 AND ARCHITECTURAL DRAWINGS.
4. ELEVATOR SHAFT, SEE FOUNDATION PLAN AND DETAILS.
5. PRE-MANUFACTURED JOISTS, SEE S0.3 AND DETAILS FOR PROFILE AND LOADS.
6. ELEVATOR FRAMING, SEE 14,15/S3.2
7. TRUSS MANUFACTURER "SSE" TO INCLUDE DEAD LOADS OF MECHANICAL UNITS IN THE DESIGN OF THE ROOF TRUSSES BELOW THE UNITS AND PANELS SHOWN ON PLAN.
8. 23/32" A.P.A. RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8d @ 6" O.C. AND ALL INTERMEDIATE FRAMING AT 12" O.C. SEE 1,2/S5.2
9. 2X BLOCKING BETWEEN 2X FLOOR JOIST.
10. OPENING IN FLOOR FOR CHUTE, SEE DETAIL 7/S3.3
11. 2X BLOCKING AT 4'-0" O.C. SEE DETAIL.2/S3.2
12. CONTINUOUS RIM TRUSS, SEE DETAIL 1,2/S3.2
13. 2"x6" T&G ROOF DECKING OVER ROOF JOISTS.
14. AT INTERIOR WALLS OF ELEV./STAIRWELLS SEE DETAIL 8/S3.2 TYPICAL.
15. TRUSS MANUFACTURER TO PROVIDED DUCT CHASES THRU ROOF TRUSSES, COORDINATE LOCATIONS WITH MECHANICAL DRAWINGS.
16. HEADER BEAM IS CONTINUOUS FROM GRID LINES "U" TO "W".
17. SIMPSON STRONG-WALL SB SWSB24X14 SEE SHEET S3.4 FOR DETAILS.

LEGEND

- JOIST CENTER LINE
- BEAMS PER HEADER AND BEAM SCHEDULES, SEE S0.3
- GRID LINE
- JOIST HANGER (LONG LEG DENOTES CONTINUOUS BEAM) SEE DETAIL 12/S5.3
- J, B, H — DENOTES JOIST, BEAM, HEADERS PER SCHEDULE S0.3
- S# — DENOTES SHEAR WALL LOCATION/TYPE, SEE S0.3 & DETAIL 4A&B/S5.3
- CONTINUOUS RIM JOIST, SEE KEYNOTE 12
- MOMENT CONNECTION DETAIL SEE SHEET 10/S3.2 & 9/S3.2

0 2 4 8 16 24

1/8" = 1'-0"

BUILDING LEVEL KEY

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DRAWING ISSUE DATES:

REVISION DATES:
10/26/23 Review Comments
2/15/24 Pool Removal, Guest Room Addition
2/21/24 Office, Network Revisions

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
SECOND FLOOR FRAMING PLAN

S1.2

SHEET: 12 OF 142

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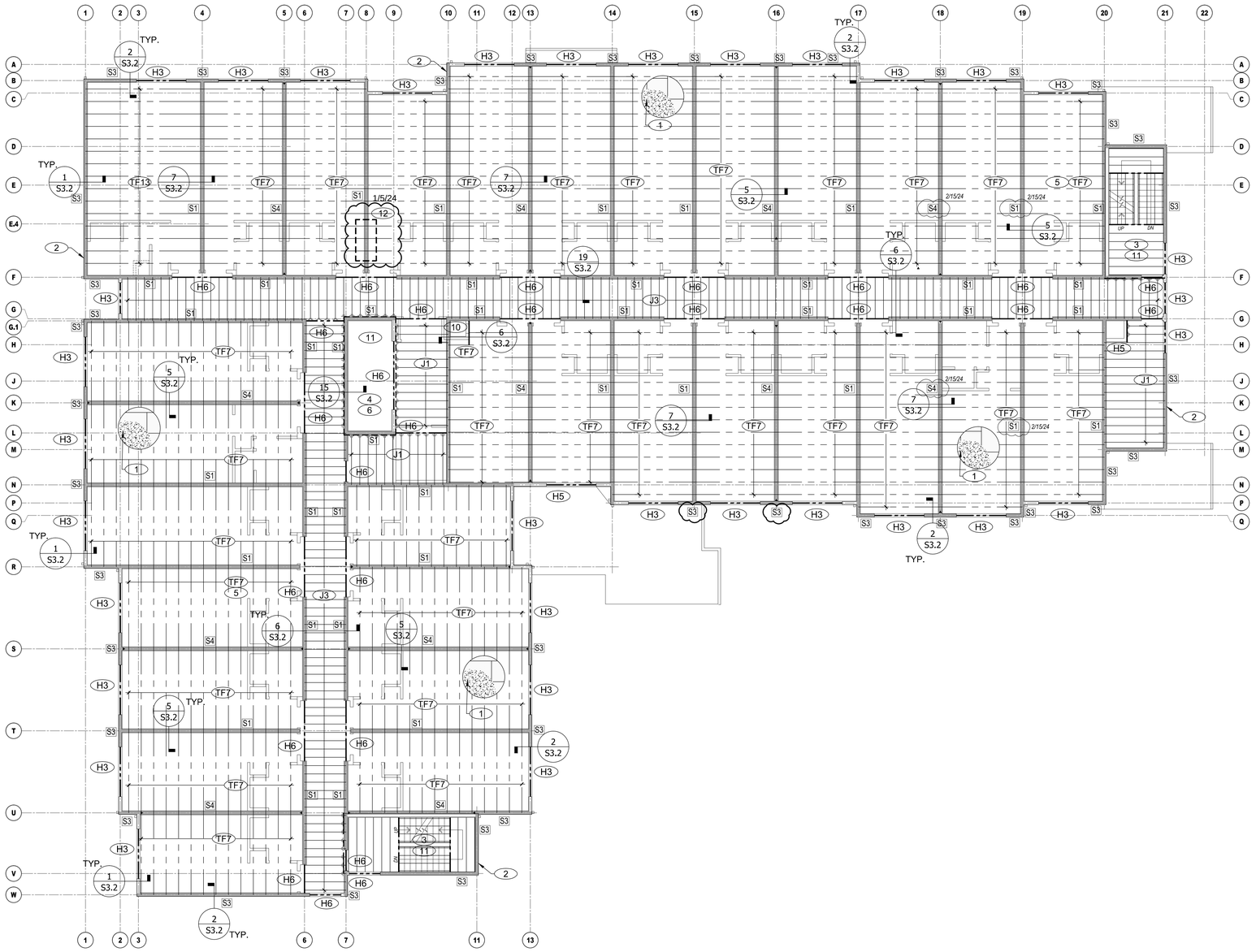

1 THIRD & FOURTH FLOOR FRAMING PLAN

Scale: 1/8"=1'-0"

GENERAL NOTES

G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.

G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FRAMING.

G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

G4. FOR JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.

G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.

G6. SEE HEADER SCHEDULE S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.

G7. SEE S5 SHEETS FOR TYPICAL TRUSS, WOOD, AND STEEL FRAMING DETAILS.

KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

1. 3/4" A.P.A RATED FLOOR SHEATHING PANELS (EXPOSURE 1, SPAN RATING 0/24). NAIL @ ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8D @ 6" O.C. AND ALL INTERMEDIATE FRAMING @ 12" O.C. SEE 1, 2/S5.2 SEE DETAILS AND ARCHITECTURAL DRAWINGS FOR GYPCRETE TOPPING THICKNESS.
2. 2X WOOD STUDS. SEE S0.3 FOR SHEAR WALL SCHEDULE FOR SIZE AND SPACING. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
3. STAIRS, SEE S5.3 AND ARCHITECTURAL DRAWINGS.
4. ELEVATOR SHAFT, SEE FOUNDATION PLAN AND DETAILS.
5. PRE-MANUFACTURED TRUSSES, SEE S0.3 AND DETAILS FOR PROFILE AND LOADS.
6. ELEVATOR FRAMING, SEE 5,6/S3.3
7. TRUSS MANUFACTURER "SSE" TO INCLUDE DEAD LOADS OF MECHANICAL UNITS IN THE DESIGN OF THE ROOF TRUSSES BELOW THE UNITS SHOWN.
8. 23/32" A.P.A. RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8d @ 6" O.C. AND ALL INTERMEDIATE FRAMING AT 12" O.C. SEE 1,2/S5.2
9. 2X BLOCKING BETWEEN FLOOR TRUSSES.
10. OPENING IN FLOOR FOR CHUTE, WITH DOUBLE FLOOR JOIST FRAMING AROUND OPENING, SEE DETAIL 7/S3.3
11. AT INTERIOR WALLS OF ELEV./STAIRWELLS SEE DETAIL 8/S3.2 TYPICAL.

1/5/24 12. SEE ARCHITECTURAL THIRD FLOOR PLAN FOR CONNECTING DOOR LOCATION. HEADER OVER DOOR SHALL BE 'H3' TYPE.

LEGEND

- JOIST CENTER LINE
- BEAMS PER HEADER AND BEAM SCHEDULES, SEE S0.3
- GRID LINE
- JOIST HANGER (LONG LEG DENOTES CONTINUOUS BEAM) SEE DETAIL 12/S5.3
- J, B, H — DENOTES JOIST, BEAM, HEADERS PER SCHEDULE S0.3
- S# — DENOTES SHEAR WALL LOCATION/TYPE, SEE S0.3 & DETAIL 4A&B/S5.3
- BLOCKING @ EXTERIOR WALL PARALLEL TO FLOOR FRAMING SEE KEYNOTE 11.
- CONTINUOUS RIM JOIST, SEE KEYNOTE 12
- HOLD DOWN ANCHOR AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3

0 2 4 8 16 24

1/8" = 1'-0"

BUILDING LEVEL KEY

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8/21/23

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DRAWING ISSUE DATES:

REVISION DATES:
1/5/24 Guest Room Header Revision
2/15/24 Pool Removal, Guest Room Addition

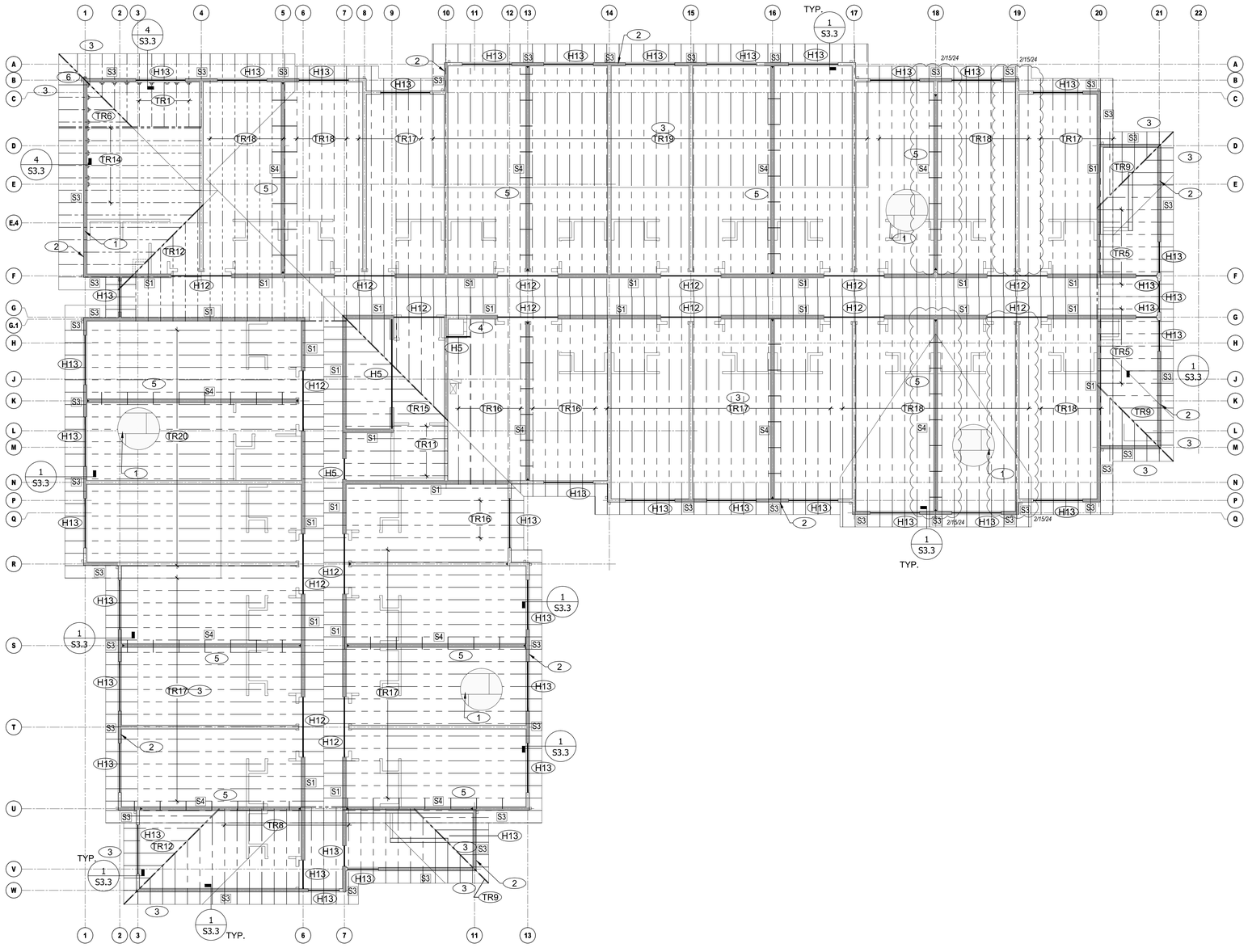
PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
THIRD & FOURTH FLOOR FRAMING PLAN

S1.3

SHEET: 13 OF 142



1 ROOF FRAMING PLAN

Scale: 1/8" = 1'-0"

GENERAL NOTES

G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.

G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FORMING.

G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

G4. FOR TRUSS/JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.

G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.

G6. ROOF TRUSS BRACING PER MANUFACTURER REQUIREMENTS. AND "BCSI TRUSS INSTALLATION & BRACING." B5-B9

G7. SEE HEADER SCHEDULE ON S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.

G8. SEE S5 SHEETS FOR TYPICAL TJI, WOOD, AND STEEL FRAMING DETAILS.

KEYED NOTES

NOT ALL KEYED NOTES MAY BE USED

1. 23/32" A.P.A RATED ROOF SHEATHING PANELS (EXPOSURE 1, SPAN RATING 24/0). NAIL AT ALL FRAMED PANEL EDGES AND OVER ALL JOISTS SHOWN ON PLAN WITH 8D @ 6" O.C. AND ALL INTERMEDIATE FRAMING @ 12" O.C. SEE 1, 2/S5.2
2. 2x_ WOOD STUDS, SEE S0.3 FOR SHEAR WALL SCHEDULE. WALL SHEATHING SHALL BE 1/2" PANELS (EXPOSURE 1) U.N.O, SEE 3/S5.2 FOR ATTACHMENT.
3. PRE-MANUFACTURED WOOD ROOF TRUSSES @ 24" O.C. SEE S0.3 FOR TRUSS PROFILE AND LOADING.
4. ROOF CHUTE OPENING WITH DOUBLE 2X12 JOIST FRAMING AROUND OPENING DETAIL 7/S3.3.
5. 2X4 BLOCKING AT SHEAR WALL SEE DETAIL 3/S5.1
6. SIMPSON TBE CONNECTOR AT RIDGE TRUSS BEARING EACH SIDE OF TRUSS.

LEGEND

- JOIST CENTER LINE
- BEAMS PER HEADER AND BEAM SCHEDULES, SEE S0.3
- GRID LINE
- JOIST HANGER (LONG LEG DENOTES CONTINUOUS BEAM) SEE DETAIL 12/S5.3
- J, B, H — DENOTES JOIST, BEAM, HEADERS PER SCHEDULE S0.3
- S# — DENOTES SHEAR WALL LOCATION/TYPE, SEE S0.3 & DETAIL 4A&B/S5.3
- BLOCKING @ EXTERIOR WALL PARALLEL TO FLOOR FRAMING SEE KEYNOTE 11.
- CONTINUOUS RIM JOIST, SEE KEYNOTE 12
- HOLD DOWN ANCHOR AT END OF SHEAR WALL SEE SCHEDULE SHEET S0.3

0 2 4 8 16 24

1/8" = 1'-0"

BUILDING LEVEL KEY

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STEAMBOAT SPRINGS, CO 80487

DRAWING ISSUE DATES:
2/15/24 Pool Removal, Guest Room Addition

REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
ROOF
FRAMING PLAN

S1.4

SHEET: 14 OF 142

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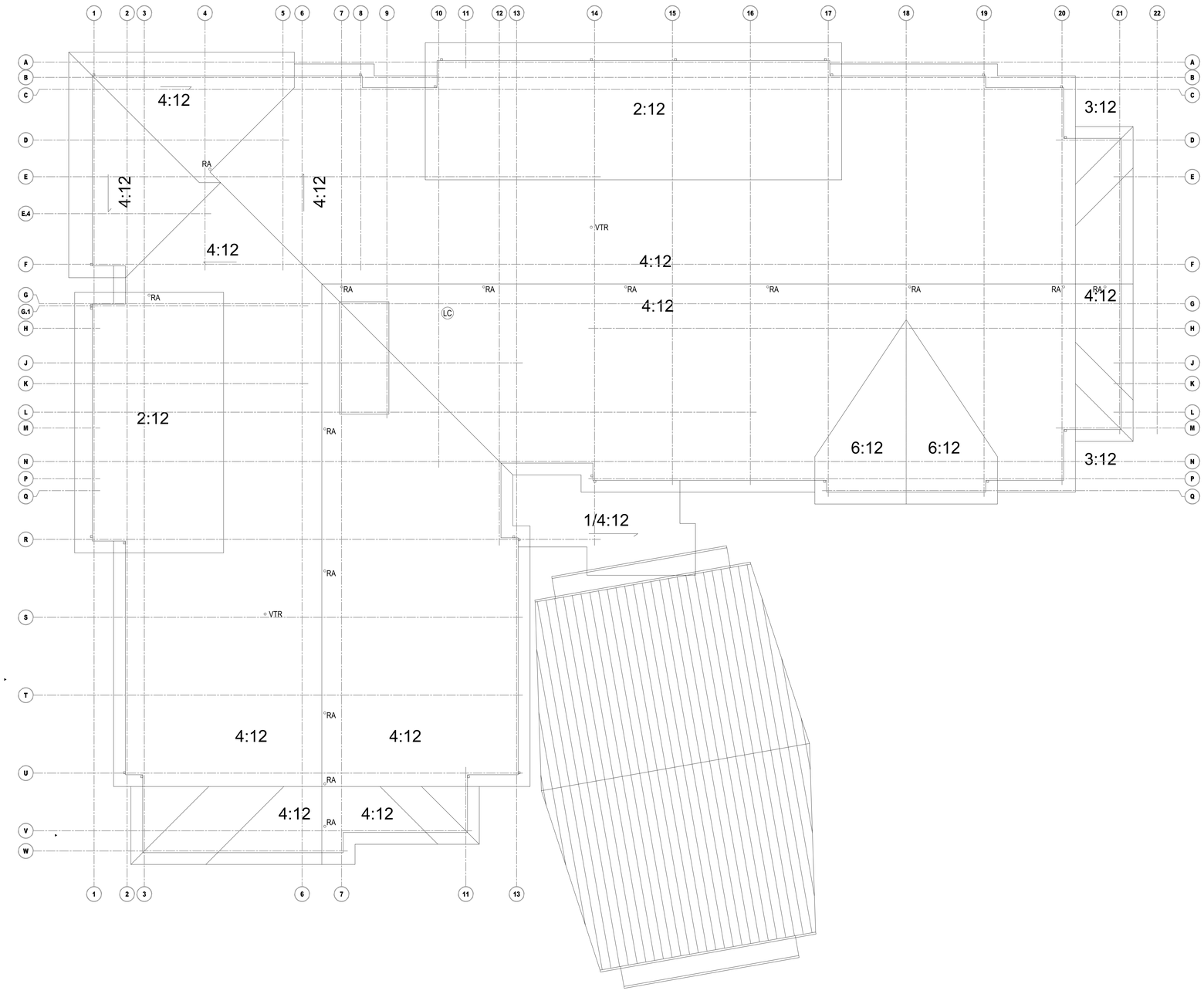

1 ROOF PLAN

Scale: 1/8" = 1'-0"

GENERAL NOTES

G1. FOR GENERAL STRUCTURAL NOTES, SEE SHEET S0.1.

