

GOVERNING CODE: 2018 INTERNATIONAL BUILDING CODE (IBC) AND ALL LOCAL AMENDMENTS

1. RISK CATEGORY:	II, LOW HAZARD, STANDARD
2. ROOF LIVE LOADS:	
A. ROOF LIVE LOAD:	20 PSF
3. ROOF SNOW LOADS:	
A. GROUND SNOW LOAD (P_g):	104 PSF
B. FLAT ROOF SNOW LOAD (P_f):	96 PSF
C. SNOW EXPOSURE FACTOR (C_e):	1.2
D. SNOW LOAD IMPORTANCE FACTOR (I_s):	1.0
E. THERMAL FACTOR (C_t):	1.0
F. SLOPE FACTOR (C_s):	1.0
G. SNOW DRIFTING AND UNBALANCED LOADS:	IN ACCORDANCE WITH ASCE 7-16 AND AS DEPICTED ON THE ROOF PLANS.
4. FLOOR LIVE LOADS:	
A. RESIDENTIAL:	40 PSF
B. BALCONIES:	60 PSF
5. ROOF AND FLOOR DEAD LOADS:	
A. ROOF:	16 PSF
B. FLOOR:	16 PSF
C. BALCONY/DECK:	16 PSF
6. WIND LOADS:	
A. BASIC WIND SPEED, 3-SECOND GUST (V):	115 MPH
B. ALLOWABLE STRESS DESIGN WIND SPEED (V):	89.1 MPH
C. INTERNAL PRESSURE COEFFICIENT ($G C_{pi}$):	+/-0.18
D. WIND EXPOSURE:	C
7. COMPONENTS AND CLADDING DESIGN WIND PRESSURES (PSF) (ASCE 7-16):	
A. WALL ZONE (25 SQ FT)	
a. 5' WITHIN 8'-0" OF CORNERS:	+27.3 PSF, -35.4 PSF
b. 4' INTERNALLY:	+27.3 PSF, -28.4 PSF
B. ROOF ZONE (25 SQ FT)	
a. 3' WITHIN 4'-0" OF CORNERS:	+23.1 PSF, -68.3 PSF
b. 3' OVERHANGS WITHIN 4'-0" OF CORNERS AND RIDGES:	+20.5 PSF, -12.0 PSF
c. 2' WITHIN 4'-0" OF EDGES AND RIDGES:	+28.5 PSF, -73.8 PSF
d. 2' OVERHANGS:	+20.5 PSF, -12.0 PSF
e. 1' INTERNALLY:	+19.9 PSF, -52.1 PSF
C. NOTE: ALL COMPONENT AND CLADDING PRESSURES ARE BASIC PRESSURES. TO CONVERT TO ALLOWABLE STRESS DESIGN PRESSURES, MULTIPLY ULTIMATE PRESSURES BY 0.6.	
8. SEISMIC LOADS:	
A. SEISMIC IMPORTANCE FACTOR (I_e):	1.0
B. SPECTRAL RESPONSE ACCELERATION COEFFICIENTS	
a. SHORT PERIOD	S_s : 59.7% G
b. ONE SECOND	S_1 : 10.3% G
C. SOILS SITE CLASS:	C
D. SEISMIC DESIGN CATEGORY:	C
E. BASIC SEISMIC-FORCE-RESISTING SYSTEM(S):	BEARING WALL SYSTEMS; LIGHT FRAME WOOD WALLS WITH STRUCTURAL WOOD SHEAR PANELS
F. DESIGN BASE SHEAR:	12.3 KIPS
G. SEISMIC RESPONSE COEFFICIENT(S) (C_d):	0.043
H. RESPONSE MODIFICATION FACTOR(S) (R):	6.5
I. ANALYSIS PROCEDURE:	EQUIVALENT LATERAL FORCE PROCEDURE

1. REFER TO SOILS REPORT NO. 20-1961 BY NORTHWEST COLORADO CONSULTANTS, INC. (NWCOC), DATED JULY 21, 2022.
2. SOIL CONDITIONS SHALL BE VERIFIED BY THE GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF FORMWORK OR CONCRETE. IF DIFFERENT SOIL CONDITIONS EXIST THE STRUCTURAL ENGINEER SHALL BE NOTIFIED TO RE-EVALUATE THE FOUNDATION DESIGN AT ADDITIONAL EXPENSE TO THE OWNER.
3. SLOPE FINAL GRADES DOWN AND AWAY FROM FOUNDATION WALLS A MINIMUM OF 12 INCHES IN FIRST 10 FEET PER IBC.
4. FOOTINGS:
 - A. FOOTINGS, SELECTED BY THE OWNER SHALL BEAR ON THE NATURAL, UNDISTURBED SOILS, OR APPROVED COMPACTED STRUCTURAL FILL.
 - B. EXTERIOR FOOTINGS SHALL BEAR BELOW FROST DEPTH; MINIMUM FROST DEPTH SHALL BE 4'-0" BELOW ADJACENT EXTERIOR FINISHED GRADE.
 - C. DESIGN OF FOOTINGS IS BASED ON:
 - a. MAXIMUM ALLOWABLE BEARING PRESSURE: 3000 PSF
5. MICROPILES:
 - A. MICROPILES SHALL BE DESIGNED AND INSTALLED IN ACCORDANCE WITH IBC SECTION 1810.4.10. MINIMUM CAPACITY PER MICROPILE SHALL SATISFY THE TENSION AND COMPRESSION LOAD REQUIREMENTS SHOWN ON THE STRUCTURAL DRAWINGS.
 - B. PILE LAYOUT, QUANTITY, ORIENTATION, AND DESIGN LOADS ARE INDICATED ON THE STRUCTURAL DRAWINGS.
 - C. MICROPILE TYPE, LENGTH, CASING SIZE, HOLE DIAMETER, BOND LENGTH, BOND STRESS, GROUT STRENGTH, GROUT TESTING, CORROSION PROTECTION SYSTEM SHALL BE DETERMINED BY A QUALIFIED CONTRACTOR BASED ON THE CRITERIA PRESENTED IN THE REFERENCED SOILS REPORT AND ON THE DESIGN LOADS INDICATED ON THE DRAWINGS.
 - D. ALL-THREAD REINFORCEMENT SHALL BE A MINIMUM OF #8 THREADED BARS DESIGNATED ASTM A615, GRADE 75 KSI WITH A MINIMUM 1/2" EMBEDMENT INTO FOUNDATIONS AS INDICATED ON THE DRAWINGS.
 - E. COMPRESSION AND TENSION LOAD TESTING SHALL BE IN ACCORDANCE WITH ASTM D1143 AND ASTM D3889. RESPECTIVELY. TEST REPORTS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER UPON COMPLETION FOR REVIEW.
 - F. SEE BORING LOGS IN THE SOILS REPORT FOR INDICATED BEDROCK DEPTH.
6. G. THE SOILS ENGINEER SHALL OBSERVE DURING MICROPILE INSTALLATION TO CONFIRM THAT THE PROPER INSTALLATION PROCEDURES ARE USED.
7. EARTH RETAINING STRUCTURES:
 - A. EARTH EQUIVALENT FLUID LATERAL PRESSURE:
 - a. WALLS RESTRAINED AT TOP (AT REST): 40 PCF
 - b. CANTILEVERED WALLS (ACTIVE): 40 PCF
 - c. PASSIVE RESISTING: 250 PCF
 - D. COEFFICIENT OF SLIDING FRICTION: 0.4

CONCRETE DESIGN IS BASED ON THE AMERICAN CONCRETE INSTITUTE' BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE 'STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" (ACI 301).

2. STATUTORY CONCRETE SHALL HAVE THE FOLLOWING PROPERTIES (NORMAL WEIGHT CONCRETE UNLESS NOTED OTHERWISE):

A. CEMENT TYPE:	III			
B. MAXIMUM AGGREGATE SIZE:	3/4"			
C. MINIMUM 28 DAY COMPRESSIVE STRENGTH (F'_c) AS FOLLOWS:				
a. FOOTINGS:	3,500 PSI	5/8" (MAX)	ENTRAINED AIR %	SUMP
b. STEM WALLS:	3,500 PSI	0.52	1.5% ($\pm 1.5\%$)	5 INCHES ($\pm 1"$)
c. GRADE BEAMS:	4,000 PSI	0.52	5.0% ($\pm 1.5\%$)	4 INCHES ($\pm 1"$)
d. WALLS:	4,000 PSI	0.50	5.0% ($\pm 1.5\%$)	4 INCHES ($\pm 1"$)
e. FORMED STRUCTURAL SLAB:	4,000 PSI	0.48	3.0% ($\pm 1.5\%$)	4 INCHES ($\pm 1"$)
f. EXTERIOR SLABS ON GROUND:	3,500 PSI	0.45	6.0% ($\pm 1.5\%$)	4 INCHES ($\pm 1"$)

(EXCLUDES FLATWORK)

3. REINFORCING STEEL SHALL BE FABRICATED AND PLACED IN ACCORDANCE WITH ACI 315 'DETAILS AND DETAILING OF CONCRETE REINFORCEMENT'.

4. WHEN COLD WEATHER CONDITIONS EXIST, PLACE AND CURE CONCRETE IN ACCORDANCE WITH ACI 306.

5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.

6. DEFORMED REINFORCING SHALL BE DOMESTIC NEW BILLET STEEL CONFORMING TO ASTM A615, GRADE 60 INCLUDING STRUTUPS AND TIES, EXCEPT THAT REINFORCING WHICH IS REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A706.

7. EPOXY COATED REINFORCING BARS SHALL CONFORM TO ASTM A775.

8. ZINC COATED (GALVANIZED) REINFORCING BARS SHALL CONFORM TO ASTM A767.

9. UNLESS OTHERWISE NOTED ON THE STRUCTURAL DRAWINGS, LAP BARS 50 DIAMETERS (50BAR DIAMETER MINIMUM).

10. REINFORCING AT ALL ABUTTING CORNERS INCLUDING FOOTINGS) SHALL BE CONTINUOUS THROUGH OR AROUND ALL CORNERS AND INTERSECTIONS, OR USE MATCHING CORNER BARS OF EQUAL SIZE AND SPACING TO REINFORCING IN THE ABUTTING MEMBERS.

11. INSTALL (2) #5 BARS (MINIMUM) AROUND ALL SIDES OF ALL OPENINGS IN CONCRETE AND EXTEND 2'-8" PAST EDGES OF OPENINGS, UNLESS OTHERWISE NOTED.

12. IN CONTINUOUS MEMBERS, SPLICE TOP BARS AT MID-SPAN BETWEEN SUPPORTS AND SPLICE BOTTOM BARS OVER SUPPORTS.

13. FORM INTERMITTENT SHEAR KEYS AT ALL CONSTRUCTION JOINTS AND AS SHOWN ON THE STRUCTURAL DRAWINGS.

14. UNLESS OTHERWISE NOTED ON THE DRAWINGS, MINIMUM CONCRETE COVER OVER REINFORCING SHALL BE AS FOLLOWS:

A. UNFORMED SURFACE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:	3"
B. FORMED SURFACE EXPOSED TO EARTH OR WEATHER:	2"
#6 THROUGH #18 BARS	2"
#5 BAR, W31 OR D31 WIRE, AND SMALLER	1-1/2"
C. FORMED SURFACE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
a. SLABS, WALLS, JOISTS: #11 BARS AND SMALLER	3/4"

D. BEAMS AND COLUMNS:

a. PRIMARY REINFORCEMENT	1-1/2"
b. STIRRUPS, TIES, SPIRALS	1-1/2"

15. INSTALL CHAIRS, BOLSTERS, ADDITIONAL REINFORCEMENT, AND ACCESSORIES NECESSARY TO SUPPORT REINFORCEMENT AT POSITION SHOWN ON DRAWINGS. SUPPORT OF REINFORCEMENT ON WOOD, BRICK, OR OTHER UNREINFORCED MATERIALS SHALL BE PROHIBITED.

16. KEEP REINFORCEMENT CLEAN AND FREE OF DIRT AND OIL. FORMS PRIOR TO PLACING REINFORCEMENT.

17. FIBER ADMIXTURE SHALL BE 10% VIRGIN POLYPROPYLENE, FIBRILLATED FIBERS, TYPE III 4.1.3, PERFORMANCE LEVEL ONE, PER ASTM C1116.

18. PROPERLY PLACE, ACCURATELY POSITION AND MAINTAIN SECURELY IN PLACE ALL EMBEDDED ITEMS PRIOR TO AND DURING CONCRETE PLACEMENT.

19. AND/OR SOLID BARS FOR BEAM AND COLUMN-BEARING PLATES SHALL BE PLACED WITH SETTING TEMPLATES.

20. UNLESS OTHERWISE SHOWN IN THE ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFERS AT ALL COLUMN, WALL, SLAB OR BEAM EDGES THAT ARE EXPOSED TO VIEW IN THE FINISHED STRUCTURE.

1. STRUCTURAL STEEL SHALL BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360) AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (AISC 303) BY THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).
2. ALL STRUCTURAL STEEL SHALL CONFORM TO THE ASTM STANDARDS AND GRADES INDICATED BELOW, UNLESS NOTED OTHERWISE ON THE DRAWINGS OR DETAILS.
 - A. STRUCTURAL STEEL WIDE FLANGE BEAMS AND WTS: ASTM A992, 50 KSI YIELD
 - B. ROLLED STEEL FLOOR PLATES: ASTM A786, COMMERCIAL GRADE
 - C. OTHER ROLLED SHAPES, INCLUDING PLATES, CHANNELS, AND ANGLES: ASTM A336, 36 KSI YIELD
 - D. HOLLOW STRUCTURAL SECTION (HSS) RECTANGULAR SHAPES: ASTM A500, GRADE B, 46 KSI YIELD
 - E. HSS ROUND SHAPES: ASTM A500, GRADE B, 46 KSI YIELD
 - F. PIPE SHAPES: ASTM A53, GRADE B, 35 KSI YIELD
 - G. ADJUSTABLE PIPE COLUMNS:
 - a. 3" DIAMETER 11 GAUGE, SHALL BE CERTIFIED BY THE MANUFACTURER FOR A SAFE LOAD CAPACITY OF 13,500 LBS AT 7'-6".
 - b. 3" DIAMETER "HEAVY DUTY" SCHEDULE 40 SHALL BE CERTIFIED FOR A SAFE LOAD CAPACITY OF 28,000 LBS AT 7'-6".
3. UNLESS OTHERWISE NOTED, FRAMED BEAM CONNECTIONS SHALL BE BEARING-TYPE WITH 3/4" DIAM. SNUG TIGHT, ASTM A325 BOLTS, DETAILED IN CONFORMANCE WITH THE STRUCTURAL DRAWINGS AND THE "STEEL CONSTRUCTION MANUAL" BY THE AISC, 16TH EDITION. INSTALL BOLTS IN ACCORDANCE WITH AISC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS".
4. ALL BEAMS SHALL HAVE FULL DEPTH WEB STIFFENERS EACH SIDE OF WEBS ABOVE AND BELOW COLUMNS (1/4" PLATE OR AS NOTED).
5. ANCHOR RODS SHALL CONFORM TO ASTM F1554, GRADE 55 AS NOTED ON THE STRUCTURAL DRAWINGS WITH WELDABILITY SUPPLEMENT S1.
6. HEADED ANCHOR STUDS (HAS) SHALL CONFORM TO ASTM A108 AND SHALL BE CONNECTED TO STRUCTURAL STEEL WITH ELEVATION APPROVED BY THE STUD MANUFACTURER ACCORDING TO THE STUD MANUFACTURER'S RECOMMENDATIONS.
7. WELDING SHALL BE DONE BY A CERTIFIED WELDER IN ACCORDANCE WITH THE AISC DOCUMENTS LISTED ABOVE, AND THE AMERICAN WELDING SOCIETY (AWS) D1.1, STRUCTURAL WELDING CODE, AND THE RECOMMENDATIONS FOR USE OF 300X ELECTRODES (MINOR) SPECIFIED ABOVE. MINIMUM WELD SHALL BE 3/16" FILL BY LENGTH OF CONTACT EDGE.
8. ALL POST-INSTALLED ANCHORS SHALL HAVE CURRENT INTERNATIONAL CODE COUNCIL EVALUATION REPORT (ICC-ES) REPORTS AND SHALL BE INSTALLED IN ACCORDANCE WITH THE MANUFACTURER'S REQUIREMENTS.
9. EXPANSION ANCHORS SHALL BE APPROVED "WEDGE" TYPE UNLESS SPECIFICALLY NOTED TO BE "SLEEVE" TYPE AS NOTED ON THE STRUCTURAL DRAWINGS.
10. ALL SHALL BE AND SHALL BE APPROVED EPOXY OR SIMILAR ADHESIVE TYPE AS APPROPRIATE FOR INSTALLATION IN SOLID AND NON-SOLID BASE MATERIALS.
11. GROUT BENEATH COLUMN BASE AND BEAM BRACING PLATES SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 5,000 PSI AND SHALL BE NON-SHRINK, NON-METALLIC, AND TESTED IN ACCORDANCE WITH ASTM C1107.

1. DESIGN IS BASED ON ANSI/AIA & NDS "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH SUPPLEMENT: DESIGN VALUES FOR WOOD CONSTRUCTION" AND ANSI/AIA & SDPW "SPECIAL DESIGN PROVISIONS FOR WIND AND SEISMIC."
2. 2X FRAMING LUMBER SHALL BE S4S HEM-FIR NO. 2 AND BETTER UNLESS NOTED OTHERWISE.
3. ALL LUMBER SHALL BE 19% OR LESS MAXIMUM MOISTURE CONTENT, UNLESS NOTED OTHERWISE.
4. SOLID TIMBER BEAMS AND POSTS SHALL BE KILN DRIED DOUGLAS FIR-LARCH NO. 1.
5. 2X STUD BEARING WALLS SHALL BE 2X6 @ 16" (10") HEM-FIR NO. 2 OR BETTER.
6. TOP AND BOTTOM PLATES SHALL BE DOUG-FIR NO. 2 OR BETTER.
7. USE OF WOOD BEARING WALLS SHOWN ON DRAWINGS WITH LATERALLY UNRESTRICTED HEIGHTS IN EXCESS OF THAT SHOWN IN IBC 2308.1 HAS BEEN JUSTIFIED BY ANTHEM'S ANALYSIS.
8. FASTENERS FOR USE WITH TREATED WOOD SHALL COMPLY WITH IBC SECTION 2304.10.5.
9. WOOD IN CONTACT WITH CONCRETE SHALL BE PRESERVED WITH 10% LACROS FIR-LARCH OR SOUTHERN YELLOW PINE.
10. PRESERVATIVE TREATED WOOD SHALL BE TREATED IN ACCORDANCE WITH AWP-A 1 AND AWP-A 14.
11. CONVENTIONAL LIGHT FRAMING SHALL COMPLY WITH IBC SECTION 2308.
12. MINIMUM NAILING SHALL BE PROVIDED AS SPECIFIED IN IBC TABLE 2308.1 "FASTENING SCHEDULE".
13. TOP FRAMING OR REQUIRED, SHALL BE PROVIDED WITH 10% LACROS FIR-LARCH OR SOUTHERN YELLOW PINE APPROVED CONNECTORS AND INSTALLED PER THE HANGER SCHEDULE. NOTE THAT HEAVY-DUTY HANGERS AND SKAGGED HANGERS MAY NOT BE STOCKED LOCALLY AND REQUIRE SPECIAL ORDER FROM THE FACTORY.
14. GLUE WOOD NAILER PLATES TO STEEL BEAMS AND ATTACH WITH EITHER 1/2"x9" BOLTS @ 32" O.C. OR 1/4"x9" POWDER DRIVEN NAILS @ 16" O.C. STAGGERED. WIDTH OF NAILER PLATE SHALL MATCH BEAM WIDTH + 1/8" MIN (1/4" MAX) OVERHANG EACH SIDE.
15. LEAD HOLES FOR LAG SCREWS SHALL BE 40%-70% OF THE SHANK DIAMETER AT THE THREADED SECTION AND EQUAL TO THE SHANK DIAMETER AT THE UNTHREADED SECTION PER NDS SECTION 12.1.4.
16. CONNECTOR BOLTS AND LAG SCREWS SHALL CONFORM TO ANSIME/A 318.2.1 AND ASTM A329 GRADE 1.
17. NAILS AND SPIKES SHALL CONFORM TO ASTM F1667.
18. WOOD SCREWS SHALL CONFORM TO ANSIME/A 318.6.1.

1. INSTALL SOLID BLOCKING BETWEEN JOISTS UNDER JAMB STUDS OR OPENINGS.
2. COLUMNS MUST HAVE A CONTINUOUS LOAD PATH TO FOUNDATION.
3. UNLESS NOTED OTHERWISE, INSTALL TWO LENGTHS OF SOLID BLOCKING X JOIST DEPTH X 12 INCHES LONG IN FLOOR FRAMING UNDER COLUMNS.
4. BUILT-UP STUD COLUMNS SHALL CONSIST OF 2X4, 2X6, OR 2X8 STUDS WITH NUMBER OF LAMINATIONS NOTED ON PLAN AND EACH LAMINATION SHALL BE NAILLED TOGETHER WITH (2) ROWS OF 12D GUN NAILS (0.131"X 3 1/4") @ 6" FULL HEIGHT OF COLUMN. DO NOT SPLIC LAMINATIONS.
5. ALL BEAMS AND TRUSSES SHALL BE BRACED AGAINST ROTATION AT POINTS OF BEARING.
6. UNLESS NOTED OTHERWISE, PROVIDE CHORDS OF GABLE END TRUSSES SHALL BE ANCHORED TO WALL PLATE WITH FRAMING ANCHORS AT 4'-0" SPACING AND LATERALLY BRACED TO ROOF FRAMING AT 8'-0" SPACING.
7. PROVIDE CONTINUOUS WALL STUDS EACH SIDE OF OPENINGS EQUAL TO ONE-HALF OR GREATER THE NUMBER OF STUDS INTERRUPTED BY OPENING UNLESS NOTED OTHERWISE.
8. ALL WALL STUDS SHALL BE CONTINUOUS FROM FLOOR TO FLOOR OR FROM FLOOR TO ROOF.
9. PROVIDE SOLID BLOCKING OR RIM JOISTS AT ALL JOIST SUPPORTS AND JOIST ENDS.
10. SOLE PLATE AT ALL PERIMETER WALLS AND AT DESIGNATED SHEAR WALLS SHALL BE NAILLED WITH (4) 0.131"X3" NAILS AT 16" MINIMUM.
11. ALL ROOF RAFTERS, JOISTS, TRUSSES, BEAMS SHALL BE ANCHORED TO SUPPORTS WITH METAL FRAMING ANCHORS.

1. PLYWOOD AND ORIENTED STRAND BOARD (OSB) FLOOR, ROOF, AND WALL SHEATHING SHALL BE APA RATED WITH STAMP INCLUDING APA TRADEMARK AND PANEL SPAN RATING.
 - A. MINIMUM FLOOR SHEATHING: SEE PLAN NOTES
 - B. MINIMUM ROOF SHEATHING: SEE PLAN NOTES
 - C. MINIMUM WALL SHEATHING: SEE PLAN NOTES
2. SHEATH ALL EXTERIOR WALLS. SHEATH INTERIOR WALLS AS SHOWN ON THE DRAWINGS.
3. SHEATHING SHALL BE CONTINUOUS FROM BOTTOM PLATE TO TOP PLATE. CUT IN "L" AND "T" SHAPES AROUND OPENINGS. LAP SHEATHING OVER RIM JOISTS A MINIMUM 4' ALL FLOORS TO THE UPPER AND LOWER STUD WALLS TOGETHER.
4. MINIMUM HEIGHT OF SHEATHING PANELS SHALL BE 16" TO ENSURE THAT PLATES ARE TIED TO STUDS
5. MACHINE APPLIED NAILING (E. GUN NAILING): THE USE OF MACHINE APPLIED NAILING IS SUBJECT TO SATISFACTORY JOINT JOINTING BY THE APPROVAL BY THE PROJECT STRUCTURAL ENGINEER. THE APPROVAL IS SUBJECT TO CONTINUED SATISFACTORY PERFORMANCE. IF NAIL HEADS PENETRATING THE OUTER PLY MORE THAN WOULD BE NORMAL FOR A HAND HAMMER OR IF MINIMUM ALLOWABLE EDGE DISTANCES ARE NOT MAINTAINED THE PERFORMANCE WILL BE DEEMED UNSATISFACTORY.

1. SERIES ROOF AND FLOOR STLS SHALL BE MANUFACTURED BY WEYERHAEUSER TRUS JOIST WITH STRUCTURAL WOOD FLANGES AND WEBS DESIGNED FOR STRUCTURAL CAPACITIES AND DESIGN PROVISIONS ACCORDING TO ASTM D 5665. SUBSTITUTION OF EQUIVALENTS BY OTHER MANUFACTURER IS ACCEPTED WITH ENGINEER APPROVAL.

2. SERIES ROOF AND FLOOR JOISTS SHALL BE INSULATED PER MANUFACTURER'S RECOMMENDATIONS. DO NOT CUT OR NOTCH CHORDS OR MAIN MEMBER. HOLES IN WEBS SHALL NOT EXCEED MANUFACTURER'S PUBLISHED LIMIT CRITERIA.

3. MEMBERS NOTED AS LVL (LAMINATED VENEER LUMBER) ON PLAN SHALL BE 1 3/4" WIDE X DEPTH INDICATED, PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

A. $F_t = 2600$ PSI $F_c = 285$ PSI $F_{t||} = 2510$ PSI $F_{c||} = 1750$ PSI $E = 2000$ KSI

4. MEMBERS NOTED AS PL (PARALLEL STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

A. BEAMS: $F_t = 2400$ PSI $F_c = 230$ PSI $F_{t||} = 2300$ PSI $F_{c||} = 1750$ PSI $E = 2000$ KSI

B. COLUMNS: $F_t = 2000$ PSI $F_c = 190$ PSI $F_{t||} = 2000$ PSI $F_{c||} = 1425$ PSI $E = 1800$ KSI

5. MEMBERS NOTED AS LSL (LAMINATED STRAND LUMBER) ON PLAN SHALL BE PLANT-FABRICATED AND HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN VALUES:

A. $E = 1121"$

B. 1 3/4" $F_t = 1700$ PSI $F_c = 400$ PSI $F_{t||} = 1400$ PSI $F_{c||} = 680$ PSI $E = 1300$ KSI

C. 1 3/4" $F_t = 2325$ PSI $F_c = 310$ PSI $F_{t||} = 2325$ PSI $F_{c||} = 800$ PSI $E = 1550$ KSI

6. BRIDGING AND BLOCKING SHALL BE INSTALLED ACCORDING TO THE FABRICATOR'S REQUIREMENTS.

1. MANUFACTURE AND INSTALLATION OF METAL PLATED WOOD TRUSSES SHALL COMPLY WITH ANSHPRI 1 "NATIONAL DESIGN STANDARD FOR METAL-PLATE CONNECTED WOOD TRUSS CONSTRUCTION," BCS (BUILDING COMPONENT SAFETY INFORMATION) GUIDE TO GOOD PRACTICE FOR HANDLING, INSTALLING & BRACING OF METAL PLATE CONNECTED WOOD TRUSSES AND DSD-88 "RECOMMENDED DESIGN SPECIFICATION FOR TEMPORARY BRACING OF METAL PLATE CONNECTED WOOD TRUSSES."
2. PRE-ENGINEERED, PREFABRICATED TRUSSES SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE STATE OF COLORADO, AND SHALL COMPLY WITH CODE REQUIREMENTS.
3. TRUSSES SHALL BE DESIGNED TO SUPPORT THE FULL DEAD LOADS AND THE SUPERIMPOSED DESIGN LOADS NOTED ABOVE OR ON THE DRAWINGS.
4. STRESSES SHALL NOT EXCEED THOSE LISTED IN THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION WITH 2018 SPECIFICATION (AF&PA NDS-2018). NO INCREASES IN STRESS ARE ALLOWED FOR DURATION OF LOAD.
5. THE FABRICATOR SHALL DETERMINE TRUSS WEB ARRANGEMENTS AND MEMBER FORCES.
6. TRUSSES TO TRUSS CONNECTIONS SPECIFIED SHALL BE BY TRUSS SUPPLIER, UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.
7. TRUSS FABRICATOR SHALL SPECIFY ALL FLOOR AND ROOF TRUSS BRACING AND BRIDGING.
8. CALCULATIONS AND SHOP DRAWINGS, INCLUDING MEMBER SIZES, LUMBER SPECIES AND GRADES, AND SUBSTANTIATING DATA FOR JOINT CAPACITIES, SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW PRIOR TO FABRICATION.

