PROJECT DESCRIPTION

- 1. Project consists of a renovation to a single story existing building. Foundations consist of spread footings. Superstructure is wood framed floors. Existing lateral system is wood shear walls. New scope of work includes steel moment frame and associated modified foundations.
- 2. This description is for general orientation only. The General Contractor is responsible for all scope items described in the drawings and project specifications as well as for all material and labor that can reasonably be inferred there from.

GENERAL APPLICATION

- 1. These drawings must be used in conjunction with the architectural drawings on the project to clearly define all requirements for construction.
- 2. No Contractor should attempt to bid nor construct any portion of this project without consulting the project architectural, mechanical, and electrical documents.
- 3. All things which, in the opinion of the Contractor, appear to be deficiencies, omissions, contradictions, or ambiguities in the drawings shall be brought to the attention of the Structural Engineer. Corrections or written interpretations shall be issued before affected work may proceed.
- 4. The Contractor shall inform the Structural Engineer, clearly and explicitly in writing of any deviation or substitution from requirements of the contract documents. Contractor shall not be relieved of any requirement of the contract documents by virtue of the Structural Engineer's review of shop drawings, project data, etc., unless the Contractor has clearly and explicitly informed the Structural Engineer in writing of any deviations or substitutions at time of submission.

EXISTING CONSTRUCTION

1. Information regarding existing structural systems is based on drawings prepared by Lykekn and Kramer dated 06/26/1998.

- 2. Existing Conditions:
- A. The current design is based on the best information available, with the understanding that not all conditions have been observed and unobserved conditions may not be fully understood.
- B. All existing information, dimensions, elevations, etc. shall be considered approximate and exiting conditions shall be field verified by the Contractor prior to ordering or fabricating material. Structural Engineer and Architect shall be informed in writing of any discrepancies.
- C. Remodeling, retrofit, renovation, or rehabilitation of an existing building requires that certain assumptions be made regarding existing conditions. The General Contractor must immediately notify the structural engineer if any existing condition deviates from those indicated in the Contract Documents.
- 3. Demolition and Shoring:
- A. The General Contractor is responsible for shoring of existing structure where required during demolition and new construction.
- B. The current interior walls and finishes may be significant to the stability of the structure and may be providing inadvertent load paths that are not observable and may impact how demolition impacts the structure.
- C. The General Contractor should be prepared to brace and shore framing during demolition.
- D. Demolition and reshoring are best accomplished prior to significant snowfall, if possible. If demolition takes place during the significant snowfall season, the Contractor's shoring design should account for this.
- 4. No openings, nor any changes or additions, shall be made in any existing structural elements without written approval of the Structural Engineer. Where the function of an existing element as structural or non-structural is unclear, the determination of its function will be made solely by the Structural Engineer.

MISCELLANEOUS NOTES

1. The Contractor is solely responsible for all safety regulations, programs, and precautions related to all work on this project.

- 2. The Contractor is solely responsible for the protection of persons and property either on or adjacent to the project and shall protect it against injury, damage, or loss.
- 3. Means and methods of construction and erection of structural materials are solely the Contractor's responsibility.
- 4. The structure is designed to function as a unit upon completion of construction of the project and then, only to support the design loads indicated. The Contractor is responsible for means, methods and sequence of construction, and the adequacy of the structure to support loads occurring during construction of the project. Furnish all temporary bracing, shoring, and/or support as may be required.
- 5. No openings, nor any change in size, dimension or location shall be made in any structural element without written approval of the Structural Engineer.
- Where dimensions or weights of MEP equipment or systems are variable from manufacturer to manufacturer, verify dimensions and weights shown on drawings with selected manufacturer prior to ordering materials. Notify Structural Engineer of discrepancies.
- 7. Do not place equipment when shipping or operating weight exceeds weight indicated on structural drawings.
- 8. Openings 1'-4" or less on a side are generally not shown on the structural drawings. Refer to drawings of other consultants for such openings.
- 9. Openings through floors and/or roofs for passage of utilities are not located nor dimensioned on structural drawings. Contractor shall obtain and coordinate such locations and dimensions with the contractor requiring the opening.
- 10. Show all openings through structural members on shop drawings and submit for review. Openings not shown on structural drawings are subject to acceptance and shall be specifically indicated for review and acceptance.
- 11. Verify elevator and escalator pit dimensions, locations, loadings, and details with the elevator/escalator supplier prior to the fabrication and/or installation of any material.