G2. CONTRACTOR TO COORDINATE ALL BUILDING DIMENSIONS WITH ARCHITECTURAL DRAWINGS PRIOR TO FORMING.

G3. CONTRACTOR TO COORDINATE ALL ROOF/FLOOR PENETRATIONS WITH MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS PRIOR TO CONSTRUCTION.

G4. FOR TRUSS/JOIST BEARING HEIGHTS SEE ARCHITECTURAL DRAWINGS.

G5. SEE S0.3 FOR BEAM, HEADER AND SHEAR WALL SCHEDULES.

G6. ROOF TRUSS BRACING PER MANUFACTURER REQUIREMENTS. AND "BCSI TRUSS INSTALLATION & BRACING." B5-B9

G7. SEE HEADER SCHEDULE ON S0.3 FOR BUILT-UP STUDS @ HEADER ENDS.

G8. SEE S5 SHEETS FOR TYPICAL TJI, WOOD, AND STEEL FRAMING DETAILS.

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8/21/23

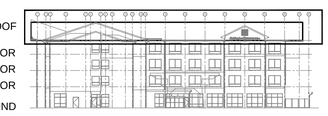
REVIEWED FOR CODE COMPLIANCE
01/12/2024

Holiday Inn Express

3400 S. LINCOLN AVE
STEAMBOAT SPRINGS, CO 80487

0 2 4 8 16 24
1/8" = 1'-0"

BUILDING LEVEL KEY



DRAWING ISSUE DATES:

REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
ROOF PLAN

S1.5

SHEET: 15 OF 140

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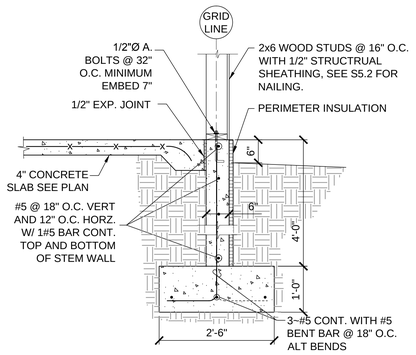
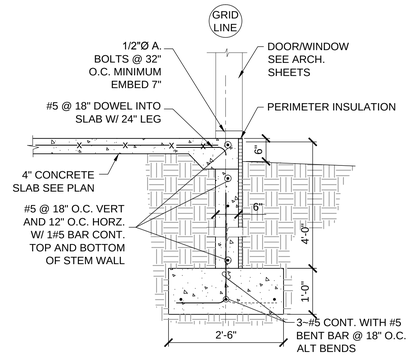


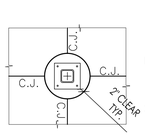
1 FOUNDATION DETAIL

Scale: 3/4"=1'-0"

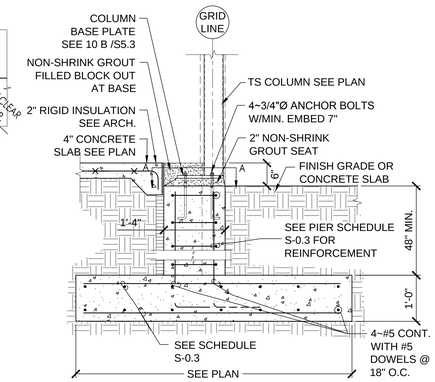


2 FOUNDATION DETAIL

Scale: 3/4"=1'-0"

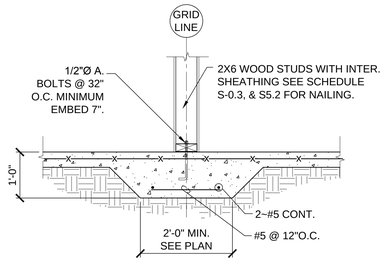


SECTION A-A



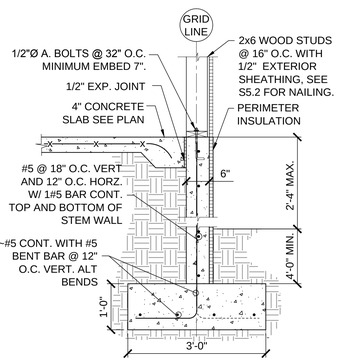
3 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



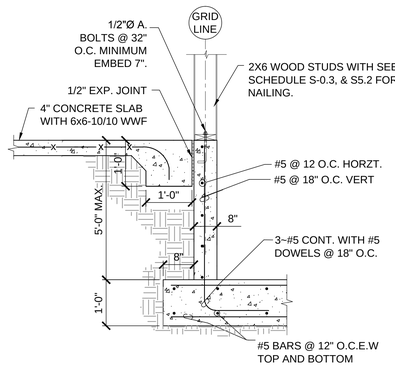
4 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



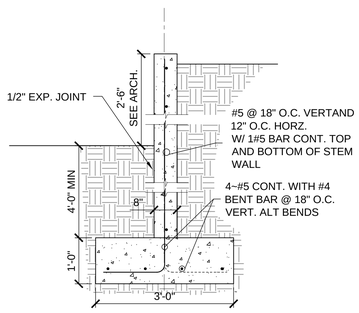
4A FOUNDATION DETAIL

Scale: 3/4"=1'-0"



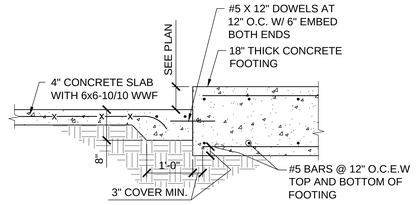
5 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



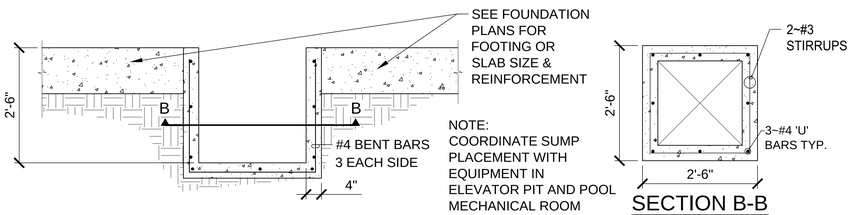
6 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



7 FOUNDATION DETAIL

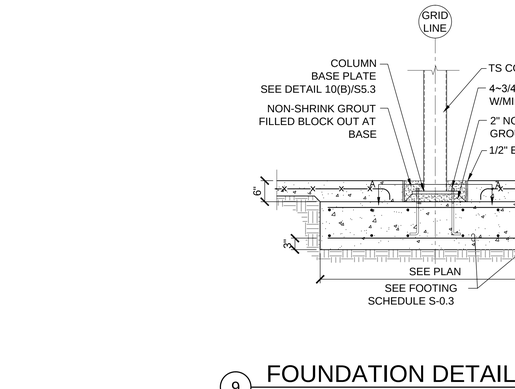
Scale: 3/4"=1'-0"



SECTION B-B

8 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



9 FOUNDATION DETAIL

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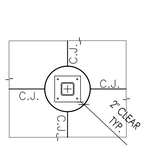
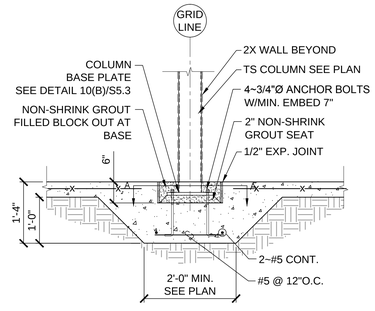
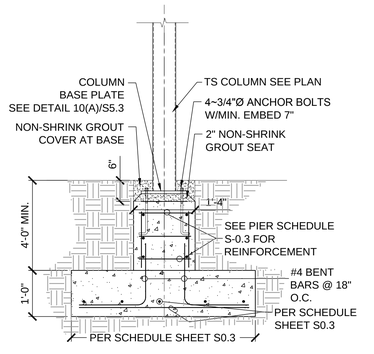


SECTION A-A



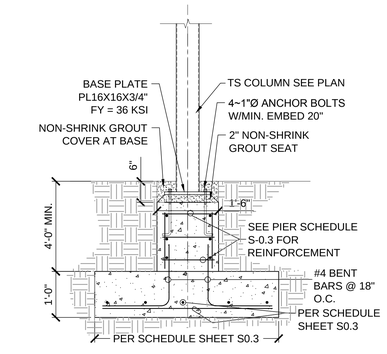
10 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



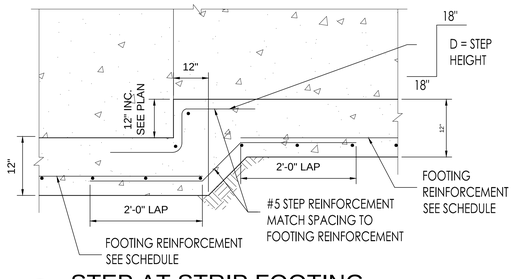
11 FOUNDATION DETAIL

Scale: 3/4"=1'-0"



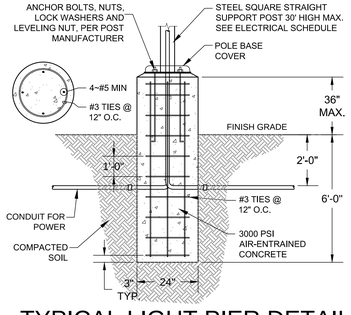
12 FOUNDATION DETAIL

Scale: N.T.S.



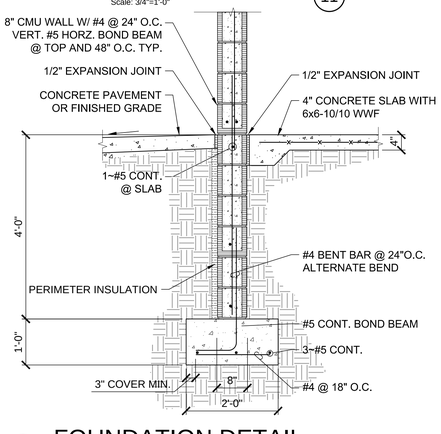
13 STEP AT STRIP FOOTING

Scale: 3/4"=1'-0"



14 TYPICAL LIGHT PIER DETAIL

Scale: N.T.S.



15 FOUNDATION DETAIL

Scale: N.T.S.

General Notes

REBAR COVER: UNLESS OTHERWISE NOTED OR DETAILED, THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT

EXPOSURE
MINIMUM COVER (IN.)
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO GROUND: 3"

B. CONCRETE EXPOSED TO OPEN AIR OR WEATHER BUT:
#6 - #18 BARS 2"
#5 - AND SMALLER 1 1/2"

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01/12/2024

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DRAWING ISSUE DATES:

REVISION DATES:

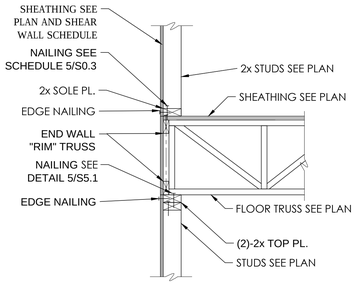
PROJECT MANAGER: NICK PIRKL

DRAWN BY: NAP

SHEET TITLE: FOUNDATION DETAILS

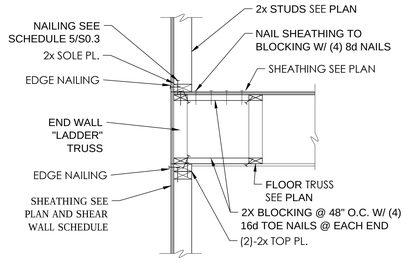
S3.1

SHEET: 16 OF 140



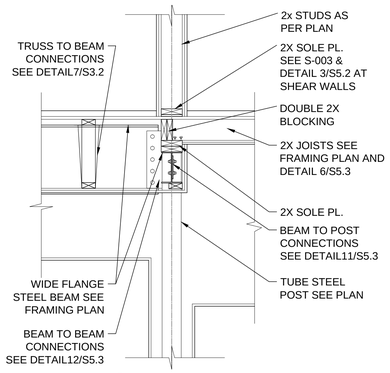
1 FRAMING DETAIL

Scale: 3/4"=1'-0"



2 FRAMING DETAIL

Scale: 3/4"=1'-0"



3 FRAMING DETAIL

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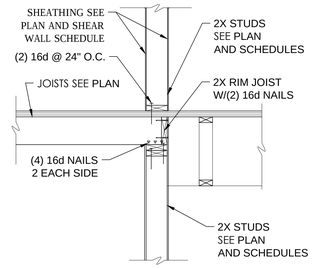
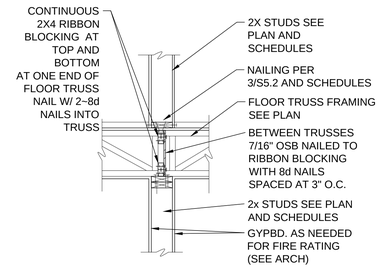


4 FRAMING DETAIL

Scale: 3/4"=1'-0"



5 FRAMING DETAIL

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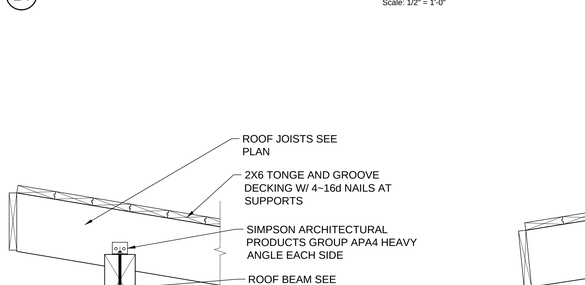
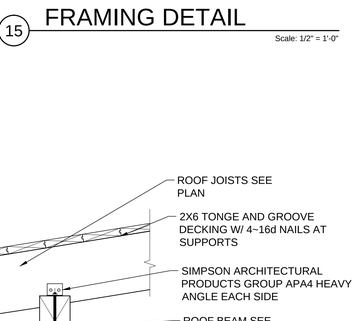


6 FRAMING DETAIL

Scale: 3/4"=1'-0"



7 FRAMING DETAIL

Scale: 3/4"=1'-0"

8 FRAMING DETAIL

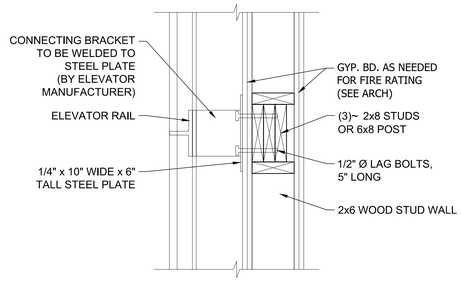
Scale: 3/4"=1'-0"

9 FRAMING DETAIL

Scale: 3/4"=1'-0"

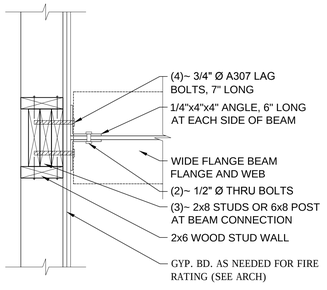
10 FRAMING SECTION

Scale: 1"=1'-0"



14 FRAMING DETAIL

Scale: 1/2" = 1'-0"



15 FRAMING DETAIL

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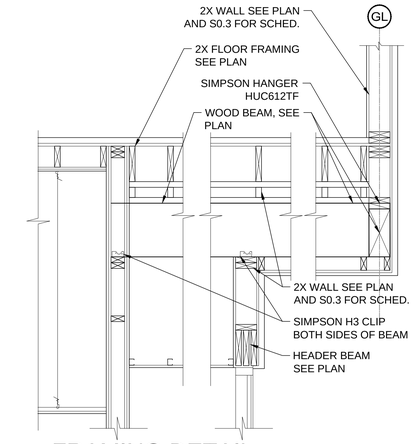
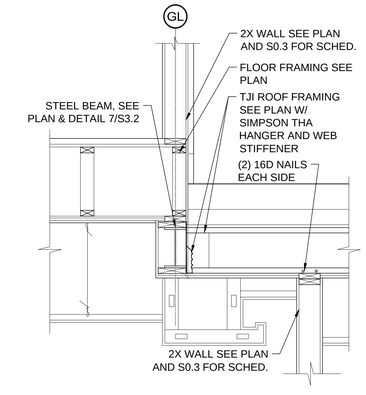


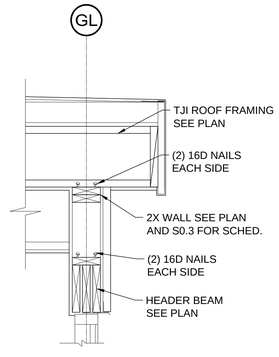
16 FRAMING DETAIL

Scale: 1/2" = 1'-0"



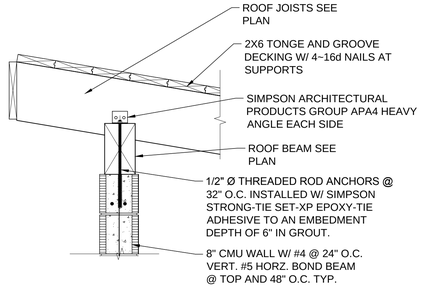
17 FRAMING DETAIL

Scale: 1/2" = 1'-0"



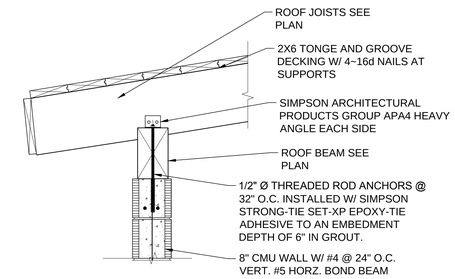
18 FRAMING DETAIL

Scale: 1/2" = 1'-0"



19 FRAMING DETAIL

Scale: 1/2" = 1'-0"



20 FRAMING DETAIL

Scale: 1/2" = 1'-0"

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PROFESSIONAL ENGINEER
8/21/23

REVIEWED FOR CODE COMPLIANCE
01/12/2024

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DRAWING ISSUE DATES:

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PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
SECTIONS AND DETAILS

