#### PROJECT LOCATION RCRBD

**RECORD SET** 

**Existing Building Description:** 

The Lower Gondola Terminal is a 60,876 s.f. Structure containing two buildings separated by a two-hour fire separation wall. The North building is 37,554 s.f. With a basement and three floors. The South building is 24,322 s.f. With no basement and three floors. The North building basement is at the same elevation as the South building first floor. Both buildings have been classified as Type 5-B, NFPA 13 fire sprinkled system.

Proposed:

Addition of 380 s.f. uncovered outdoor deck area for climbing wall (provided by owner).

Occupancy group: A-5 TABLE 503, Area unlimited.

TABLE 508.4 No separation requirement.

OWNER:

2305 Mt Werner Cir,

Steamboat Springs, CO 80487

ARCHITECTS/PLANNERS:

**ESA ARCHITECTURE -- PLANNING** 1919 7TH STREET BOULDER, CO 80302 303.442-5458

600 S. LINCOLN AVE. #201, STEAMBOAT SPRINGS CO 80487

#### STRUCTURAL ENGINEER:

Alpenglow Engineering Solutions 1901 Pine Grove Road, Suite 202 P.O. Box 773501 Steamboat Springs, Co 80477 (970) 879-1181 BEN SCHUTT ben@alpengloweng.com

