Structural Closeout Assistance

KL&A, Inc.
1717 Washington Avenue, Suite 100
Golden, Colorado 80401
Telephone: (303) 384-9910

October 25, 2023

Matt Disney
Deneuve Construction Services
2334 Spruce St., Suite B
Boulder, CO 80302

Re: Failed Inspection Report

Dear Mr. Disney:

The purpose of this letter is to provide a response to a failed county inspection report dated July 31st, 2023, for the Steamboat Basecamp project located at 1901 Curve Plaza in Steamboat Springs, Colorado. A copy of the inspection report is enclosed for record.

KL&A understands that multiple light pole standards were installed around the site. The light pole bases are 18” diameter, extend 2’-0” above grade and embed 5’-0” minimum into grade. The light poles are 4” diameter x20’-0” tall. Anchor bolts supplied by the light pole manufacturer with the poles are ¾” diameter x17” long ‘L-Bolts’.

KL&A understands that the concrete bases were cast in place before the light poles and anchor bolts were delivered to the site. KL&A is aware that Deneuve substituted 5/8” diameter all-thread rod for the manufacturer supplied L-Bolts. The threaded rod was installed with a nut at the bottom and 18” embedment.

KL&A performed calculations for the pole anchorage, attached. Per the calculation, the 5/8” diameter all-thread rods are adequate for the demand loads on the light standards.

If you have any questions, please feel free to call.

COLORADO LICENSED
PETER MAITLAND KELLY
56031
PROFESSIONAL ENGINEER

Sincerely,

Andrew Smith, PE, SE
Project Engineer

Reviewed By: Peter Kelly, PE, SE
Project Manager, Engineer of Record

FOR REFERENCE ONLY

INSPECTION REPORT

Report Issued: Jul 31, 2023

Inspection Result: **FAILED**

Permit Number: SPREL221585
Issuance Date: Oct 25, 2022
Property Address: 1901 CURVE PLZ;
Applicant: CENTRAL ELECTRIC, LLC
Contractor: CENTRAL ELECTRIC, LLC 2659 Jacob Cir #4 STEAMBOAT SPGS,, CO 80487 (970) 846-8379
To construct: Steamboat basecamp
Temp power and new electrical for new building

On this day we have inspected your construction for **Electrical Rough-in Inspection** and we find the following result of the inspection. If there are any questions, please call the inspector at the address below.

Comments: While on site for ceiling rough inspection noticed uninspected parking lot pole bases on closer examination notice telltale sign of yellow paint on the end of all thread as opposed to UL listed anchor bolts. Correctional will require demolition of pole bases and inspected installation or an engineer special inspection letter stating that all thread for anchor bolts will suffice.

Corrections:

Sincerely,

Tom Cook
Electrical Plan Reviewer/Inspector
tcook@co.routt.co.us

KL&A
Engineers & Builders

Title STEAMBOAT BASECAMP Date 10/23 Job no. 21304
Subject LIGHTPOLE ANCHORAGE By APS Sheet 1 of