DESIGN ALL ROOF TRUSSES TO CLEAR SPAN TO BEARING WALLS AND SUPPORT BEAMS AS INDICATED ON PLAN.

DESIGN LOADS ARE AS FOLLOWS:	
DEAD LOAD BOTTOM CHORD:	6 PSF
DEAD LOAD TOP CHORD:	10 PSF
SNOW LOAD TOP CHORD:	88 PSF (FOR ADDITIONAL DRIFT LOAD, SEE LOADING PLANS)

LIMIT DEFLECTIONS TO:
SPAN/360 FOR LIVE LOAD
SPAN/240 FOR TOTAL LOAD

DESIGN TRUSSES FOR DRAG, POINT AND UNIFORM LOADS CALLED OUT ON PLANS.

ALL TRUSS TO TRUSS CONNECTIONS TO BE DESIGNED AND SUPPLIED BY THE TRUSS MANUFACTURER UNO. TRUSS FABRICATOR SHALL SUBMIT SHOP DRAWINGS AND CALCULATIONS STAMPED BY A REGISTERED PROFESSIONAL ENGINEER IN THE STATE IN WHICH THE PROJECT IS LOCATED TO ENGINEER OF RECORD (EOR) FOR REVIEW PRIOR TO FABRICATION.

REFER TO ARCHITECTURAL DRAWINGS FOR TRUSS HEEL HEIGHTS AND PROFILES.

DO NOT ALTER TRUSS LAYOUT WITHOUT EOR PRIOR APPROVAL.

BEARING DESIGN SHALL ASSUME DOUG-FIR WALL TOP PLATES (625 psi). USE MULTIPLE PLIES, BEARING BLOCKS, OR BEARING ENHANCERS TO ACCOMMODATE BEARING LENGTHS PER PLAN. THE USE OF END GRAIN BEARING IS NOT ACCEPTABLE, UNLESS SPECIFICALLY APPROVED BY THE EOR FOR THE PROJECT.

SI 2018- SPECIAL INSPECTION AND VERIFICATION OF SOILS				
SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	IBC REFERENCE
Y	1. VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.		X	1705.6
Y	2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.		X	1705.6
Y	3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.		X	1705.6
Y	4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESS DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	X		1705.6
Y	5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.		X	1705.6

SI 2018 - REQUIRED SPECIAL INSPECTIONS AND TESTS OF CONCRETE CONSTRUCTION					
SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCE STANDARD	IBC REFERENCE
Y	1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT		X	ACI 318 CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1906.4
	2. REINFORCING BAR WELDING:				
Y	a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706		X	AWS D1.4; ACI 318: 26.6.4	
Y	b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16"; AND		X	AWS D1.4; ACI 318: 26.6.4	
Y	c. INSPECT ALL OTHER WELDS	X		AWS D1.4; ACI 318: 26.6.4	
Y	3. INSPECT ANCHORS CAST IN CONCRETE		X	ACI 318: 17.8.2	
	4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS				
Y	a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS	X		ACI 318: 17.8.2.4	
Y	b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.		X	ACI 318: 17.8.2	
Y	5. VERIFY USE OF REQUIRED DESIGN MIX		X	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
Y	6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	X		ASTM C172, ASTM C31; ACI 318: 26.4, 26.12	1908.10
Y	7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES	X		ACI 318: 26.5	1908.6-1908.8
Y	8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES		X	ACI 318: 26.5.3-26.5.5	1908.9
	9. INSPECT PRESTRESSED CONCRETE FOR:				
Y	a. APPLICATION OF PRESTRESSING FORCES; AND	X		ACI 318: 26.10	
Y	b. GROUTING OF BONDED PRESTRESSING TENDONS	X		ACI 318: 26.10	
Y	10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS		X	ACI 318: CH. 26.9	
Y	11. VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS		X	ACI 318: 28.11.2	
Y	12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED		X	ACI 318: 26.11.1.2(b)	
	13. WELDING OF REINFORCING BARS				
Y	a. INSPECTION OF WELDING AND QUALIFICATIONS OF SPECIAL INSPECTORS SHALL BE IN ACCORDANCE WITH AWS D.1.4 FOR SPECIAL INSPECTION AND AWS D1.4 FOR SPECIAL INSPECTOR QUALIFICATION			AWS D1.4	1705.3.1
	14. MATERIAL TESTS				
Y	a. IN THE ABSENCE OF SUFFICIENT DATA OR DOCUMENTATION PROVIDING EVIDENCE OF CONFORMANCE TO QUALITY STANDARDS FOR MATERIALS IN CHPT. 19 AND 20 OF ACI 318-14, TESTING SHALL BE DONE OF MATERIALS IN ACCORDANCE WITH THE APPROPRIATE STANDARDS AND CRITERIA FOR THE MATERIAL IN CHAPTERS 19 AND 20 OF ACI 318-14			ACI 318: CH 19, 20	1705.3.2

SI 2018 - SPECIAL INSPECTION OF WIND FORCE RESISTING SYSTEMS (REQUIRED WHEN Vasd IS 120MPH OR GREATER IN EXP. CAT. B OR WHEN Vasd IS 110MPH OR GREATER IN EXP. CAT. C OR D)				
SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	FREQUENCY OF INSPECTION CONTINUOUS SPECIAL INSPECTIONS	PERIODIC SPECIAL INSPECTIONS	REFERENCE FOR CRITERIA: IBC SECTION
	1. STRUCTURAL WOOD			
Y	a. FIELD GLUING OF ELEMENTS OF THE MAIN WINDFORCE-RESISTING SYSTEM	X		1705.11.1
Y	b. NAILING, BOLTING, ANCHORING AND OTHER FASTENING ELEMENTS OF THE MAINFORCE-RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, WOOD DIAPHRAGMS, DRAG STRUTS, BRACES AND HOLD-DOWNS, EXCEPT WHEN FASTENER SPACING IS MORE THAN 4 INCHES ON CENTER		X	1705.11.1
	2. COLD-FORMED STEEL LIGHT-FRAME CONSTRUCTION			
Y	a. WELDING OPERATIONS OF ELEMENTS OF THE MAINFORCE-RESISTING SYSTEM.		X	1705.11.2
Y	b. SCREW ATTACHMENT, BOLTING, ANCHORING AND OTHER FASTENING OF ELEMENTS OF MAIN WINDFORCE-RESISTING SYSTEM, INCLUDING SHEAR WALLS, BRACES, DIAPHRAGMS, COLLECTORS (DRAG STRUTS) AND HOLD-DOWNS EXCEPT WHEN SHEATHING IS GYPSUM BOARD OR FIBER BOARD OR IF SHEATHING IS WOOD STRUCTURAL PANEL OR STEEL SHEETS ON ONLY ONE SIDE OF THE SHEAR WALL, OR DIAPHRAGM ASSEMBLY AND THE FASTENER SPACING OF THE SHEATHING IS MORE THAN 4 INCHES ON CENTER		X	1705.11.2
	3. WIND-RESISTING COMPONENTS			
Y	a. ROOF COVERING, ROOF DECK AND ROOF FRAMING CONNECTIONS		X	1705.11.3
Y	b. EXTERIOR WALL COVERING AND WALL CONNECTIONS TO ROOF AND FLOOR DIAPHRAGM AND FRAMING		X	1705.11.3

SI 2018 - SPECIAL INSPECTION AND VERIFICATION OF MASONRY - LEVEL 2					
SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA	
		CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED	TMS 402	TMS 602
	MINIMUM VERIFICATION				
	1. PRIOR TO CONSTRUCTION, VERIFICATION OF COMPLIANCE OF SUBMITTALS	-	-	-	ART. 1.5
	2. PRIOR TO CONSTRUCTION, VERIFICATION OF Fm AND f _{rac} , EXCEPT WHERE SPECIFICALLY EXEMPTED BY CODE	-	-	-	ART. 1.4B
	3. DURING CONSTRUCTION, VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) WHEN SELF-CONSOLIDATING GROUT IS DELIVERED TO THE PROJECT SITE	-	-	-	ART. 1.5, 1.6.3
	INSPECTION TASK				
	1. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE				
Y	a. PROPORTIONS OF SITE-PREPARED MORTAR	-	X		ART. 2.1, 2.6A, 2.6C
Y	b. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	-	X		ART. 2.4B, 2.4H
Y	c. GRADE, TYPE AND SIZE OF REINFORCEMENT, CONNECTORS, ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X		ART. 3.4, 3.6A
Y	d. PRESTRESSING TECHNIQUE	-	X	-	ART. 3.6B
Y	e. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X	-	-	ART. 2.1C.1
Y	f. SAMPLE PANEL CONSTRUCTION	-	X	-	ART. 1.6D
	2. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE				
Y	a. GROUT SPACE IS CLEAN	-	X	-	ART. 3.2D, 3.2F
Y	b. PLACEMENT OF PRESTRESSING TENDONS AND ANCHORAGES	-	X	SEC. 10.8, 10.9	ART. 2.4, 3.6
Y	c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND ANCHOR BOLTS	-	X	SEC. 6.1, 6.3.1, 6.3.6,	ART. 3.2E, 3.4
Y	d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	-	X	-	ART. 2.6B, 2.4G.1b
	3. VERIFY COMPLIANCE OF THE FOLLOWING DURING CONSTRUCTION				
Y	a. MATERIALS AND PROCEDURES WITH THE APPROVED SUBMITTALS	-	X	-	ART. 1.5
Y	b. PLACEMENT OF MASONRY UNITS AND MORTAR JOINT CONSTRUCTION	-	X	-	ART. 3.3B
Y	c. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	-	X	-	ART. 3.3F
Y	d. TYPE, SIZE, AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION	-	X	SEC. 12.1(e), 6.2.1, 6.3.1	-
Y	e. WELDING OF REINFORCEMENT	X	-	SEC.6.16.12	-
Y	f. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F)	-	X	-	ART. 1.8C, 1.8D
Y	g. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE	X	-	-	ART. 3.6B
Y	h. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	-	-	ART. 3.5, 3.6C
Y	i. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X	-	-	ART. 3.3B.9, 3.3F.1b
Y	4. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS		X	-	ART. 1.4B.2 a.3, 1.4B.2 b.3, 1.4B.2 c.3, 1.4B.3, 1.4B.4

SI 2018 - SPECIAL INSPECTIONS AND TESTS OF CAST-IN-PLACE DEEP FOUNDATION ELEMENTS			
SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION
Y	1. INSPECT DRILLING OPERATIONS AND MAINTAIN COMPLETE AND ACCURATE RECORDS FOR EACH ELEMENT.	X	
Y	2. VERIFY PLACEMENT LOCATIONS AND PLUMBNESS, CONFIRM ELEMENT DIAMETERS, BELL DIAMETERS (IF APPLICABLE), LENGTHS, EMBEDMENT INTO ROCK (IF APPLICABLE), AND ADEQUATE END-BEARING STRATA CAPACITY, RECORD CONCRETE OR GROUT VOLUMES.	X	
Y	3. FOR CONCRETE ELEMENTS, PERFORM TESTS AND ADDITIONAL, SPECIAL INSPECTIONS IN ACCORDANCE WITH SECTION 1705.3.	X	
Y	4. DETERMINE CAPACITIES OF TEST ELEMENTS AND CONDUCT LOAD TESTS AS REQUIRED.	X	
Y	5. FOR SPECIALTY ELEMENTS, PERFORM ADDITIONAL INSPECTIONS AS DETERMINED BY THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE.		

NOTE: FINAL MICROPILE SPECIAL INSPECTION REPORT MUST BE SUBMITTED TO THE BUILDING OFFICIAL PRIOR TO INSPECTION OF THE GRADE BEAMS.

SPECIAL INSPECTION REQUIRED Y/N	VERIFICATION AND INSPECTION TASK	QC		QA	
		CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTIONS	CONTINUOUS SPECIAL INSPECTIONS	PERIODIC SPECIAL INSPECTIONS
	1. INSPECTION TASKS PRIOR TO WELDING - TABLE N5.4-1 / AWS D1.1				
Y	a. WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	X			X
Y	b. WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	X		X	
Y	c. MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	X		X	
Y	d. MATERIAL IDENTIFICATION (TYPE/GRADE)		X		X
Y	e. WELDER IDENTIFICATION SYSTEM		X		X
Y	f. FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)				
Y	f.1. JOINT PREPARATION, DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOF FACE, BEVEL), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION), BAKING TYPE AND FIT (IF APPLICABLE)		X		X
Y	g. FIT-UP OF CJP GROOVE WELDS OF HSS T-, Y- AND K-JOINTS WITHOUT BAKING (INCLUDING JOINT GEOMETRY)				
Y	g.1. JOINT PREPARATION, DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOF FACE, BEVEL), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION)	X			X
Y	h. CONFIGURATION AND FINISH OF ACCESS HOLES		X		X
Y	i. FIT UP OF FILLET WELDS				
Y	i.1. DIMENSIONS (ALIGNMENT, GAPS AND ROOT), CLEANLINESS (CONDITION OF STEEL SURFACES), TACKING (TACK WELD QUALITY AND LOCATION)		X		X
Y	j. CHECK WELDING EQUIPMENT		X		
	2. INSPECTION TASKS DURING WELDING - TABLE N5.4-2 / AWS D1.1				
Y	a. CONTROL AND HANDLING OF WELDING CONSUMABLES				
Y	a.1. PACKAGING, EXPOSURE CONTROL		X		X
Y	b. NO WELDING OVER CRACKED TACK WELDS		X		X
Y	c. ENVIRONMENTAL CONDITIONS				
Y	c.1. WIND SPEED WITHIN LIMITS, PRECIPITATION AND TEMPERATURE		X		X
Y	d. WPS FOLLOWED				
Y	d.1. SETTINGS ON WELDING EQUIPMENT, TRAVEL SPEED, SELECTED WELDING MATERIALS, SHIELDING GAS TYPE/FLOW RATE, PREHEAT APPLIED, INTERPASS TEMPERATURE MAINTAINED (MIN, MAX), PROPER POSITION (F,V,H,OH)		X		X
Y	e. WELDING TECHNIQUES				
Y	e.1. INTERPASS AND FINAL CLEANING, EACH PASS WITHIN PROFILE LIMITATION, EACH PASS MEETS QUALITY REQUIREMENTS		X		X
Y	f. PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	X		X	
	3. INSPECTION TASKS AFTER WELDING - TABLE N5.4-3 / AWS D1.1				
Y	a. WELDS CLEANED		X		X
Y	b. SIZE, LENGTH AND LOCATION OF WELDS	X		X	
Y	c. WELDS MEET VISUAL ACCEPTANCE CRITERIA				
Y	c.1. CRACK PROHIBITION, WELD/BASE-METAL FUSION, CRATER CROSS SECTION, WELD PROFILES, WELD SIZE, UNDERCUT, POROSITY	X		X	
Y	d. ARC STRIKES	X		X	
Y	e. K-AREA	X		X	
Y	f. WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES	X		X	
Y	g. BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	X		X	
Y	h. REPAIR ACTIVITIES	X		X	
Y	i. DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	X		X	
Y	j. NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR		X		X
	4. INSPECTION TASKS PRIOR TO BOLTING - TABLE 5.6-1				
Y	a. MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS			X	X
Y	b. FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS			X	X
Y	c. CORRECT FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)			X	X
Y	d. CORRECT BOLTING PROCEDURE SELECTED FOR JOINT DETAIL		X		X
Y	e. CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS		X		X
Y	f. PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	X			X
Y	g. PROTECTED STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS		X		X
	5. INSPECTION TASKS DURING BOLTING - TABLE 5.6-2				
Y	a. FASTENER ASSEMBLIES OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED), ARE POSITIONED AS REQUIRED		X		X
Y	b. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION		X		X
Y	c. FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING		X		X
Y	d. FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES		X		X
	6. INSPECTION TASKS AFTER BOLTING - TABLE 5.6-3				
Y	a. DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	X		X	

APPROVAL STAMPS:

**REVIEWED
FOR
CODE
COMPLIANCE**
12/27/2023

[illegible]

2	05.12.23	RCRBD CORRECTIONS NOTICE 1
1	09.09.22	ISSUED FOR PERMIT
No.	Date	Description

SUBMISSIONS & REVISIONS

OWNER

ARCHITECT

K | A | S | A

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GENERAL CONTRACTOR

CIVIL ENGINEER

LANDSCAPE ARCHITECT



M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO 80487
DRAWING TITLE

SPECIAL INSPECTION SCHEDULES

SEA



DATE:

DRAWN BY:

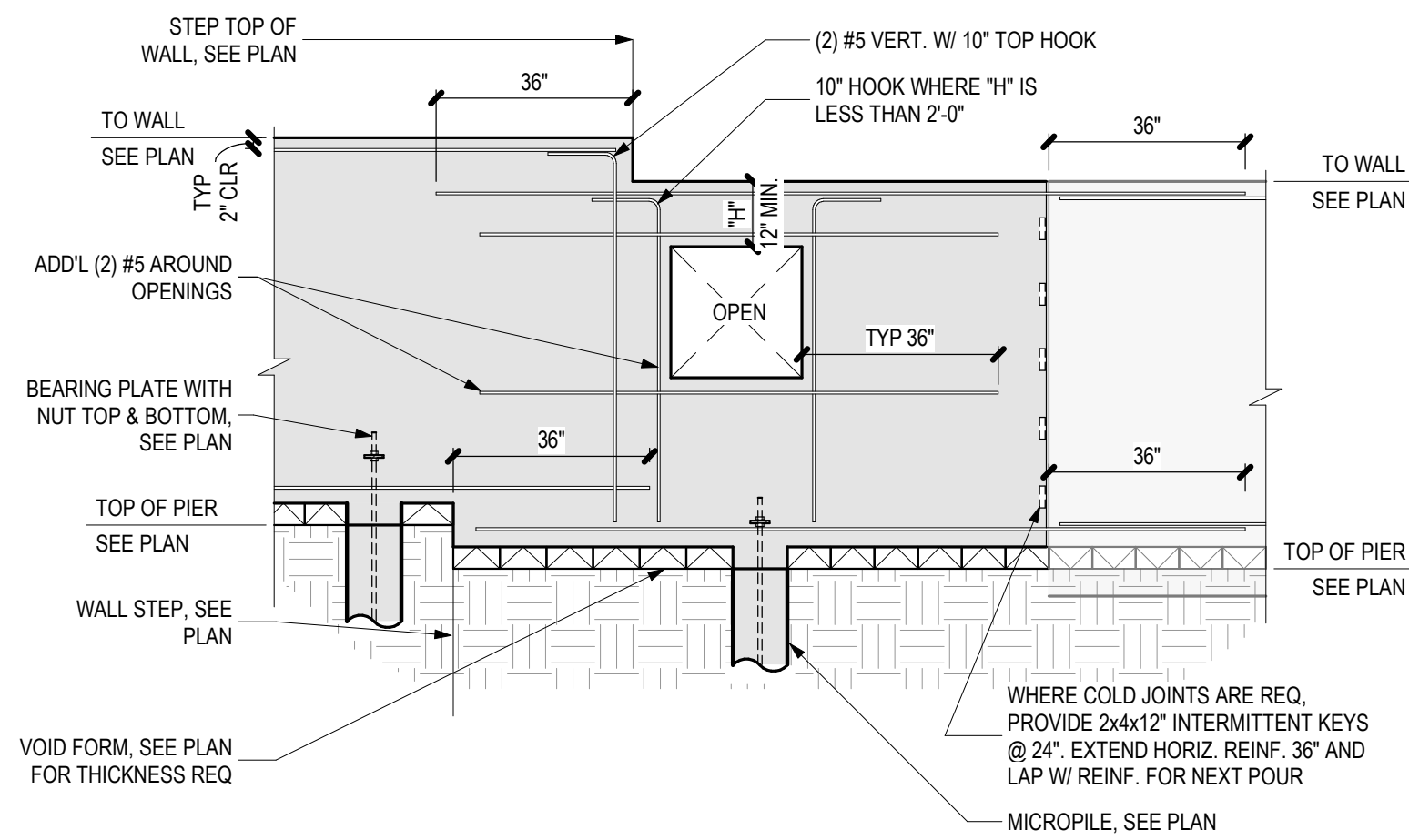
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PROJECT NO:	22-048
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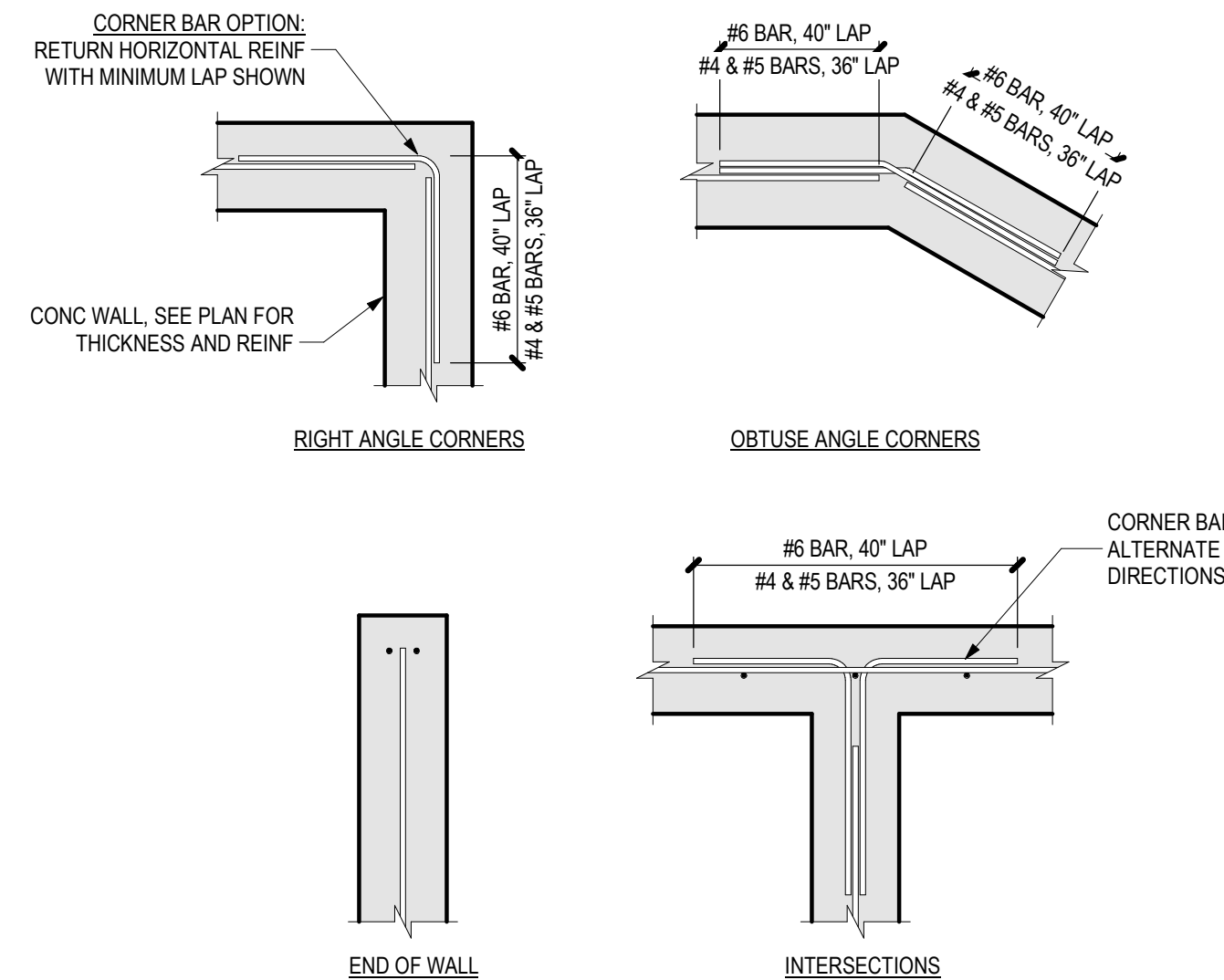
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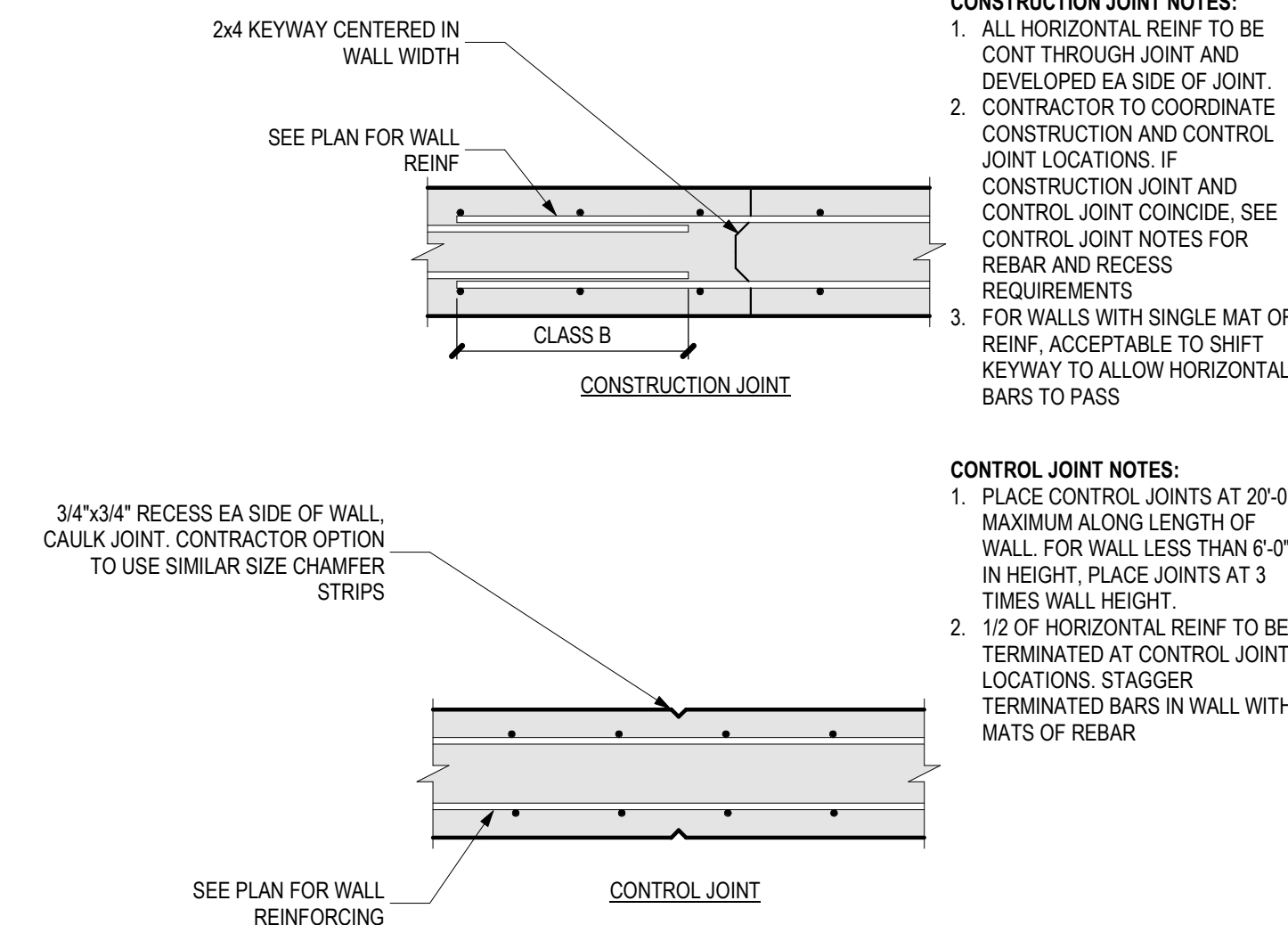
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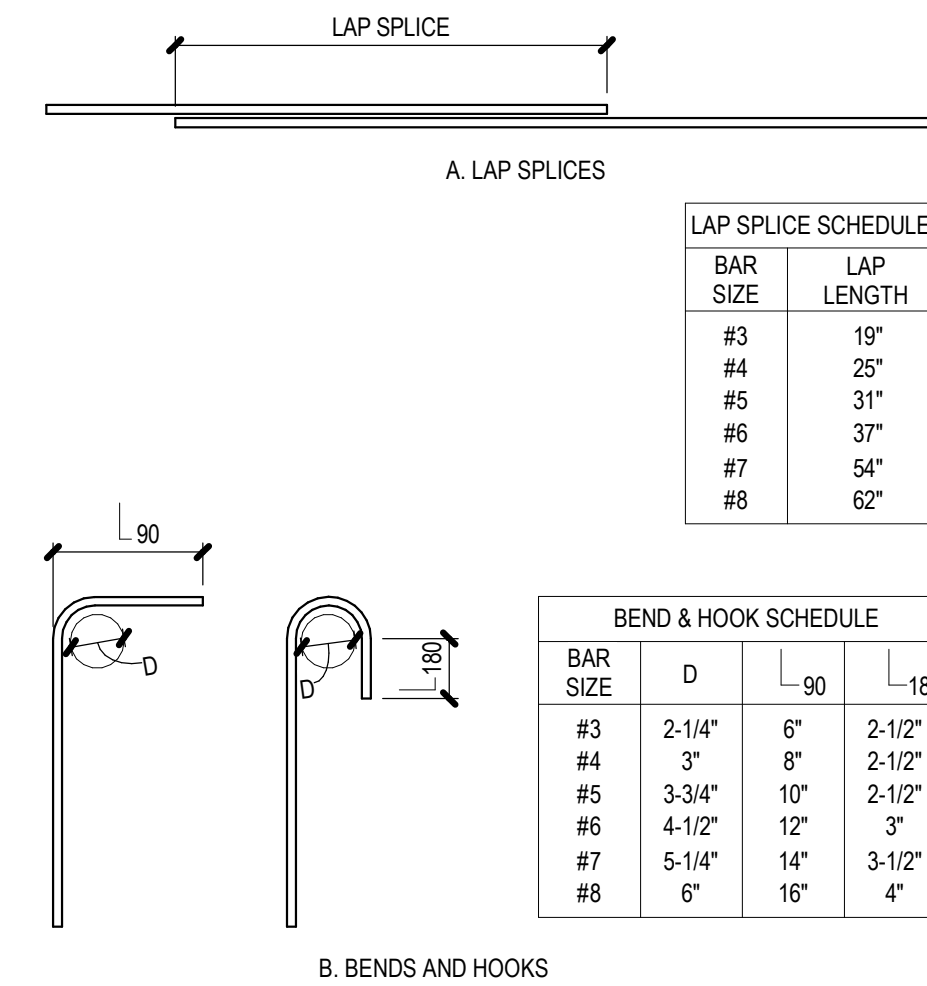
1 TYPICAL REINFORCING AT WALL OPENINGS AND STEPS - MICROPILES
NOT TO SCALE