#### SHEET INDEX

001 **COVER SHEETS** A-001 1ST LEVEL PLAN A-002 2ND LEVEL PLAN A-003 **ELEVATION & SECTION** 

**RAILING PLAN** A-004 **RAILING DETAILS** A-005

S-1 **NEW DECK FRAMING PLAN** 

PJ2406-1 **Fire Prevention** In: 05/10/2017 Out: 05/12/2017

**PROJECT TEAM** 

STEAMBOAT SKI & RESORT CORP. SSRC 2305 MT. WERNER CIRCLE STEAMBOAT SPRINGS, CO 80487 970 871 5436 MICHAEL GUMBINER

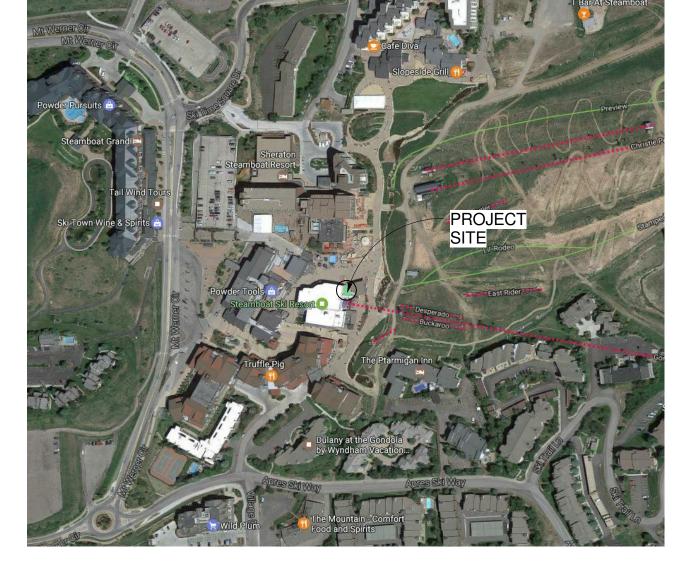