S3.2

SHEET: 17 OF 140

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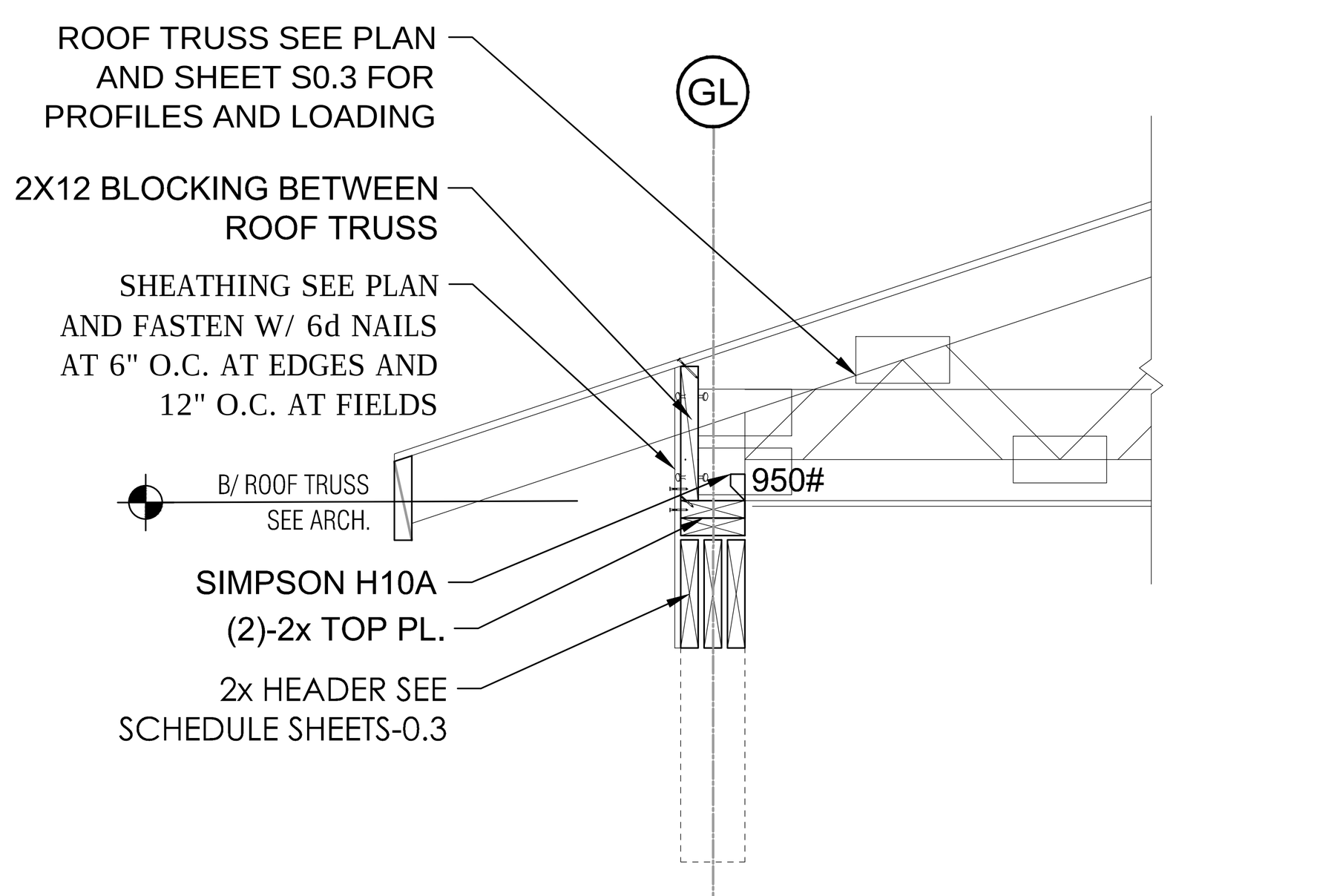
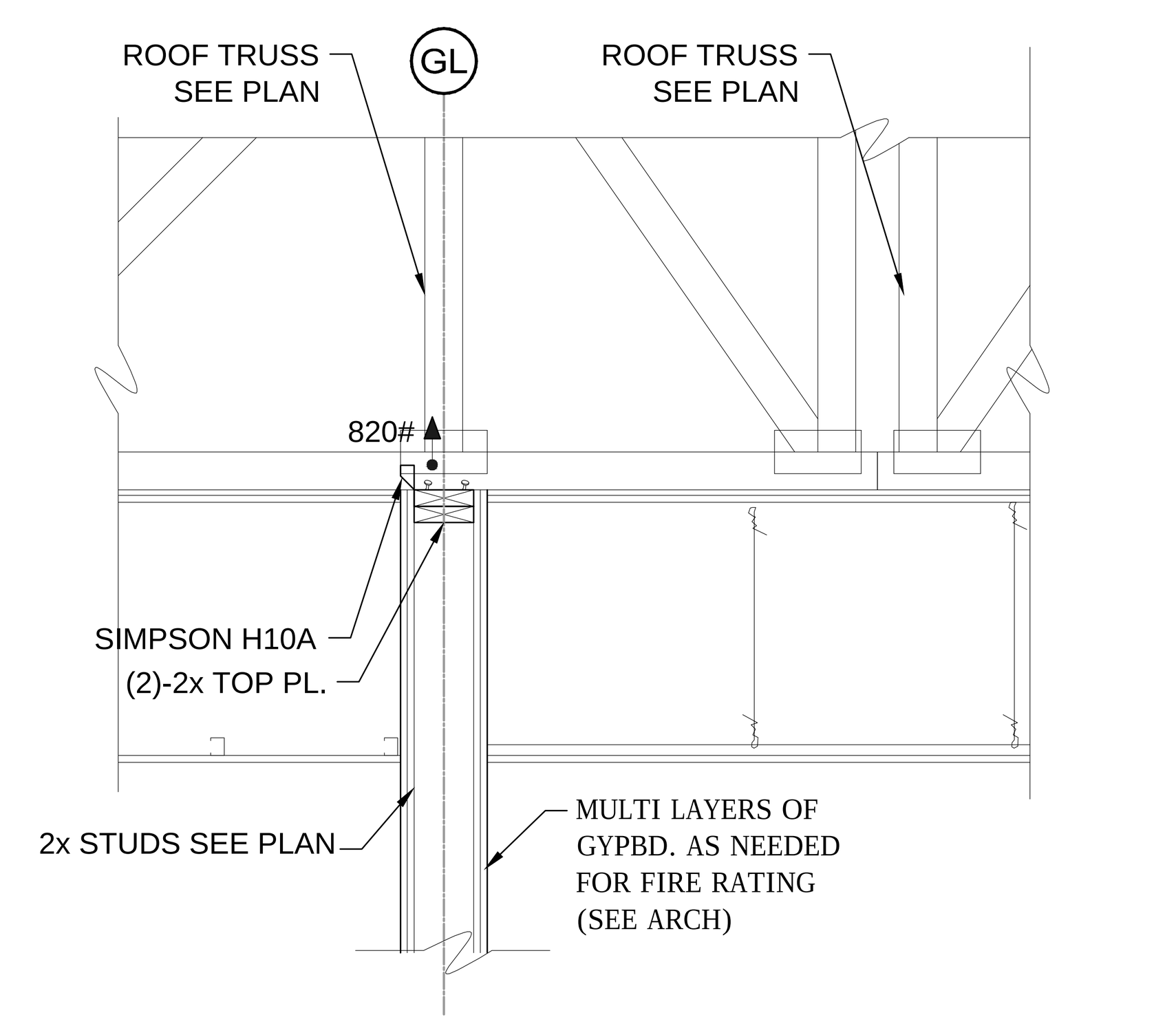


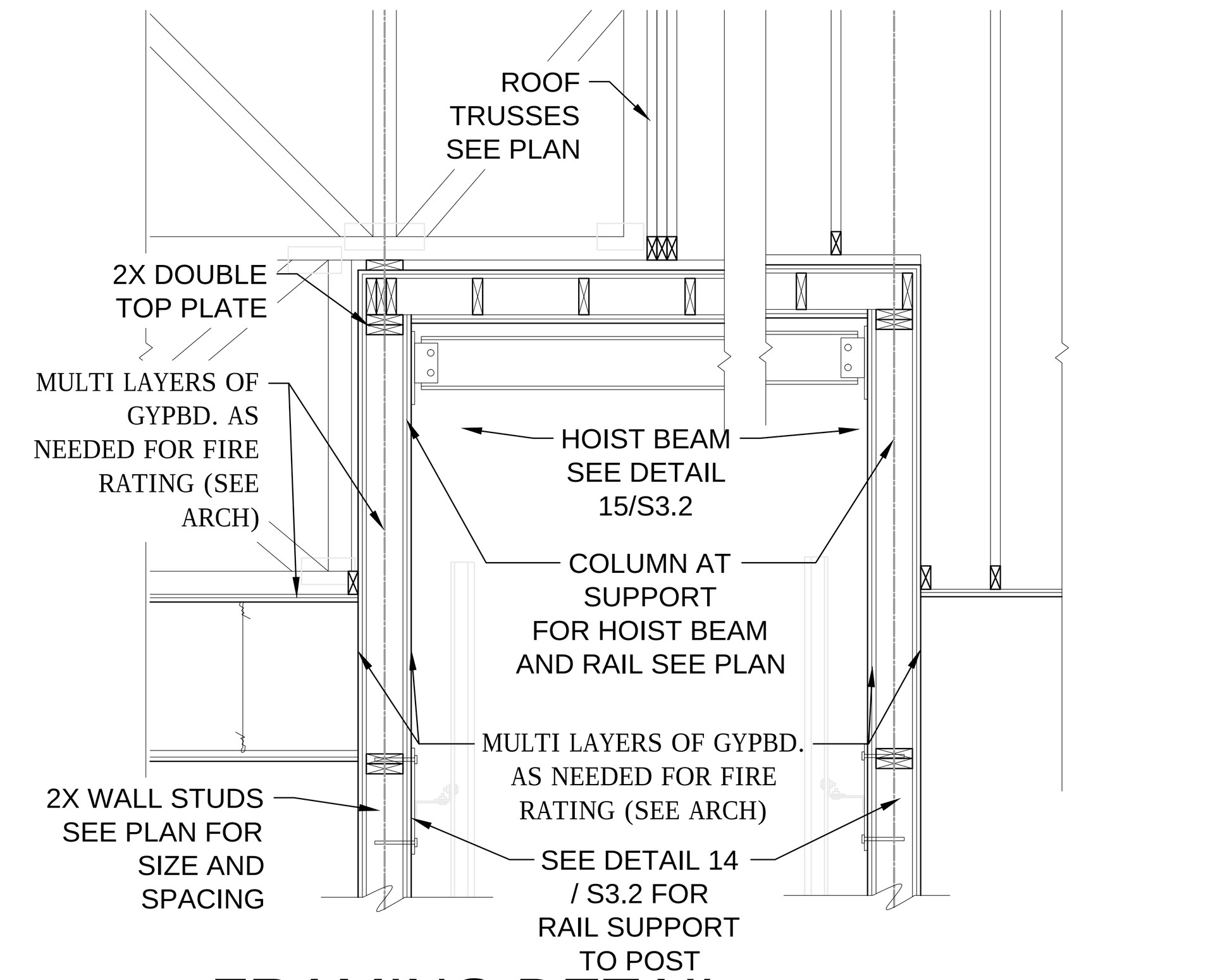
1 FRAMING DETAIL

Scale: 3/4" = 1'-0"



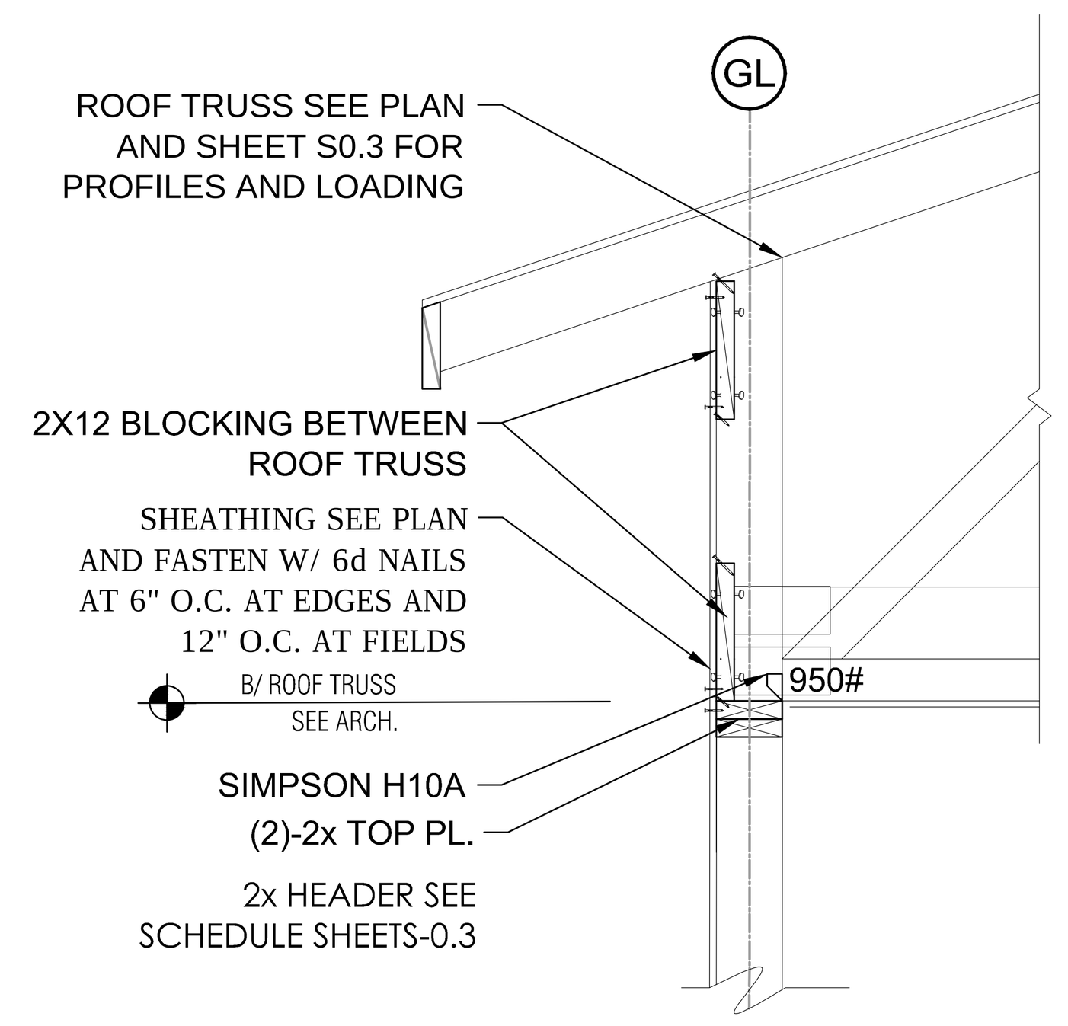
2 FRAMING DETAIL

Scale: 3/4" = 1'-0"



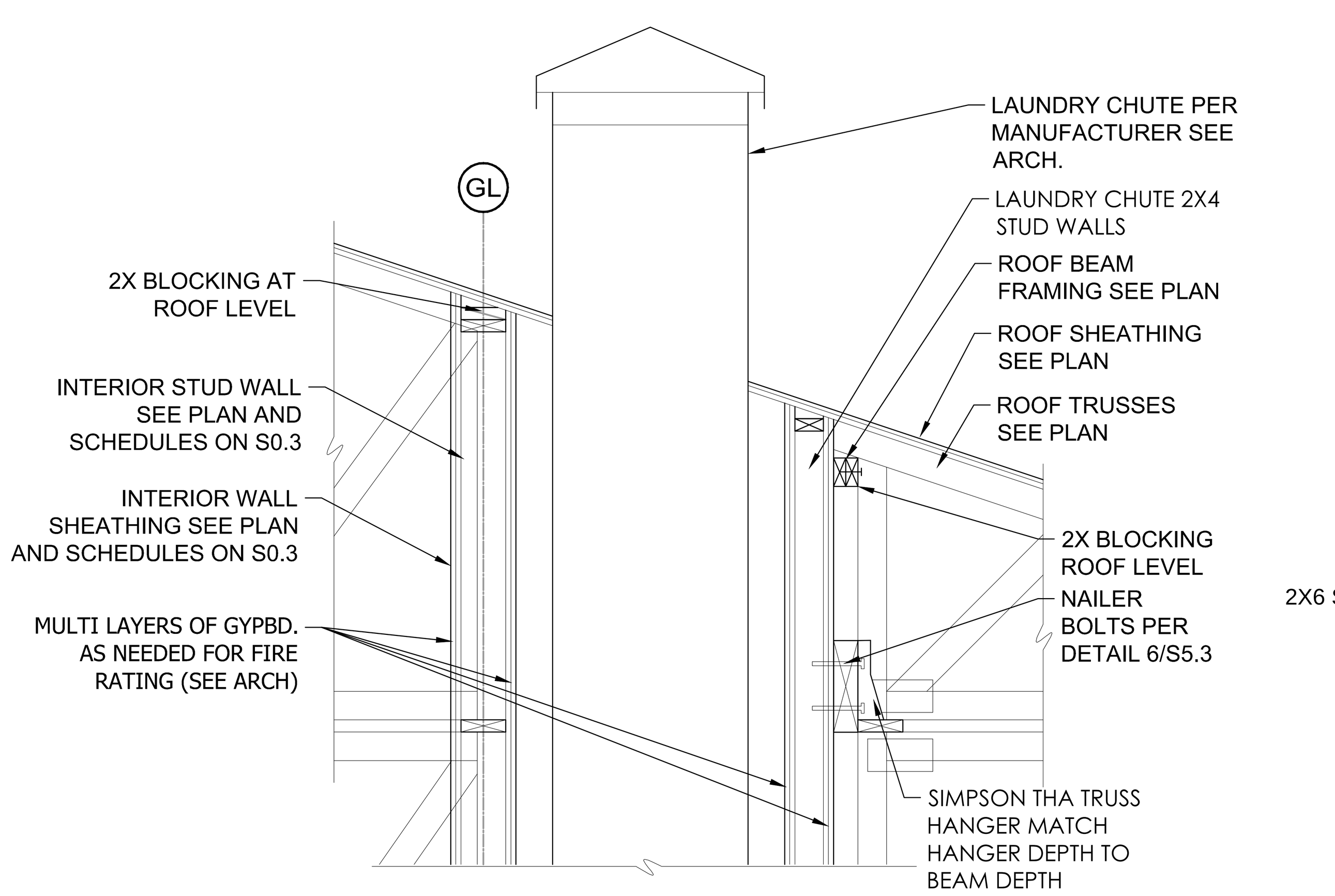
3 FRAMING DETAIL

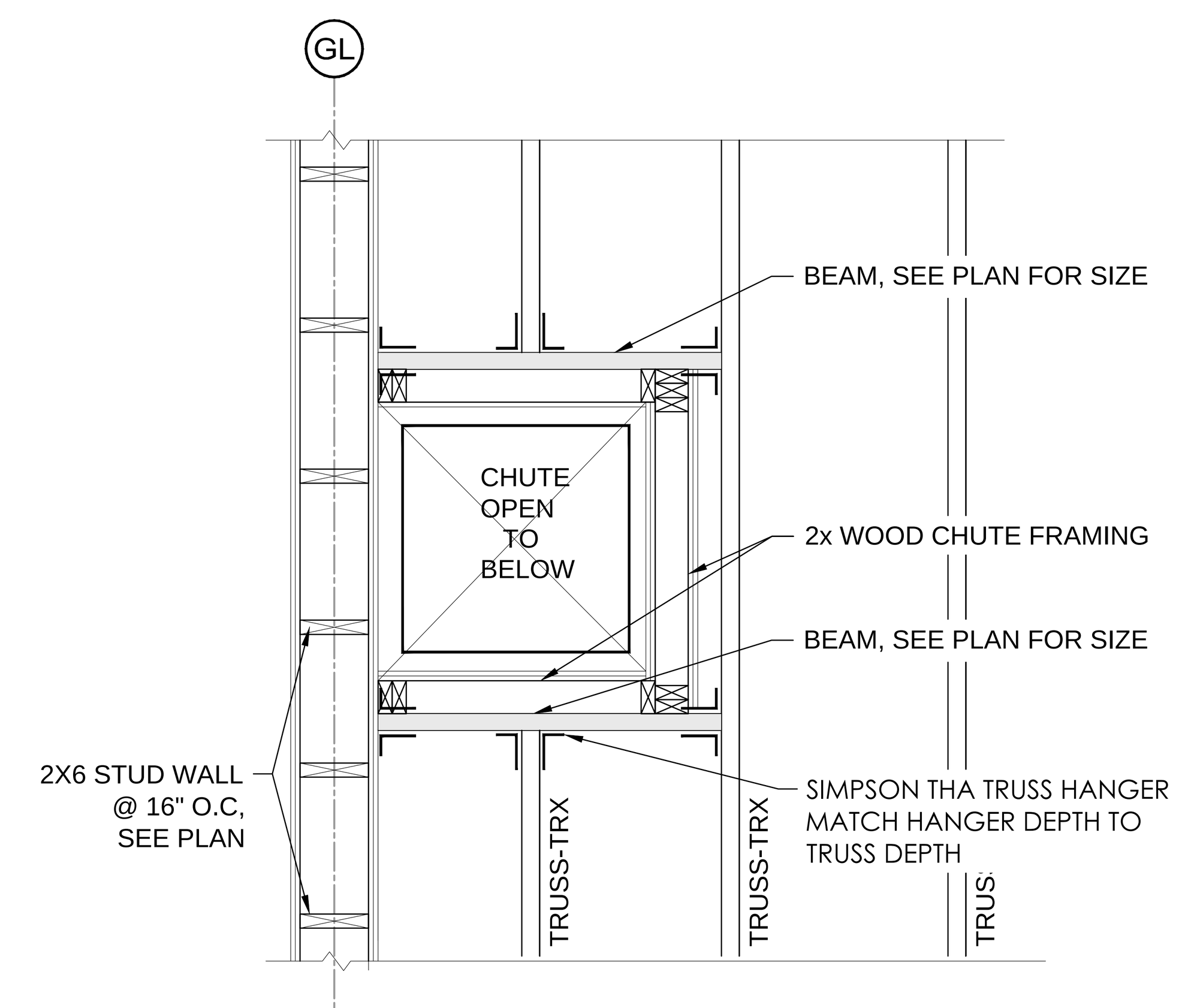
Scale: 1/2" = 1'-0"