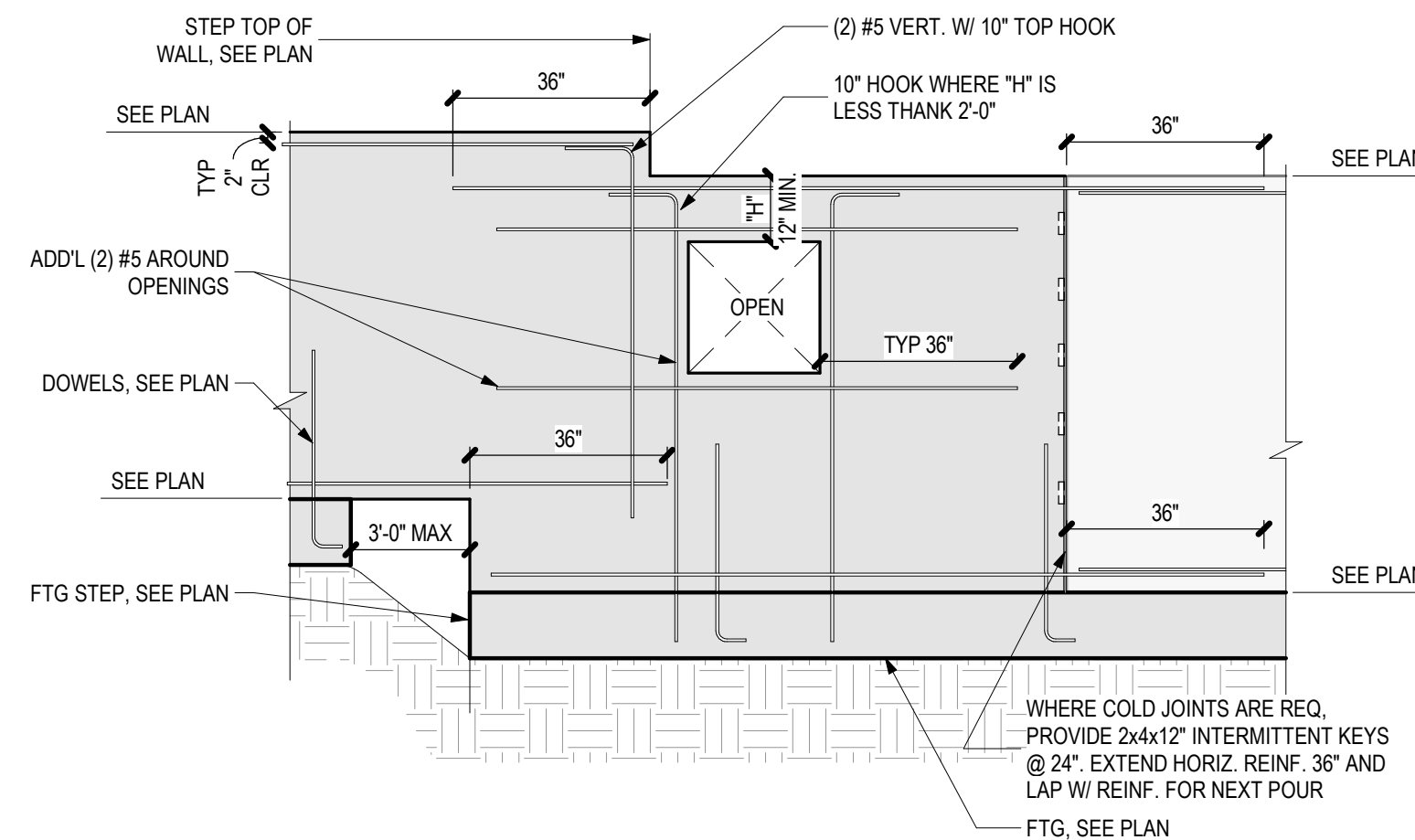
2 WALL AND GRADE BEAM CORNER REINFORCEMENT
NOT TO SCALE



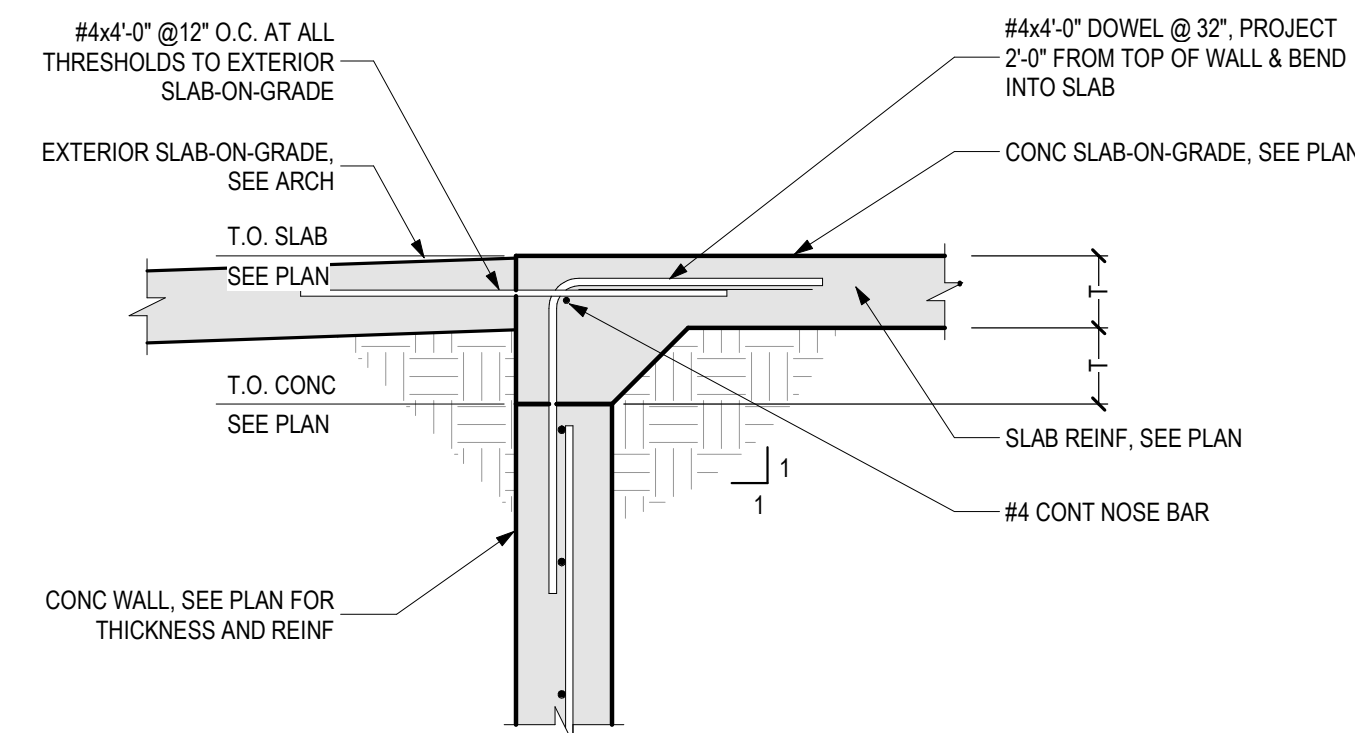
3 CONCRETE WALL JOINTS



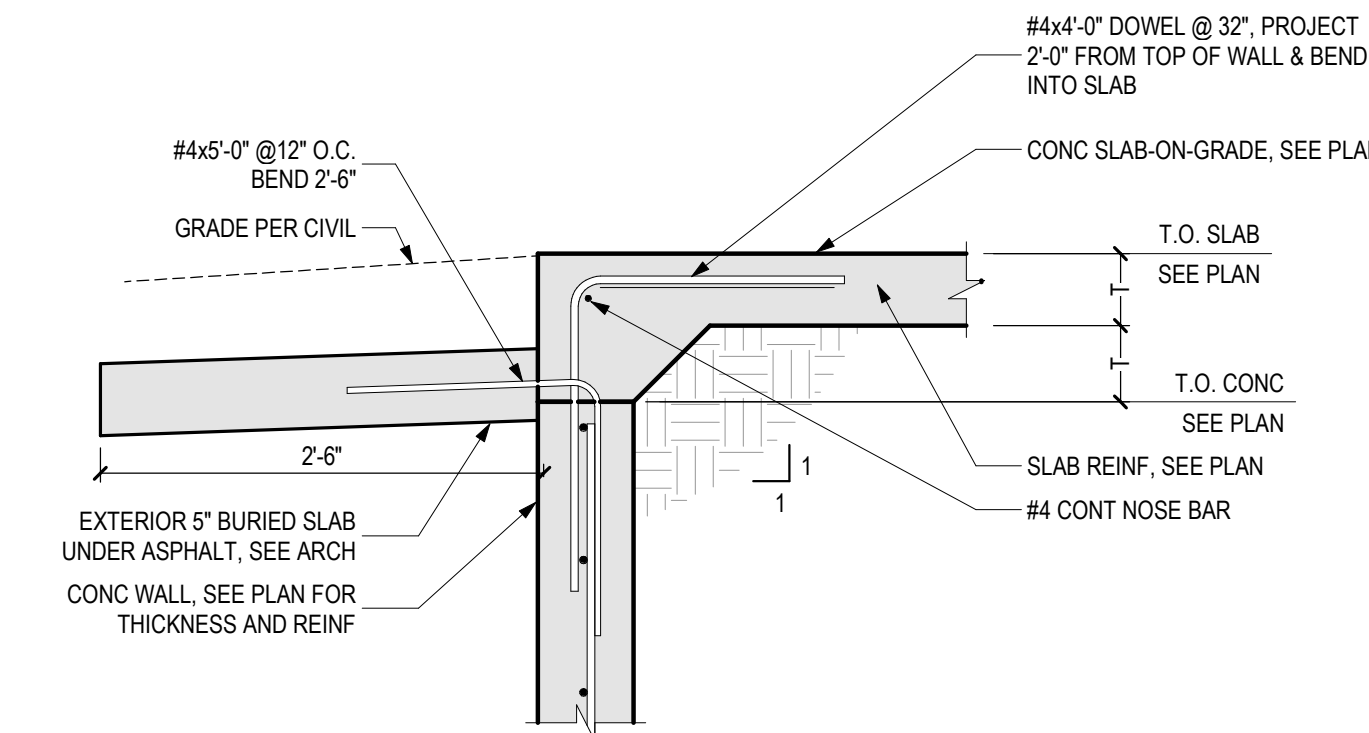
4 TYPICAL REBAR BEND & HOOK SCHEDULE
NOT TO SCALE



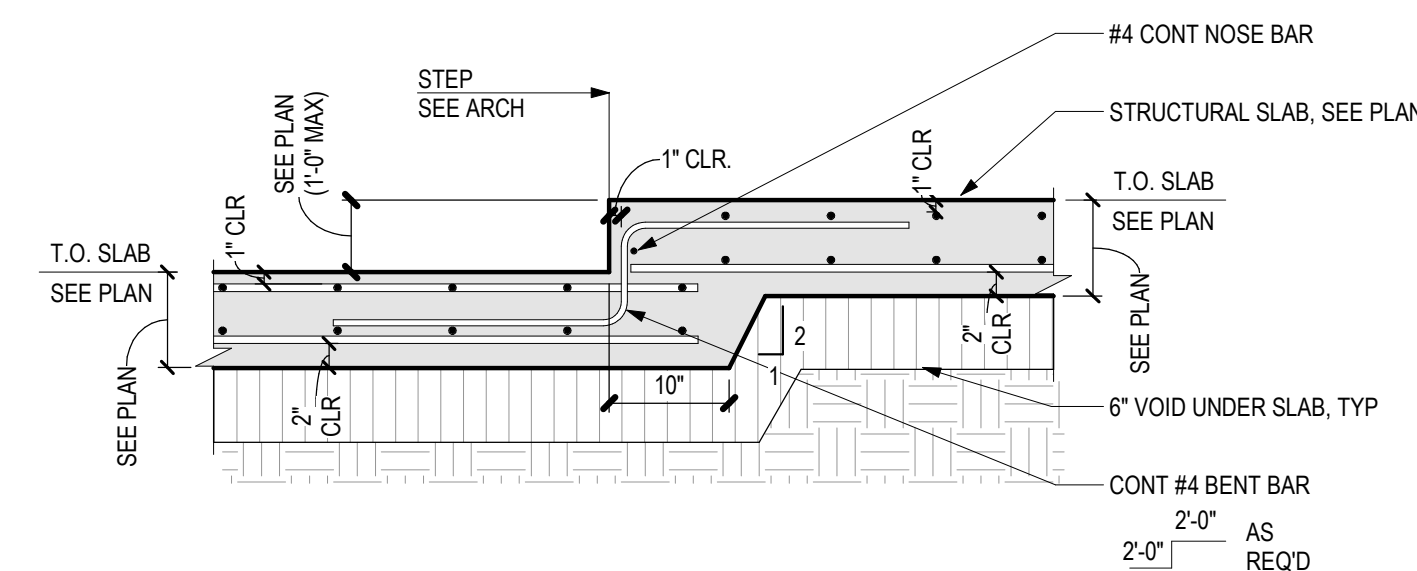
5 TYPICAL REINFORCING AT WALL OPENINGS AND STEPS
NOT TO SCALE



6 SLAB POUROVER AT DOOR
NOT TO SCALE



7 SLAB THRESHOLD AT DRIVEWAY
NOT TO SCALE



8 SLAB STEP WITH REINFORCING
3/4" = 1'-0"

APPROVAL STAMPS:

**REVIEWED
FOR
CODE
COMPLIANCE**
12/27/2023

[illegible]

No.	Date	Description
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OWNER

ARCHITECT

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GENERAL CONTRACTOR

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LANDSCAPE ARCHITECT



M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

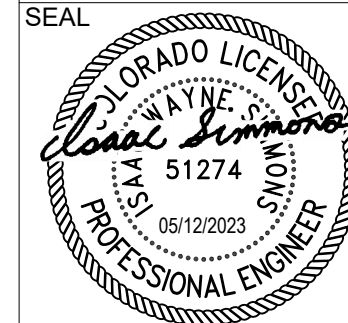
PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO 80487
DRAWING TITLE

TYPICAL DETAILS

SEA



DATE:

09/09/2022

DRAWN BY

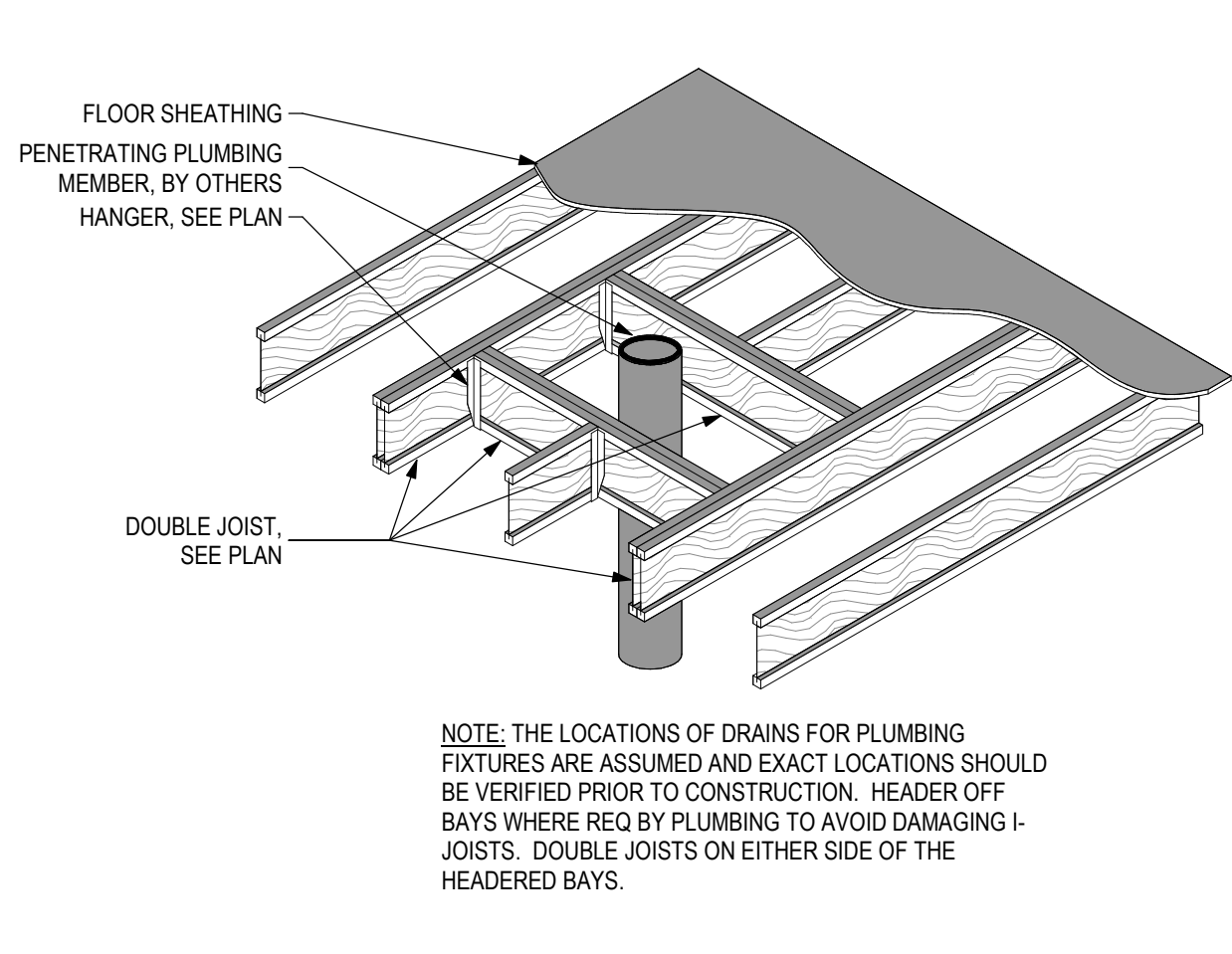
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PROJECT NO:	22-048
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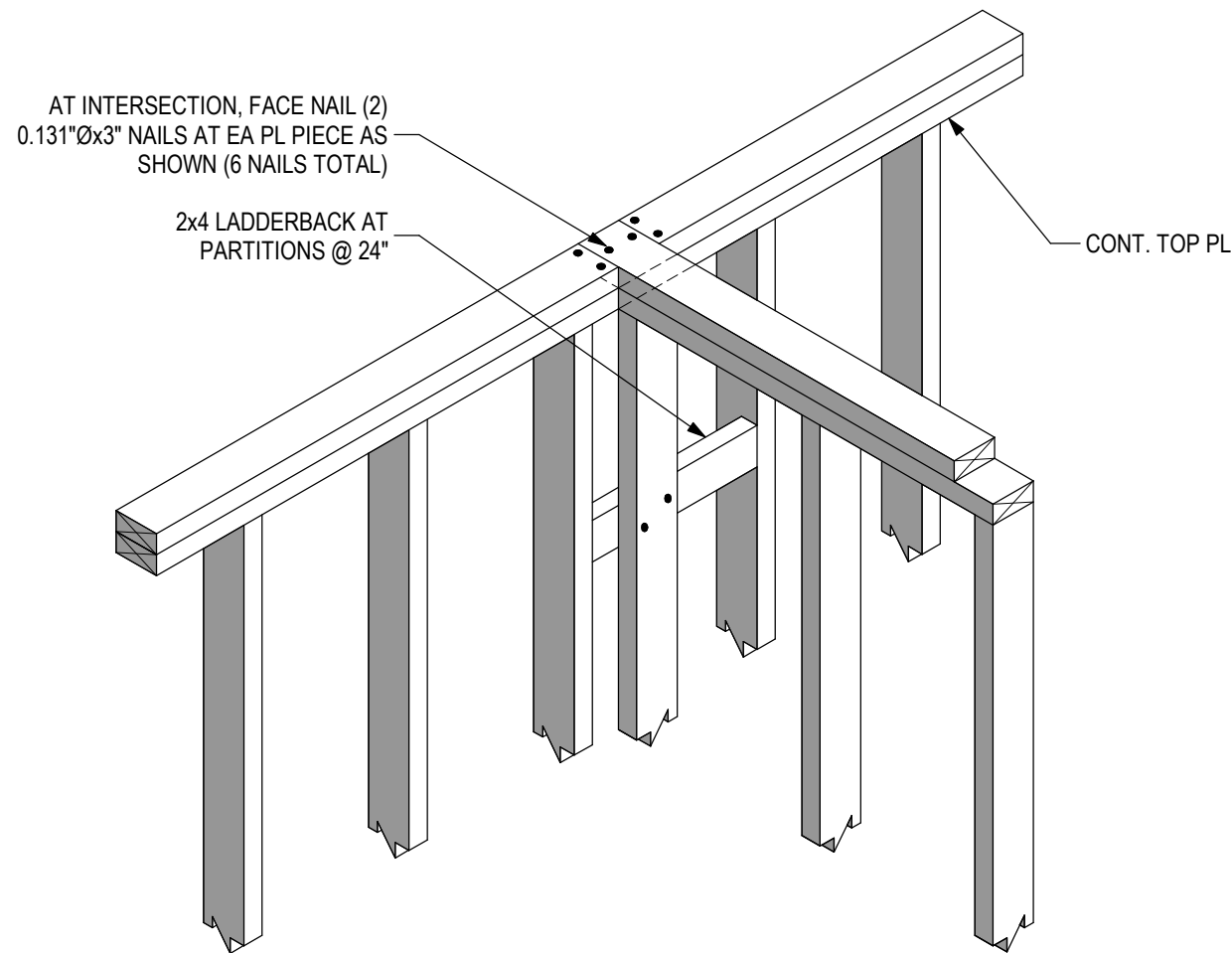
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S0030