970 879 5458

PROJECT MANAGER: LEE FISCHER Lee@esapc.com



Project Phase
CONSTRUCTION

DOCUMENT COVER SHEETS

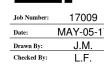


**CLIMBING WAI** 

RCRBD







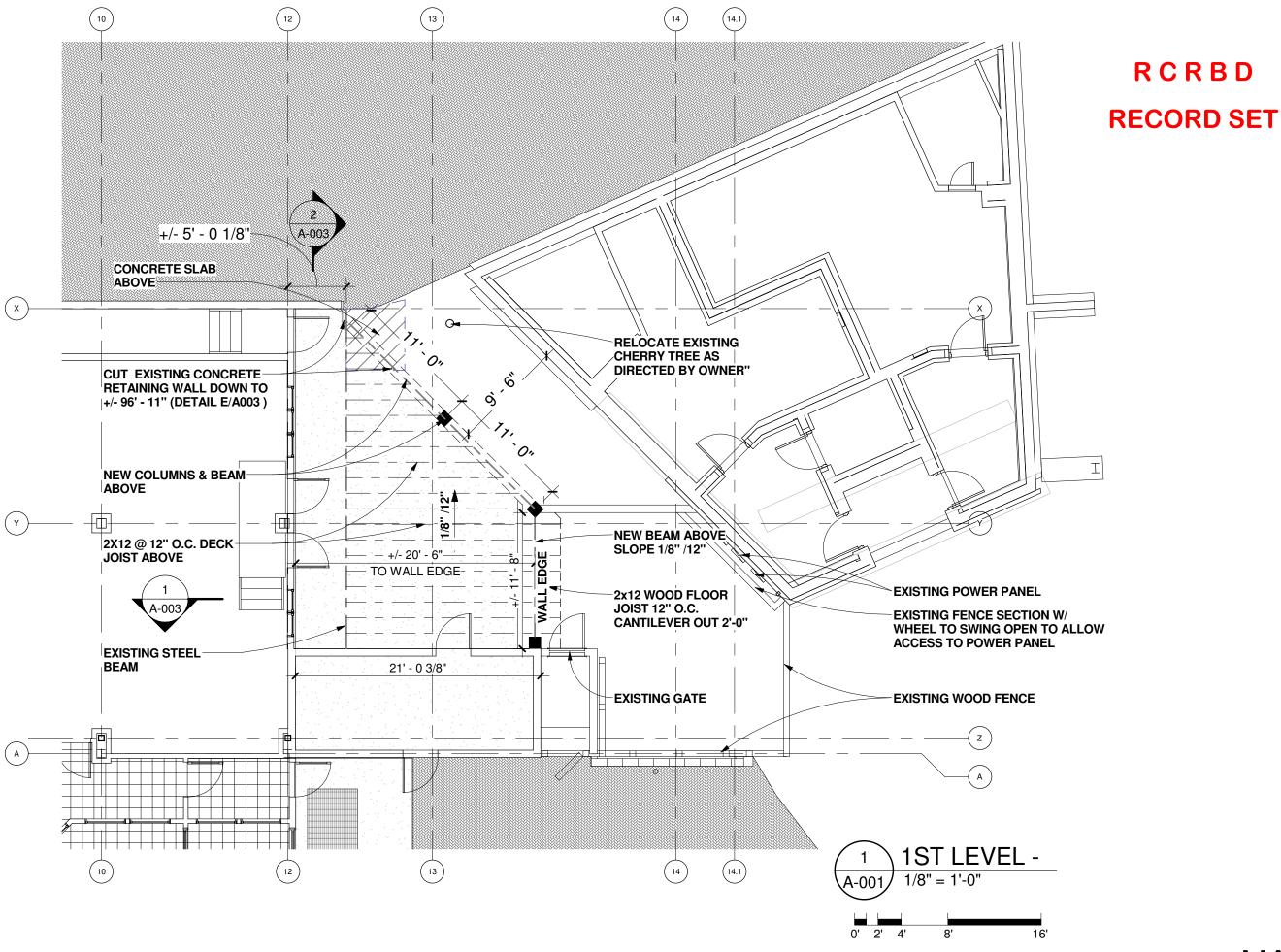
Project Phase

CONSTRUCTION

DOCUMENT

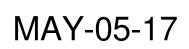
1ST LEVEL PLAN

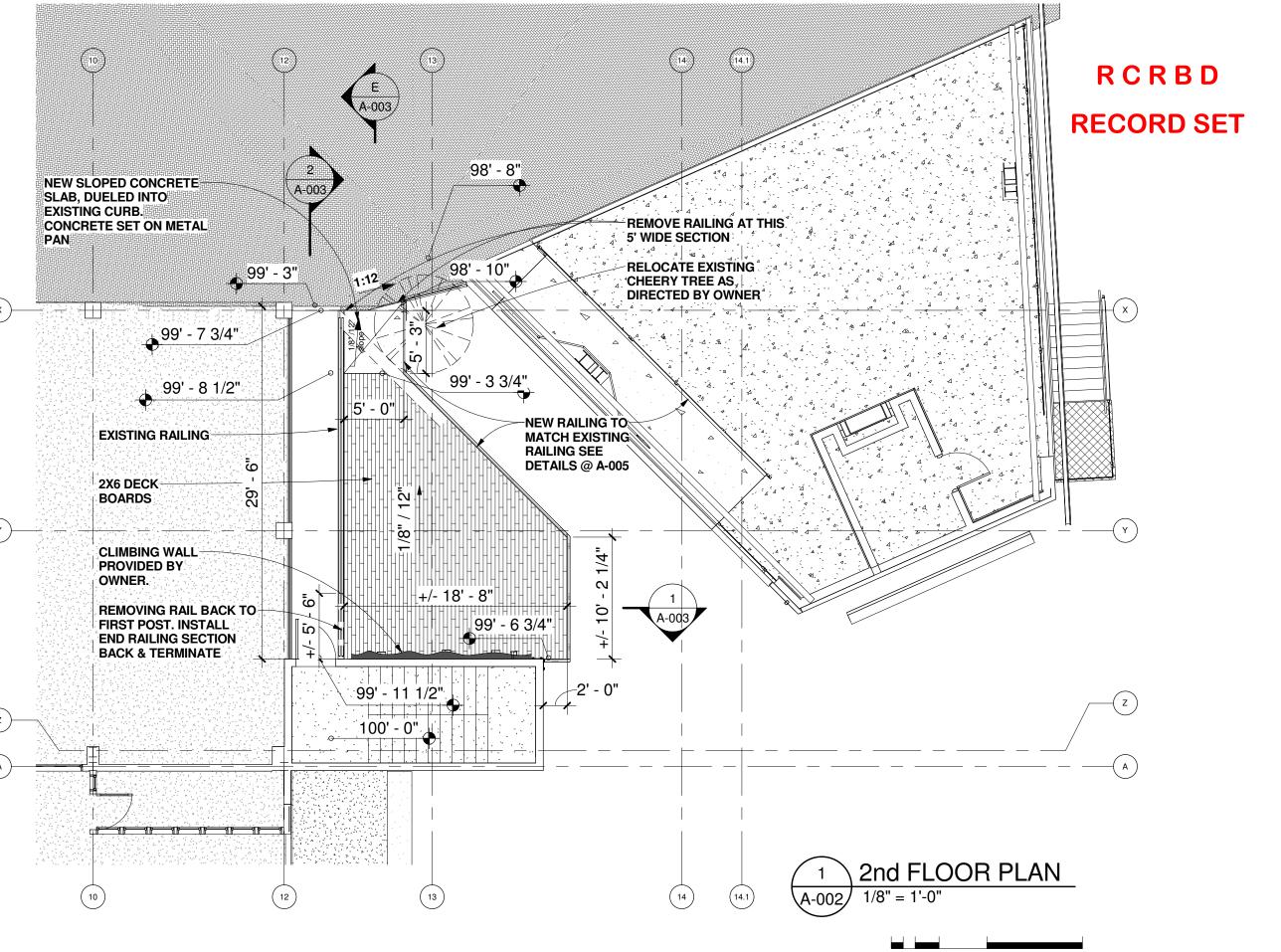
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2ND LEVEL PLAN