4 FRAMING DETAIL

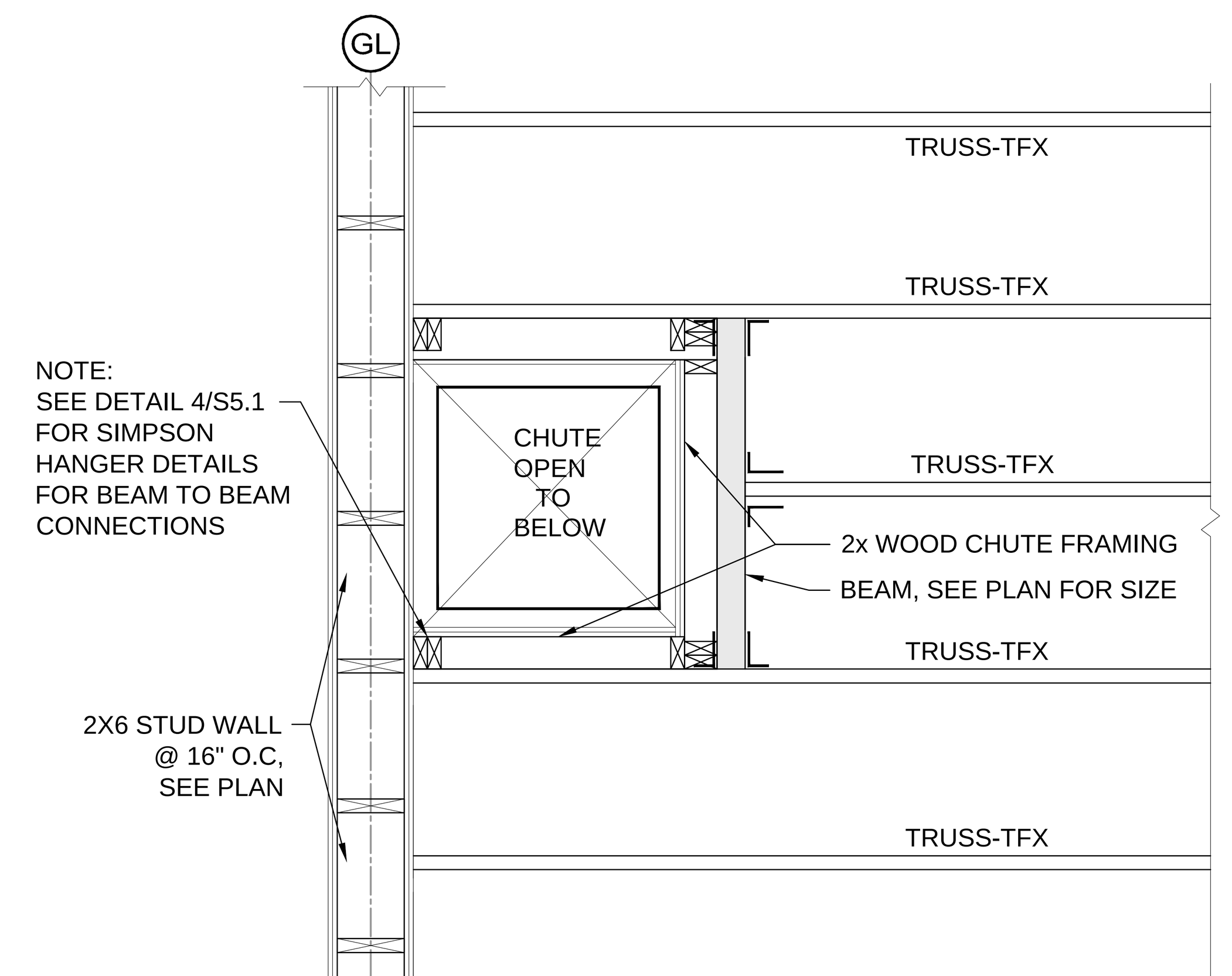
Scale: 3/4" = 1'-0"





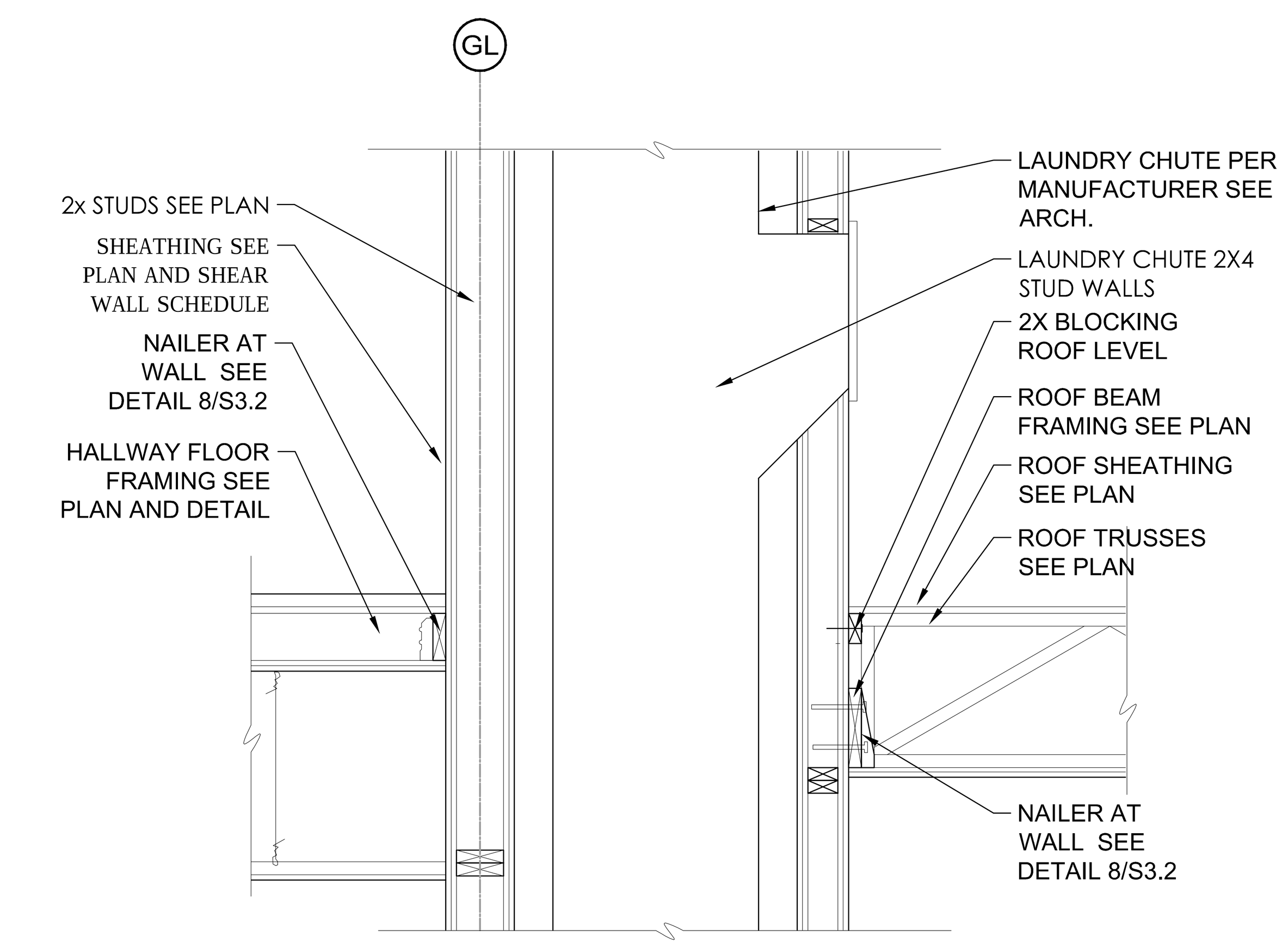
NOTE:
SEE DETAIL 4/S5.1 FOR SIMPSON HANGER DETAILS FOR BEAM TO BEAM CONNECTIONS

PLAN AT ROOF



NOTE:
SEE DETAIL 4/S5.1 FOR SIMPSON HANGER DETAILS FOR BEAM TO BEAM CONNECTIONS

PLAN AT FLOOR



7 FRAMING SECTION & DETAIL AT LAUNDRY CHUTE

Scale: 3/4" = 1'-0"

General Notes

1. SEE PLAN AND DETAIL 2/S5.2 FOR ROOF SHEATHING FASTENING PATTERNS FOR FIELD AND EDGES.
2. SEE ARCHITECTURAL FOR WALL LAYERED GYP. BOARD FIRE PROTECTION ASSEMBLY.
3. SEE S0.3 FOR WALL STUD AND SHEAR WALL SCHEDULE.
4. ALL FASTENERS SHALL BE PER IBC 2015 NAILING SCHEDULE FOR WOOD FRAMING MEMBERS UNLESS OTHERWISE SHOWN IN DETAILS.

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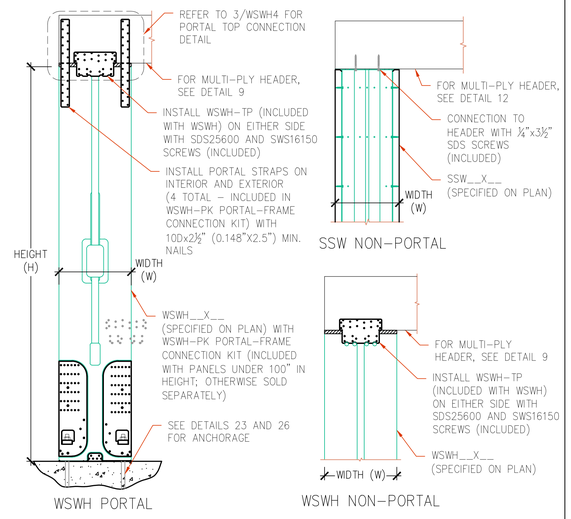
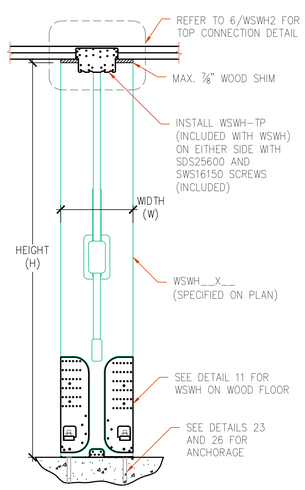
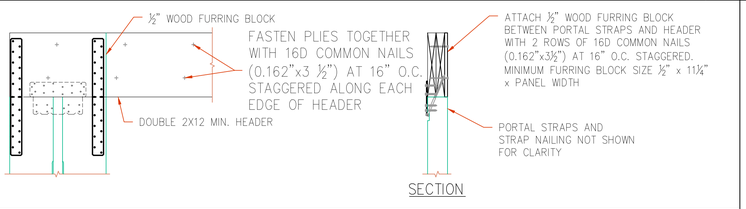
S3.3



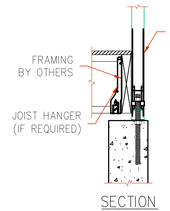
STRONG-WALL-FRONT | 5

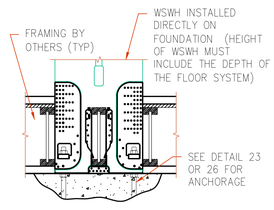


STRONG-WALL FIRST-STORY | 6

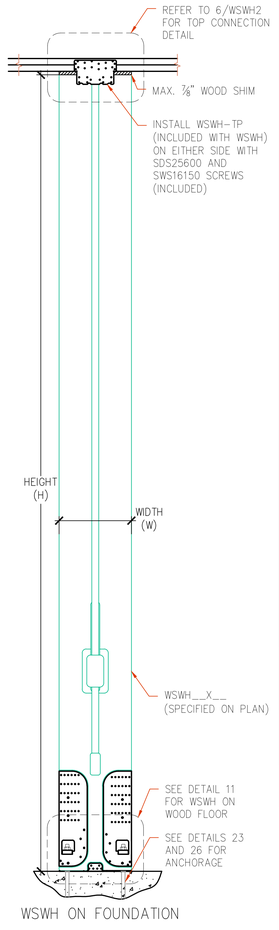


WSWH WITH MULTI-PLY HEADER | 9

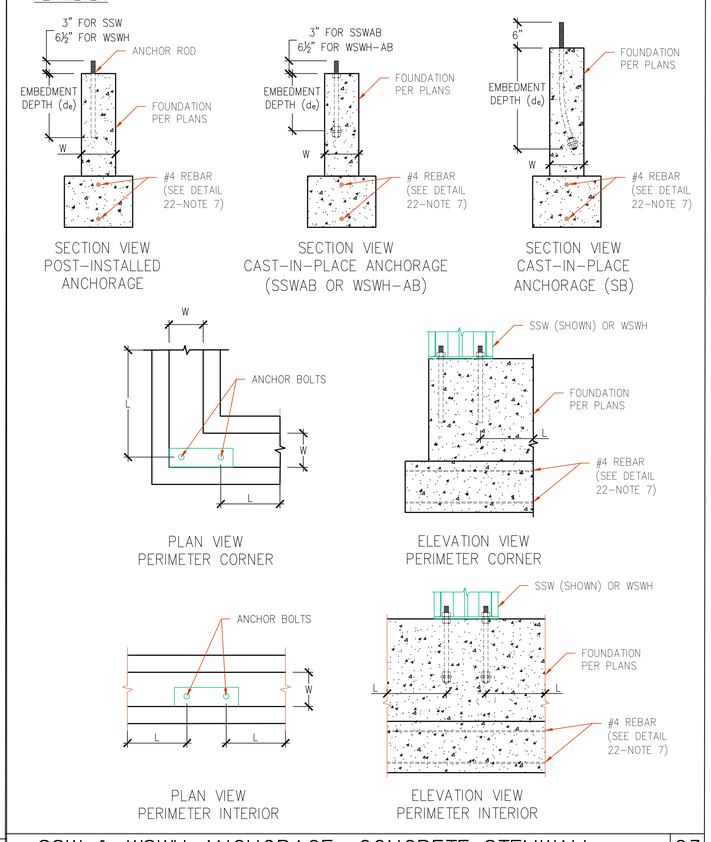




10 | WSWH ON WOOD FLOOR | 11



STRONG-WALL BALLOON-FRAMING | 8



SSW & WSWH ANCHORAGE: CONCRETE STEMWALL | 23

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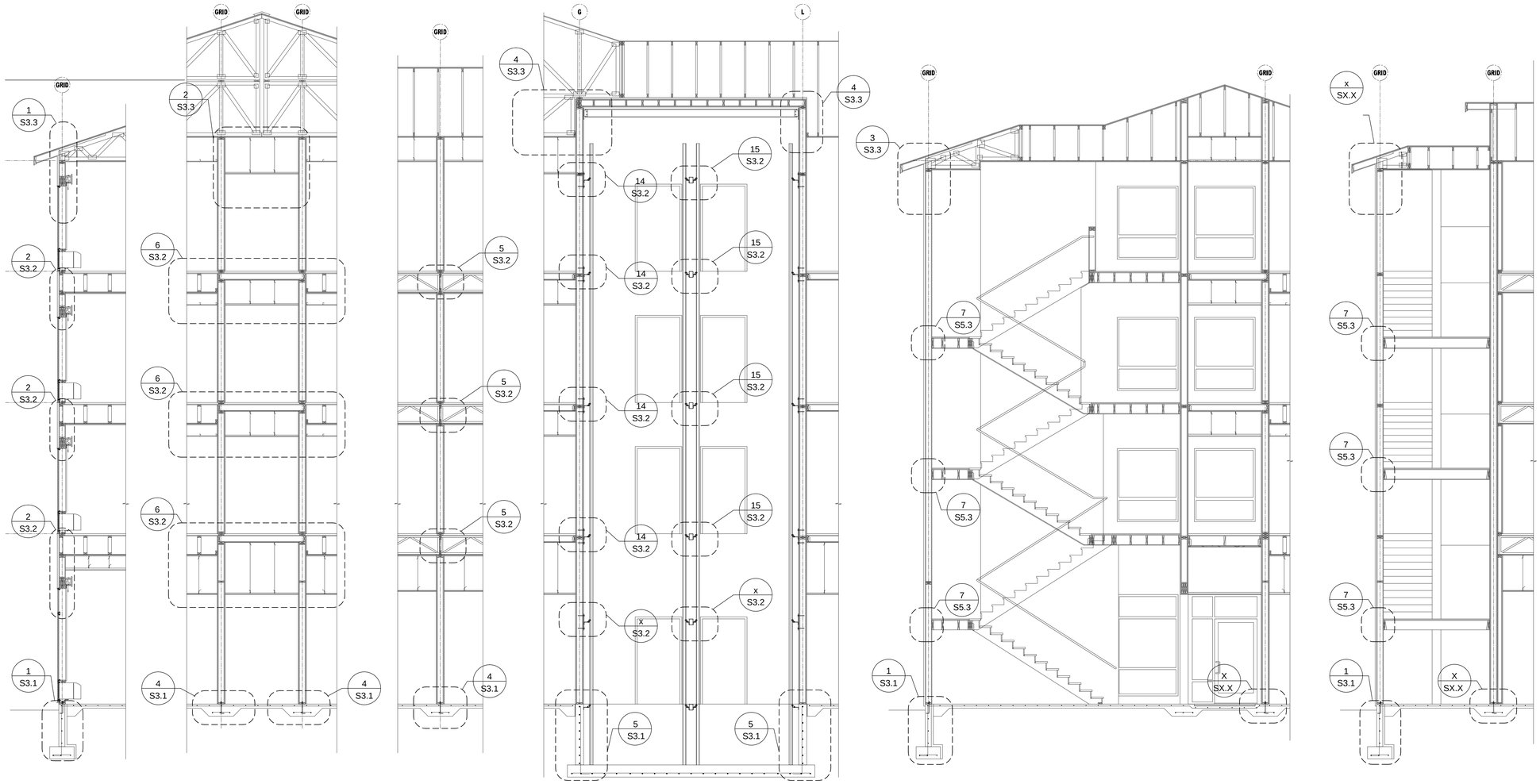
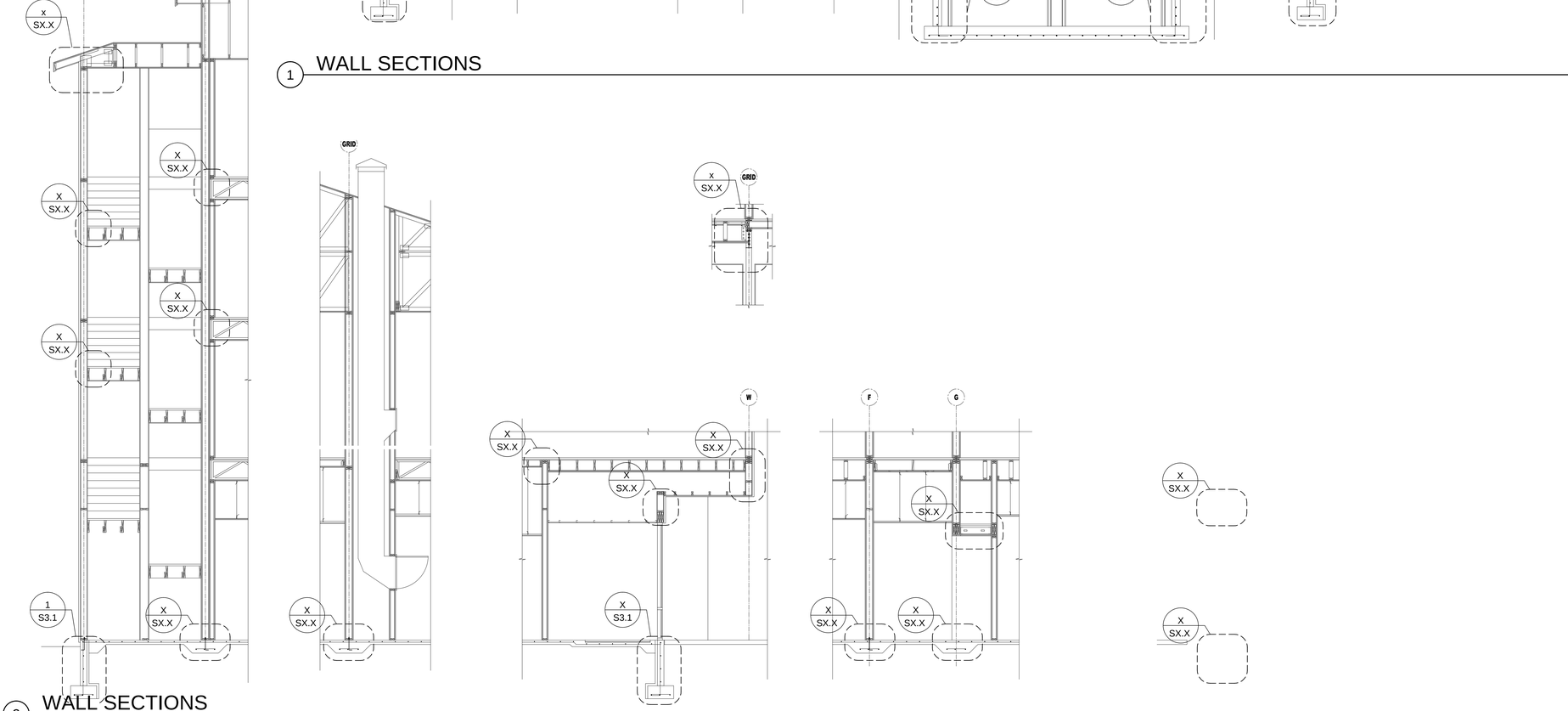