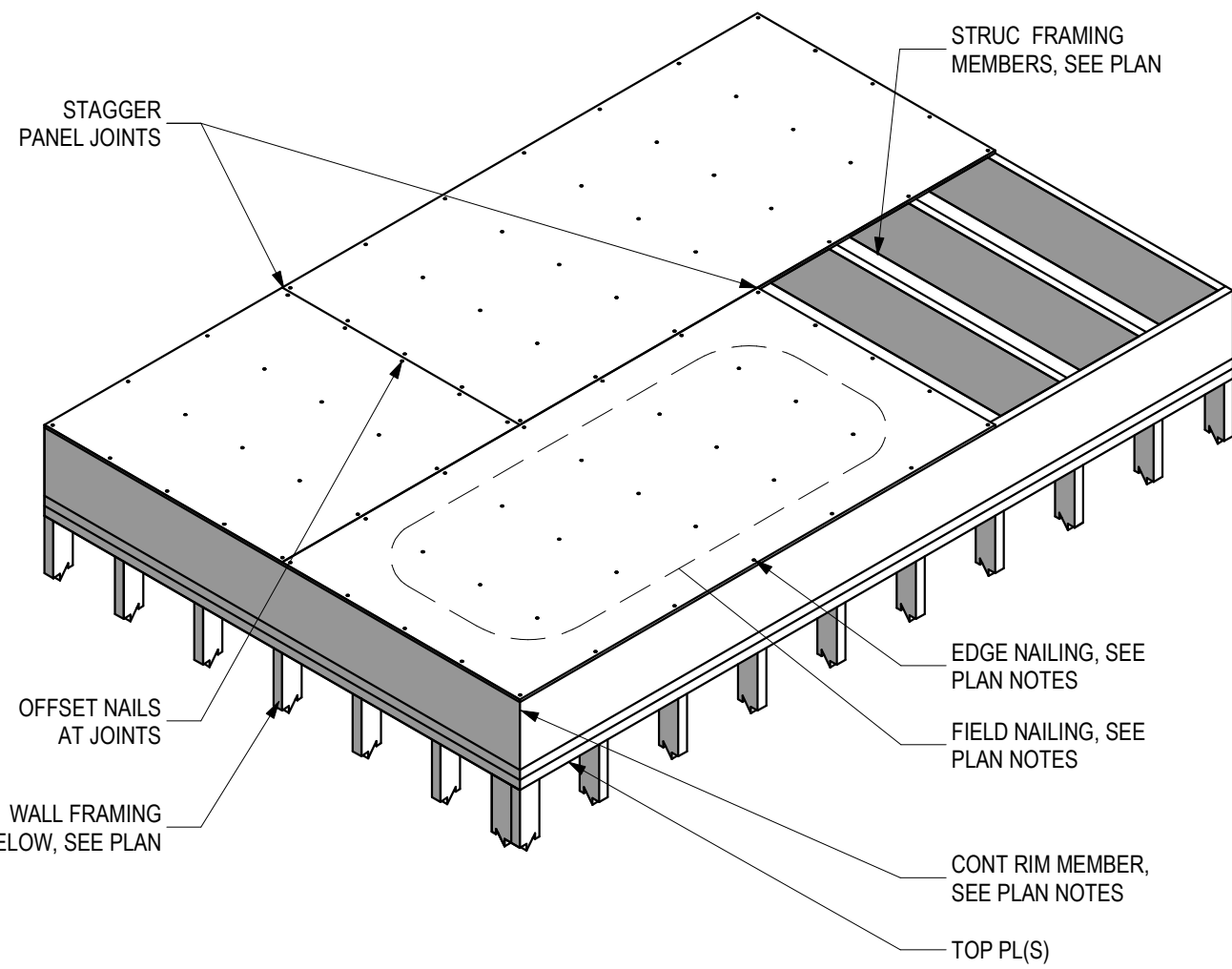
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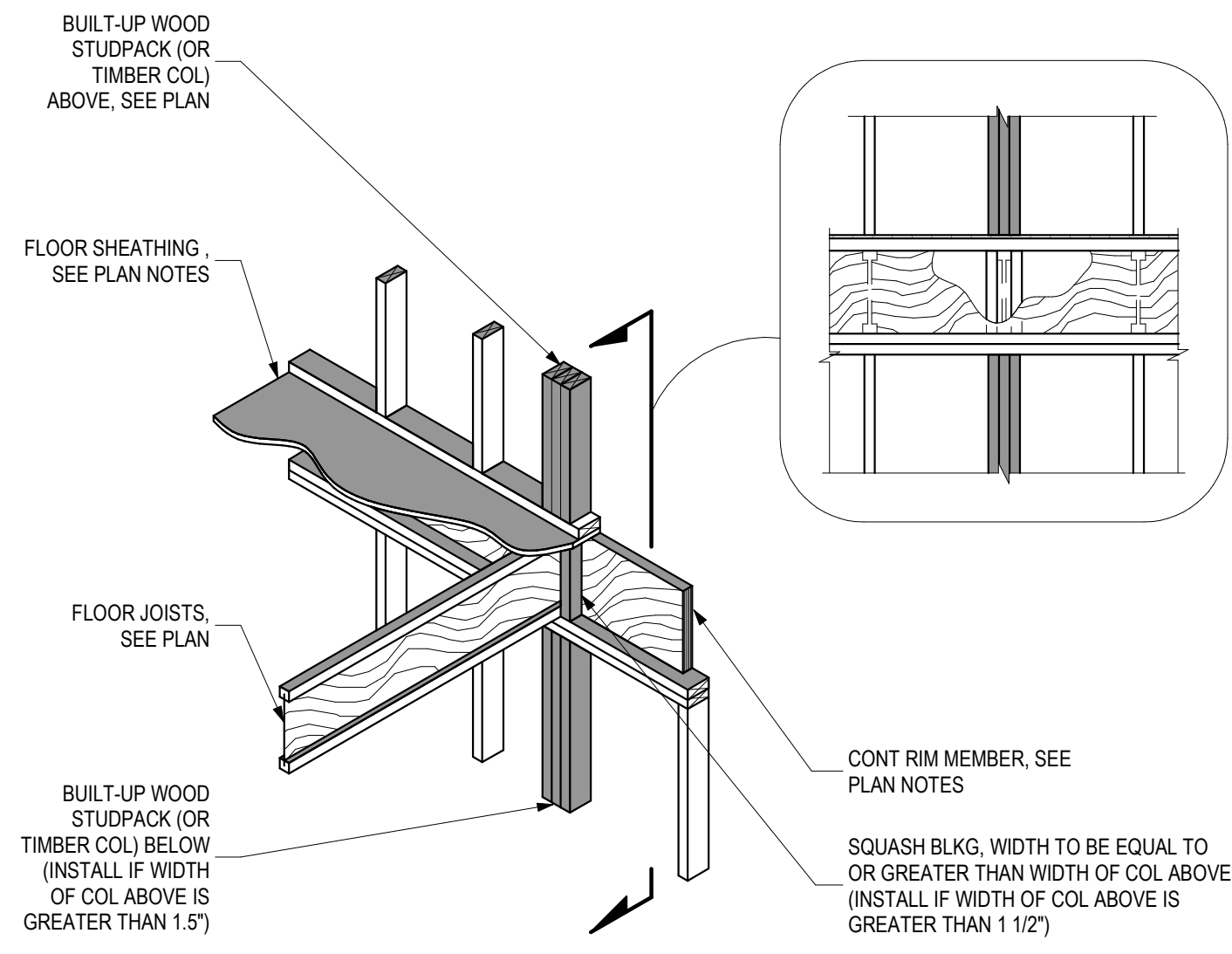
1 FLOOR PENETRATION
NOT TO SCALE



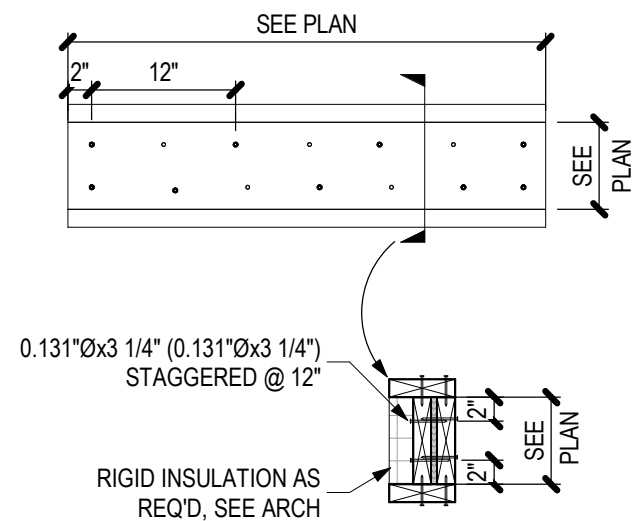
2 INTERSECTING WALL L FRAMING AT TOP PLATE
NOT TO SCALE



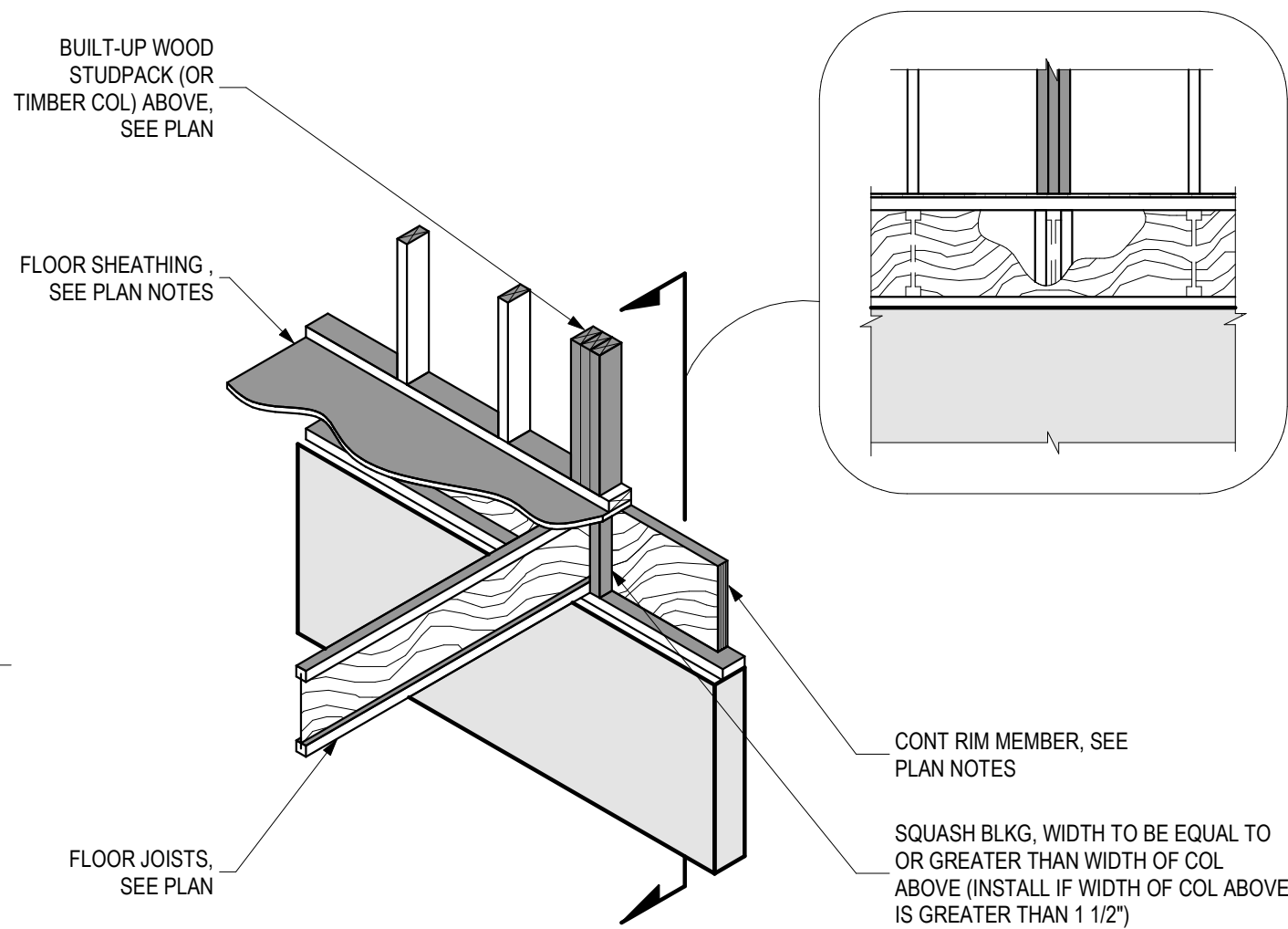
3 SUBFLOOR TO WOOD STRUCTURAL PANEL
NOT TO SCALE



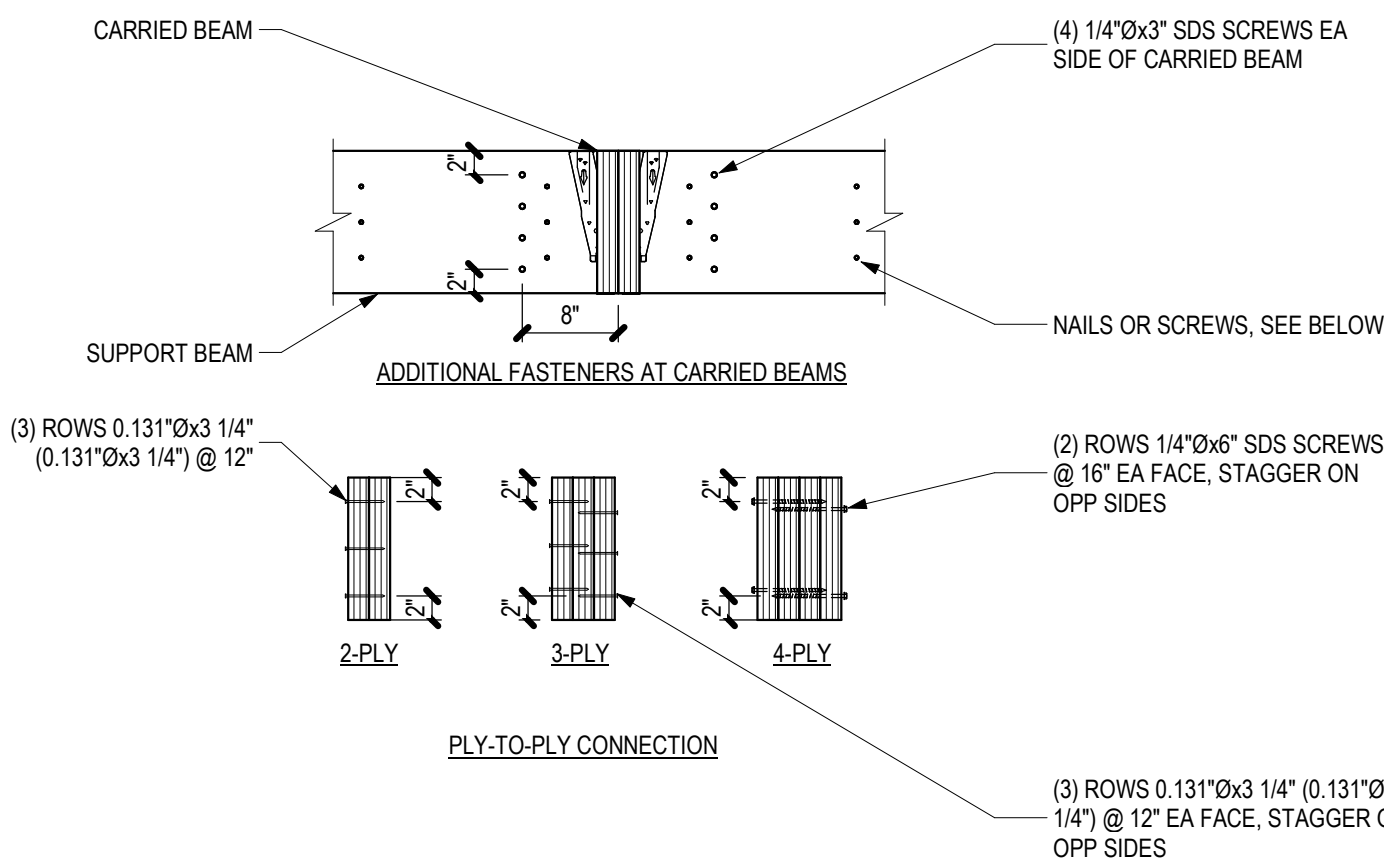
4 TYPICAL SQUASH BLOCK
NOT TO SCALE



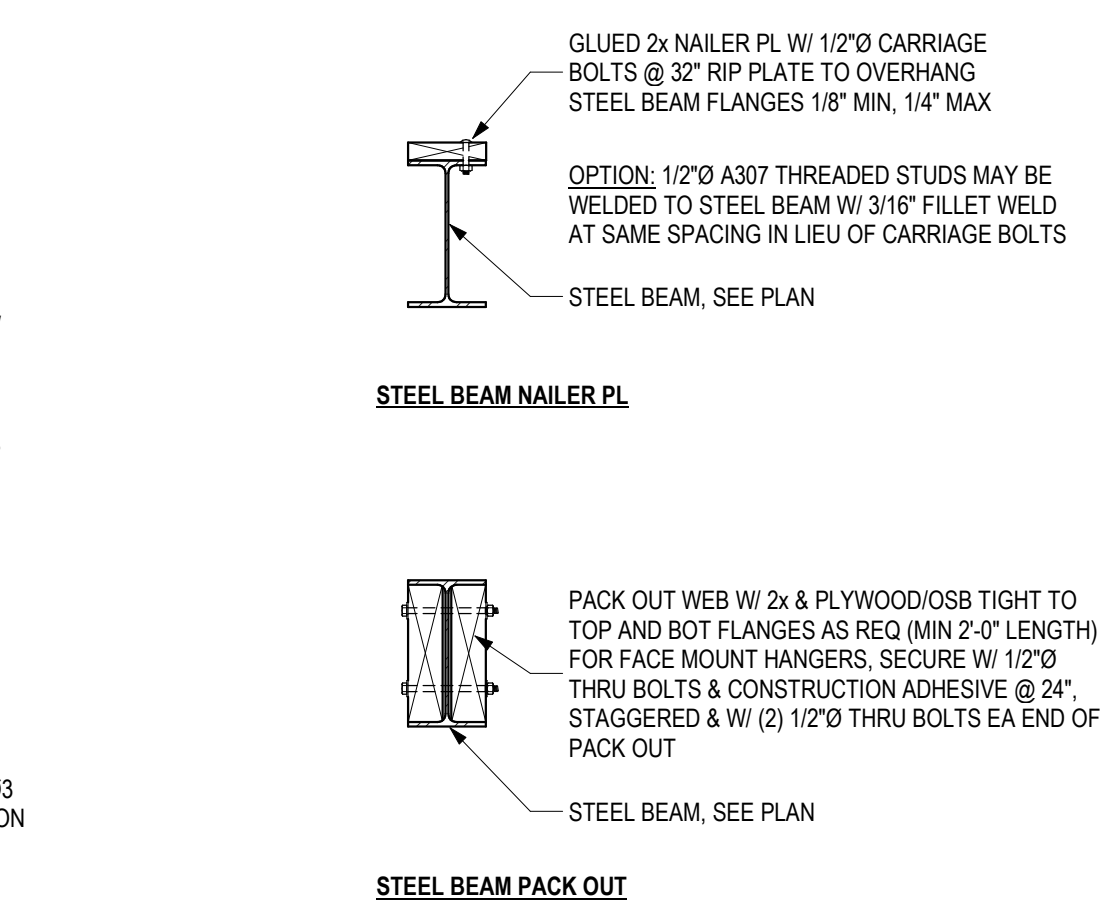
5 TYP HEADER CONSTRUCTION
NOT TO SCALE



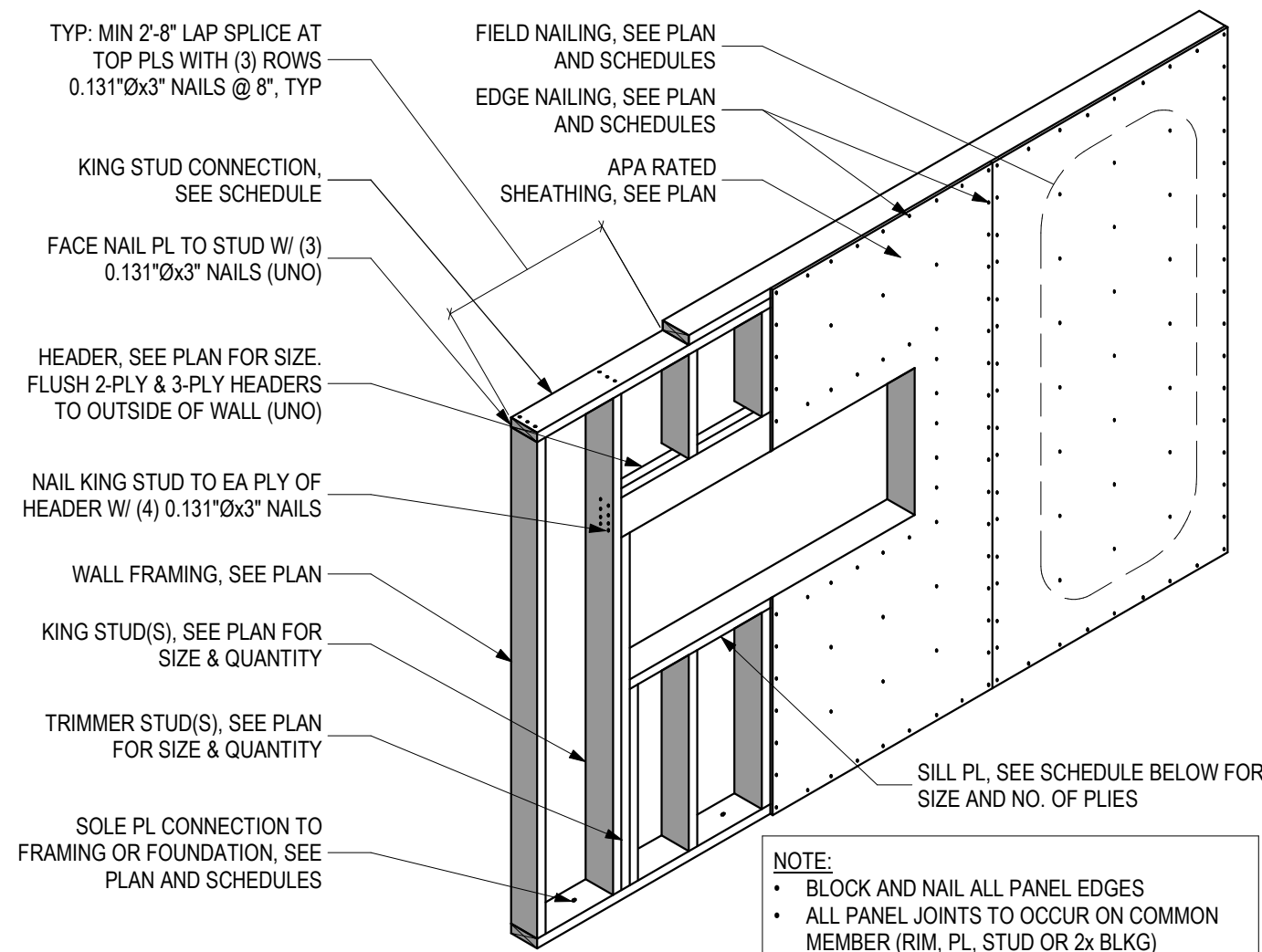
6 TYPICAL SQUASH BLOCK ON CONCRETE
NOT TO SCALE



7 MULTIPLE LVL CONNECTIONS
NOT TO SCALE

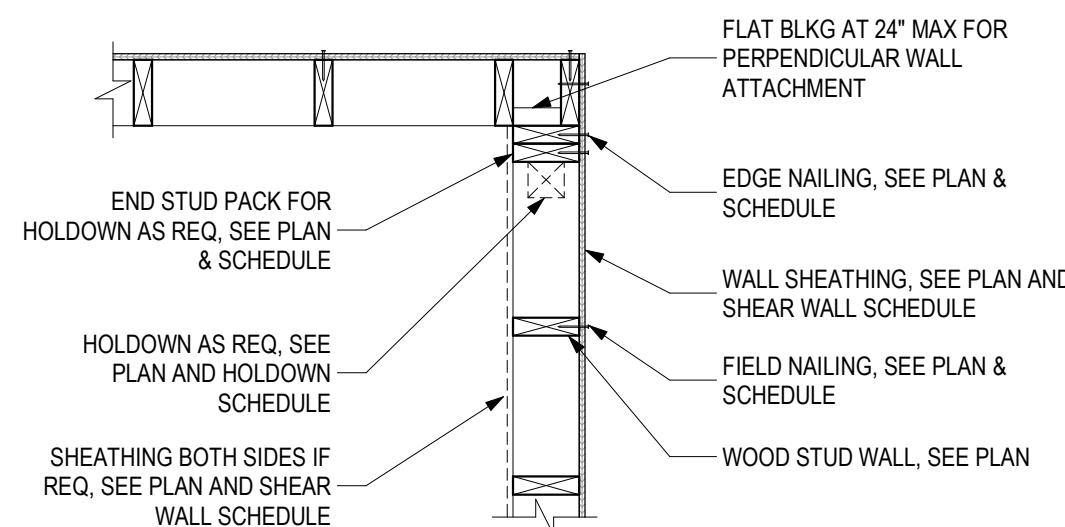


8 STEEL BEAM NAILER PLATE & PACK OUT
NOT TO SCALE

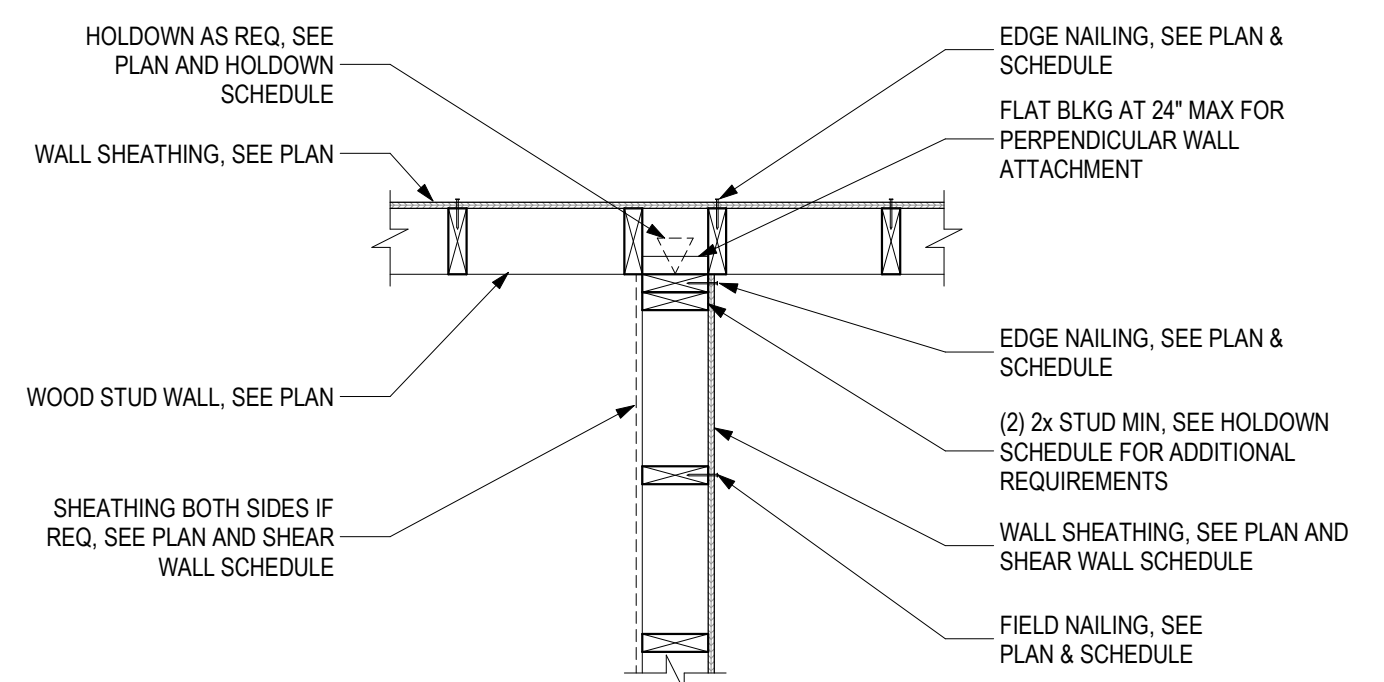


WOOD WALL OPENING SCHEDULE				
OPENING WIDTH	KING STUDS (UNO)		WINDOW SILL (UNO)	
	FRAMING	END CONNECTION	FRAMING	END CONNECTION
≤1'-6"	SEE PLAN	(3) 12d GUN NAILS	(1) 2x6	(3) 12d GUN NAILS
≤4'-5"	SEE PLAN	(3) 12d GUN NAILS PER STUD	(2) 2x6	(3) 12d GUN NAILS PER PLATE
≤7'-5"	SEE PLAN	(3) 12d GUN NAILS PER STUD	(3) 2x6	(3) 12d GUN NAILS PER PLATE
≤10'-3"	SEE PLAN	(3) 12d GUN NAILS PER STUD	(4) 2x6	(3) 12d GUN NAILS PER PLATE