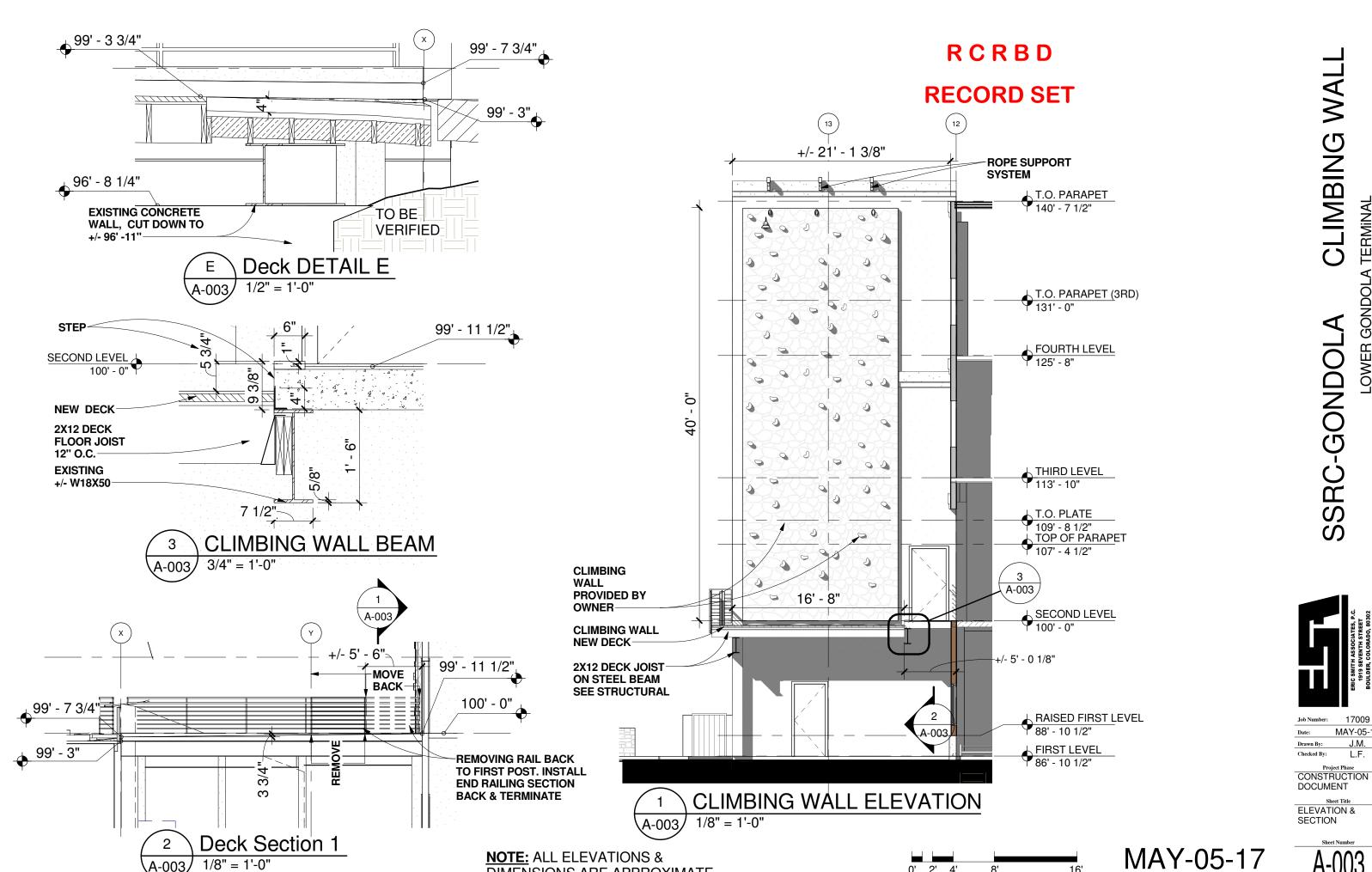


**NOTE:** ALL ELEVATIONS & DIMENSIONS ARE APPROXIMATE.

LOWER GONDOLA TERMINAL STEAMBOAT SPRINGS, COLORADO

17009

MAY-05-17



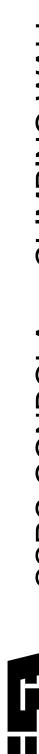
**NOTE:** ALL ELEVATIONS &

DIMENSIONS ARE APPROXIMATE.

0' 2' 4'

1/8" = 1'-0"