1 WALL SECTIONS

Scale: 1/4"=1'-0"



2 WALL SECTIONS

Scale: 1/4"=1'-0"

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info@design2functionllc.com
505-823-6481

NEW MEXICO LICENSED 0054782 PROFESSIONAL ENGINEER
8/21/23

REVIEWED FOR CODE COMPLIANCE
01/12/2024

Holiday Inn Express

3400 S. LINCOLN AVE
STEAMBOAT SPRINGS, CO 80487

DRAWING ISSUE DATES:

REVISION DATES:

PROJECT MANAGER:
NICK PIRKL

DRAWN BY:
NAP

SHEET TITLE:
BUILDING SECTIONS

S4.1

SHEET: 20 OF 140

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Civil ▪ Structural ▪ Mechanical ▪ Plumbing ▪ Electrical
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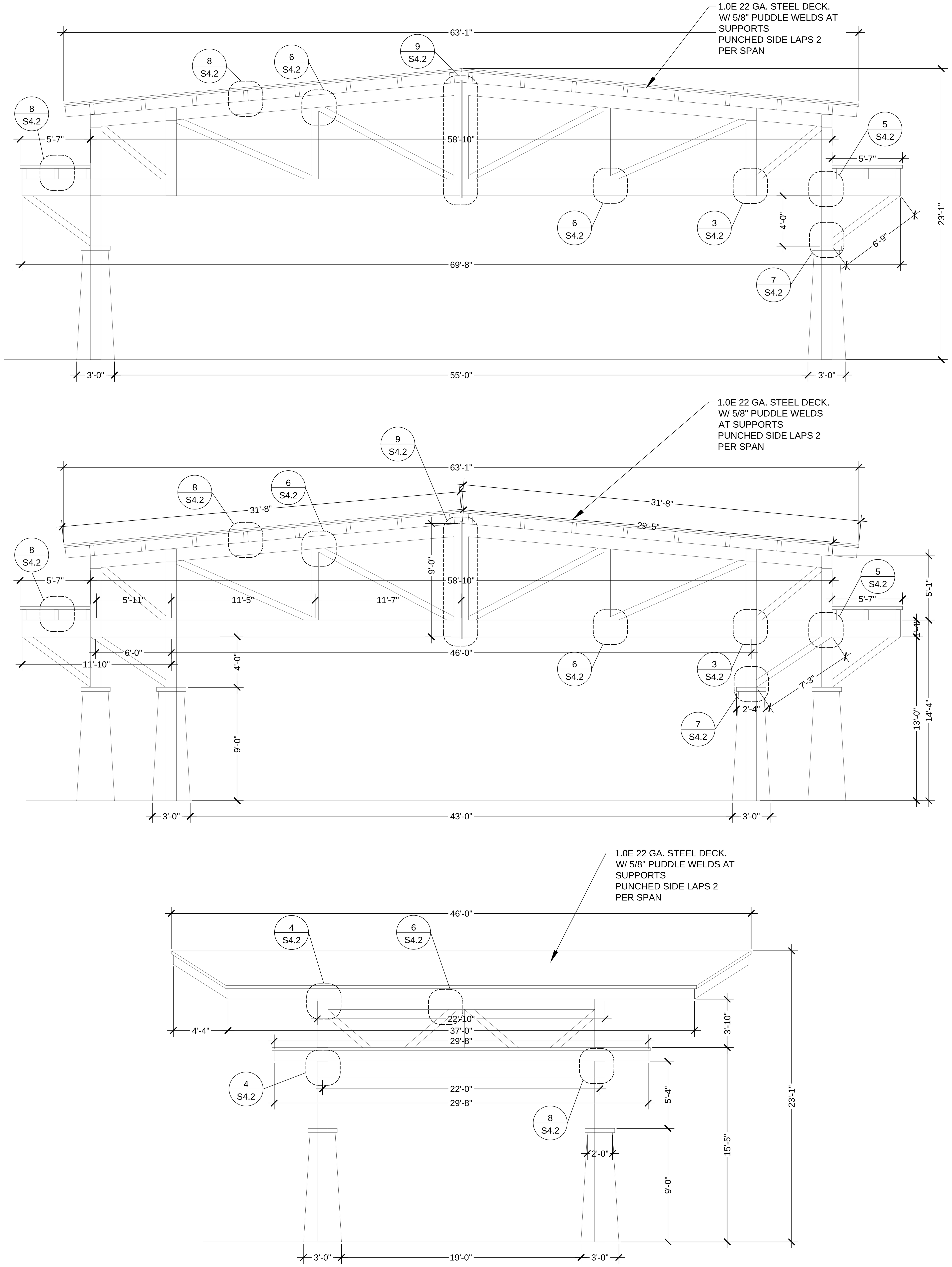
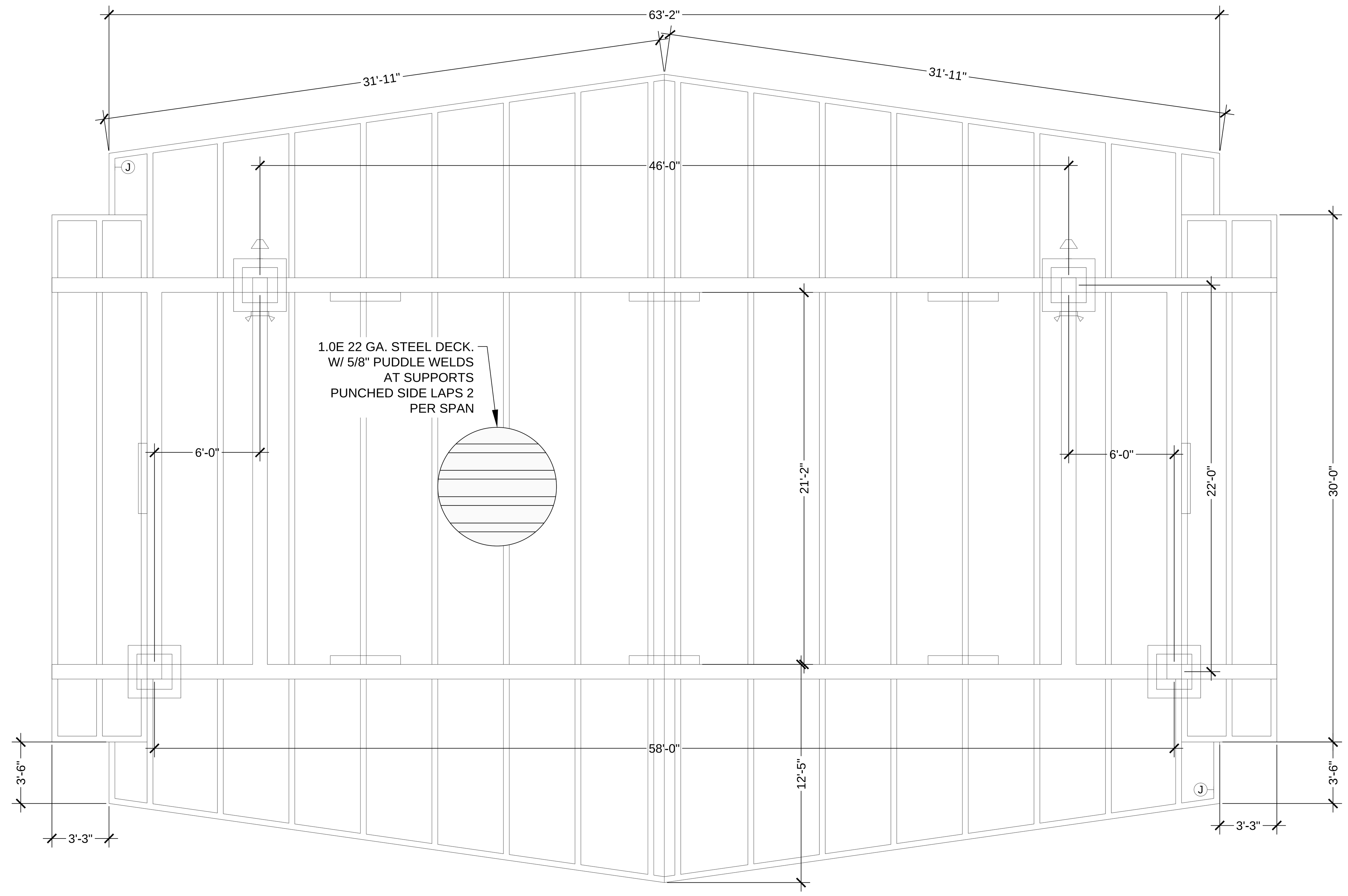
tele: 505.255.7802 Proj. No.: 23-015 www.abqeng.com



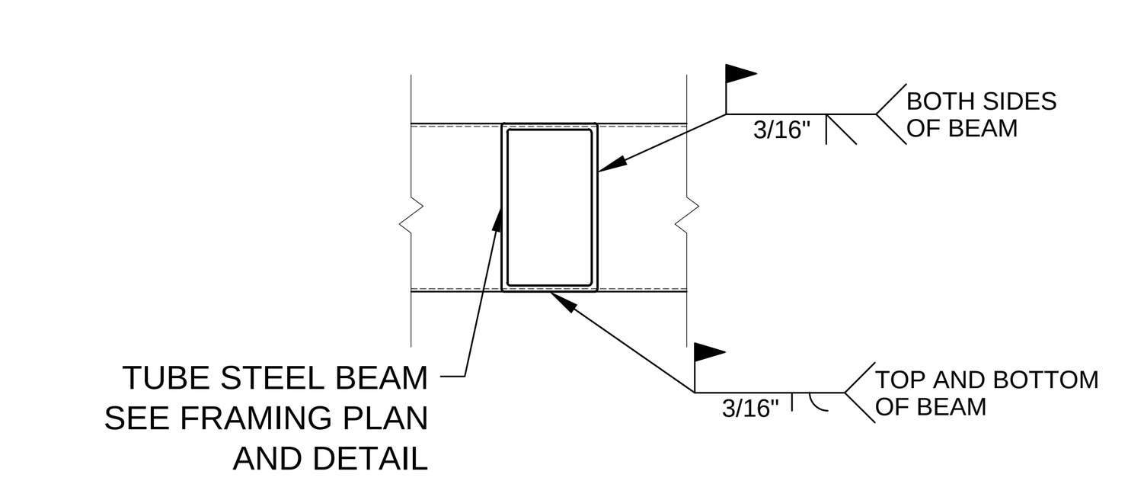
1 ELEVATIONS

Scale: 1/4"=1'-0"



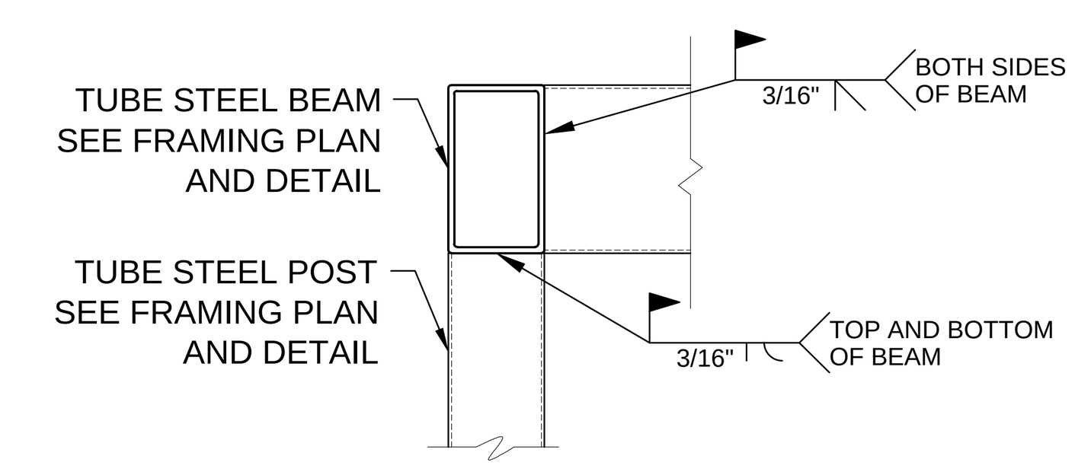
2 PLAN VIEW

Scale: 1/4"=1'-0"



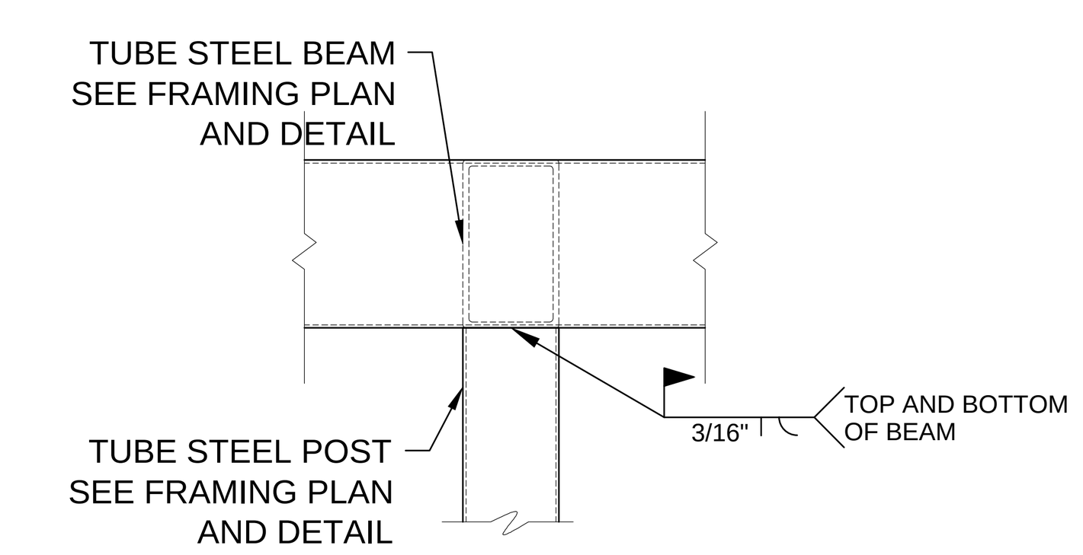
3 DETAIL

Scale: 3/4"=1'-0"



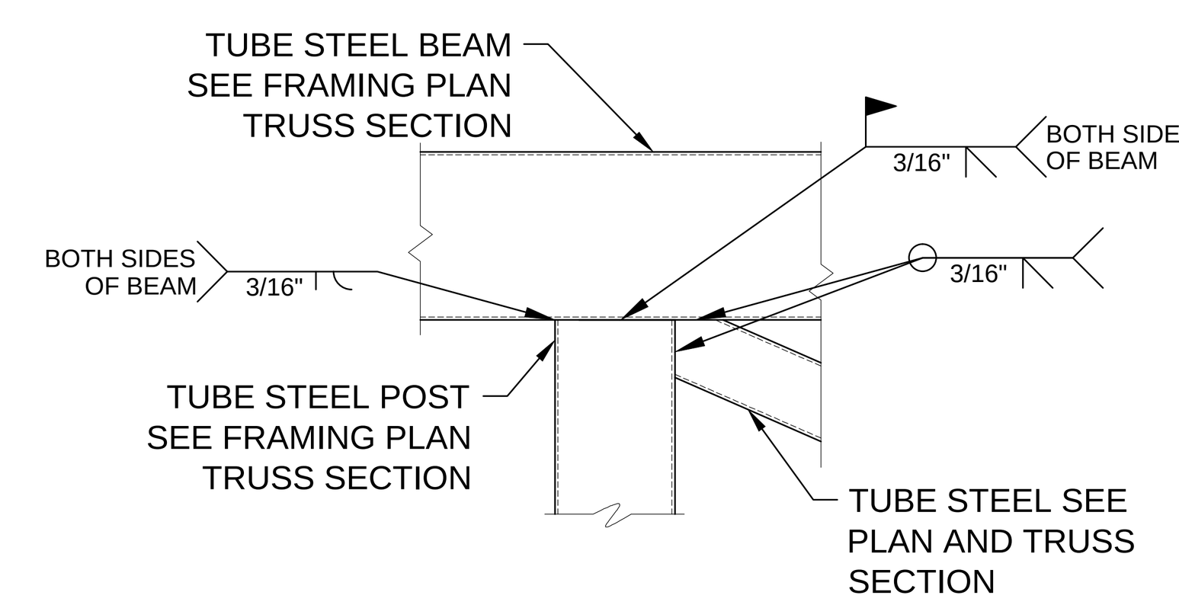
4 DETAIL

Scale: 3/4"=1'-0"



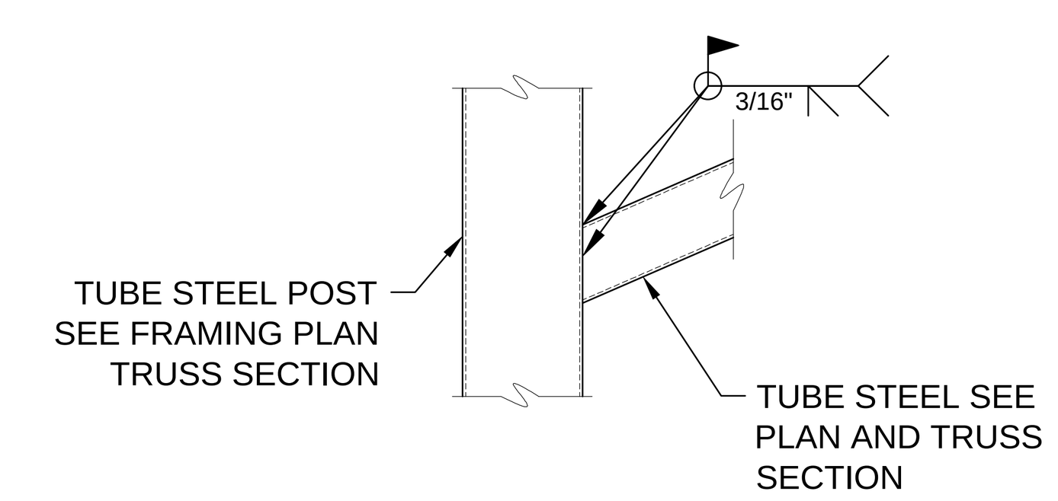
5 DETAIL

Scale: 3/4"=1'-0"



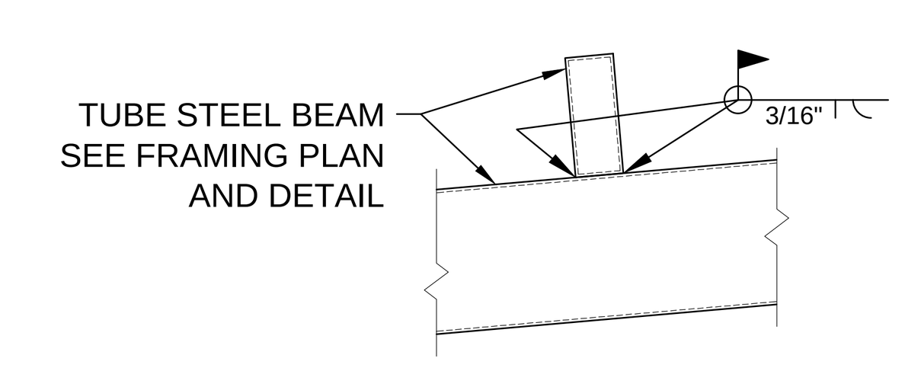
6 DETAIL

Scale: 3/4"=1'-0"