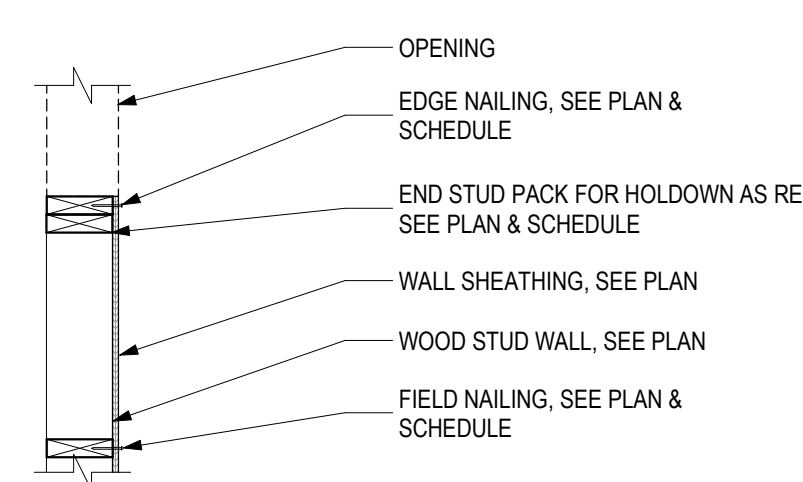
9 TYPICAL WALL FRAMING
3/8" = 1'-0"



10 WOOD SHEAR WALL CORNER INTERSECTION
NOT TO SCALE



11 WOOD SHEAR WALL INTERSECTION
NOT TO SCALE



12 SHEAR WALL END PANEL
NOT TO SCALE

APPROVAL STAMPS:

REVIEWED
FOR
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COMPLIANCE
12/27/2023

Submissions & Revisions

Owner

Architect

KASA
KEVIN & ASAKO SPERRY ARCHITECTURE
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General Contractor

Civil Engineer

Landscape Architect

anthem
structural engineers
303-848-8497 970-300-3338
anthemstructural.com Job #22-048

M.E.P. & F.P. Engineers

Interior Designer

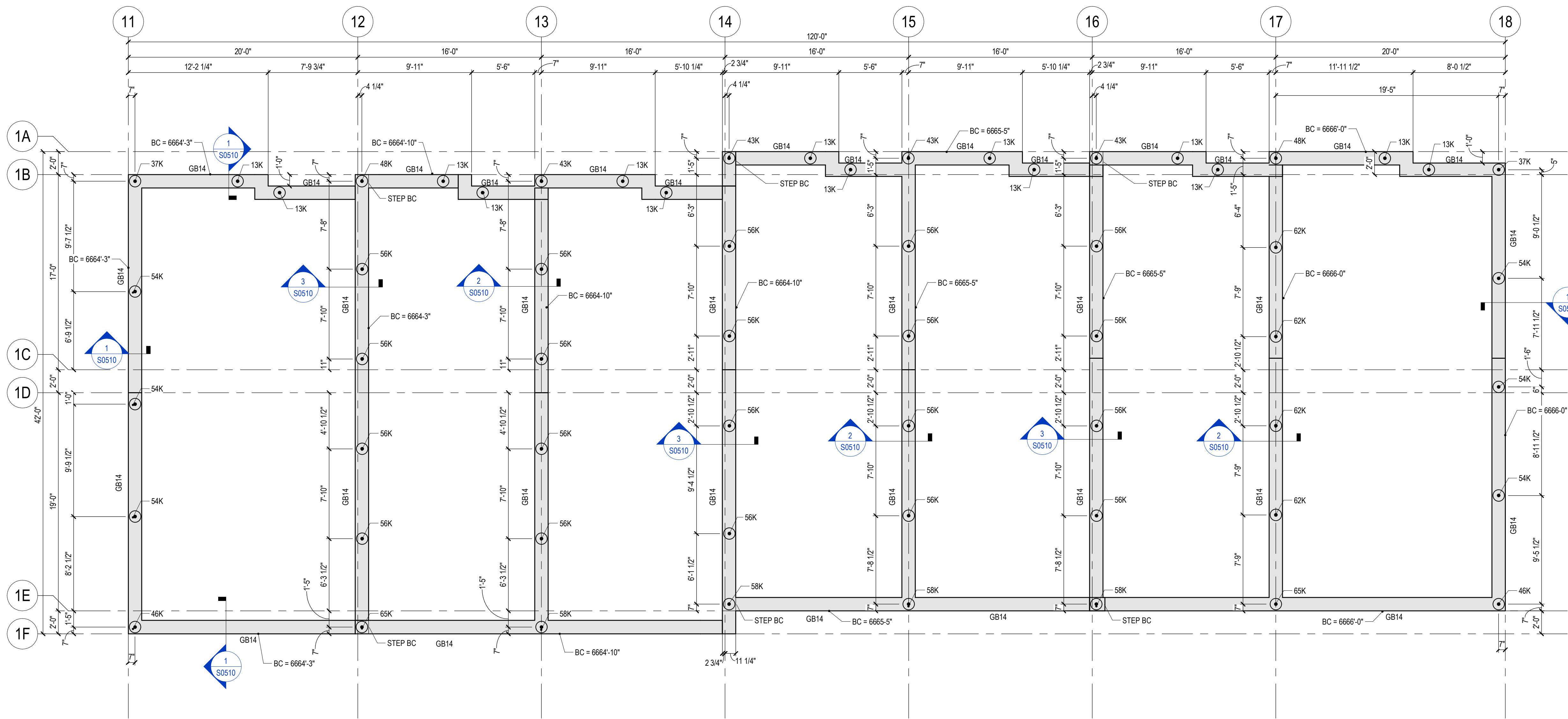
Project Location

Basecamp Townhome
1950 Curve Court
Steamboat Springs, CO 80487

Drawing Title

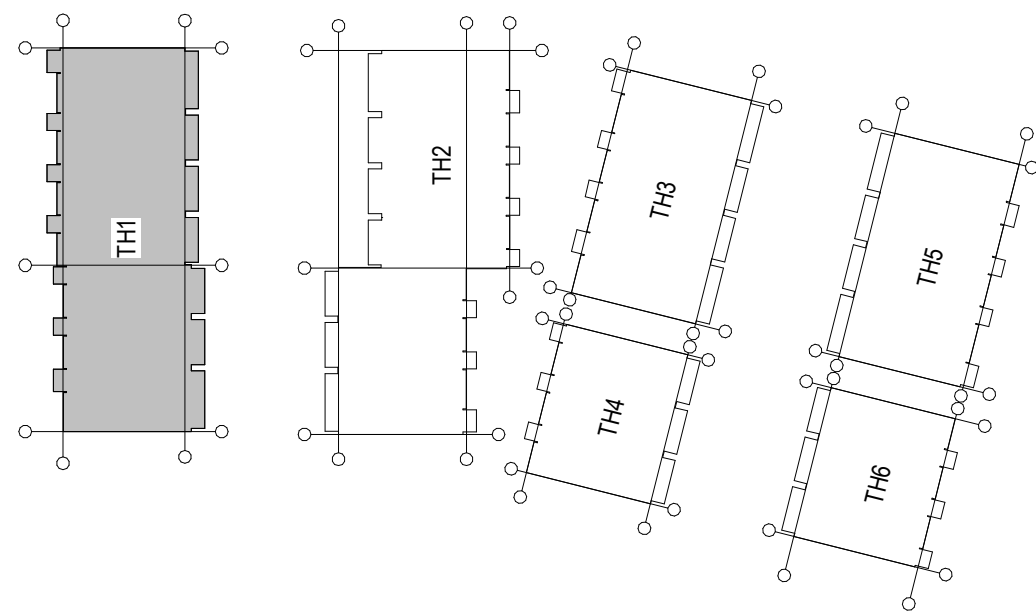
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DATE: 09/09/2022
DRAWN BY:
CHECKED BY:
PROJECT NO: 22-048

Drawing No: S0060



FOUNDATION PLAN
3/16" = 1'-0"

CONCRETE GRADE BEAM SCHEDULE					
TYPE	WIDTH	DEPTH	TOP BARS	BOT BARS	STIRRUPS
GB14	1'-2"	SEE PLAN	4- #7	4- #7	#4 @ 22" O.C.



KEY PLAN

FOUNDATION PLAN NOTES:

- SEE S0001 AND S0002 FOR GENERAL STRUCTURAL NOTES, ABBREVIATIONS KEY AND LEGEND.
- SEE S0030 FOR TYPICAL FOUNDATION DETAILS
- SEE S0600 AND S0601 FOR SCHEDULES
- DO NOT BACKFILL FOUNDATION WALLS UNTIL 7 DAYS AFTER LEVEL 1 SLAB IS INSTALLED
- SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS OF RAMPS, SLAB SLOPES, STEPPED SLABS, AND PARTITION WALLS. SLAB ELEVATIONS SHOWN ARE APPROXIMATE.
- REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.
- CONCRETE GRADE BEAMS:** SEE SCHEDULE ON SHEET
- LEVEL 1 STRUCTURAL SLAB:** 8" THICK CONCRETE SLAB 6" VOID, REINFORCED PER PLAN. INSTALL (3) #4 x 5'-0" DIAGONAL BARS AT MID-DEPTH OF SLAB AT ALL RE-ENTRANT CORNER. DO NOT SAW CUT OR TROWEL JOINTS IN SLABS UNO.
- MICROPILE:** INDICATES MICROPILE, SEE PLAN FOR LOADING, SIZE AND REINFORCING BY OTHERS.
- INDICATES TOTAL AXIAL LOAD AT MICROPILE.
XXX POSITIVE VALUE = COMPRESSION (KIPS, ASD)
XXX NEGATIVE VALUE = TENSION (KIPS, ASD)

APPROVAL STAMPS:

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FOR
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12/27/2023

No. Date Description

SUBMISSIONS & REVISIONS

OWNER

ARCHITECT

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Arlington, VA 22207
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GENERAL CONTRACTOR

CIVIL ENGINEER

LANDSCAPE ARCHITECT

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303-848-8497 970-300-3338
anthemstructural.com Job #22-048

M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO. 80487

TH1 FOUNDATION
PLAN

SEAL



DATE:

09/09/2022

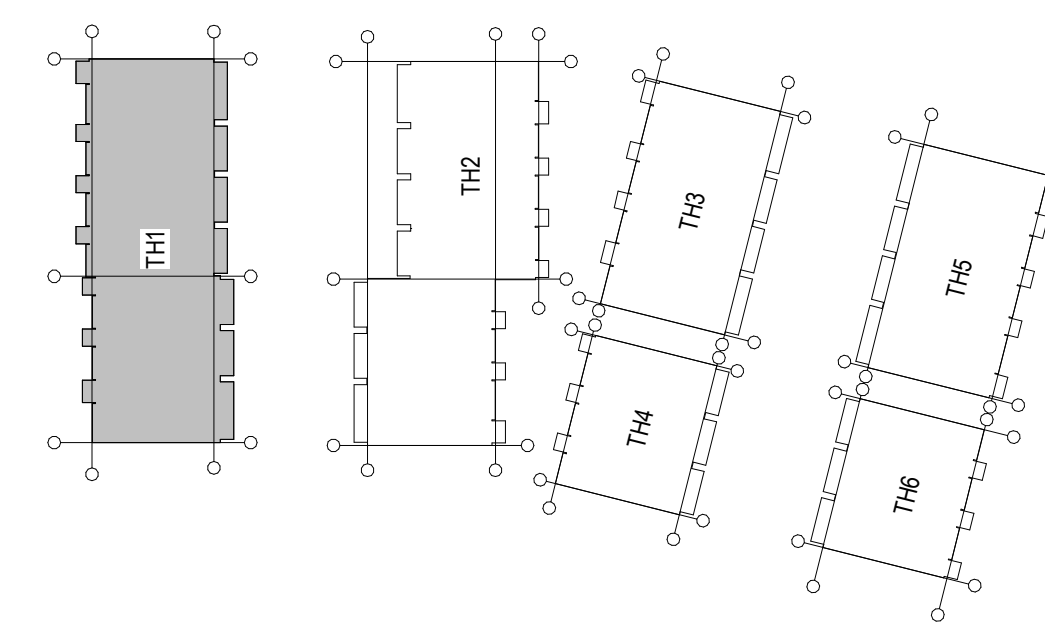
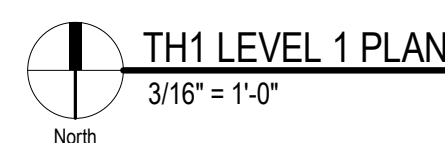
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PROJECT NO:
22-048

DRAWING NO:

S0210-TH1



KEY PLAN

1. SEE S0001 AND S0002 FOR GENERAL STRUCTURAL NOTES,
ABBREVIATIONS KEY AND LEGEND.
2. SEE S0003 FOR FOUNDATION DETAIL
3. SEE S0060 AND S0061 FOR SCHEDULES
4. DO NOT BACKFILL FOUNDATION WALLS UNTIL 7 DAYS AFTER
LEVEL 1 SLAB IS INSTALLED
5. REFER ARCHITECTURAL DRAWINGS FOR LOCATIONS OF RAMPS,
SLAB SLOPES, STEPPED SLABS, AND PARTITION WALLS. SLAB
ELEVATIONS SHOWN ARE APPROXIMATE.
6. REFER TO ARCHITECTURE DRAWINGS FOR ADDITIONAL
INFORMATION AND DIMENSIONS.
7. **TYPICAL CONCRETE WALL DRUMING:** #8 WALLS: #5 @ 16" EACH
WAY, CENTERED IN SLAB; #5 BARS TOP AND BOT HORIZ.
#4 WALLS: 12" X 12" ANCHOR BOLTS, 12" X 12" ANCHOR VERTICAL
WALL REINFORCING, 12" X 12" ANCHOR BOLTS, AND WASHERS
TO WOOD WALL ABOVE WITH "V" PROJECTION @32" AND #8 MAX
FROM CORNERS AND STEPS IN WALL.
8. **LEVEL 1 STRUCTURAL SLAB:** #8 THICK CONCRETE SLAB OVER #6"
WOOD JOISTED FLOOR. PROVIDE 12" X 12" ANCHOR BOLTS DIAGONAL
BARS AT MID DEPTH OF SLAB AT ALL EN-RE-ENTRANCES CORNERS.
DO NOT SAW CUT OR TROWEL JOINTS IN SLABS UNDO.
9. INDICATES HOLDOUTS LOCATED AT THE TOP OF THE
FOUNDATION WALL. SEE S0600 FOR SCHEDULE AND
MINIMUM END STUD REQUIREMENTS

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12/27/2023

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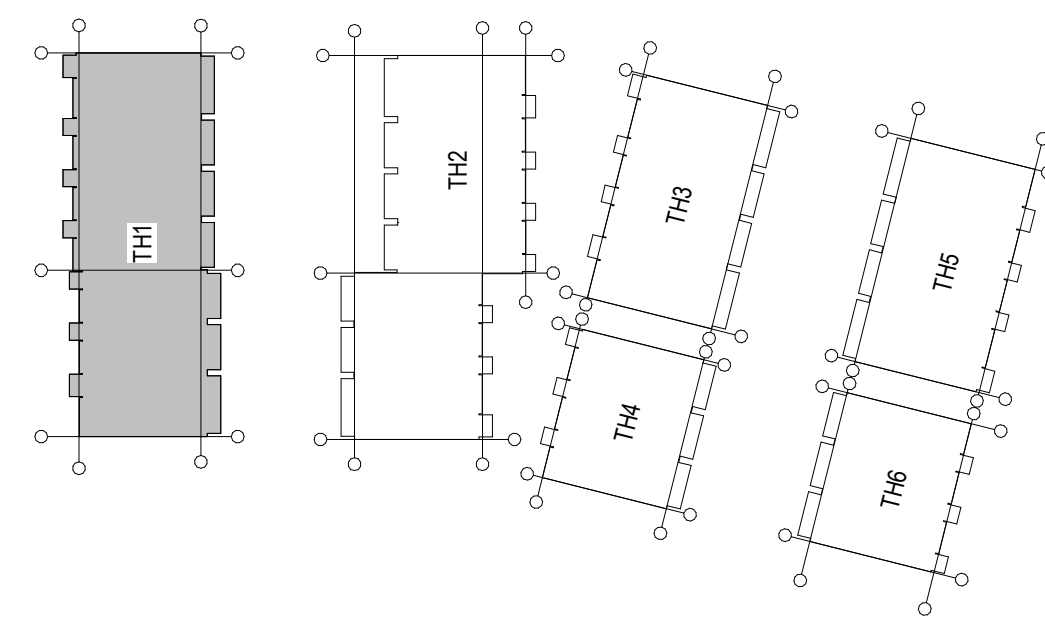
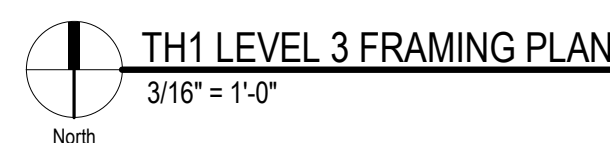
DRAWING NO:
S0211-TH1

5. ITS HANGER ASSUMES 2x SINGLE NAILER PLATES AS SUPPORT WITH 0.148"x1 1/2" NAILS INTO PLATE. IF ALTERNATE SUPPORT CONDITION OCCURS, SEE PLAN FOR ADDITIONAL HANGER INFORMATION

MARK	JOIST	SPACING
J1	14" TJI 360	16" O.C.
J2	2x12 HF	16" O.C.
J3	14" LVL	16" O.C.
J4	(2) 14" LVL	16" O.C.
J5	14" TJI 110	16" O.C.

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PROJECT NO:	22-048

S0212-TH1



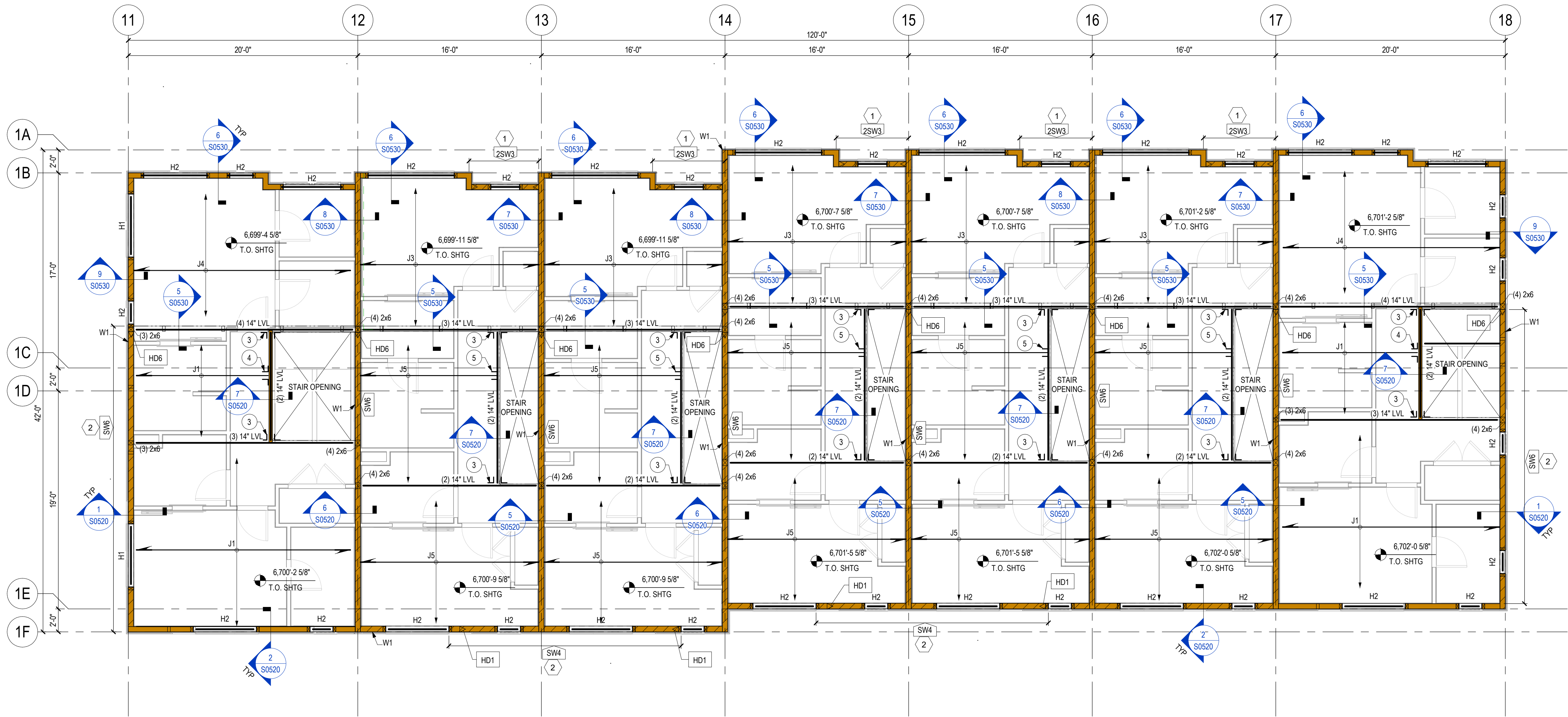
KEY PLAN

MARK	JOIST	SPACING
J1	14" TJI 360	16" O.C.
J2	2x12 HF	16" O.C.
J3	14" LVL	16" O.C.
J4	(2) 14" LVL	16" O.C.
J5	14" TJI 110	16" O.C.

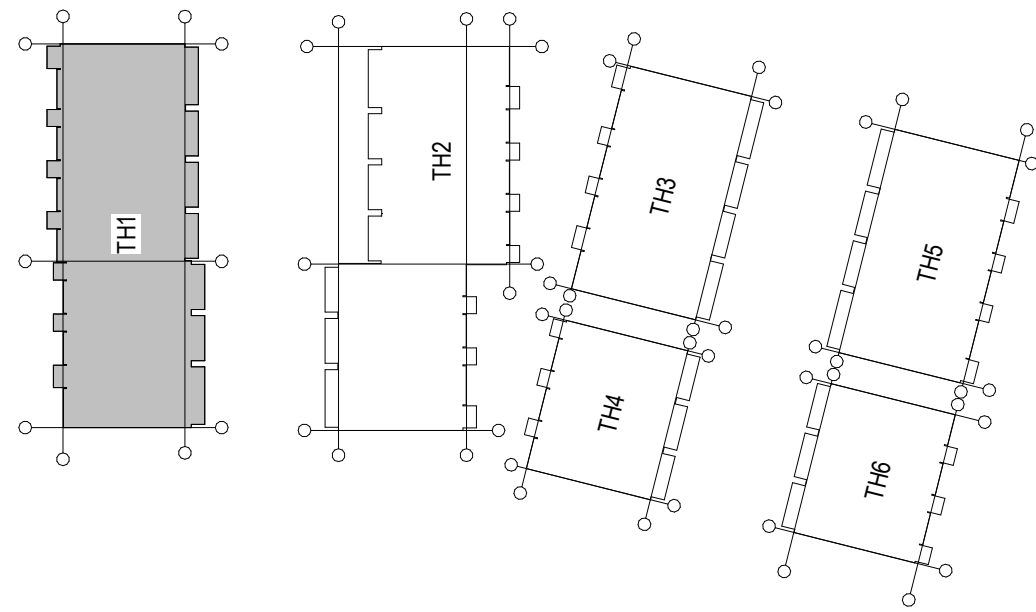
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S0213-TH1



TH1 LEVEL 4 FRAMING PLAN
3/16" = 1'-0"



KEY PLAN

FRAMING LEVEL PLAN NOTES:

- SEE S0001 AND S0002 FOR GENERAL STRUCTURAL NOTES, ABBREVIATIONS KEY AND LEGEND
- SEE S0060 FOR TYPICAL DETAILS
- SEE S0600 FOR SCHEDULES
- REFER TO ARCHITECTURAL DRAWINGS FOR ADDITIONAL INFORMATION AND DIMENSIONS.
- TOP OF SUB-FLOOR SHEATHING = SEE PLAN
- TOP OF PLATE HEIGHT = SEE ARCH
- WALL FRAMING AND COLUMNS SHOWN SUPPORT THE FRAMING ON THIS LEVEL
- FLOOR CONSTRUCTION (UNO):** 3/4" STURD-I-FLOOR APA RATED @ 24" OC TONGUE AND GROOVE SHEATHING, OVER WOOD I-JOISTS / TRUSSES W/ 1 1/4" LSL RIM, SEE PLAN. GLUE AND FASTEN SHEATHING TO JOISTS, RIM, FLUSH BEAMS, AND LEDGERS WITH 8d GUN NAILS (0.131"Ø x 2.38") @ 4" O.C. AND @ 8" O.C. ALONG INTERMEDIATE FRAMING MEMBERS. LAY PANELS PERPENDICULAR TO FRAMING MEMBERS AND STAGGER PANEL JOINTS.
- EXTERIOR WALL CONSTRUCTION (UNO):** 2x STUDS SHEATHED WITH 7/16" CDX PLYWOOD OR OSB, APA 24/16 ON EXTERIOR FACE. NAIL WALL SHEATHING WITH 8d GUN NAILS (0.131"Ø x 2.38") @ 4" AT PANEL EDGES AND BOUNDARIES AND @ 12" IN FIELD OF PANEL BLOCK AND NAIL ALL EDGES BETWEEN STUDS. SEE BEARING WALL SCHEDULE FOR STUD QUANTITIES AND SPACING
- INTERIOR BEARING WALL CONSTRUCTION (UNO):** 2x STUDS SHEATHED WITH 1/2" MIN. GYPSUM WALLBOARD ON EACH FACE. ATTACH W/ NO. 6 x 1 1/4" DRYWALL SCREWS @ 8" AT PANEL EDGES AND BOUNDARIES AND @ 12" IN FIELD OF PANEL. SEE BEARING WALL SCHEDULE FOR STUD QUANTITIES AND SPACING
- WALL OPENING CONSTRUCTION (UNO):** (2) 2x6 HEADER W/ MINIMUM (1) 2x6 TRIM AND (1) 2x6 KING STUD EACH END. HEADERS ARE DROPPED UNO.
- TYPICAL DECK CONSTRUCTION (UNO):** 2x EXTERIOR DECKING OVER WOOD JOISTS PER PLAN. LAY DECKING PERPENDICULAR TO FRAMING AND FASTEN DECKING TO JOIST W/ (2) #8x3" EXTERIOR DECK SCREWS PER BOARD. FLASH TOP OF MULTI-PLY JOISTS / BEAMS.
- INDICATES HOLDOWN THROUGH LEVEL SHOWN, SEE S0600.
- CONTRACTOR TO VERIFY LOCATIONS AND LAYOUT WITH FRAMING ABOVE
- INDICATES SHEAR WALL TO BE SHEATHED ON SIDE INDICATED BY ARROW (UNO) WITH SHEATHING PER SHEAR WALL SCHEDULE. SEE S0600
- HX INDICATES HEADER. SEE SCHEDULE ON S0600
- JX INDICATES JOIST TYPE. SEE SCHEDULE ON THIS SHEET
- INDICATES FTAO SHEAR WALL ANALYSIS WITH HORIZONTAL CS20 STRAPS TOP AND BOTTOM OF OPENINGS, FULL LENGTH OF WALL.
- INDICATES PERFORATED SHEAR WALL ANALYSIS

HANGER SCHEDULE

1. ALL HANGERS NOTED TO BE INSTALLED WITH NUMBER AND SIZE FASTENERS SPECIFIED BY MNFR. ANY SUBSTITUTIONS SHALL BE REVIEWED AND APPROVED BY ANTHEM
2. INSTALL HANGERS NOTED OR APPROVED EQUIVALENT

X	DESCRIPTION	FACE FASTENERS	JOIST FASTENERS
1	LUS28	(6) 10d x 3" NAILS	(4) 10d x 3" NAILS
2	LUS28-2	(6) 10d x 3" NAILS	(4) 10d x 3" NAILS
3	HHUS410	(30) 10d x 3" NAILS	(10) 10d x 3" NAILS
4	ISU2.37/14	(12) 10d x 3" NAILS	-
5	IUS1.81/14	(12) 10d x 3" NAILS	-
7	HUC212-2	(14) 10d x 3" NAILS	(6) 10d x 3" NAILS

WOOD BEARING WALL SCHEDULE

WALL	DESCRIPTION
W1	2x6 @ 16" O.C.
W2	(2) 2x6 @ 16" O.C.
W3	2x4 @ 16" O.C.