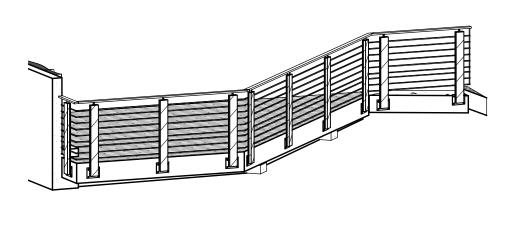
A-003,

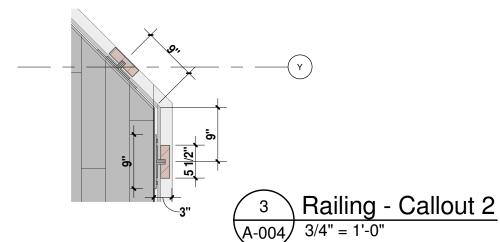


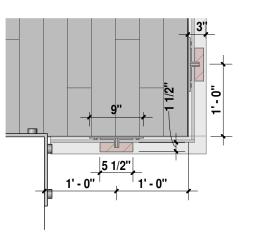


A-004





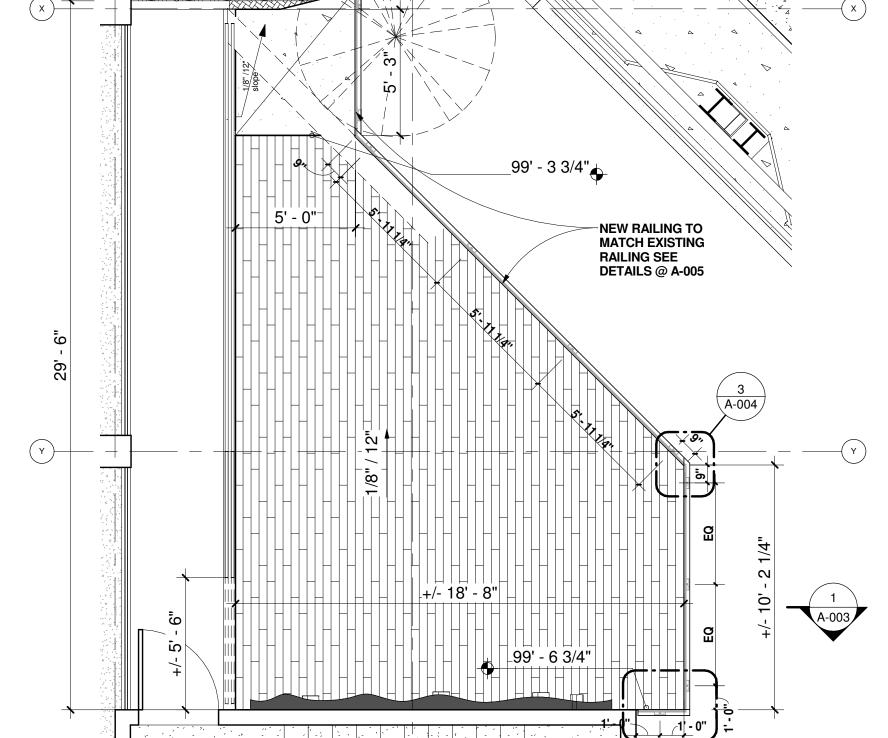




2 A-004

Railing - Callout 1

MAY-05-17



( 13 )

Railing PLAN

1/4" = 1'-0"

\_98' - 10"\_\_

A-003

99' - 3"\_

1:12

LOWER GONDOLA TERMINAL STEAMBOAT SPRINGS, COLORADO









Checked By:

Project Phase
CONSTRUCTION DOCUMENT

RAILING DETAILS

6' - 0" MAX. 3" X1" RECTANGULAR 5 3/8" **TUBULAR STEEL RAIL WELD** TO STEEL MOUNTING PLATE 4" X1 1/2" X3/8" MOUNTING-3" 41/4" 41/4" 41/4" 41/4" 41/4" 41/4" 41/4" 41/4" PLATE WELD TO STEEL RAIL & STEEL ANGEL **GALVANIZED SCREW THRU** STEEL ANGLE INTO WOOD 1/2" SQUARE TUBULAR STEEL RAIL. TYP. WELD TO STEEL ANGEL 2 1/2" X1 1/2"X1/4" STEEL **ANGLE WELD TO PLATE** 2X6 WOOD PLANK 6"x6"x1/2" STEEL PLATE TOP OF DECK **WOOD PLANK SIDE** STEEL ANGEL SIDE **ELEVATION ELEVATION** RAILING ELEVATION 3" X1" RECTANGULAR **TUBULAR STEEL RAIL** 1" = 1'-0" A-005 **WELD TO STEEL GALVANIZED SCREW MOUNTING PLATE** THRU STEEL ANGLE 2"x6" PLANK INTO WOOD 4" X1 1/2" X3/8" 1/2" SQUARE TUBULAR MOUNTING PLATE WOOD **STEEL** STEEL RAIL, TYP. WELD **WELD TO STEEL RAIL ANGEL PLANK TO ANGLE** & STEEL ANGEL SIDE SIDE ROUTER INSIDE 1/2" SQUARE TUBULAR 2 1/2" X1 1/2"X1/4" STEEL **EDGE** STEEL RAIL. TYP. WELD 3/8" 3" X1" RECTANGULAR 3/8" 1 1/2" 1 1/8" ANGLE WELD TO PLATE 3/16" **TUBULAR STEEL RAIL** TO STEEL ANGEL 3/16"-**RAILING PLAN WELD TO STEEL** 4 1/2" 4 1/2" 9"x6"x1/2" STEEL 2 1/2" X1 1/2"X1/4" **MOUNTING PLATE** PLATE 3" = 1'-0" STEEL ANGLE WELD TO A-005 4" X1 1/2" X3/8" PLATE **MOUNTING PLATE WELD** RCRBD TO STEEL RAIL & STEEL 1. REFERENCE SPECIFICATIONS FOR METAL TYPE & FINISH. **ANGEL** 2. CONTRACTOR SHALL PROVIDE SHOP DRAWING OF RAILING & 2X6 WOOD PLANK 2X6 WOOD PLANK **RECORD SET** WELDS FOR APPROVAL BY OWNER'S REP. PRIOR TO CONSTRUCTION, DESIGN OF CONNECTIONS & WELDS BY 1/2" SQUARE FABRICATOR, UNLESS OTHERWISE NOTED. **TUBULAR STEEL RAIL.** 3. DRAWING TO REFLECT DESIGN INTENT, CONTRACTOR SHALL TOP OF DECK TYP. WELD TO STEEL (2X6 REDWOOD **ANGEL** 