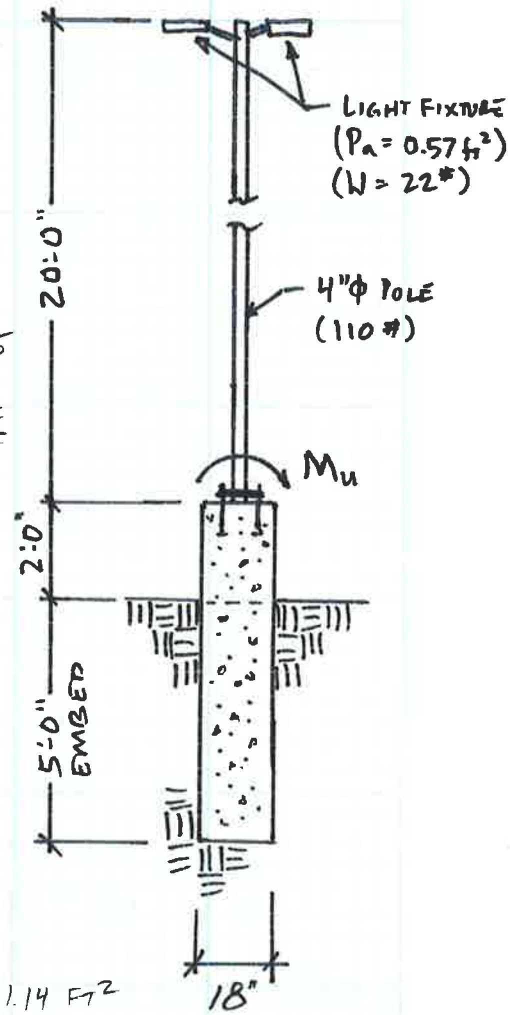
STANDARD LIGHTPOLE ANCHORAGE

WIND DESIGN LOADS

RISK CATEGORY : II
BASIC WIND SPEED, $V = 115$ MPH ($V_{ASD} = 89$ MPH)
EXPOSURE CATEGORY : C

WIND DIRECTIONALITY FACTOR, $K_d = 1.0$
TOPOGRAPHIC FACTOR, $K_{zt} = 1.0$
VELOCITY PRESSURE EXPOSURE COEFFICIENT, $K_z = 0.94$
GROUND ELEVATION FACTOR, $K_e = 0.785$

VELOCITY PRESSURE, $q_h = 0.00256 K_z K_{zt} K_d K_e V^2 = \underline{25 \text{ PSF}}$



ASCE 7-16 §29.4 - WIND LOADS ON OTHER STRUCTURES

$F = q_z G C_f A_f$

GUST EFFECT FACTOR, $G = 0.85$

POLE : FORCE COEFFICIENT, $C_f = 1.2$
PROJECTED AREA, $A_f = 0.33$ FT/FT

$F_{POLE} = q_h G C_f A_f = 8.5$ PLF

FIXTURES : FORCE COEFFICIENT, $C_f = 1.3$
PROJECTED AREA, $A_f = 2(0.57 \text{ FT}^2) = 1.14 \text{ FT}^2$

$F_{FIXTURES} = q_h G C_f A_f = 32\#$

MOMENT @ BASE, $M_u = F_{FIXTURE} H_{FIXTURE} + \frac{F_{POLE} H_{POLE}^2}{2}$

$\underline{M_u = 2340 \text{ FT-LB}}$

SHEAR @ BASE $V_u = F_{FIXTURE} + F_{POLE} H_{POLE}$

$\underline{V_u = 202\#}$

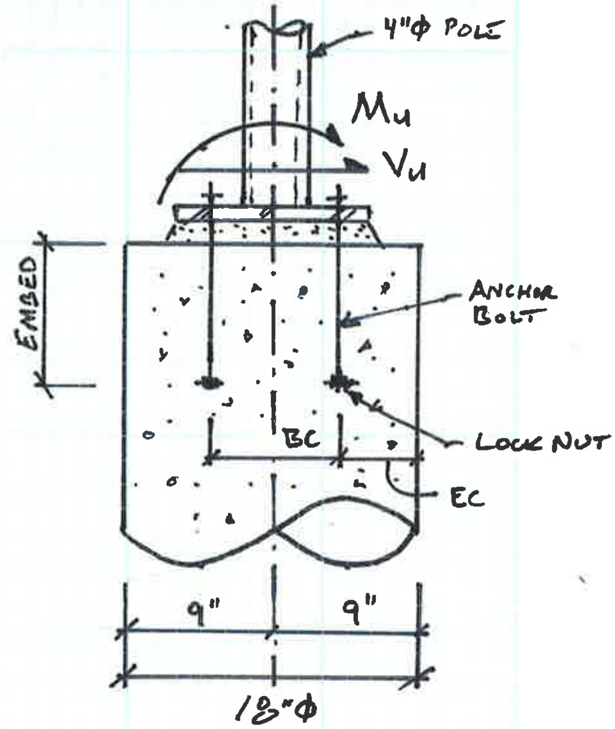
CALCULATIONS CONTINUE FOLLOWING PAGES

STANDARD LIGHTPOLE ANCHORAGE (CONT.)