MISCELLANEOUS NOTES (continued)

- 12. Fireproofing of structural elements is not shown on the structural drawings. Refer to the specifications and architectural documents.
- 13. Do not scale these drawings, use the dimensions shown. In case of conflict, request clarification from Architect and Structural Engineer.
- 14. No structural modifications, alterations, or repairs shall be made without prior review by Structural Engineer. Submit details and calculations prepared by a professional engineer registered in state where project is located and employed by Contractor.
- 15. Where structure is to be used for staging or temporary storage area, the Contractor shall verify that unit loads do not exceed the design loads for the structure.

QUALITY ASSURANCE AND QUALITY CONTROL

- 1. The Contractor is responsible for assuring quality, including workmanship and materials furnished by subcontractors and suppliers.
- 2. Inspection or testing by the Owner does not relieve the Contractor of the responsibility to perform the work in accordance with the Contract Documents.
- 3. Workmanship: The Contractor is responsible and shall bear the cost of correcting work which does not conform to the specified requirements. 4. Correct deficient work by means acceptable to the Architect. The cost of extra work incurred by the Architect to approve corrective work shall be
- borne by the Contractor. The Owner's Testing Agency shall perform testing and special inspections
- required by the structural documents, building code, and the local authority. The Testing Agency shall comply with ASTM E329 and upon completion of work, the Testing Agency shall furnish a certificate of compliance, signed by the professional engineer overseeing special inspections and testing. The professional engineer must be registered and licensed in the state where the project is located.
- 6. The individual employed by the Testing Agency, responsible for overseei testing and inspection of soils and foundations shall be a professional engineer practicing the discipline of geotechnical engineering, referred to as the Geotechnical Engineer in the structural portion of the construction documents. The Geotechnical Engineer is responsible for testing and inspections of soils, earthwork, and foundations for conformance to the foundation design and the geotechnical report. See foundation section o the General Notes.
- 7. See special inspections section of the General Notes for required testing and inspection.

SPECIAL INSPECTION

- Special inspection and testing shall be performed as required by the local jurisdiction, the building code, and the construction documents. See quality assurance section of the General Notes.
- 2. Coordinate and schedule inspection and testing prior to the start of work requiring inspection and testing while providing special inspector reasonable notice.
- 3. All deficiencies shall be corrected for acceptance by the Testing Agency. 4. Inspections performed by the local jurisdiction do not replace inspection
- or testing required by the Owner's Testing Agency. 5. Special inspection and testing is required for the items shown in the "Special Inspections and Testing" Table.

SPECIFICATIONS

These General Notes are intended to function as the structural portion of project specifications.

SUBMITTALS

- 1. See Material sections of these General Notes for required shop drawings 2. Submit one (1) copy of the required information (Manufacturers Data,
- Shop Drawings, etc) via electronic media (PDF or similar). 3. Reproducible copies of contract documents shall not be used.
- 4. Submittals shall be sent directly to the Architect for review and distribution.
- 5. Submittals shall be reviewed by Contractor and Subcontractor prior to submission. Drawings shall bear Contractor's approval stamp accepting responsibility for coordination of dimensions shown in the contract documents, quantities, and coordination with other trades.
- 6. Allow 14 calendar days in the Structural Engineer's office for review of submittals.
- 7. Submittals will be returned to the Architect with Structural Engineer's review comments via electronic media.

EEN PREPAREL I NOT SHOWN NATURE AND ,

BE IN ,

ANY ANY PRE

THAT INFO NOR FOR . RED TO BE

OTHERS

BE BE

ONS PE EPARED SHALL

ORMATION SHOWN AND THE CALCU RESPONSIBILITY FOR INFORMATION S THAT ARE ISSUED BUT NOT SEAL

INDICATES THAT THE INFO SEAL DOES NOT IMPLY F SED PROJECTS, DRAWINGS

ORD.

ERS SEAL C ENGINEER-ICALLY DISC

ЩΥ

N.