> RAILING SECTION 1'' = 1'-0''

DECKING)

**RIM JOINTS** 

9"x6"x1/2" STEEL

PLATE BOLTED TO

5/8"-

1/2"\_\_\_\_\_1 1/2"\_\_

2 1/2" X1 1/2"X1/4"

**TO PLATE** 

**STEEL ANGLE WELD** 

- DEVELOP PROPOSED FABRICATION UNIT SECTION AND INTERFACING CONNECTION, AND PROVIDE ALL NECESSARY ACCESSORIES, ATTACHMENTS, & WELDS.
- 4. ALL GUARDRAILS SHALL BE CONSTRUCTED IN ACCORDANCE WITH ALL APPLICABLE CODES AND STANDARDS.
- 5. CONTRACTOR SHALL GRIND AND SMOOTH EDGES TO MINIMIZE BURRS AND POINT.
- 6. CONTRACTOR TO COORDINATE WITH OWNERS REP. STEEL PLATE LOCATION PRIOR TO ANY CONCRETE POURING RELEVANT TO GUARDRAIL /HANDRAIL LOCATIONS.
- 7. GUARDRAIL SHALL BE LEVEL AND PLUMB.

# GENERAL NOTES

### DESIGN LIVE LOADS

- a. Deck......100 ps

# FOUNDATION DESIGN

a. Design of footings is based on a maximum allowable bearing pressure of 2000 psf dead load plus live load placed on the natural undisturbed soils below frost depth.

# REINFORCED CONCRETE

- a. Structural concrete shall have a minimum 28 day compressive strength of 3000 psi Type I.
- b. Reinforcing bars shall conform to ASTM Specification A615-79 and shall be Grade 60.
  c. All anchor bolts are to meet ASTM Specification F1554 Grade 36.
- d. At splices, lap bars 38 diameters. At corners and intersections, make horizontal bars continuous or provide matching corner bars. Around openings in walls and slabs, provide 2-#5, extending 2'-0" beyond edge of opening.

## STRUCTURAL STEEL

- a. Structural steel rolled shapes shall conform to ASTM A572, Grade 50. Plates and angles shall conform to ASTM A36. Tube shapes shall conform to ASTM A500 Grade B., 46 ksi yield. Pipe shapes shall conform to ASTM A53, Grade B.
- b. All bolts shall conform to ASTM Specification A307.
- c. Expansion bolts called for on the drawings shall be "WEG-IT", "RED HEAD", or approved wedge type, with the
- following minimum embedments: 5/8" diameter bolts -23/4", 1/2" diameter bolts -21/4". d. All welding shall be done by a certified welder.

# STRUCTURAL WOOD FRAMING

- a. Except where noted otherwise, all 2" lumber shall be Douglas Fir—Larch S4S No.2 or better, and all solid timber beams and posts shall be Douglas Fir—Larch No. 1. Grade shall be as approved by TPI in accordance with ASTM D=3957=84
- b. Except as noted otherwise, minimum nailing shall be provided as specified in Table 2304.9.1 "Fastening Schedule" of the I.B.C., 2009 edition.
- c. Where light gage framing anchors are shown or required, they shall be Simpson "Strong Tie" or equal ICBO approved connectors and shall be installed with the number and type of nails recommended by the manufacturer to develop the rated capacity.

# EPOXY ADHESIVE ANCHORING SYSTEM

- a. Epoxy adhesive anchoring system shall be 1/2"ø HILTI HIT HY 150 MAX anchors w/ HAS threaded rods or
- b. Anchor rods shall be furnished with chamfered ends so that either end will accept a nut and washer and meet
- the requirements of ISO 898 Class 5.8.