BASE PLATE REACTIONS

$M_u = 2{,}340$ FT-LB $\approx$ 28080 IN-LB

$V_u = 202^{\#}$



BASE PLATE & ANCHOR BOLT DIMENSIONS

BOLT CIRCLE, BC = 8" (DIAGONAL)

EDGE COVER, EC = 5"

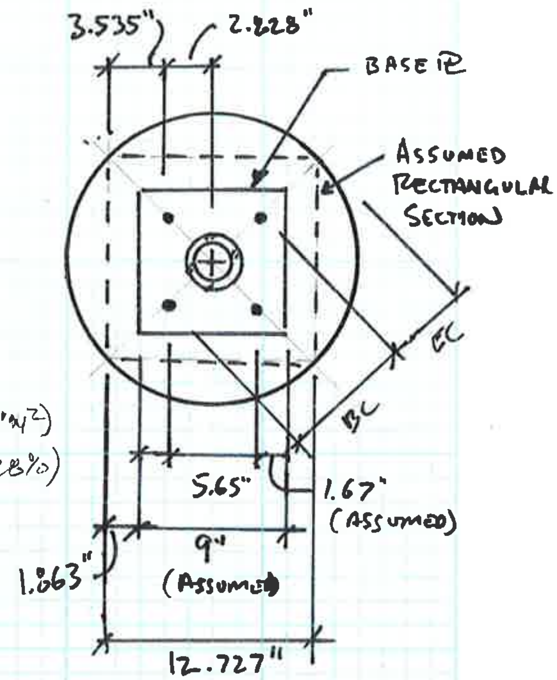
EMBEDMENT = 18"

ANCHOR BOLT DIAMETER, d_b = 5/8"Φ

ANCHOR BOLT GRADE = F1554, GR 36 (ASSUMED)

5/8"Φ ALL THREAD ROD W/ 18" EMBED OKAY

SEE HILTI PROFIS OUTPUT FOLLOWING PAGES



$T_{BOLT} = 2{,}100^{\#}$

$\phi R_N = \phi F_N A_b = 0.75(\underbrace{0.75(58\text{ KSI})}_{F_N})(0.226\text{ IN}^2)$

$= 7{,}373^{\#}$ (DCR = 28%)

Company:		Page:	1
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

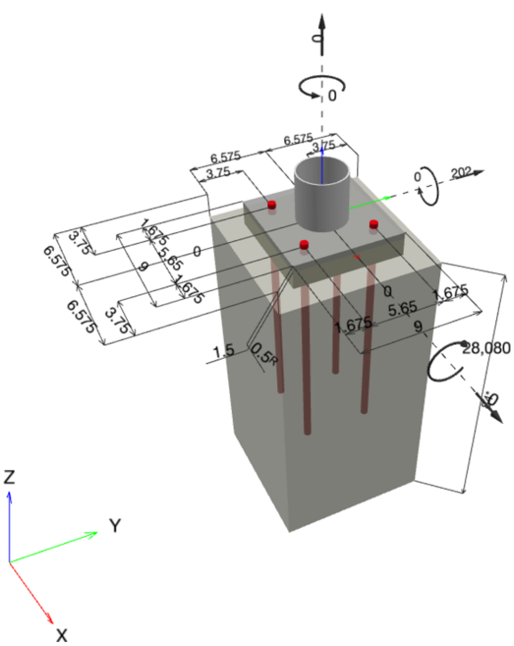
Specifier's comments:

1 Input data

Anchor type and diameter:	**Hex Head ASTM F 1554 GR. 36 5/8**
Item number:	not available
Effective embedment depth:	h_{ef} = 18.000 in.
Material:	ASTM F 1554
Evaluation Service Report:	Hilti Technical Data
Issued I Valid:	- | -
Proof:	Design Method ACI 318-14 / CIP
Stand-off installation:	without clamping (anchor); restraint level (anchor plate): 2.00; e_b = 1.500 in.; t = 0.500 in.
	Hilti Grout: CB-G EG, epoxy, $f_{c,Grout}$ = 14,939 psi
Anchor plate[R] :	l_x x l_y x t = 9.000 in. x 9.000 in. x 0.500 in.; (Recommended plate thickness: not calculated)
Profile:	Round HSS (AISC), HSS4X.125; (L x W x T) = 4.000 in. x 4.000 in. x 0.125 in.
Base material:	cracked concrete, 4000, f_c' = 4,000 psi; h = 84.000 in.
Reinforcement:	tension: condition B, shear: condition B;
	edge reinforcement: none or < No. 4 bar

[R] - The anchor calculation is based on a rigid anchor plate assumption.