JOIST SCHEDULE

1. SUBSTITUTION OF JOISTS OR HANGERS SHALL BE APPROVED BY EOR
2. DAB SUBFLOOR ADHESIVE IN SEAT OF HANGER
3. DO NOT CUT OR NOTCH TJI CHORDS IN ANY MANNER
4. HOLES IN WEB SHALL NOT EXCEED MANUFACTURER'S PUBLISHED LIMIT CRITERIA
5. ITS HANGER ASSUMES 2x SINGLE NAILER PLATES AS SUPPORT WITH 0.148x1 1/2" NAILS INTO PLATE. IF ALTERNATE SUPPORT CONDITION OCCURS, SEE PLAN FOR ADDITIONAL HANGER INFORMATION

MARK	JOIST	SPACING
J1	14" TJI 360	16" O.C.
J2	2x12 HF	16" O.C.
J3	14" LVL	16" O.C.
J4	(2) 14" LVL	16" O.C.
J5	14" TJI 110	16" O.C.

APPROVAL STAMPS:

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12/27/2023

SUBMISSIONS & REVISIONS

OWNER

ARCHITECT

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GENERAL CONTRACTOR

CIVIL ENGINEER

LANDSCAPE ARCHITECT

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structural engineers
303-848-8497 970-300-3338
anthemstructural.com Job #22-048

M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

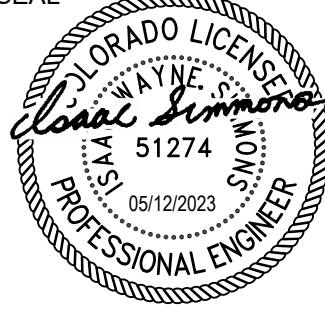
PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO. 80487

TH1 LEVEL 4
FRAMING PLAN

SEAL



DATE:

09/09/2022

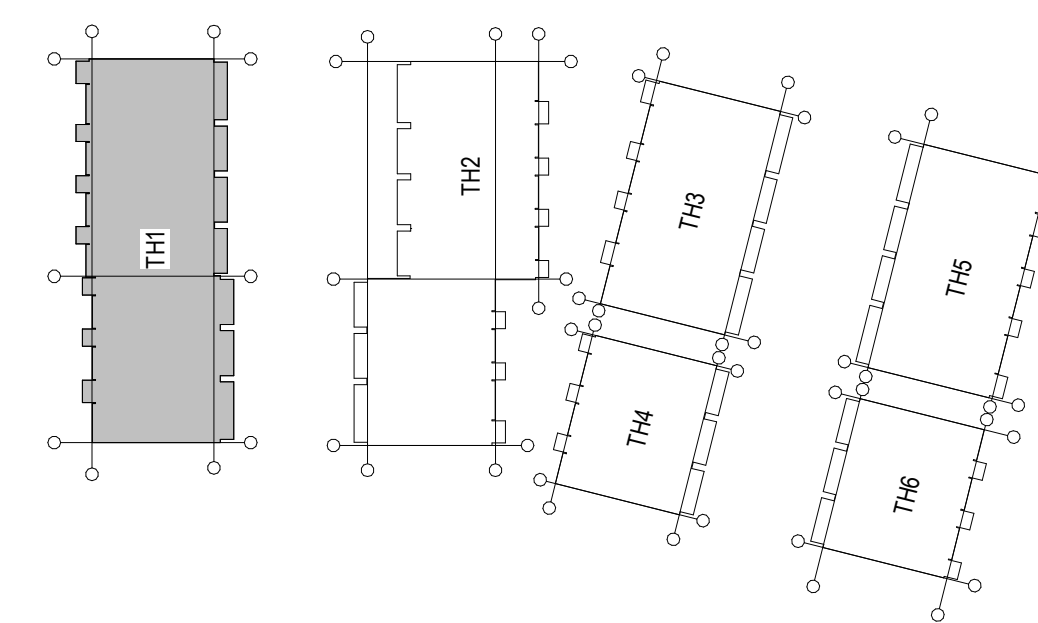
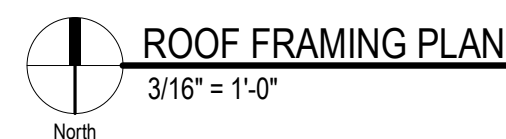
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PROJECT NO:
22-048

DRAWING NO:

S0214-TH1



KEY PLAN

- | WOOD BEARING WALL SCHEDULE | |
|----------------------------|--------------------|
| WALL | DESCRIPTION |
| W1 | 2x6 @ 16" O.C. |
| W2 | (2) 2x6 @ 16" O.C. |
| W3 | 2x4 @ 16" O.C. |

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OWNER

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www.kasa-arch.com

GENERAL CONTRACTOR

CIVIL ENGINEER

LANDSCAPE ARCHITECT



M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO 80487
DRAWING TITLE

TH1 ROOF FRAMING
PLAN

SEAL

DATE: _____

09/09/2022

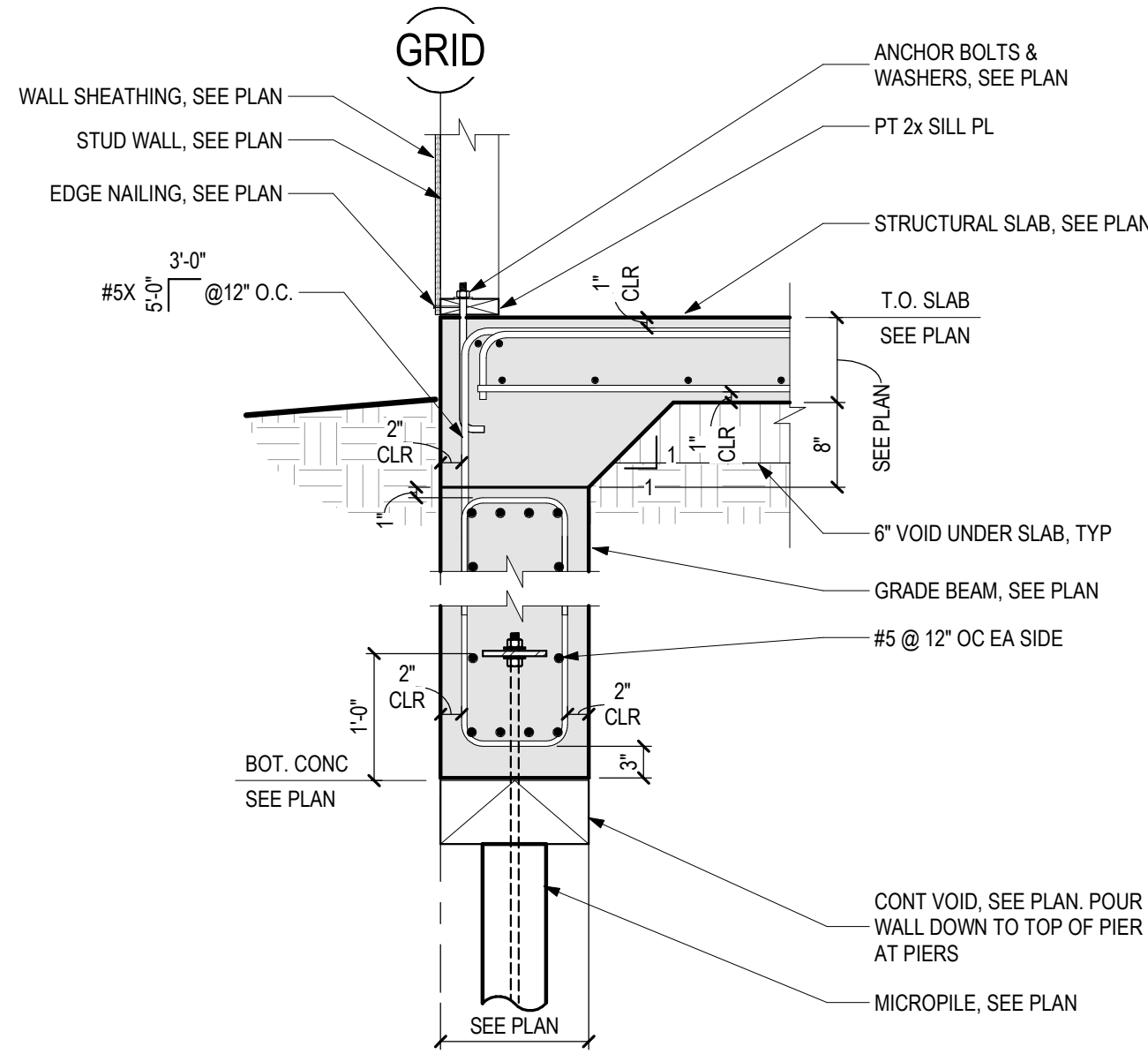
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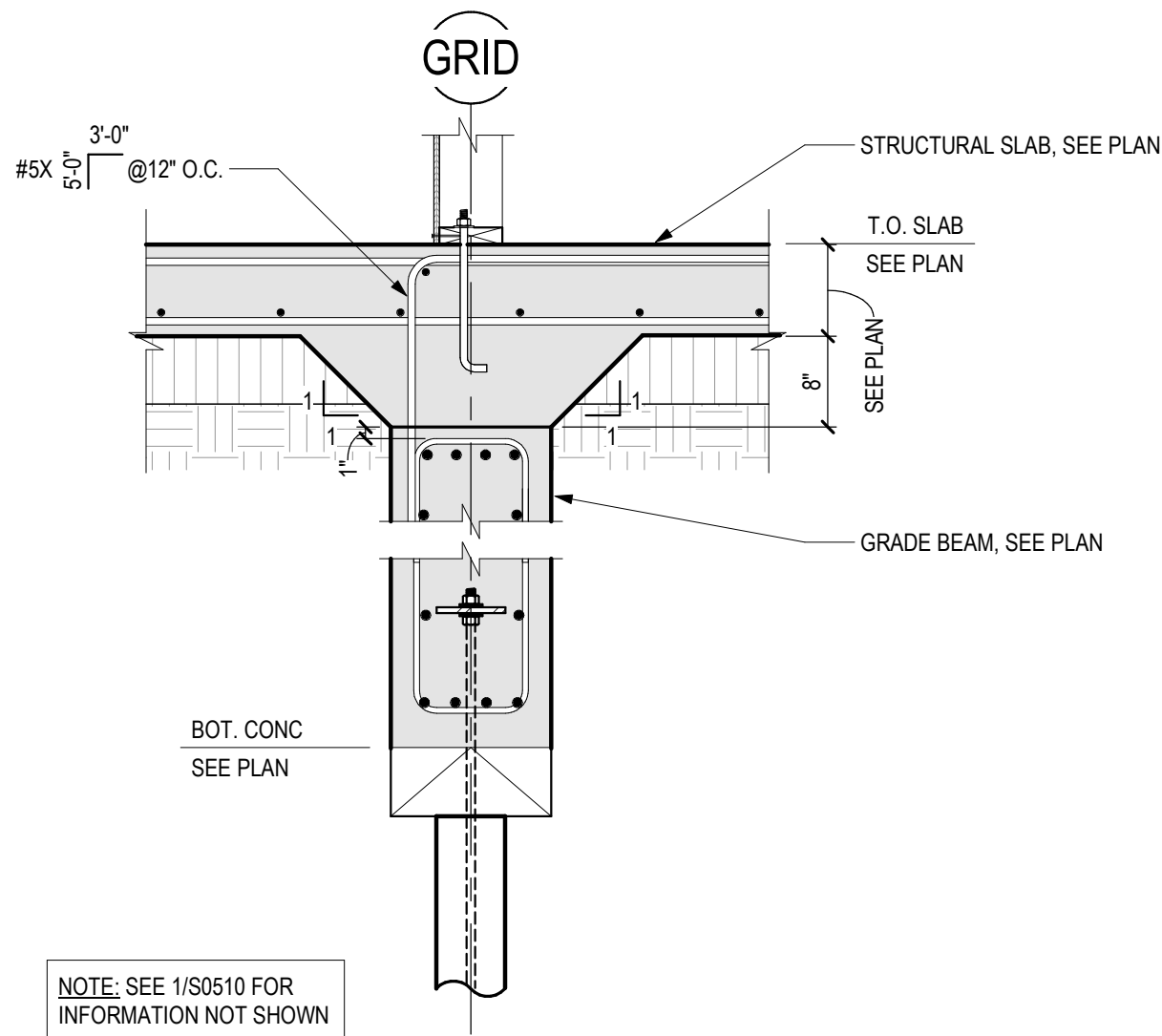
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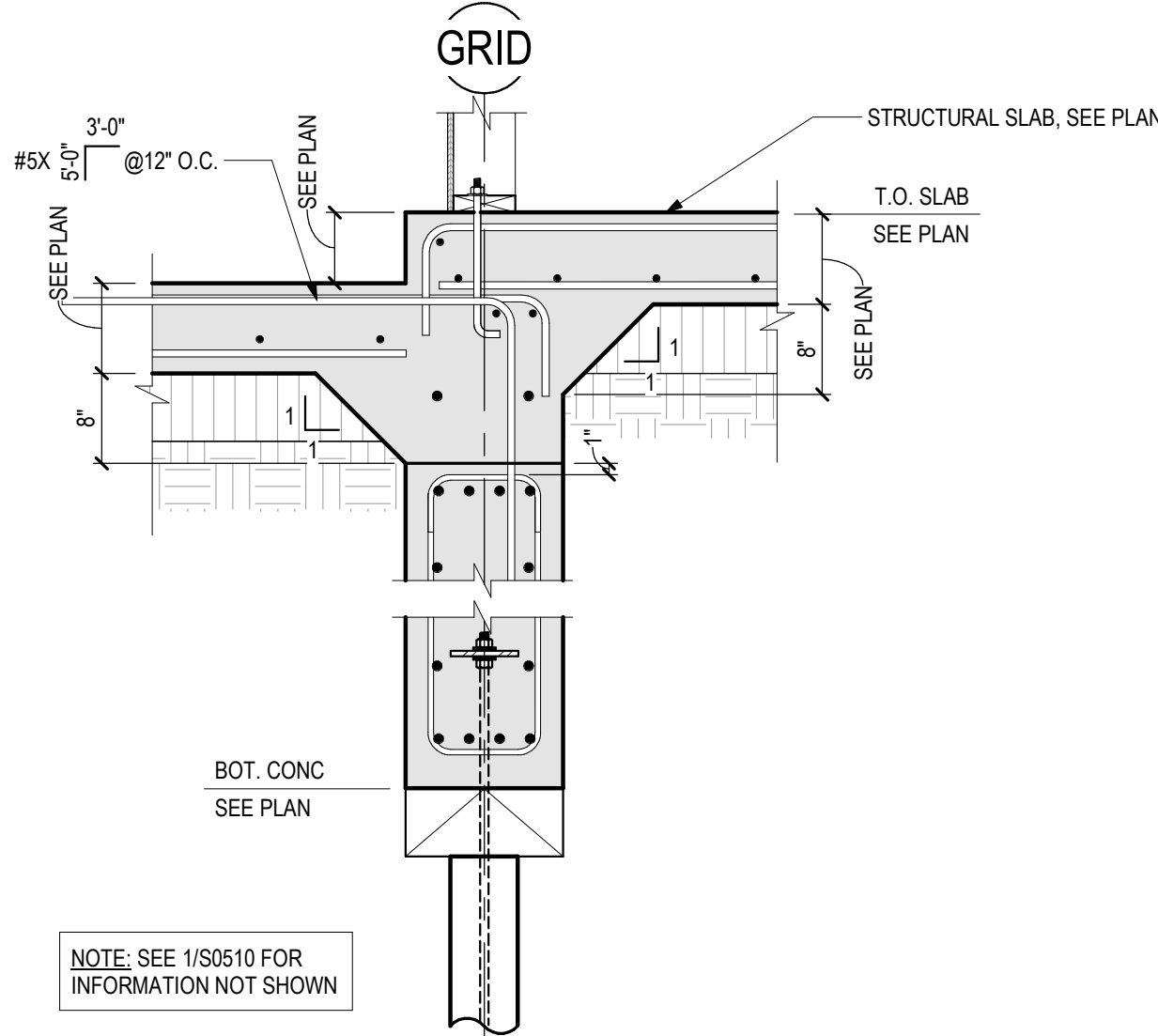
S0215-TH1