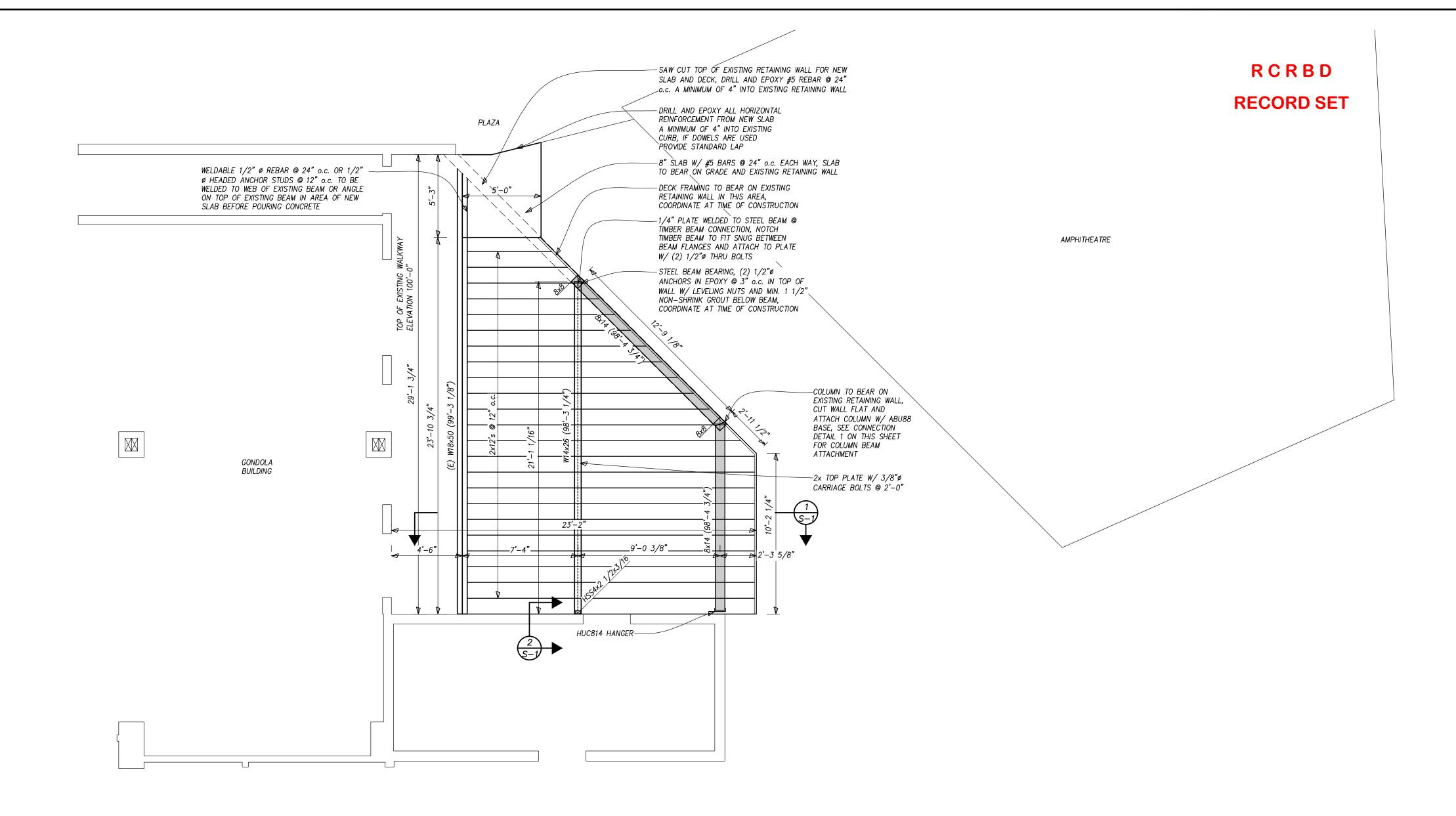
  c. Anchors shall have the following minimum embedments: 3/4"ø 6 3/4", 5/8"ø 5 5/8", 1/2"ø 4 1/2".