Geometry [in.] & Loading [lb, in.lb]



www.hilti.com

Company:		Page:	2
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

1.1 Design results

Case	Description	Forces [lb] / Moments [in.lb]	Seismic	Max. Util. Anchor [%]
1	Combination 1	$N = 0; V_x = 0; V_y = 202; M_x = -28{,}080; M_y = 0; M_z = 0;$	no	63

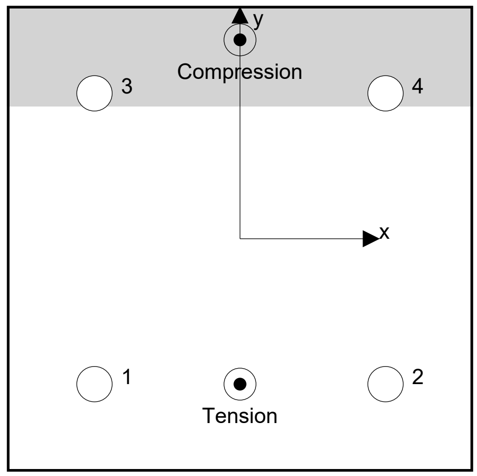
2 Load case/Resulting anchor forces

Anchor reactions [lb]

Tension force: (+Tension, -Compression)

Anchor	Tension force	Shear force	Shear force x	Shear force y
1	2,099	50	0	50
2	2,099	50	0	50
3	0	50	0	50
4	0	50	0	50

max. concrete compressive strain: 0.11 [‰]
max. concrete compressive stress: 490 [psi]
resulting tension force in (x/y)=(0.000/-2.825): 4,197 [lb]
resulting compression force in (x/y)=(0.000/3.865): 4,197 [lb]



Anchor forces are calculated based on the assumption of a rigid anchor plate.

3 Tension load

	Load N_{ua} [lb]	Capacity ϕ N_n [lb]	Utilization $\beta_N = N_{ua}/\phi$ N_n	Status
Steel Strength*	2,099	9,831	22	OK
Pullout Strength*	2,099	10,170	21	OK
Concrete Breakout Failure**	4,197	6,685	63	OK
Concrete Side-Face Blowout, direction y-**	4,197	22,393	19	OK

* highest loaded anchor **anchor group (anchors in tension)

3.1 Steel Strength

$N_{sa} = A_{se,N}\, f_{uta}$	ACI 318-14 Eq. (17.4.1.2)
$\phi\ N_{sa} \geq N_{ua}$	ACI 318-14 Table 17.3.1.1

Variables

$A_{se,N}$ [in.^{2}]	f_{uta} [psi]
0.23	58,000

Calculations

N_{sa} [lb]
13,108

Results

N_{sa} [lb]	ϕ_{steel}	$\phi\ N_{sa}$ [lb]	N_{ua} [lb]
13,108	0.750	9,831	2,099

3.2 Pullout Strength

$N_{pN} = \psi_{c,p}\, N_p$	ACI 318-14 Eq. (17.4.3.1)
$N_p = 8\, A_{brg}\, f'_c$	ACI 318-14 Eq. (17.4.3.4)
$\phi\ N_{pN} \geq N_{ua}$	ACI 318-14 Table 17.3.1.1

Variables

$\psi_{c,p}$	A_{brg} [in.^{2}]	λ_a	f'_c [psi]
1.000	0.45	1.000	4,000

Calculations

N_p [lb]
14,528

Results

N_{pn} [lb]	$\phi_{concrete}$	$\phi\ N_{pn}$ [lb]	N_{ua} [lb]
14,528	0.700	10,170	2,099