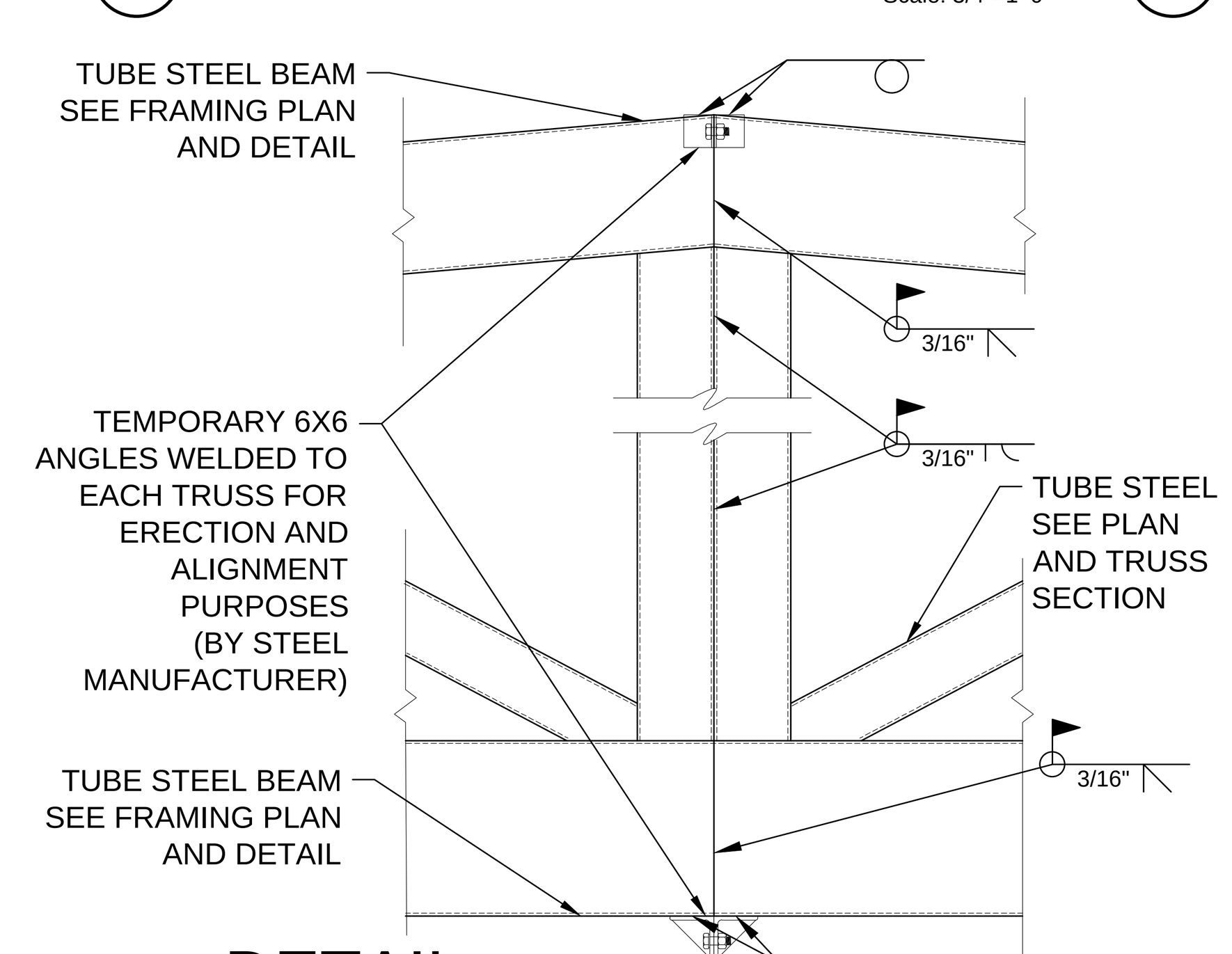
7 DETAIL

Scale: 3/4"=1'-0"



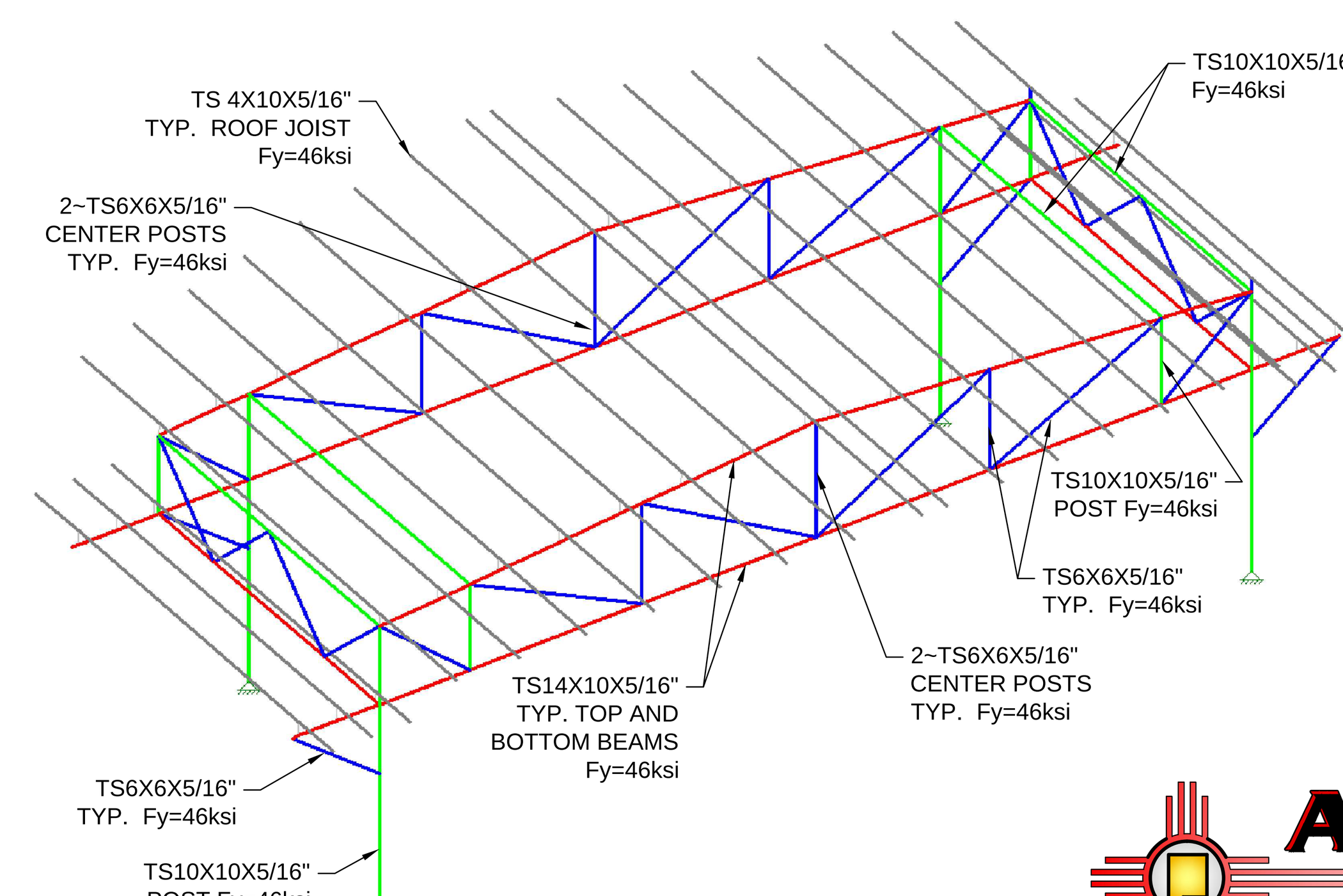
8 DETAIL

Scale: 3/4"=1'-0"



9 DETAIL

Scale: 3/4"=1'-0"



General Notes

ALL SHOP AND FIELD WELDS WILL BE GROUND SMOOTH TO MATCH THE SURFACE OF BEAM MEMBERS, AND PRIME AND FINISH SURFACE. FINAL FINISH PRIMER AND PAINT PER ARCHITECTURAL REQUIREMENTS.

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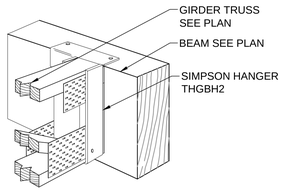
PROJECT MANAGER: NICK PIRKL

DRAWN BY: NAP

SHEET TITLE: BUILDING SECTIONS

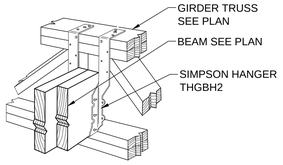
S4.2

SHEET: 21 OF 140



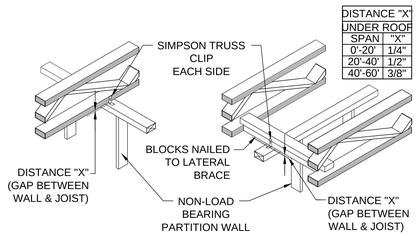
1 TRUSS/2X JOIST @ OPENING

Scale: 3/4" = 1'-0"



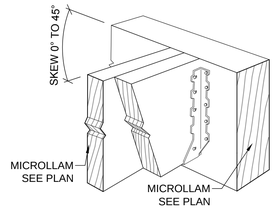
2 TRUSS/2X BEARING BLOCKING

Scale: 3/4" = 1'-0"



3 LATERAL SUPPORT

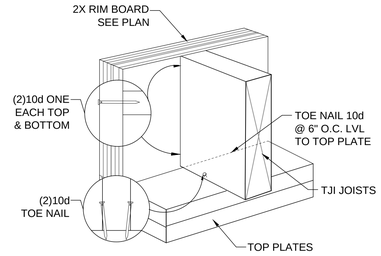
Scale: 3/4" = 1'-0"



SIMPSON HU9 FOR SINGLE HEADERS
SIMPSON HHUS410 FOR DOUBLE HEADERS

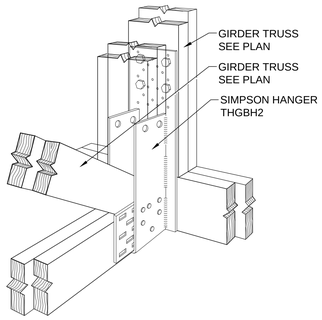
4 PARALLAM TO PARALLAM

Scale: 3/4" = 1'-0"



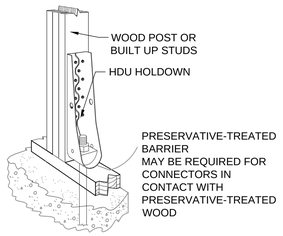
5 2X JOISTS TO TOP PLATE

Scale: 3/4" = 1'-0"



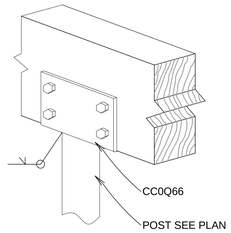
6 TJI/2X JOIST @ HEADER

Scale: 3/4" = 1'-0"



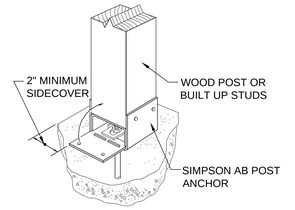
7 HOLD DOWN ANCHOR

Scale: 3/4" = 1'-0"



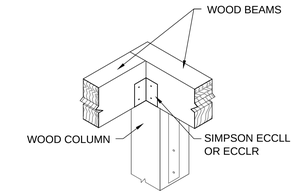
8 POST CAP AT BEAM

Scale: 3/4" = 1'-0"



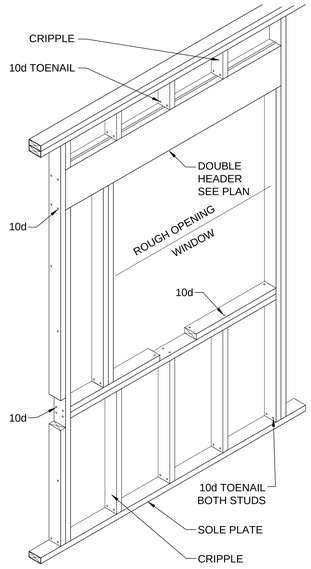
9 OFFSET POST BASE

Scale: 3/4" = 1'-0"



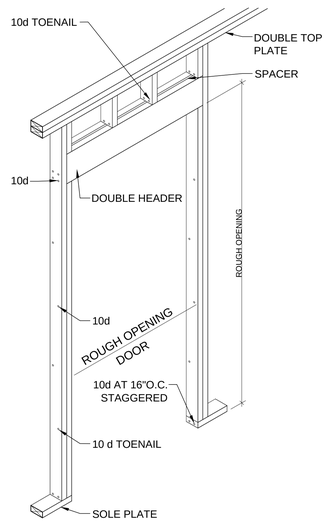
10 BEAM TO COLUMN CONNECTION

Scale: 3/4" = 1'-0"



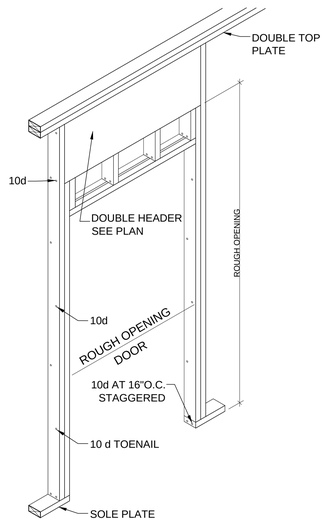
11 TYPICAL WINDOW FRAMING

Scale: 3/4" = 1'-0"



12 TYP. NON-BEARING DOOR FRAMING

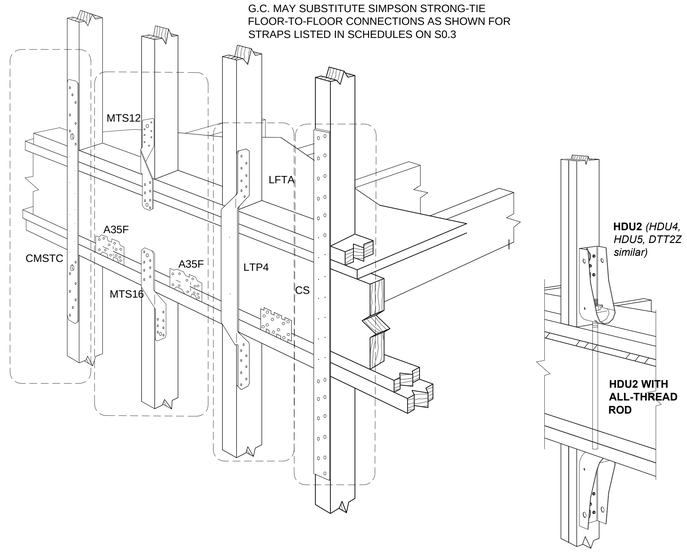
Scale: 3/4" = 1'-0"



13 TYP. BEARING DOOR FRAMING

Scale: 3/4" = 1'-0"

G.C. MAY SUBSTITUTE SIMPSON STRONG-TIE
FLOOR-TO-FLOOR CONNECTIONS AS SHOWN FOR
STRAPS LISTED IN SCHEDULES ON S0.3



14 TYP. FLOOR STRAPS

Scale: 3/4" = 1'-0"

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STRUCTURAL DETAILS

S5.1

SHEET: 22 OF 140

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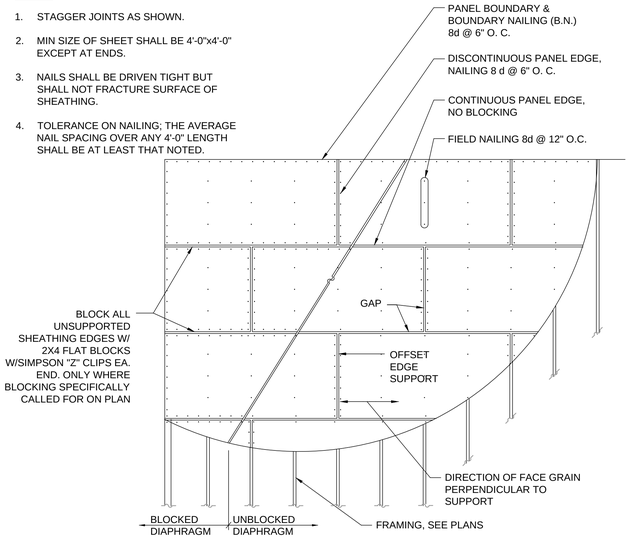
NOTES:

1. STAGGER JOINTS AS SHOWN.
2. MIN SIZE OF SHEET SHALL BE 4'-0"x4'-0" EXCEPT AT ENDS.
3. NAILS SHALL BE DRIVEN TIGHT BUT SHALL NOT FRACTURE SURFACE OF SHEATHING.
4. TOLERANCE ON NAILING; THE AVERAGE NAIL SPACING OVER ANY 4'-0" LENGTH SHALL BE AT LEAST THAT NOTED.

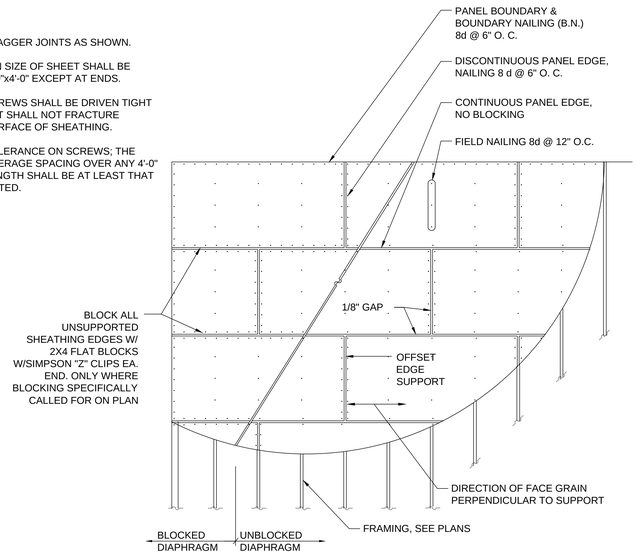


1 2ND FLOOR SHEATHING (NAILING)

Scale: 3/4" = 1'-0"

NOTES:

1. STAGGER JOINTS AS SHOWN.
2. MIN SIZE OF SHEET SHALL BE 4'-0"x4'-0" EXCEPT AT ENDS.
3. SCREWS SHALL BE DRIVEN TIGHT BUT SHALL NOT FRACTURE SURFACE OF SHEATHING.
4. TOLERANCE ON SCREWS; THE AVERAGE SPACING OVER ANY 4'-0" LENGTH SHALL BE AT LEAST THAT NOTED.

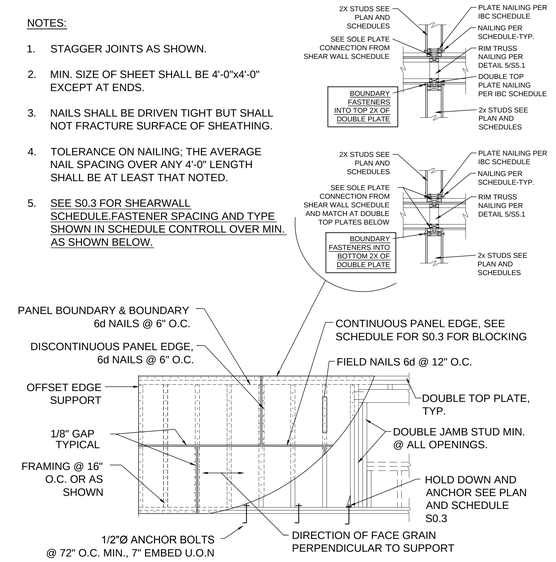


2 3RD, 4TH FLOOR & ROOF SHEATHING (NAILING)

Scale: 3/4" = 1'-0"

NOTES:

1. STAGGER JOINTS AS SHOWN.
2. MIN. SIZE OF SHEET SHALL BE 4'-0"x4'-0" EXCEPT AT ENDS.
3. NAILS SHALL BE DRIVEN TIGHT BUT SHALL NOT FRACTURE SURFACE OF SHEATHING.
4. TOLERANCE ON NAILING; THE AVERAGE NAIL SPACING OVER ANY 4'-0" LENGTH SHALL BE AT LEAST THAT NOTED.
5. SEE S0.3 FOR SHEARWALL SCHEDULE.FASTENER SPACING AND TYPE SHOWN IN SCHEDULE CONTROLL OVER MIN. AS SHOWN BELOW.



NOTE: EXTERIOR WALLS SHALL HAVE STRUCTURAL GRADE I SHEATHING. (OSB/PLYWOOD) CONTINUOUS.

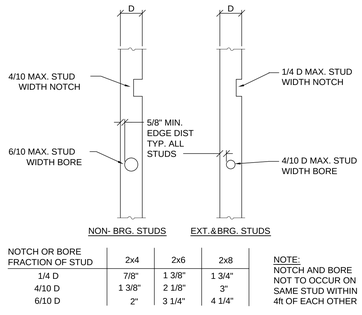
3 SHEAR WALLS - MINIMUM FASTENER DETAIL

Scale: 3/4" = 1'-0"

NOTE:
1. FRAMING NOTED ON PLANS GOVERNS. TYPICAL FRAMING SHOWN HERE.
2. STUD SHALL BE SPF NO. 3 OR BETTER AND HEADERS SHALL BE SPF NO. 2 OR BETTER.