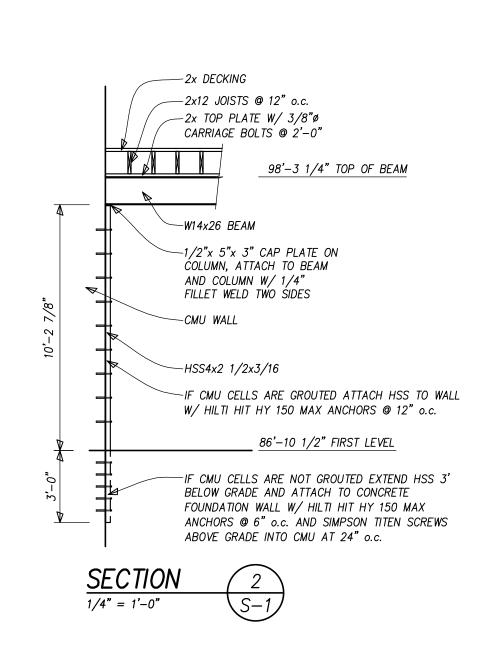
# STRUCTURAL ERECTION AND BRACING REQUIREMENTS

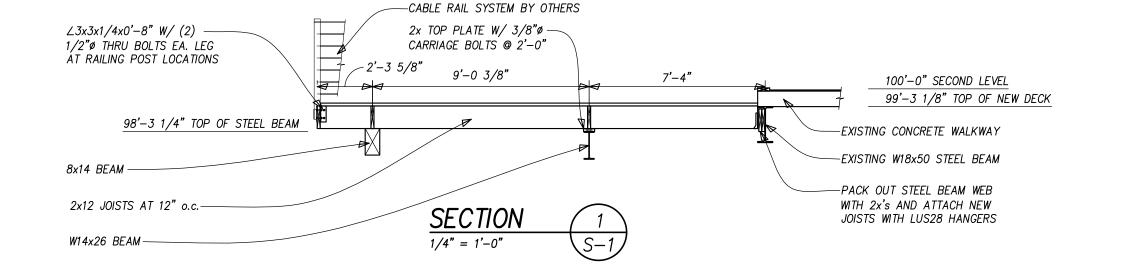
- a. The structural drawings illustrate the completed structure with all elements in their final positions, properly supported and braced.
- b. The Contractor, in the proper sequence, shall provide proper shoring and bracing as may be required during construction to achieve the final completed structure.
- c. The Contractor shall submit a shoring plan for approval prior to construction and all shoring shall be inspected and approved by Engineer prior to demolition.

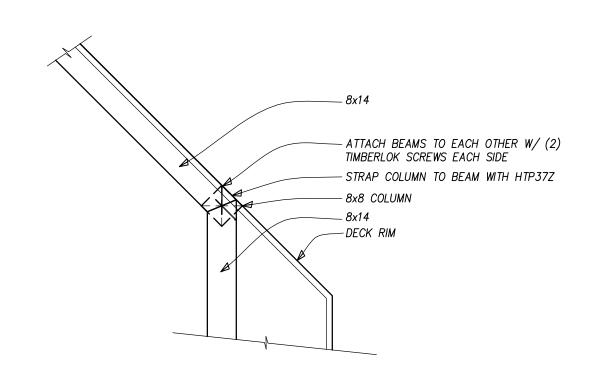
### MASONRY

- a. Hollow load—bearing concrete masonry shall be normal weight units conforming to ASTM C90, Grade N. Minimum ultimate compressive strength (fm) = 1900 psi.
- b. Mortar shall be Type S consisting of a mixture of portland cement, hydrated lime and aggregate conforming to the proportion specifications of IBC Table 2103.7(1) or the property specifications of IBC Table 2103.7(2) and conforming to ASTM C270. Masonry cement shall not be used. Admixtures shall not be added for any reason unless approved by the Architect. Minimum 28-day cube strength = 1800 psi.
- c. Grout used in masonry walls and block cells shall be coarse grout conforming to proportion specifications of IBC Table 2103.10 or as defined by ATSM C476. Grout shall be placed by vibrating or puddling, if grout lifts exceed 4'-0" in height clean—out holes shall be provided in each grouted cell. Minimum cube strength = 2000 psi.
- d. Use continuous joint reinforcing in all masonry walls with a maximum vertical spacing of 16".
   e. Except for lintels, bond beam units shall be produced from standard vertically voided units with pre-cut knock-out cross walls.
- f. Reinforcing shall be grade 60. Stagger splices in the same block course by 4'-0" minimum and lap bars in all splices a minimum of 48 diameters. At corners and intersections, make horizontal bars continuous or provide matching corner bars. Provide dowels from the foundation to match size and location of all vertical reinforcing in walls
- g. Special Inspection is required for all structural masonry construction.









CONNECTION DETAIL 1

SHEET