3.3 Concrete Breakout Failure

$N_{cbg} = \left(\frac{A_{Nc}}{A_{Nc0}}\right) \psi_{ec,N}\ \psi_{ed,N}\ \psi_{c,N}\ \psi_{cp,N}\ N_b$	ACI 318-14 Eq. (17.4.2.1b)
$\phi\ N_{cbg} \geq N_{ua}$	ACI 318-14 Table 17.3.1.1
A_{Nc} see ACI 318-14, Section 17.4.2.1, Fig. R 17.4.2.1(b)	
$A_{Nc0} = 9\ h_{ef}^2$	ACI 318-14 Eq. (17.4.2.1c)
$\psi_{ec,N} = \left(\frac{1}{1 + \frac{2\ e'_N}{3\ h_{ef}}}\right) \leq 1.0$	ACI 318-14 Eq. (17.4.2.4)
$\psi_{ed,N} = 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5h_{ef}}\right) \leq 1.0$	ACI 318-14 Eq. (17.4.2.5b)
$\psi_{cp,N} = \text{MAX}\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5h_{ef}}{c_{ac}}\right) \leq 1.0$	ACI 318-14 Eq. (17.4.2.7b)
$N_b = k_c\ \lambda_a\ \sqrt{f'_c}\ h_{ef}^{1.5}$	ACI 318-14 Eq. (17.4.2.2a)

Variables

h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]	$\psi_{c,N}$
6.267	0.000	0.000	3.750	1.000

c_{ac} [in.]	k_c	λ_a	f'_c [psi]
-	24	1.000	4,000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\psi_{ec1,N}$	$\psi_{ec2,N}$	$\psi_{ed,N}$	$\psi_{cp,N}$	N_b [lb]
172.92	353.44	1.000	1.000	0.820	1.000	23,812

Results

N_{cbg} [lb]	$\phi_{concrete}$	$\phi\ N_{cbg}$ [lb]	N_{ua} [lb]
9,549	0.700	6,685	4,197

3.4 Concrete Side-Face Blowout, direction y-

$N_{sb} = 160\ c_{a1} \sqrt{A_{brg}}\ \lambda_a \sqrt{f'_c}$ ACI 318-14 Eq. (17.4.4.1)

$N_{sbg} = \alpha_{group}\ N_{sb}$ ACI 318-14 Eq. (17.4.4.2)

$\phi\ N_{sbg} \geq N_{ua}$ ACI 318-14 Table 17.3.1.1

$\alpha_{group} = \left(1 + \frac{s}{6\ c_{a1}}\right)$ see ACI 318-14, Section 17.4.4.2, Eq. (17.4.4.2)

Variables

c_{a1} [in.]	c_{a2} [in.]	A_{brg} [in.^{2}]	λ_a	f'_c [psi]	s [in.]
3.750	3.750	0.45	1.000	4,000	5.650

Calculations

α_{group}	N_{sb} [lb]
1.251	25,569

Results

N_{sbg} [lb]	$\phi_{concrete}$	$\phi\ N_{sbg}$ [lb]	$N_{ua,edge}$ [lb]
31,989	0.700	22,393	4,197

4 Shear load

	Load V_{ua} [lb]	Capacity ϕ V_n [lb]	Utilization $\beta_V = V_{ua}/\phi$ V_n	Status
Steel Strength*	50	4,090	2	OK
Steel failure (with lever arm)*	50	495	11	OK
Pryout Strength**	202	25,823	1	OK
Concrete edge failure in direction y+**	202	2,837	8	OK

* highest loaded anchor   **anchor group (relevant anchors)

4.1 Steel Strength

$V_{sa} = 0.6\ A_{se,V}\ f_{uta}$ ACI 318-14 Eq. (17.5.1.2b)

$\phi\ V_{steel} \geq V_{ua}$ ACI 318-14 Table 17.3.1.1

Variables

$A_{se,V}$ [in.^{2}]	f_{uta} [psi]
0.23	58,000

Calculations

V_{sa} [lb]
7,865

Results

V_{sa} [lb]	ϕ_{steel}	ϕ_{eb}	$\phi\ V_{sa}$ [lb]	V_{ua} [lb]
7,865	0.650	0.800	4,090	50