4 TYPICAL HEADER FRAMING ELEVATION

Scale: 3/4" = 1'-0"

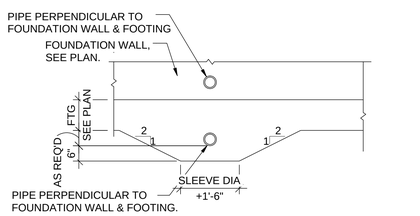


NOTCH OR BORE FRACTION OF STUD	2x4	2x6	2x8
1/4 D	7/8"	1 3/8"	1 3/4"
4/10 D	1 3/8"	2 1/8"	3"
6/10 D	2"	3 1/4"	4 1/4"

NOTE:
NOTCH AND BORE NOT TO OCCUR ON SAME STUD WITHIN 4ft OF EACH OTHER

6 STUD NOTCH/HOLE SPEC.

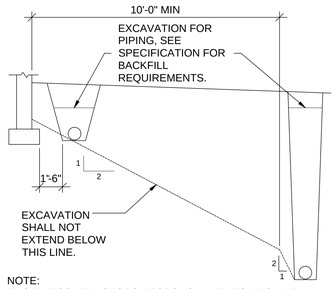
Scale: 3/4" = 1'-0"



NOTES:
1. FOR PIPES WITHIN FOOTING DEPTH STEP FOOTING PER 1/S500 SO PIPES PASS THROUGH WALL. PROVIDE SLEEVE & GROUT INTO WALL.
2. FOR PIPES BELOW FOOTING PROVIDE SLEEVE & THICKEN CONCRETE FOOTING AS SHOWN OR STEP FOOTING BELOW PIPE.
3. SLEEVE DIAMETER TO BE 2" GREATER THAN PIPE OUTSIDE DIAMETER.

7 PIPE SLEEVE AT FOOTING, TYP

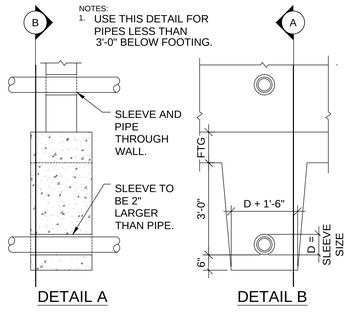
Scale: N.T.S.



NOTE:
IT IS THE CONTRACTORS RESPONSIBILITY TO PROVIDE SHORING, SHEATHING, ETC. TO MAINTAIN THE SIDES OF THE EXCAVATION FROM CAVING IN UNTIL ALL WORK AND BACKFILL IS COMPLETE.

8 TRENCH AT WALLS, TYP

Scale: N.T.S.



9 PIPE AT STEM WALLS, TYP

Scale: N.T.S.

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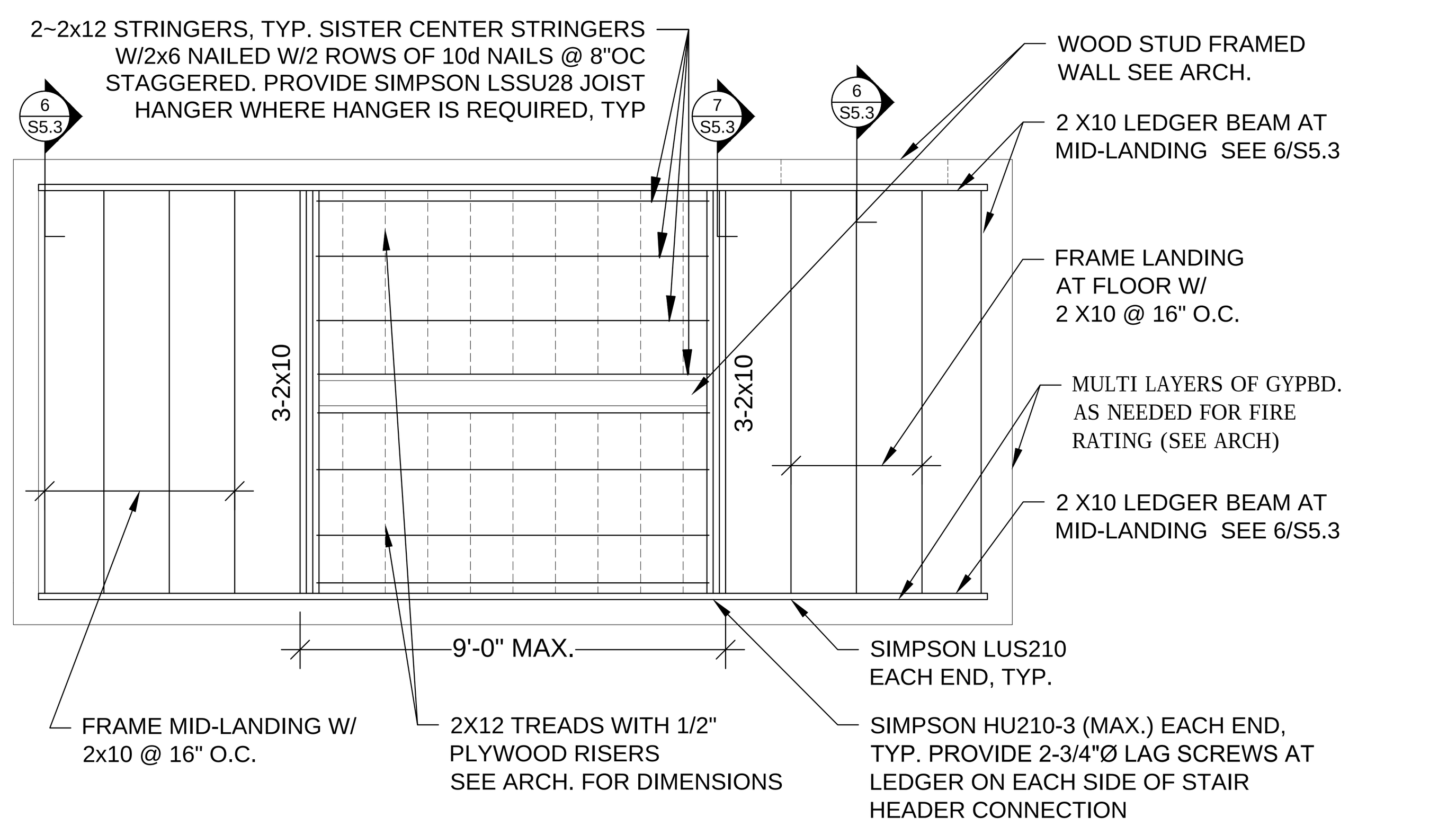
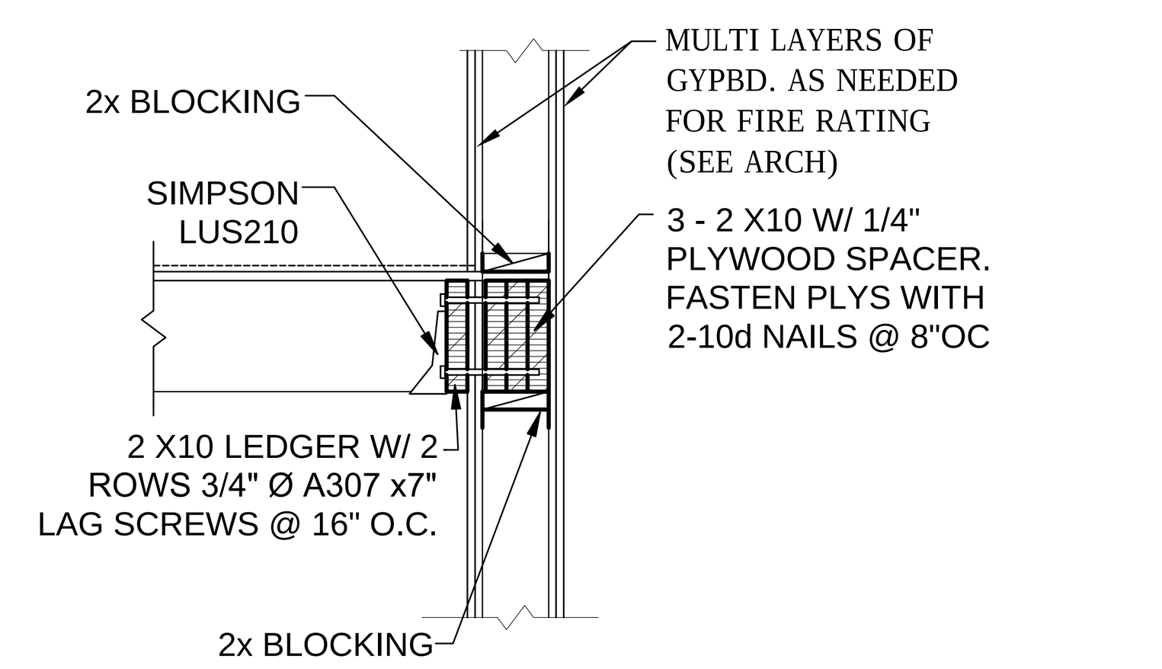


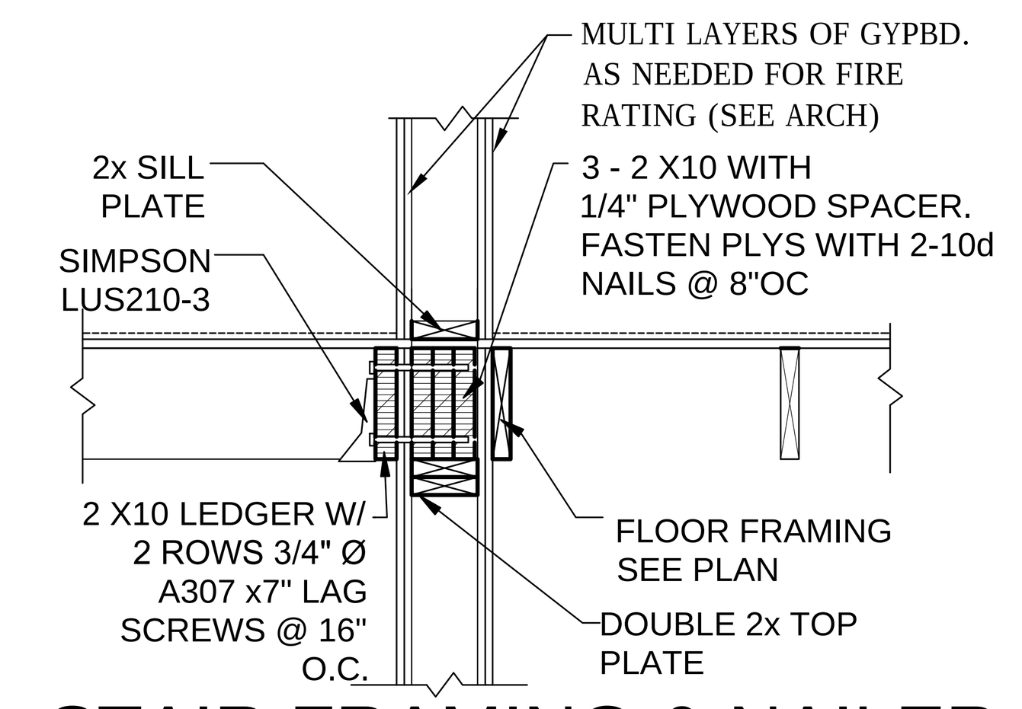
5 STAIR FRAMING PLAN DETAIL

Scale: 3/8"=1'-0"



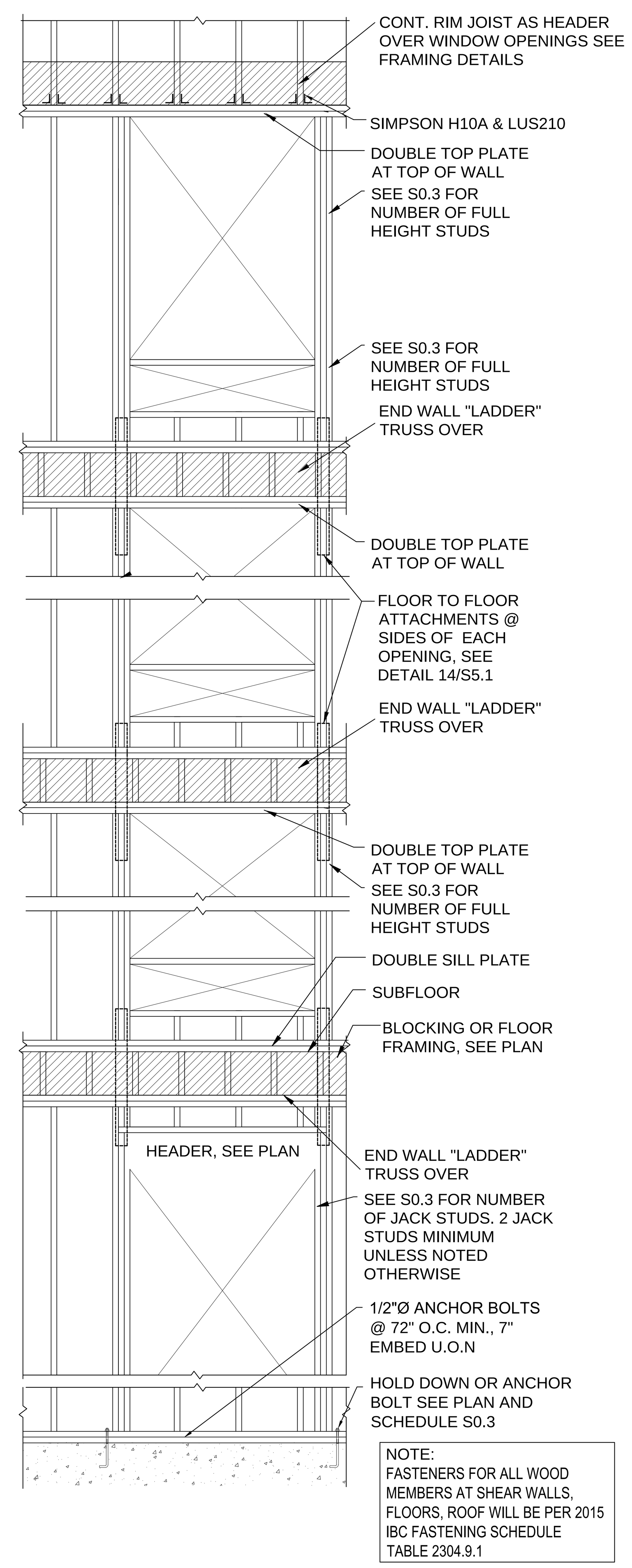
6 STAIR FRAMING & NAILER

Scale: 3/4"=1'-0"



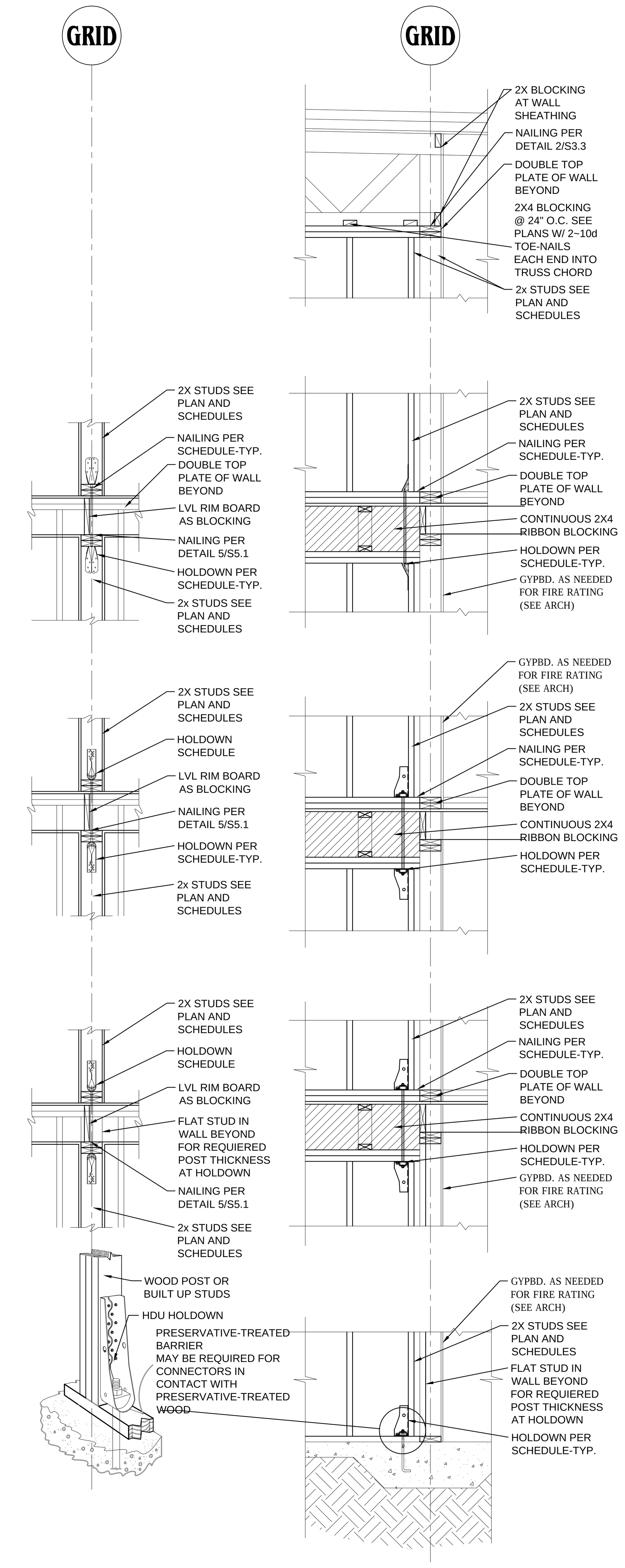
7 STAIR FRAMING & NAILER

Scale: 3/4"=1'-0"



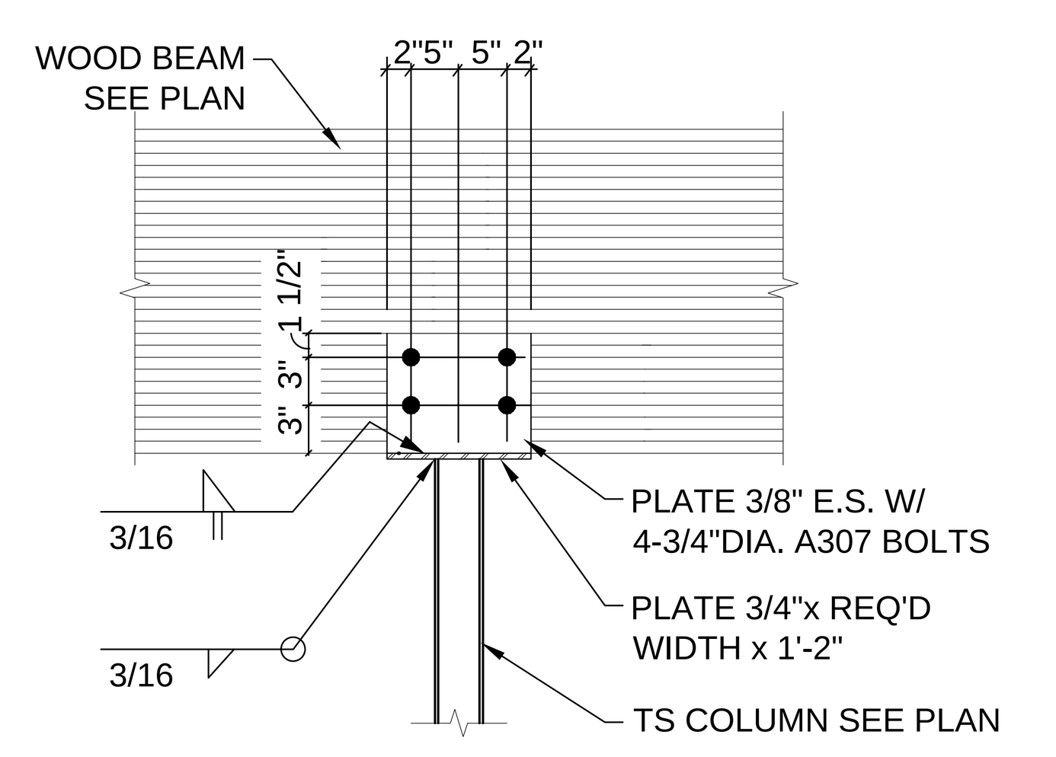
4A TYP. SHEAR WALL HOLDOWN & LOAD PATH

Scale: N.T.S.