1 EXTERIOR GRADE BEAM
3/4" = 1'-0"



2 INTERIOR GRADE BEAM (NO STEP)
3/4" = 1'-0"



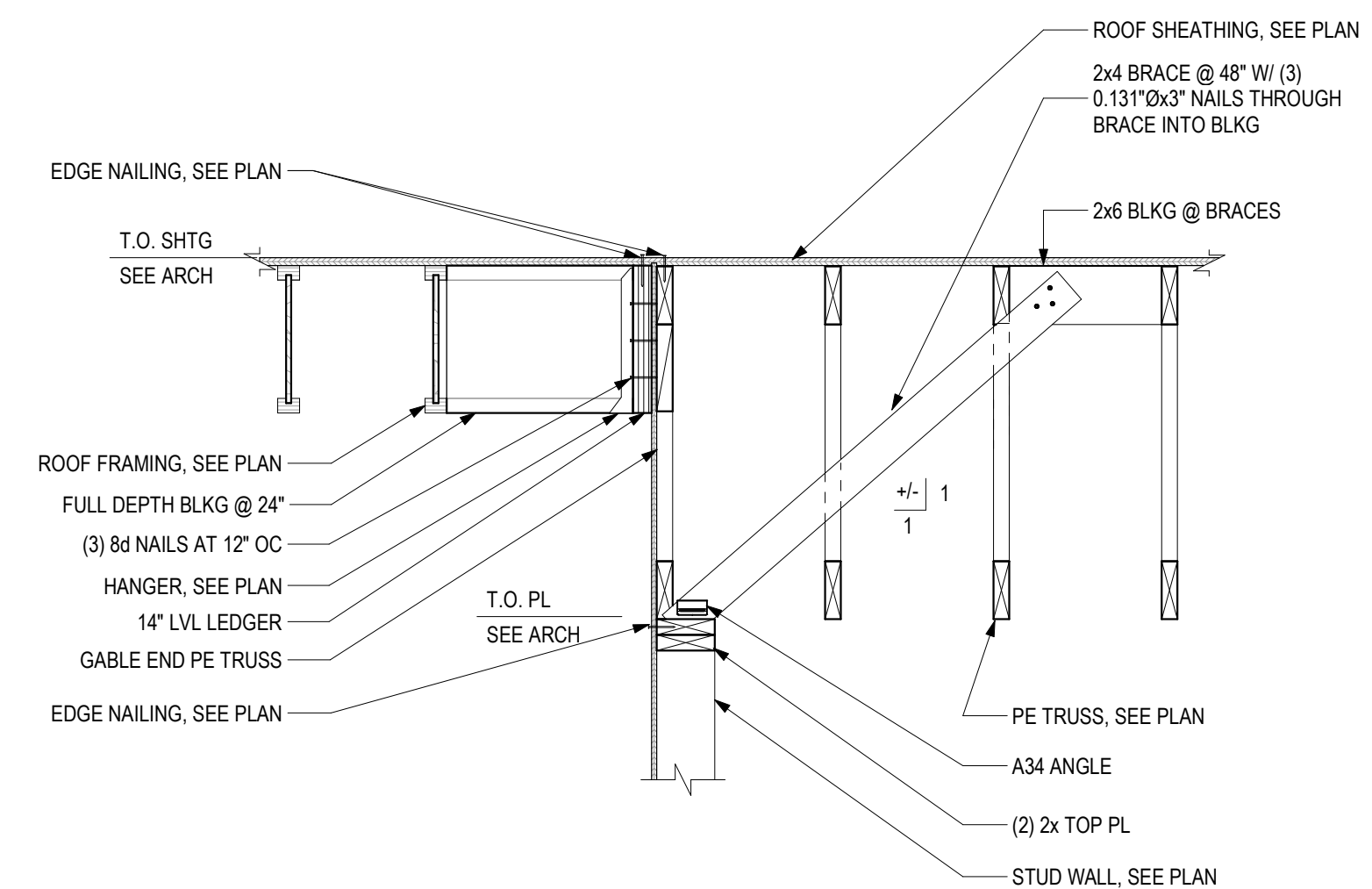
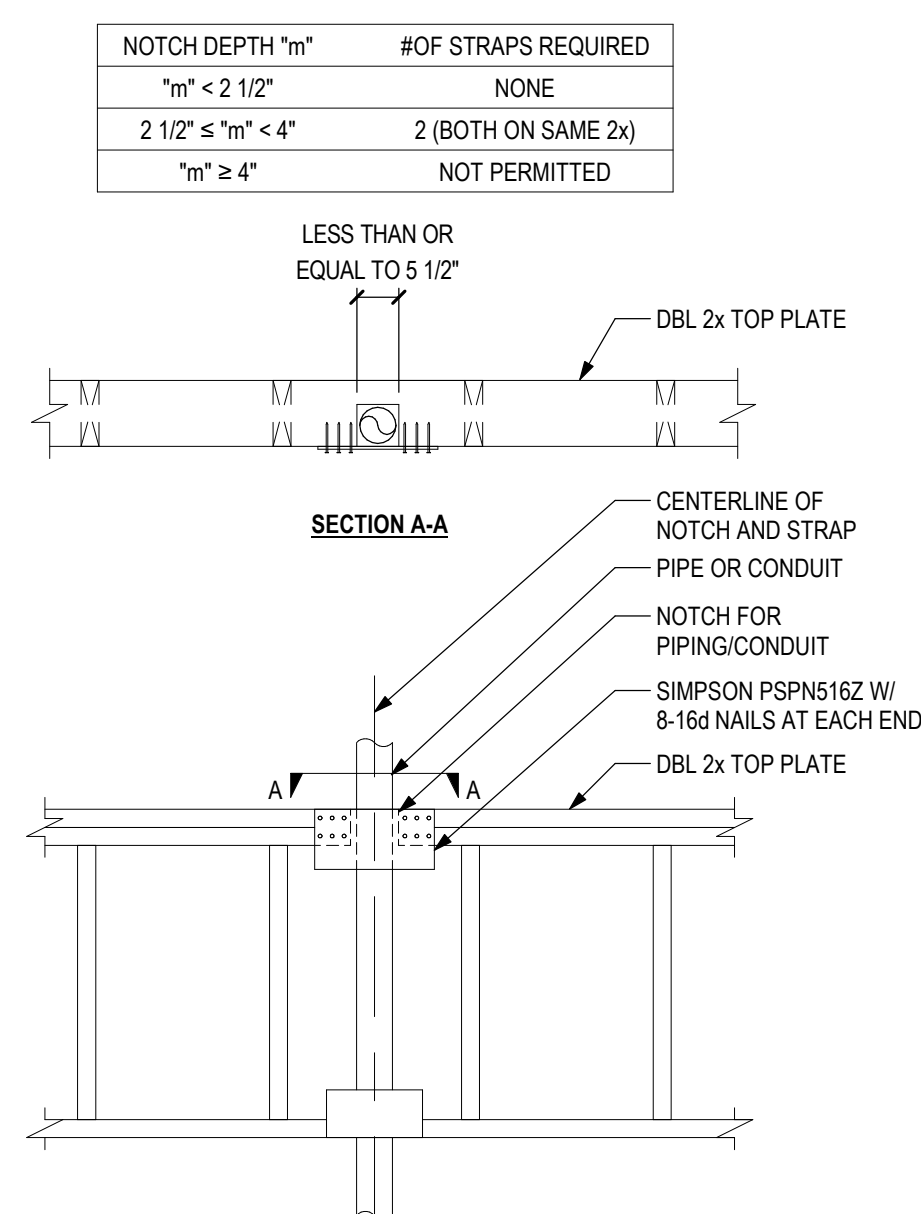
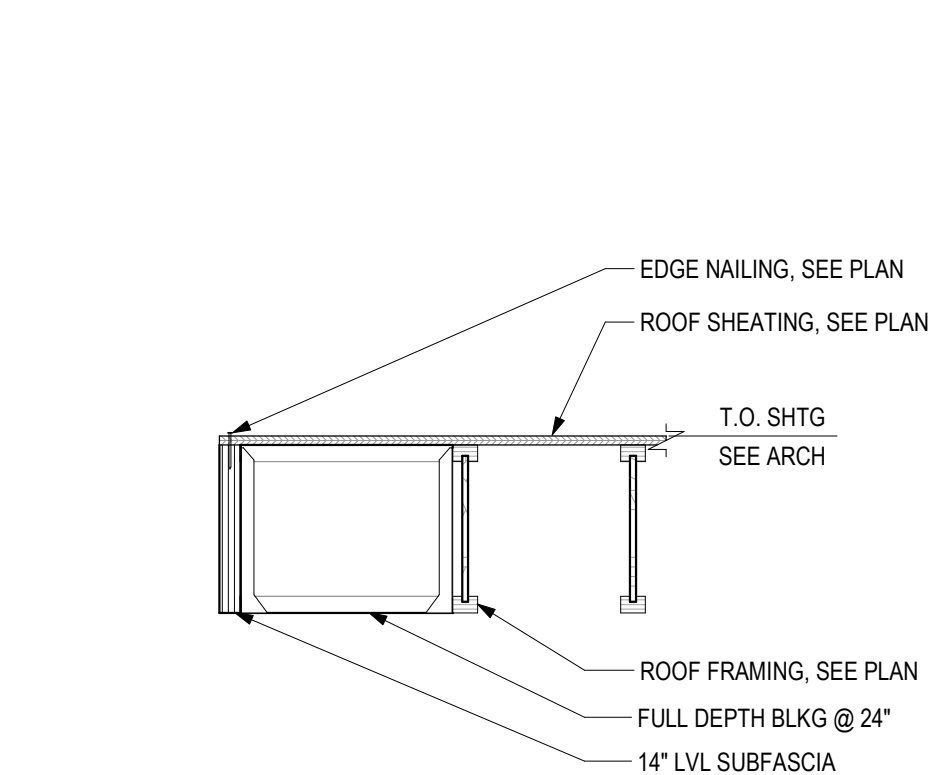
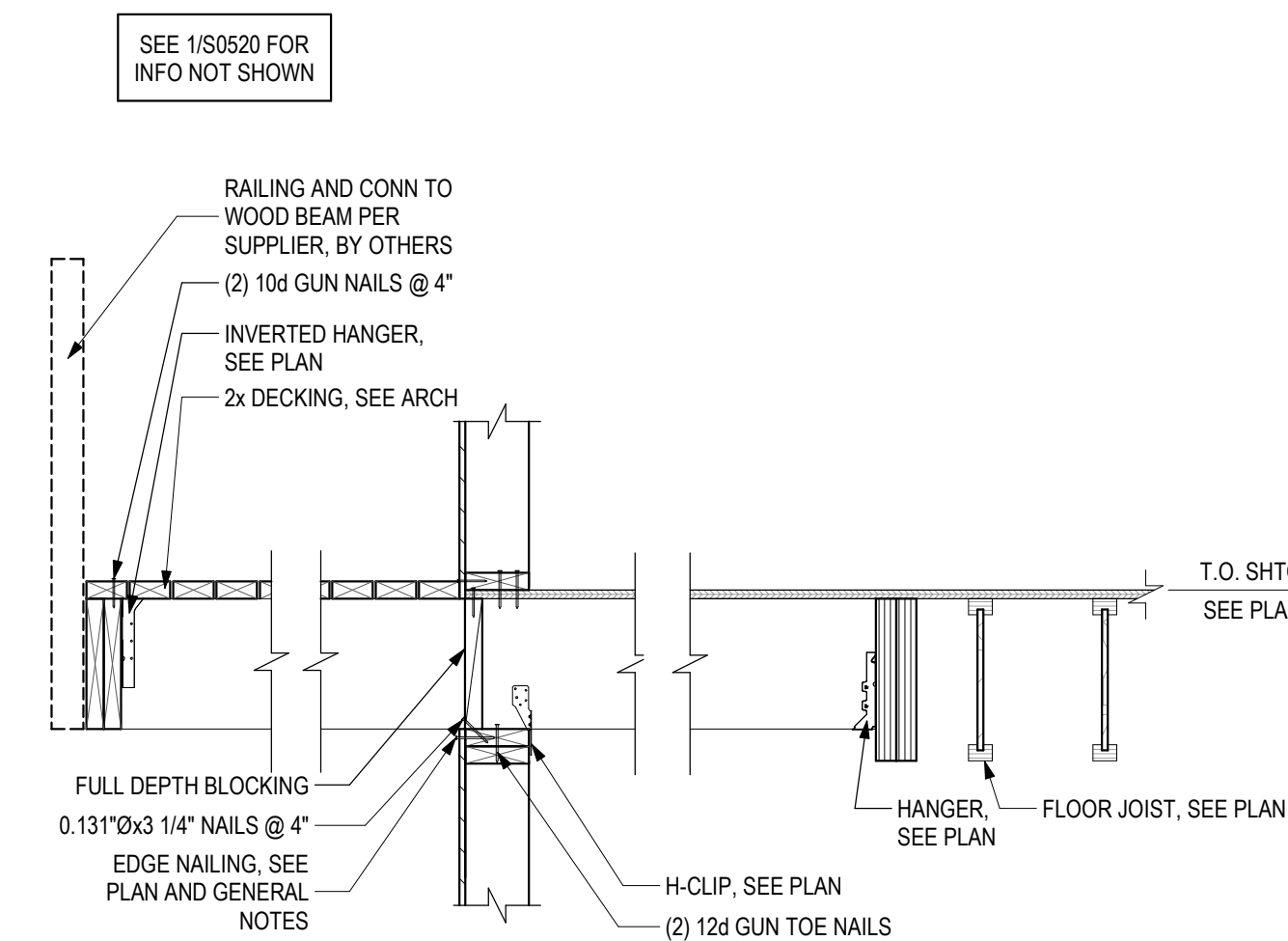
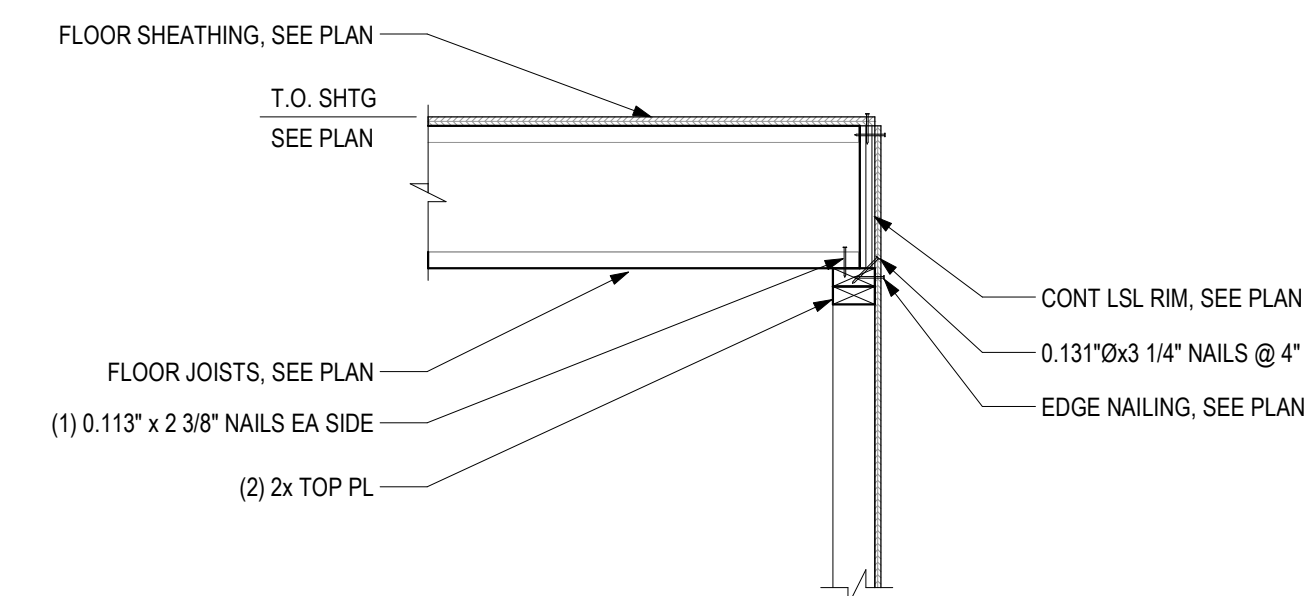
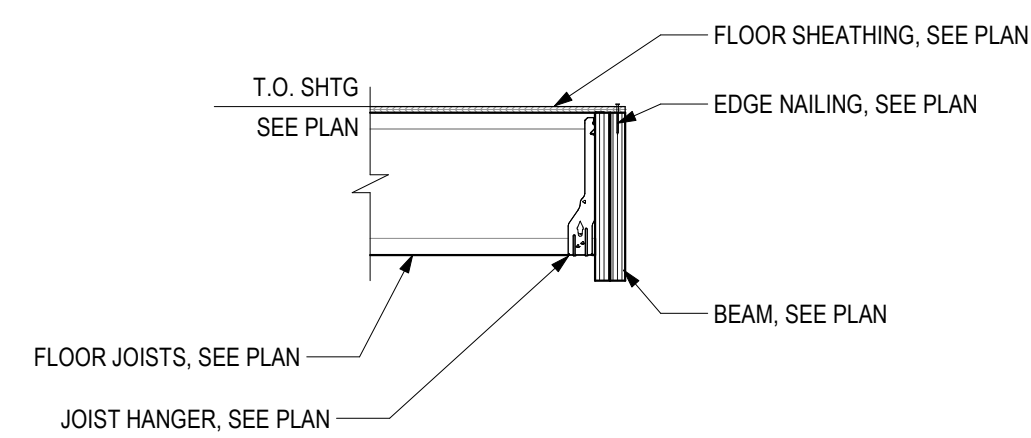
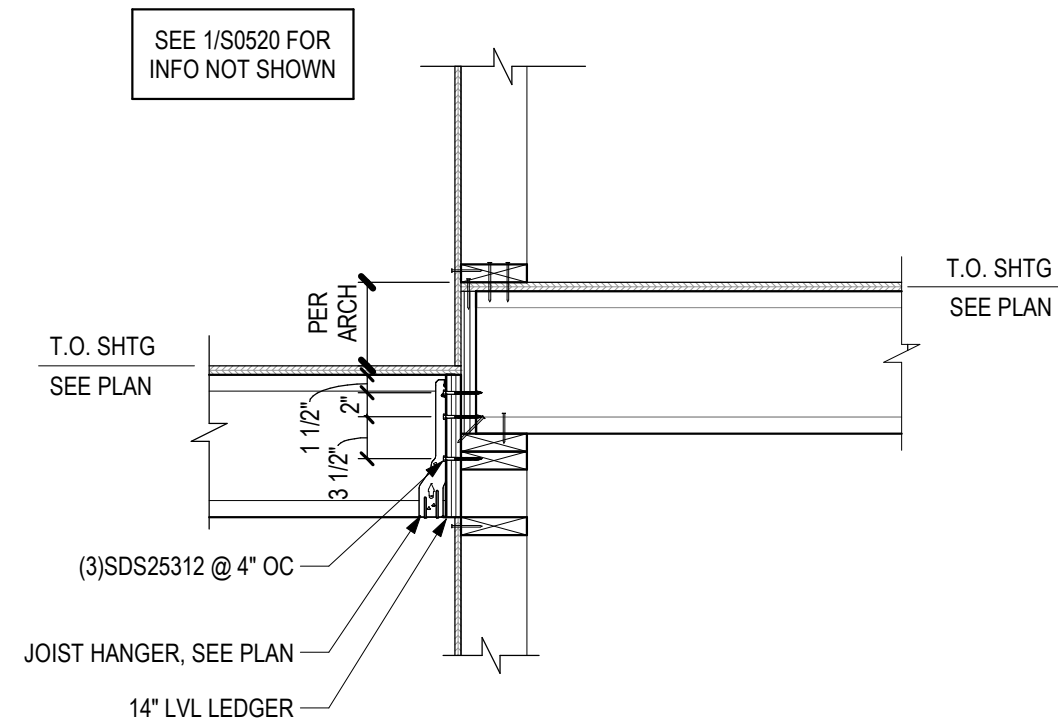
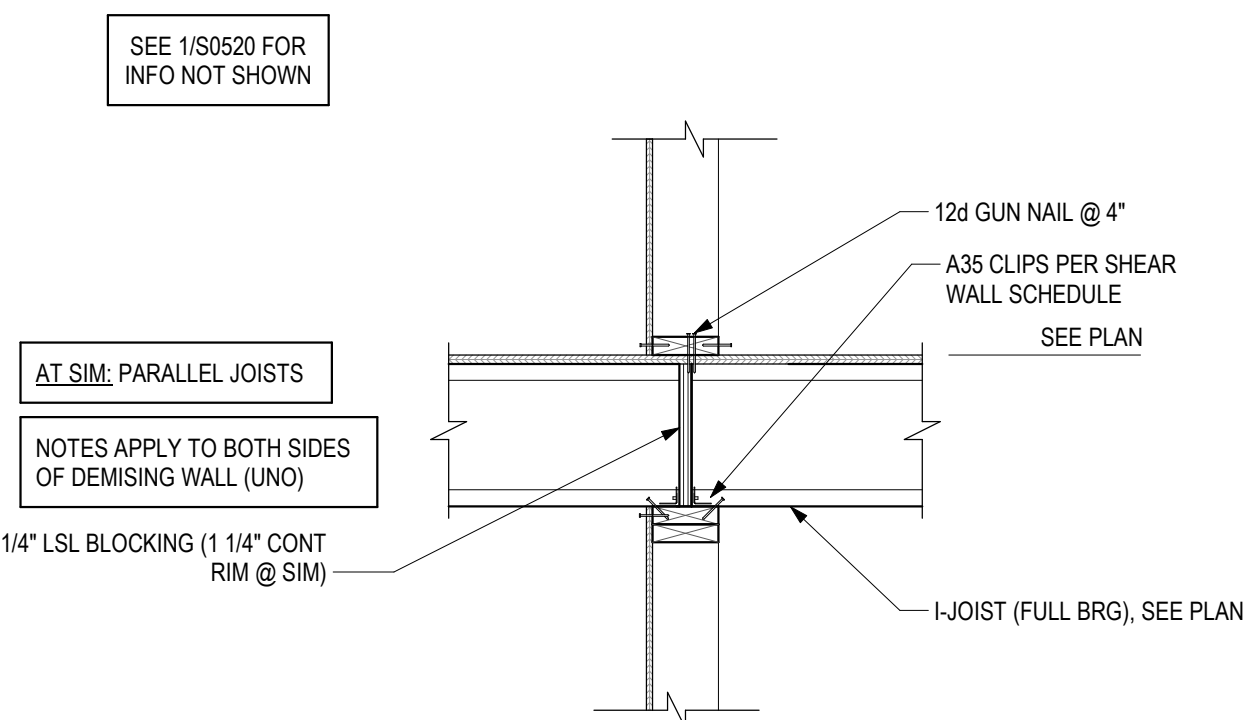
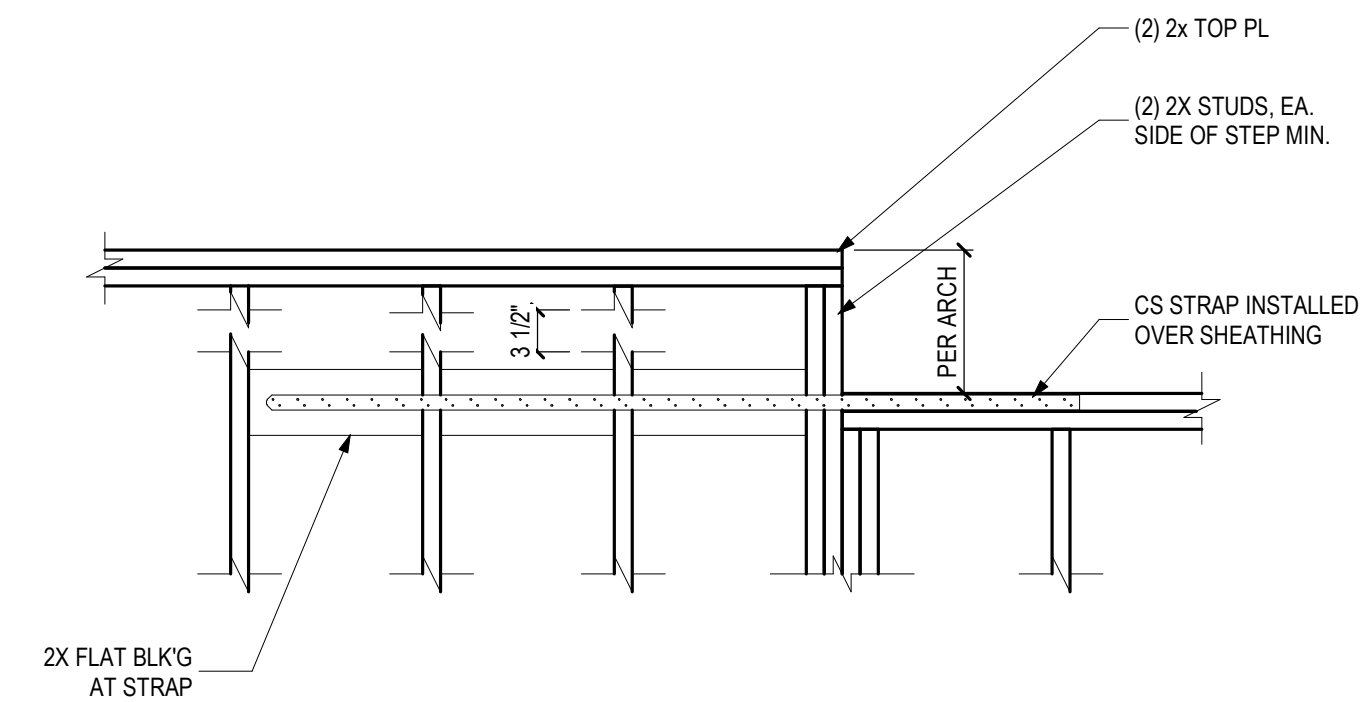
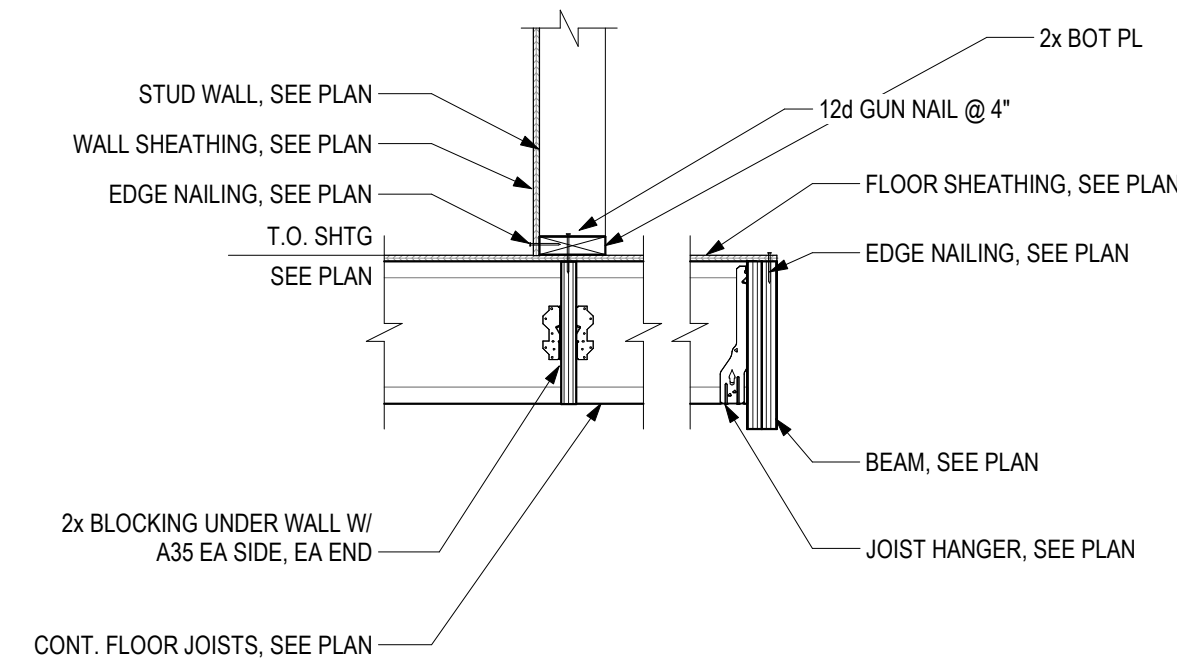
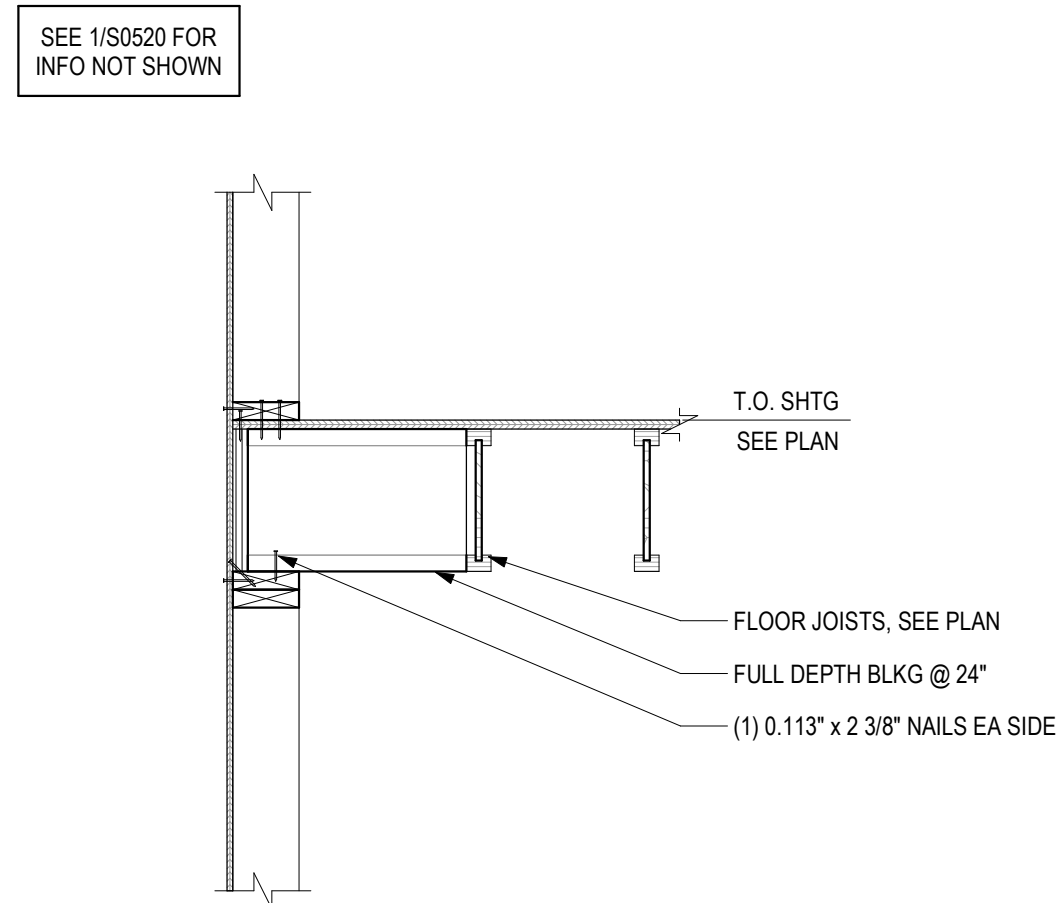
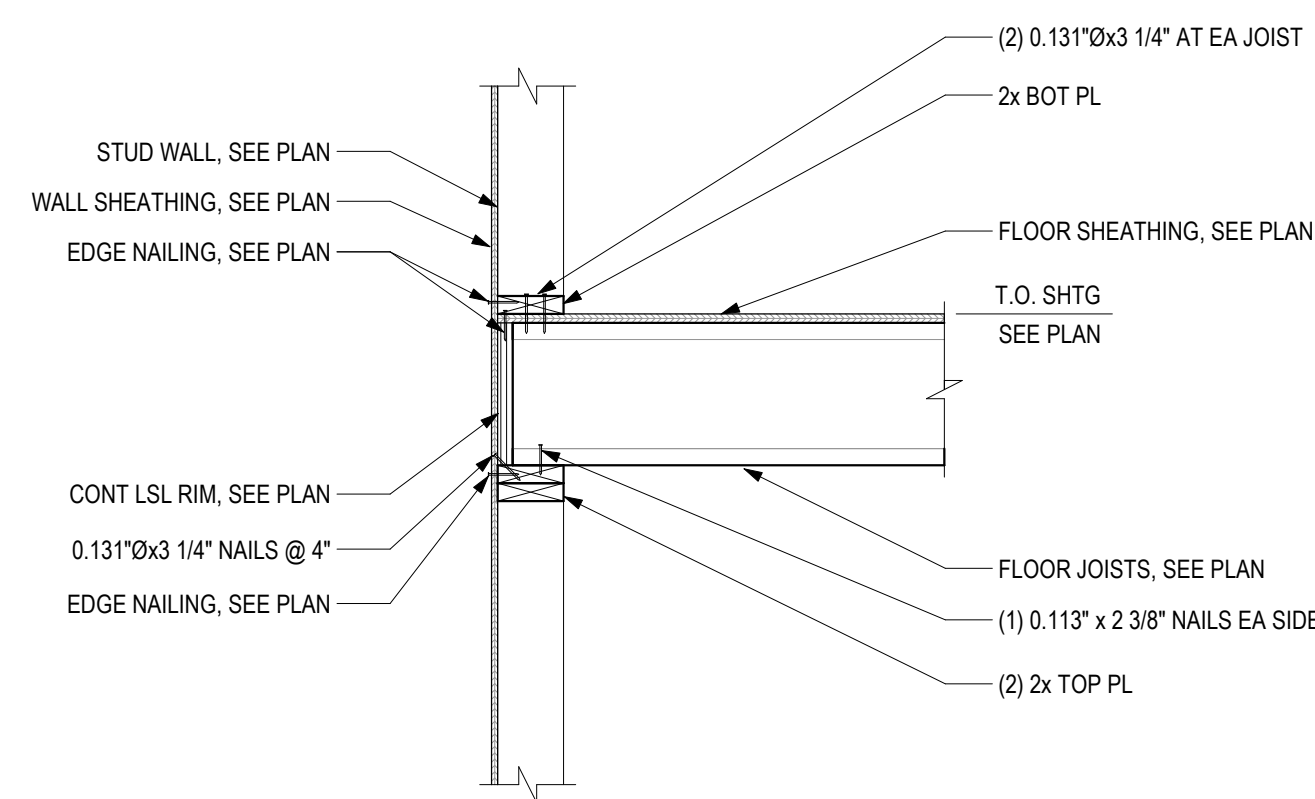
3 INTERIOR GRADE BEAM (W/ STEP)
3/4" = 1'-0"

APPROVAL STAMPS:

REVIEWED
FOR
CODE
COMPLIANCE
12/27/2023

No.	Date	Description
SUBMISSIONS & REVISIONS		
OWNER		
ARCHITECT		
KEVIN & ASAKO SPERRY ARCHITECTURE 3318 N. Columbus Street Arlington, VA 22207 T.312.636.3248 / 312.636.4252 www.kasa-arch.com		
GENERAL CONTRACTOR		
CIVIL ENGINEER		
LANDSCAPE ARCHITECT		
anthem structural engineers 303-848-8497 970-300-3338 anthemstructural.com Job #22-048		
M.E.P. & F.P. ENGINEERS		
INTERIOR DESIGNER:		
PROJECT LOCATION		
BASECAMP TOWNHOME 1950 CURVE COURT STEAMBOAT SPRINGS, CO 80487		
DRAWING TITLE		
FOUNDATION DETAILS		
SEAL	DATE: 09/09/2022	
PROFESSIONAL ENGINEER 51274 09/12/2023	DRAWN BY:	
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	PROJECT NO: 22-048	
DRAWING NO: S0510		

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anthemstructural.com Job #22-048

M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

PROJECT LOCATION

BASECAMP TOWNHOME

1950 CURVE COURT
STEAMBOAT SPRINGS, CO 80487

DRAWING TITLE

FLOOR FRAMING DETAILS

SEAL

DATE: _____

09/09/2022

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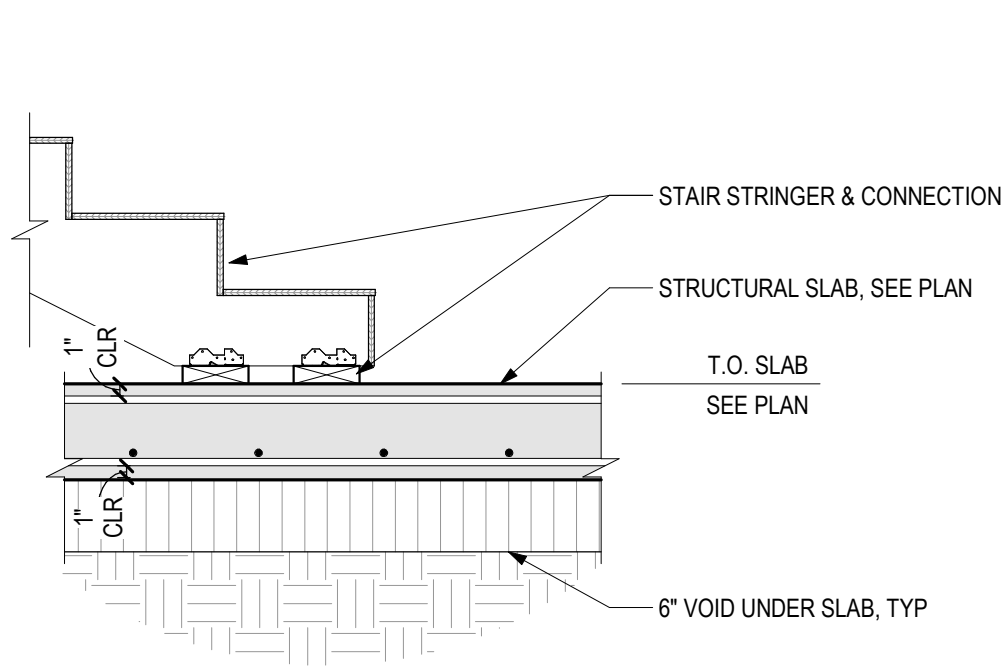
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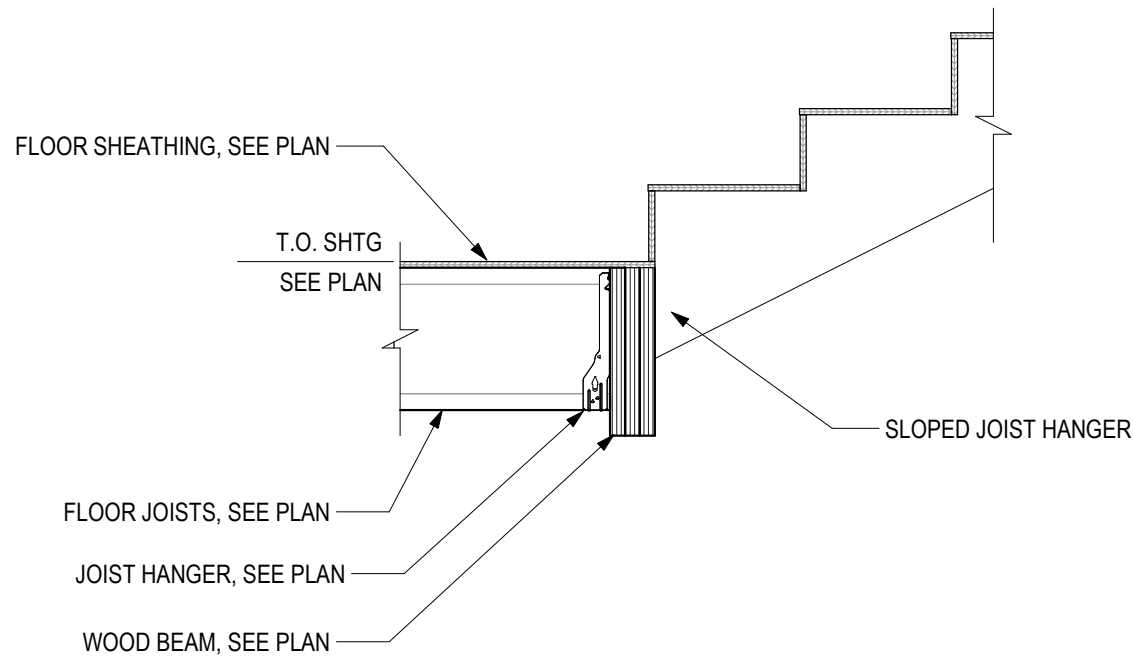
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S0520

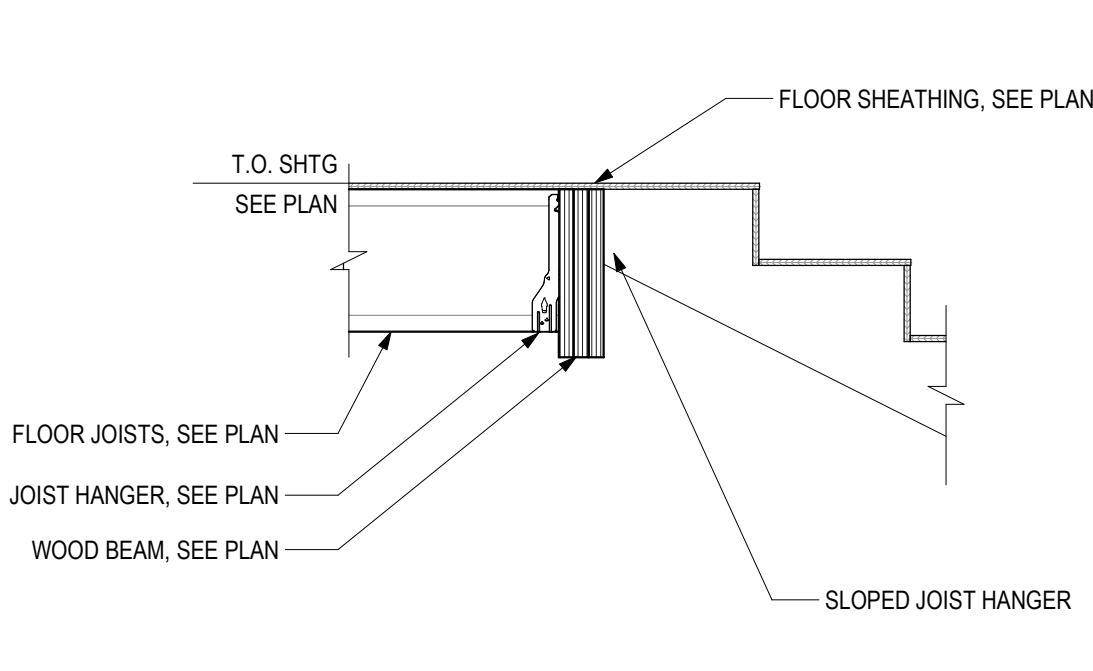
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1 SLAB STAIR
NOT TO SCALE



2 STAIR STRINGER TO LANDING
3/4" = 1'-0"



3 STAIR STRINGER TO LANDING
3/4" = 1'-0"

APPROVAL STAMPS:

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FOR
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No.	Date	Description

SUBMISSIONS & REVISIONS

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M.E.P. & F.P. ENGINEERS

INTERIOR DESIGNER:

PROJECT LOCATION

BASECAMP TOWNHOME
1950 CURVE COURT
STEAMBOAT SPRINGS, CO 80487

DRAWING TITLE

STAIR DETAILS

SEAL



DATE:

09/09/2022

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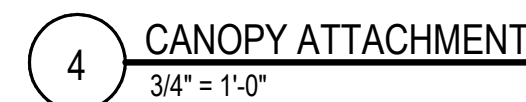
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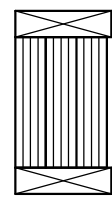
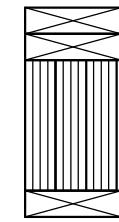
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22-048

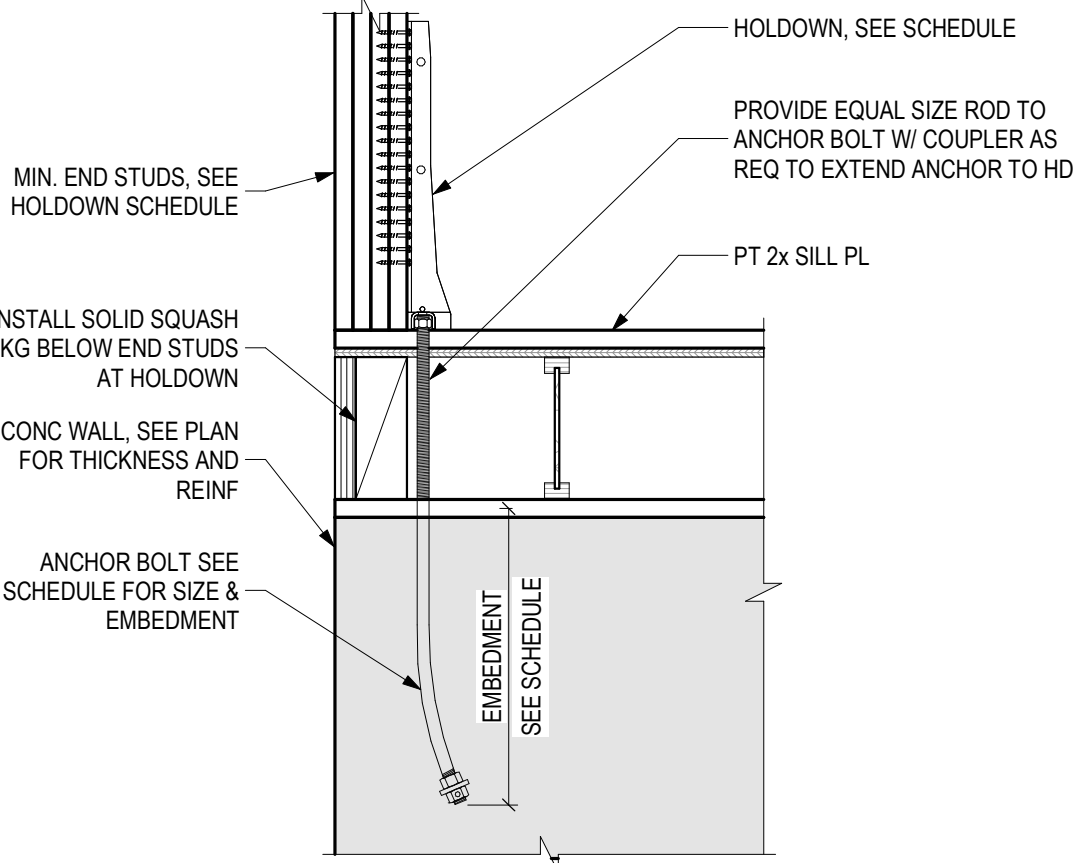
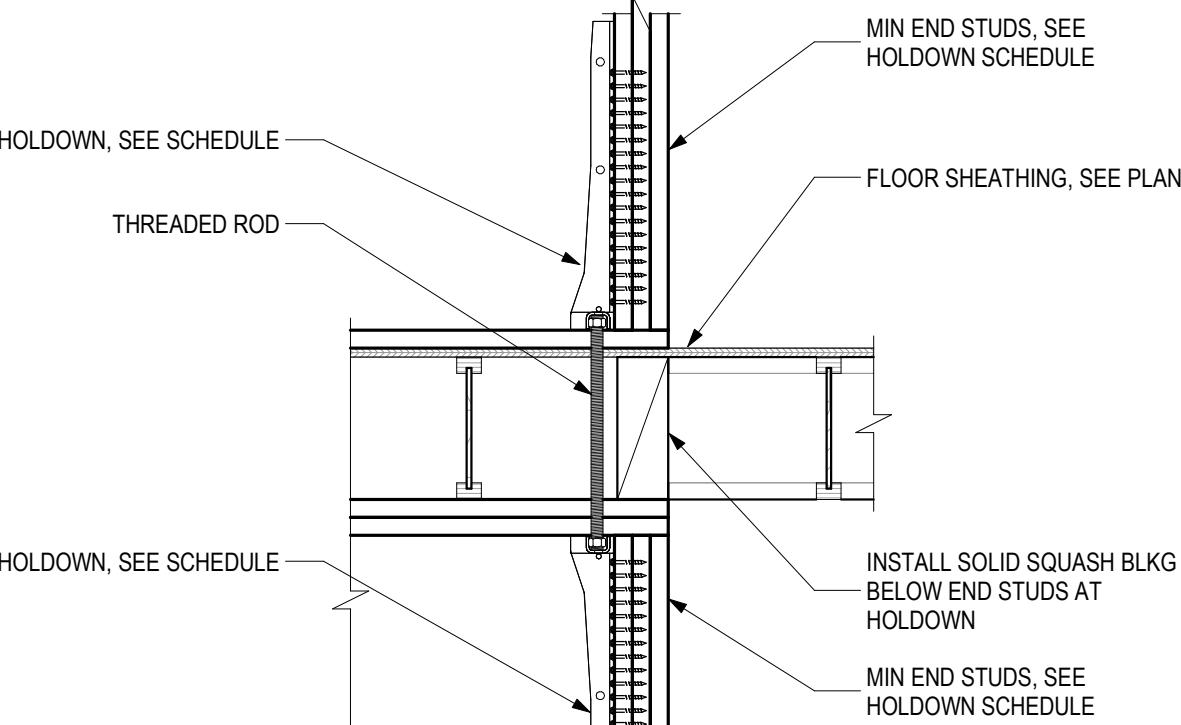
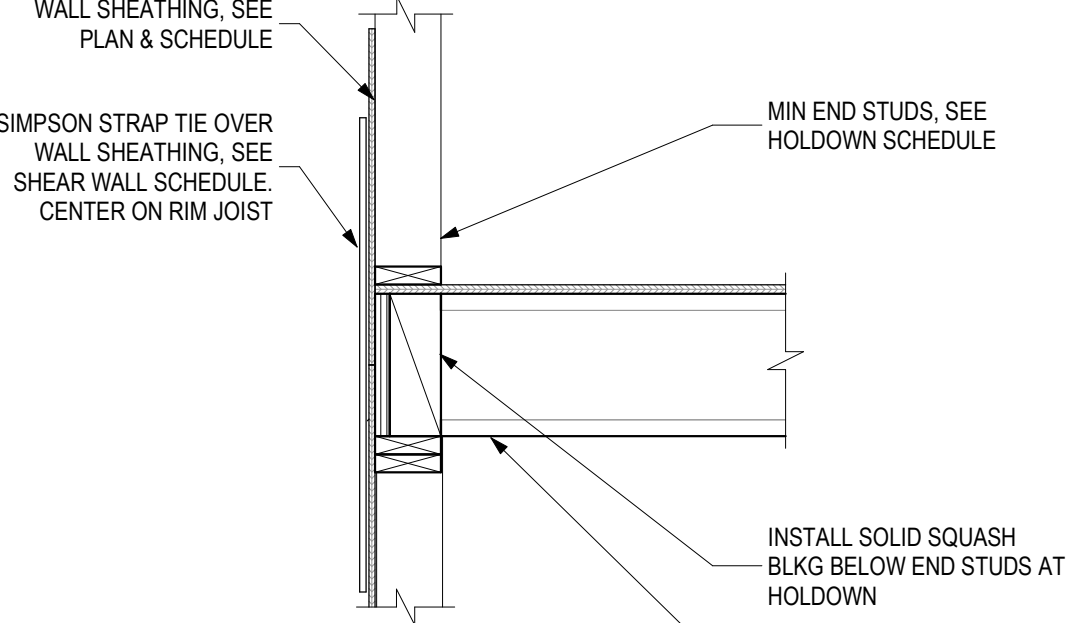
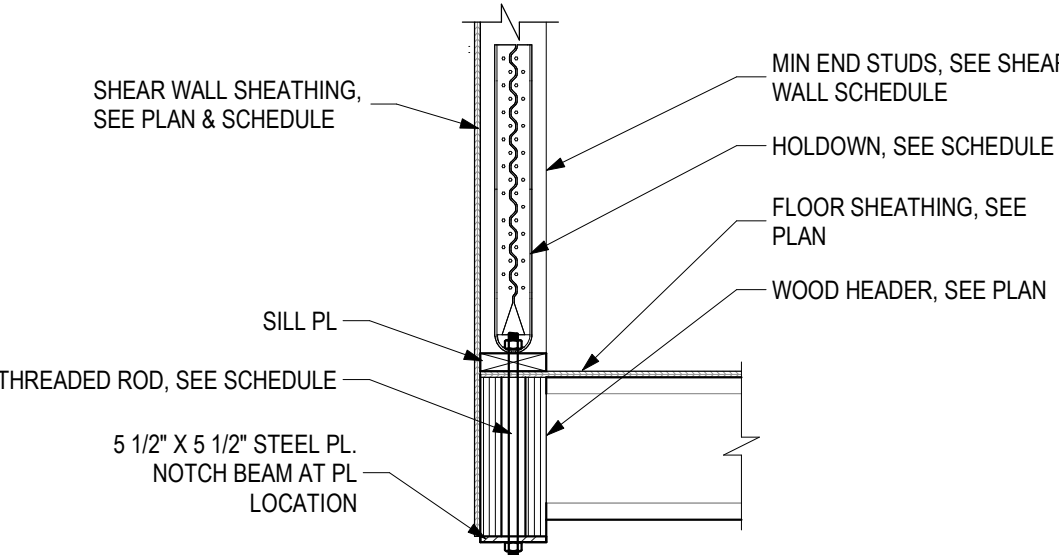
DRAWING NO:

S0521



HEADER SCHEDULE							
MARK	TYPE	OPENING WIDTH	HORIZONTAL MEMBERS	VERTICAL MEMBERS	KING STUDS	TRIMMERS	END CONNECTION
H1	B	-	(2) 2x	(2) 9 1/2" LVL	(3) 2x	(2) 2x	LS50 T&B
H2	A	-	(2) 2x	(2) 2x10	(2) 2x	(1) 2x	LS50
H3	C	-	(3) 2x	(3) 16" LVL	(5) 2x	(4) 2x	LS50 T&B
H4	A	-	(2) 2x	(2) 14" LVL	(3) 2x	(3) 2x	LS50 T&B
NOTES: 1. SEE DETAIL 12/S5.01 AND XX/S5.01 FOR TYPICAL ASSEMBLY. 2. REPLACE 2X8 WITH 2X6 IN 2X6 FRAMED WALLS 2. AT FLUSH HEADER, PROVIDE STUD PACK EA END THAT EQUALS THE COMBINED HEADER AND TRIMMER STUDS. FLUSH HEADER SHALL BEAR ON ENTIRE STUD PACK. 3. USE 1-2X6 SILL PLATE AT OPENINGS <= 6'-0". USE 2-2X6 SILL PLATES AT OPENINGS > 6'-0". 4. FHx INDICATES FLUSH HEADER.				HEADER TYPES  2 HORIZONTAL MEMBERS / 2 VERTICAL MEMBERS (TYPE A)  2 HORIZONTAL MEMBERS / 3 VERTICAL MEMBERS (TYPE B)  3 HORIZONTAL MEMBERS / 3 VERTICAL MEMBERS (TYPE C)			

WOOD SHEAR WALL SCHEDULE							
1.OSB OR PLYWOOD WOOD STRUCTURAL PANEL CONFORMING TO THE REQUIREMENTS FOR ITS TYPE IN DOC PS 1 OR PS 2 UNO. 2.MINIMUM WIDTH OF SHEATHING PANELS AT ENDS OF SHEAR WALLS SHALL BE 16" 3.8d GUN: 0.113"x2x 3/8" 10d GUN: 0.131"x3/4" 4.FASTENERS TO BE MINIMUM 3/8" FROM PANEL EDGE 5.BLOCK AND NAIL ALL PANEL EDGES 6.PER IBC / AWC SDPWS, SHEATHING NAILS SHALL BE DRIVEN FLUSH BUT SHALL NOT FRACTURE THE SURFACE OF THE SHEATHING. SHEATHING PANEL NAILING NOT CONFORMING TO THIS SECTION WILL NOT BE ACCEPTABLE AND WILL HAVE TO BE REINSTALLED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT THE NAIL GUNS USED FOR FASTENING ARE SET AT THE PROPER DEPTH AND/OR AIR PRESSURE TO ACHIEVE THE REQUIRED PENETRATION 7.MINIMUM 7" EMBED FOR CAST IN PLACE ANCHORS. AT CONTRACTORS OPTION, IT IS ACCEPTABLE TO SUBSTITUTE TITEN HD SCREW ANCHORS x 4 1/2" EMBED OF THE SAME SIZE AND SPACING AS INDICATED IN THE ANCHOR TO CONCRETE COLUMN SHOWN ABOVE. 8.INSTALL A MINIMUM OF (2) ANCHOR BOLTS PER EACH SILL PLATE PIECE, SEE PLAN NOTES 9.WHERE SHEATHING IS REQUIRED ON BOTH SIDES, STAGGER END JOINTS OF SHEATHING 16" MIN EA SIDE OF WALL. 10.WALL STUDS TO BE SPACED @ 16" MAX 11.SEE PLAN FOR HOLDOWN TYPE AND LOCATION 12.NO PENETRATIONS GREATER THAN 12"x12" IN SHEAR WALLS. BLOCK AND NAIL ALL EDGES AROUND OPENING 13.NO MECHANICAL OR PLUMBING PENETRATIONS IN TOP AND BOTTOM PLATES 14.SEE DETAILS FOR ATTACHMENT OF DIAPHRAGMS TO SHEAR WALL PLATES *****WHERE SHEAR WALL ASPECT RATIO IS LESS THAN 2:1 AND LOAD IS >400 PLF (ALL TYPES EXCEPT SW1,2,3) THEN 0.229"x3"x3" WASHERS (SIMPSON BPS-3) ARE REQUIRED*****							
TAG	SHEATHING (1,2)	FASTENERS (3,4,5,6)			SOLE PLATE CONNECTION (UNO)		TOP OF WALL CONN TO BLOCKING OR FRAMING (UNO)
		TYPE/SIZE	EDGE SPACING	FIELD SPACING	ANCHORS TO CONCRETE (7,8)	FASTENERS TO WOOD	
SW6	7/16" (ONE SIDE)	8d GUN NAIL	6"	12"	1/2"x10 @ 32" or 5/8"x10 @ 48"	12d GUN NAILS @ 6"	12d GUN TOENAIL @ 4"
SW4	7/16" (ONE SIDE)	8d GUN NAIL	4"	12"	1/2"x10 @ 24" or 5/8"x10 @ 32"	12d GUN NAILS @ 4"	A35 CLIPS @ 16"
SW2	7/16" (ONE SIDE)	8d GUN NAIL	2"	12"	1/2"x10 @ 16" or 5/8"x10 @ 24"	SDS25412 SCREWS @ 8"	A35 CLIPS @ 8"
2SW3	7/16" (BOTH SIDES) (9)	8d GUN NAIL	3"	12"	1/2"x10 @ 12" or 5/8"x10 @ 16"	SDS25412 SCREWS @ 6"	A35 CLIPS @ 6"
2SW2	7/16" (BOTH SIDES) (9)	8d GUN NAIL	2"	12"	1/2"x10 @ 8" or 5/8"x10 @ 12"	SDS25412 SCREWS @ 4"	A35 CLIPS @ 4"

HOLDOWN SCHEDULE					
1. COORDINATE HOLDOWN LOCATIONS WITH ARCHITECTURAL FLOOR PLANS & FRAMING ROUGH OPENINGS AT DOOR/WINDOW LOCATIONS FOR PROPER PLACEMENT AT TRIMIKING STUD ASSEMBLIES. 2. REFER TO THE DETAIL FOR TYPE INDICATED FOR INSTALLATION REQUIREMENTS AND COMPLY WITH ADDITIONAL MNFR'S INSTALLATION RECOMMENDATIONS. 3. WHERE STRAPS ARE INSTALLED OVER MAX 1/2" NOMINAL SHEATHING, USE MINIMUM 2 1/2" LONG FASTENERS. 10d SHORT (0.148" DIA x 1 1/2"). ACCEPTABLE ONLY AT STRAPS DIRECTLY ATTACHED TO FRAMING. NOTE: AT ZIP-R SHEATHING OR GYP SHEATHING, STRAPS MUST BE ATTACHED DIRECTLY TO FRAMING.					
 HOLDOWN TYPE I					
 HOLDOWN TYPE II					
 HOLDOWN TYPE III					
 HOLDOWN TYPE IV					
MARK	HOLDOWN	HOLDOWN TYPE	CONNECTION ABOVE	CONNECTION BELOW	END STUDS
HD1	HDU2	II	(6) 1/4 X 2 1/2" SDS SCREWS	(6) 1/4 X 2 1/2" SDS SCREWS	(2) 2x
HD2	HDU4	II	(10) 1/4 X 2 1/2" SDS SCREWS	(10) 1/4 X 2 1/2" SDS SCREWS	(2) 2x
HD5	HDU14	I	(36) 1/4 X 2 1/2" SDS SCREWS	1" ANCHOR ROD W/ 18" MIN EMBEDMENT INTO CONCRETE	(4) 2x
HD6	MST48	III	(18) 0.162 X 2 1/2" NAILS	(18) 0.162 X 2 1/2" NAILS	(2) 2x
HD7	HST5	III	(6) 5/8" DIA. BOLTS	(6) 5/8" DIA. BOLTS	(3) 2x
HD8	HDU14	IV	(36) 1/4 X 2 1/2" SDS SCREWS	1" ANCHOR ROD W/ PL (SEE DETAIL)	(4) 2x

APPROVAL STAMPS:

REVIEWED
FOR
CODE
COMPLIANCE
12/27/2023

Submissions & Revisions

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M.E.P. & F.P. Engineers

Interior Designer:

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STEAMBOAT SPRINGS, CO. 80487

Drawing Title

Schedules

Seal
Professional Engineer
51274
05/12/2023

Date: 09/09/2022
Drawn By:
Checked By:
Project No: 22-048

Drawing No: S0600