Company:		Page:	7
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

4.2 Steel failure (with lever arm)

V_s^M	$= \frac{\alpha_M \cdot M_s}{L_b}$	bending equation for stand-off
M_s	$= M_s^0 \left(1 - \frac{N_{ua}}{\phi\, N_{sa}}\right)$	resultant flexural resistance of anchor
M_s^0	$= (1.2)\,(S)\,(f_{u,min})$	characteristic flexural resistance of anchor
$\left(1 - \frac{N_{ua}}{\phi\, N_{sa}}\right)$		reduction for tensile force acting simultaneously with a shear force on the anchor
S	$= \frac{\pi (d)^3}{32}$	elastic section modulus of anchor bolt at concrete surface
L_b	$= z + (n)(d_0)$	internal lever arm adjusted for spalling of the surface concrete
$\phi\, V_s^M$	$\geq V_{ua}$	ACI 318-14 Table 17.3.1.1

Variables

α_M	$f_{u,min}$ [psi]	N_{ua} [lb]	$\phi\, N_{sa}$ [lb]	z [in.]	n	d_0 [in.]
2.00	58,000	2,099	9,831	1.750	0.500	0.625

Calculations

M_s^0 [in.lb]	$\left(1 - \frac{N_{ua}}{\phi\, N_{sa}}\right)$	M_s [in.lb]	L_b [in.]
998	0.787	785	2.062

Results

V_s^M [lb]	ϕ_{steel}	$\phi\, V_s^M$ [lb]	V_{ua} [lb]
761	0.650	495	50

4.3 Pryout Strength

$V_{cpg} = k_{cp} \left[\left(\frac{A_{Nc}}{A_{Nc0}} \right) \psi_{ec,N} \; \psi_{ed,N} \; \psi_{c,N} \; \psi_{cp,N} \; N_b \right]$ ACI 318-14 Eq. (17.5.3.1b)

$\phi \; V_{cpg} \geq V_{ua}$ ACI 318-14 Table 17.3.1.1

A_{Nc} see ACI 318-14, Section 17.4.2.1, Fig. R 17.4.2.1(b)

$A_{Nc0} = 9 \; h_{ef}^2$ ACI 318-14 Eq. (17.4.2.1c)

$\psi_{ec,N} = \left(\frac{1}{1 + \frac{2 \; e'_N}{3 \; h_{ef}}} \right) \leq 1.0$ ACI 318-14 Eq. (17.4.2.4)

$\psi_{ed,N} = 0.7 + 0.3 \left(\frac{c_{a,min}}{1.5 h_{ef}} \right) \leq 1.0$ ACI 318-14 Eq. (17.4.2.5b)

$\psi_{cp,N} = \mathrm{MAX}\left(\frac{c_{a,min}}{c_{ac}}, \frac{1.5 h_{ef}}{c_{ac}} \right) \leq 1.0$ ACI 318-14 Eq. (17.4.2.7b)

$N_b = k_c \; \lambda_a \; \sqrt{f'_c} \; h_{ef}^{1.5}$ ACI 318-14 Eq. (17.4.2.2a)

Variables

k_{cp}	h_{ef} [in.]	$e_{c1,N}$ [in.]	$e_{c2,N}$ [in.]	$c_{a,min}$ [in.]
2	2.500	0.000	0.000	3.750

$\psi_{c,N}$	c_{ac} [in.]	k_c	λ_a	f'_c [psi]
1.000	∞	24	1.000	4,000

Calculations

A_{Nc} [in.^{2}]	A_{Nc0} [in.^{2}]	$\psi_{ec1,N}$	$\psi_{ec2,N}$	$\psi_{ed,N}$	$\psi_{cp,N}$	N_b [lb]
172.92	56.25	1.000	1.000	1.000	1.000	6,000

Results

V_{cpg} [lb]	$\phi_{concrete}$	$\phi \; V_{cpg}$ [lb]	V_{ua} [lb]
36,890	0.700	25,823	202

4.4 Concrete edge failure in direction y+

$V_{cbg} = \left(\frac{A_{Vc}}{A_{Vc0}}\right) \psi_{ec,V}\ \psi_{ed,V}\ \psi_{c,V}\ \psi_{h,V}\ \psi_{parallel,V}\ V_b$ ACI 318-14 Eq. (17.5.2.1b)