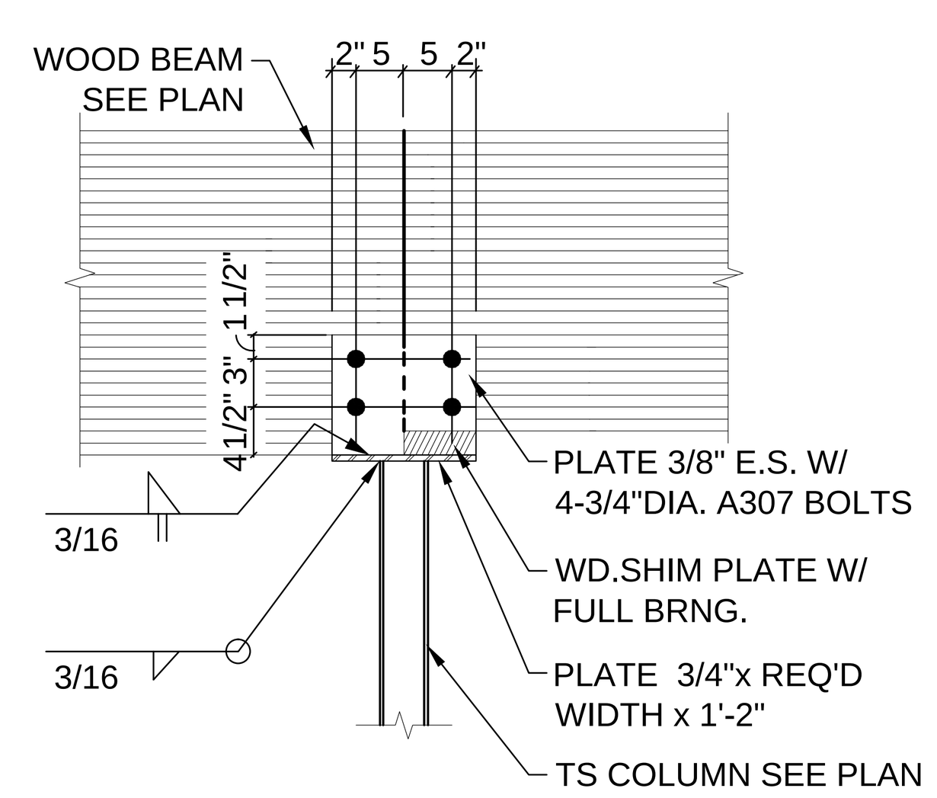
4B SHEAR WALL HOLDOWN & LOAD PATH

Scale: N.T.S.



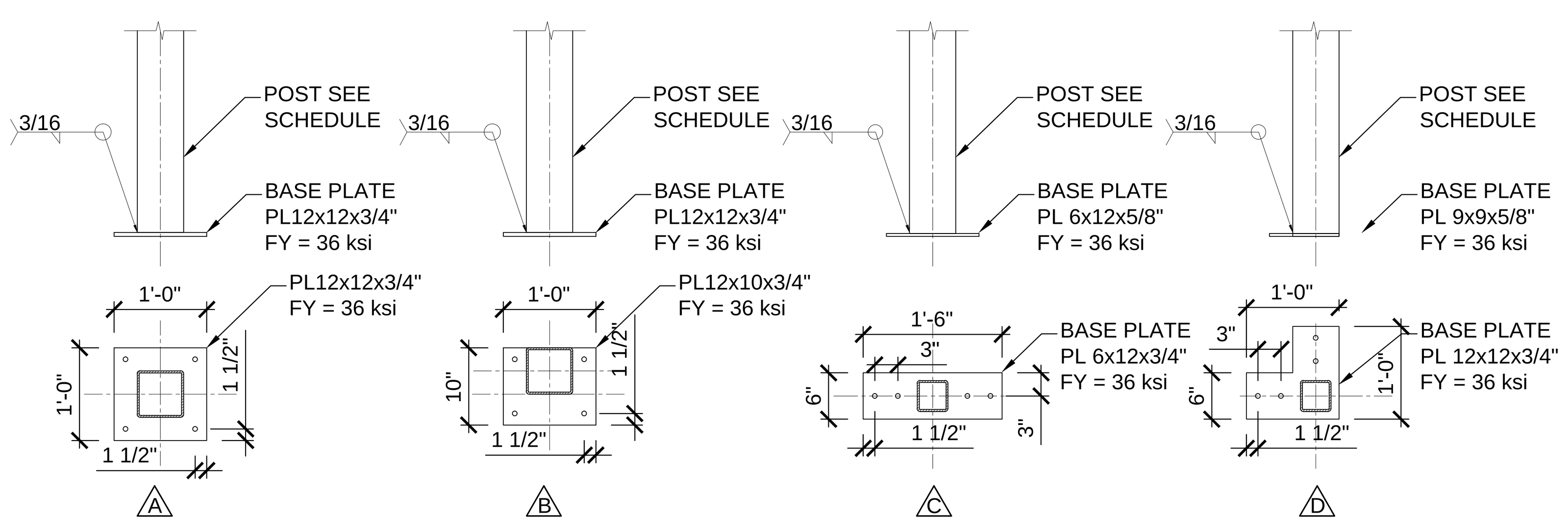
8 BEAM TO HSS COL. CONN.

Scale: 3/4"=1'-0"



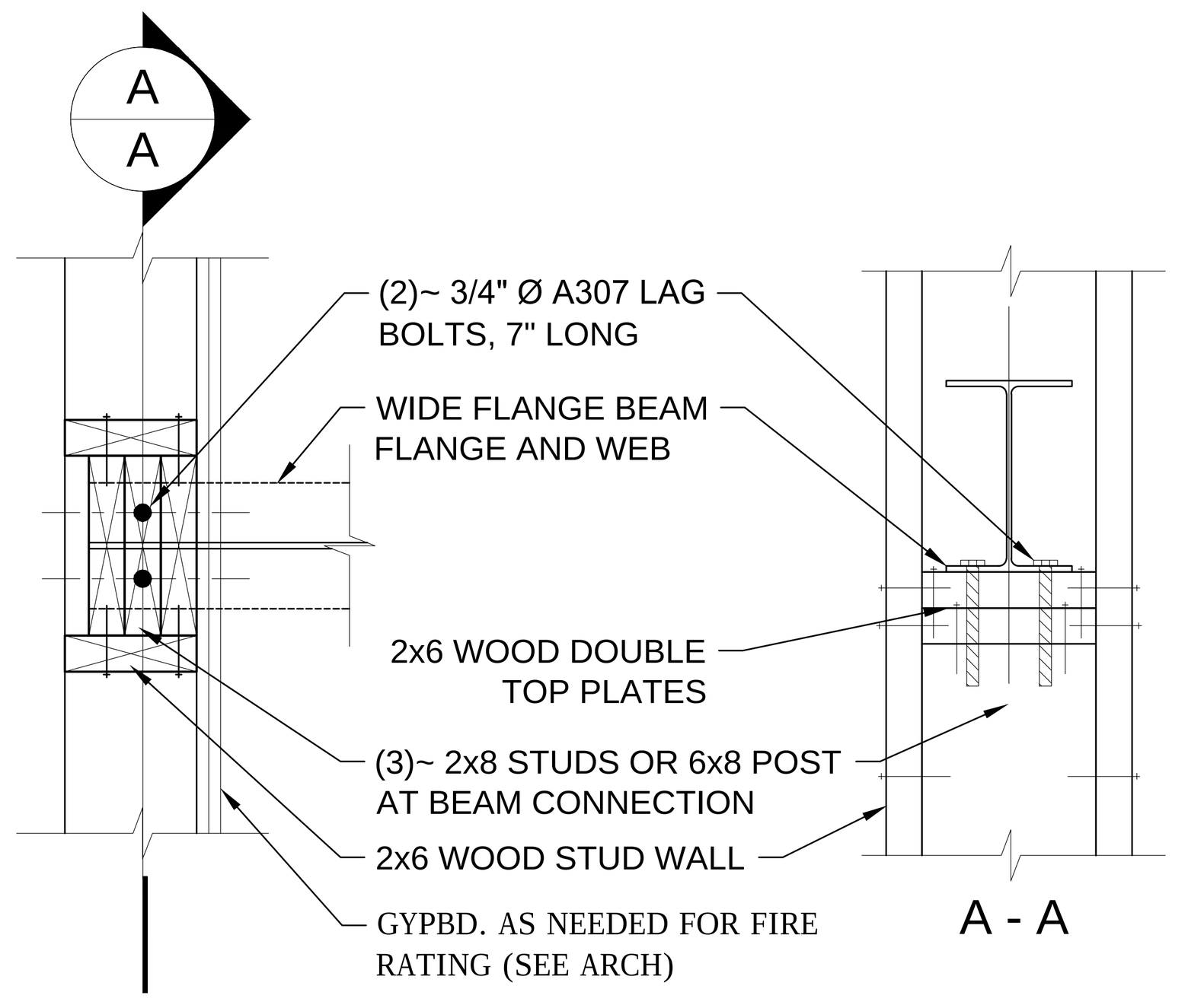
9 BEAM TO HSS COL.

Scale: 3/4"=1'-0"



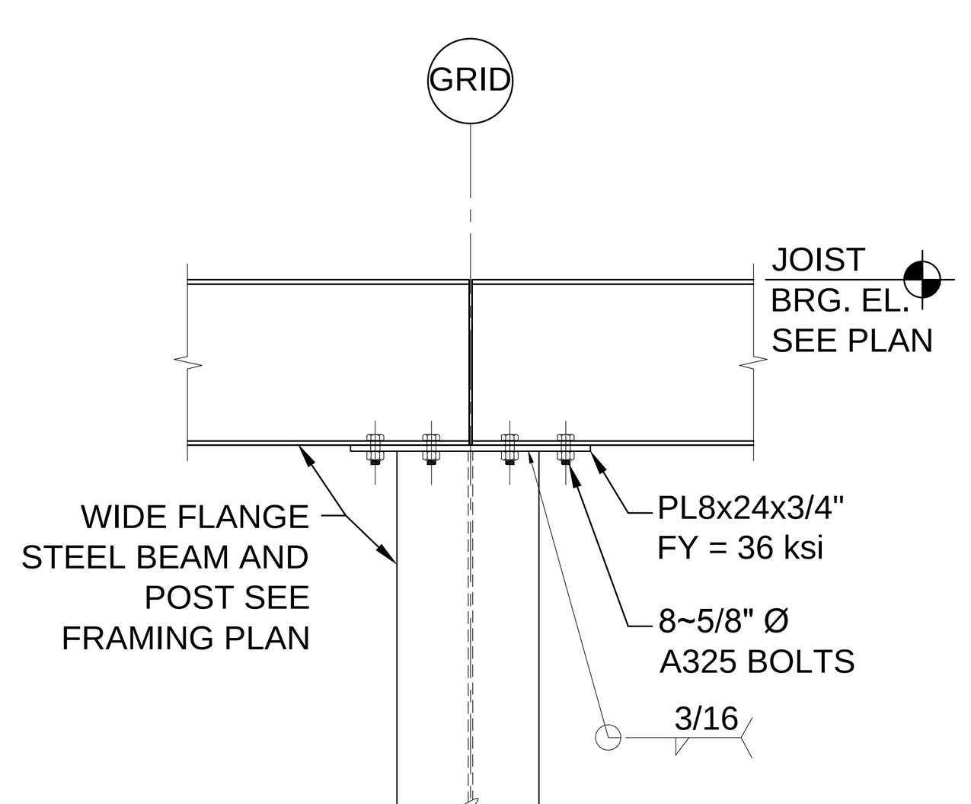
10 TS COLUMN BASE PLATES TYP.

Scale: 3/4"=1'-0"



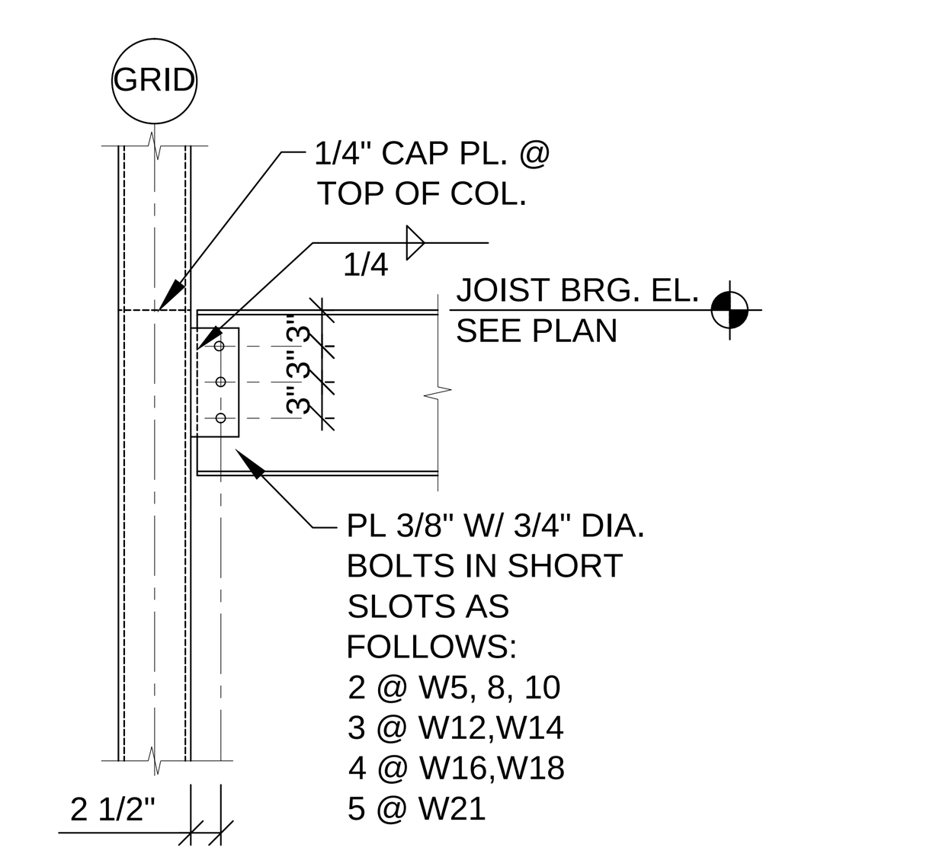
13 FRAMING DETAIL

Scale: 1/2" = 1'-0"



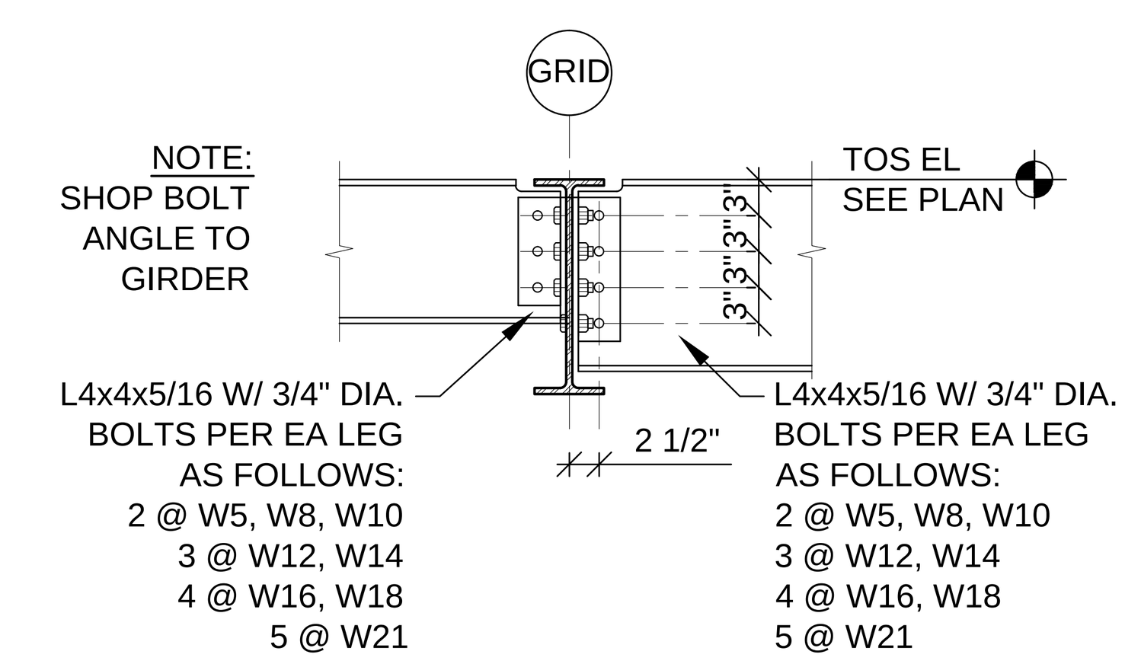
14 WF BEAM @ WF COL.

Scale: 3/4"=1'-0"



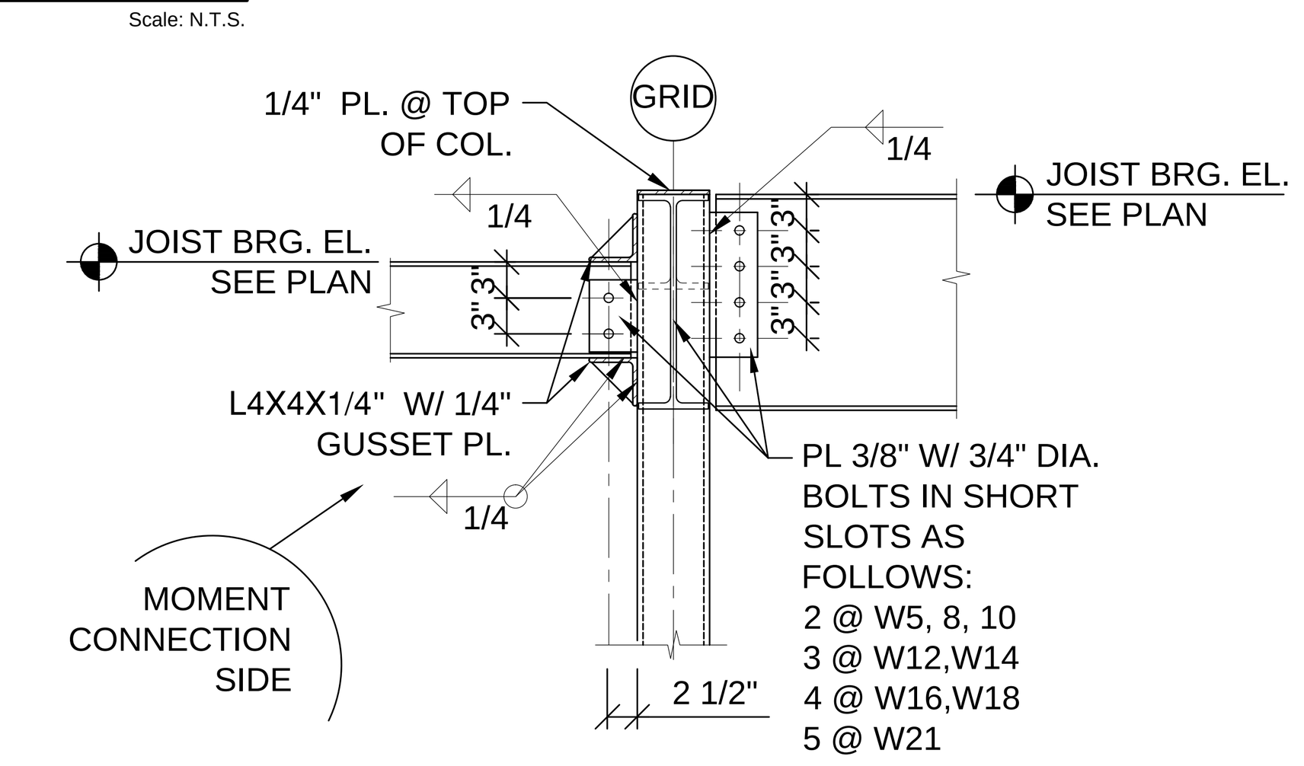
11 BEAM TO HSS COL. CONN.

Scale: 3/4"=1'-0"



12 WF TO WF BEAM CONN.

Scale: 3/4"=1'-0"



13 WF TO TS POST MOMENT CONN.

Scale: 3/4"=1'-0"

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8/21/23

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