$\phi\ V_{cbg} \geq V_{ua}$ ACI 318-14 Table 17.3.1.1

A_{Vc} see ACI 318-14, Section 17.5.2.1, Fig. R 17.5.2.1(b)

$A_{Vc0} = 4.5\ c_{a1}^2$ ACI 318-14 Eq. (17.5.2.1c)

$\psi_{ec,V} = \left(\frac{1}{1 + \frac{2e_V'}{3c_{a1}}}\right) \leq 1.0$ ACI 318-14 Eq. (17.5.2.5)

$\psi_{ed,V} = 0.7 + 0.3\left(\frac{c_{a2}}{1.5c_{a1}}\right) \leq 1.0$ ACI 318-14 Eq. (17.5.2.6b)

$\psi_{h,V} = \sqrt{\frac{1.5c_{a1}}{h_a}} \geq 1.0$ ACI 318-14 Eq. (17.5.2.8)

$V_b = \left(7\left(\frac{l_e}{d_a}\right)^{0.2} \sqrt{d_a}\right) \lambda_a \sqrt{f_c'}\ c_{a1}^{1.5}$ ACI 318-14 Eq. (17.5.2.2a)

Variables

c_{a1} [in.]	c_{a2} [in.]	e_{cV} [in.]	$\psi_{c,V}$	h_a [in.]
3.750	3.750	0.000	1.000	84.000

l_e [in.]	λ_a	d_a [in.]	f_c' [psi]	$\psi_{parallel,V}$
5.000	1.000	0.625	4,000	1.000

Calculations

A_{Vc} [in.^{2}]	A_{Vc0} [in.^{2}]	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{h,V}$	V_b [lb]
73.97	63.28	1.000	0.900	1.000	3,852

Results

V_{cbg} [lb]	$\phi_{concrete}$	$\phi\ V_{cbg}$ [lb]	V_{ua} [lb]
4,053	0.700	2,837	202

5 Combined tension and shear loads

β_N	β_V	ζ	Utilization $\beta_{N,V}$ [%]	Status
0.628	0.102	5/3	49	OK

$\beta_{NV} = \beta_N^{\zeta} + \beta_V^{\zeta} <= 1$

Company:		Page:	10
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

6 Warnings

- The anchor design methods in PROFIS Engineering require rigid anchor plates per current regulations (AS 5216:2021, ETAG 001/Annex C, EOTA TR029 etc.). This means load re-distribution on the anchors due to elastic deformations of the anchor plate are not considered - the anchor plate is assumed to be sufficiently stiff, in order not to be deformed when subjected to the design loading. PROFIS Engineering calculates the minimum required anchor plate thickness with CBFEM to limit the stress of the anchor plate based on the assumptions explained above. The proof if the rigid anchor plate assumption is valid is not carried out by PROFIS Engineering. Input data and results must be checked for agreement with the existing conditions and for plausibility!
- Condition A applies where the potential concrete failure surfaces are crossed by supplementary reinforcement proportioned to tie the potential concrete failure prism into the structural member. Condition B applies where such supplementary reinforcement is not provided, or where pullout or pryout strength governs.
- ACI 318 does not specifically address anchor bending when a stand-off condition exists. PROFIS Engineering calculates a shear load corresponding to anchor bending when stand-off exists and includes the results as a shear Design Strength!
- For additional information about ACI 318 strength design provisions, please go to https://submittals.us.hilti.com/PROFISAnchorDesignGuide/

Fastening meets the design criteria!

Company:		Page:	11
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

7 Installation data

Profile: Round HSS (AISC), HSS4X.125; (L x W x T) = 4.000 in. x 4.000 in. x 0.125 in.

Hole diameter in the fixture: d_f = 0.687 in.

Plate thickness (input): 0.500 in.

Recommended plate thickness: not calculated

Anchor type and diameter: Hex Head ASTM F 1554 GR. 36 5/8

Item number: not available

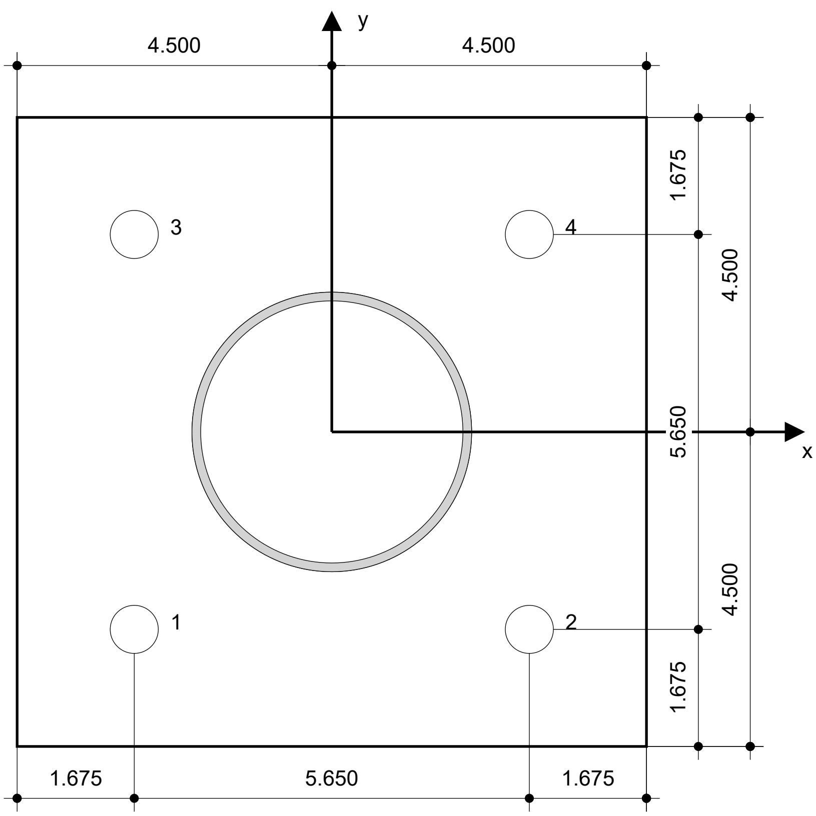
Maximum installation torque: -

Hole diameter in the base material: - in.

Hole depth in the base material: 18.000 in.

Minimum thickness of the base material: 18.922 in.

Hilti Hex Head headed stud anchor with 18 in embedment, 5/8, Steel galvanized, installation per instruction for use



Coordinates Anchor [in.]

Anchor	x	y	c_{-x}	c_{+x}	c_{-y}	c_{+y}
1	-2.825	-2.825	3.750	9.400	3.750	9.400
2	2.825	-2.825	9.400	3.750	3.750	9.400
3	-2.825	2.825	3.750	9.400	9.400	3.750
4	2.825	2.825	9.400	3.750	9.400	3.750

Company:		Page:	12
Address:		Specifier:	
Phone I Fax:	|	E-Mail:	
Design:	Concrete - Oct 25, 2023	Date:	10/25/2023
Fastening point:			

8 Remarks; Your Cooperation Duties

- Any and all information and data contained in the Software concern solely the use of Hilti products and are based on the principles, formulas and security regulations in accordance with Hilti's technical directions and operating, mounting and assembly instructions, etc., that must be strictly complied with by the user. All figures contained therein are average figures, and therefore use-specific tests are to be conducted prior to using the relevant Hilti product. The results of the calculations carried out by means of the Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors, the completeness and the relevance of the data to be put in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an expert, particularly with regard to compliance with applicable norms and permits, prior to using them for your specific facility. The Software serves only as an aid to interpret norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- You must take all necessary and reasonable steps to prevent or limit damage caused by the Software. In particular, you must arrange for the regular backup of programs and data and, if applicable, carry out the updates of the Software offered by Hilti on a regular basis. If you do not use the AutoUpdate function of the Software, you must ensure that you are using the current and thus up-to-date version of the Software in each case by carrying out manual updates via the Hilti Website. Hilti will not be liable for consequences, such as the recovery of lost or damaged data or programs, arising from a culpable breach of duty by you.