FOR FOR ICATIC

CHITEC: MENTS CLARIFI

Local Jurisdiction: ROUTTE COUNTY BUILDING DEF	PARTMENT	
IEBC Alteration Level: LEVEL 3		
Historic Designation: No		
Wind Loading		
Basic Wind Speed	Vult= 115 MPH	Vasd= 89.1 MP
Exposure Category	(2
GCpi	+/-	0.18
Ground Elevation Factor, Ke	0.7	852
Ultimate Wind Base Shear	0,00	
East/West	22	kips
North/South	48	kips
Ultimate Wind Design Pressure Components & Cladding, PSF	20ft ² 50)ft ² 100ft ²
Interior Roof Zone (Zone 1)	16/-47.7 16/-	29.0 16/-16.
Roof End Zone (Zone 2)	16/-60.1 16/-	47.7 16/-38.
Corner Roof Zone (Zone 3)	16/-70.8 16/-	55.2 16/-43
Interior Wall Zone (Zone 4)	24.6/-26.8 23.1/	/-25.3 21.9/-24
Wall end Zone (Zone 5)	24.6/-32.3 23.1/	-29.2 21.9/-26
Parapet	/0.8/-43.0 01.7/	-38.9 50.3/-35
Soismic Loading		
Mapped Spectral Response Acceleration		
Ss	0 -	325
S1	0.0)83
Site Class	(C
Spectral Response Coefficients		
Sds	0.3	334
Sd1	0.1	L44
Seismic Design Category	(N/S Direction
Basic Seismic Force Resisting System	E/W Direction: Wood shear walls	Steel not detail for seismic
Response Modification Factor, R	6.5	3
Over-Strength Factor, Ω_0	2.5	3
Deflection Amplification Factor, Cd	4	3
Analysis Procedure Used	Equivalent Latera	U.III
Illtimate Seismic Base Shear	15.4	15.4
Snow Loading	(Notes	2,3,5)
Ground Snow Load, Pg	105	psf
Importance Factor, Is	1	.0
Terrain Category	(С
Exposure Factor, Ce	1	.0
Thermal Factor, Ct	1	.0
Slope Factor, Cs	1	.0
Live Loads and Superimposed Dead Loads	(Note	es 4,5)
Foundations Geotechnical Engineer Information:	Timothy S. Tr	
Geoteennear Engineer miormation.	Northwest Colora	ado Consultants
	2580 Coppe	r Ridge Drive
	Steamboat Spr	ings. CO 80487
	(970) 8	79-7888
	Geotech Proje	ect #20-11961
	Date of Repor	t: 03/15/2021
Active Equivalent Fluid Pressure	45 p	osf/ft
At-Rest Equivalent Fluid Pressure	55 p	osf/ft
Passive Equivalent Fluid Pressure	250	psf/ft
Sliding Friction Coefficient	0	.4
Allowable Bearing Capacity	3,00	0 psf
Minimum Frost Depth	48	in
Foundation Schedule Located on:	SOS	501
Referenced Datum	100'-0'' = First = 6668	Floor Elevation
NOTES:		-
 The governing building code defines the applica standards. Where governing building code does standards, the latest adition shall be used. 	ble edition of referen not define reference	ced codes and d codes and
stanuarus, the latest euition shall be used.	inty Snow Load Mapp 21. I sloped roofs are calc	er provied by the ulated in
 Ground snow load is according to the Routt Cou Routt County Building Department on 07/07/20 All snow loads on the structure for both flat and accordance with the 2018 IBC and based on the snow loads consider the following loads on the 	ground snow load sta	



-PLAN KEYS AND SYMBOLS		ABBREVIATIONS] [ABBREVIATIONS	
	AB	anchor bolt	KLF	1000 pounds per lineal foot	
	ADDNL AFF	additional above finish floor	1	length	
	ALT	alternate	LAT	lateral	
	ARCH	architectural	LBS LLH	pounds long leg horizontal	
	B, B/	bottom of	LLV	long leg vertical	
	BLDG BM	beam	LONG	low point	
	BOT BRG	bottom bearing	LSL I VI	laminated strand lumber	
	BS	both sides	LW, LWT	lightweight	
	BTWN	between	MAS	masonry	
	CFS	cold-formed steel	МАХ	maximum	
	CJ	construction / control joint	MFR	manufacturer	
	CJP CLR	complete joint penetration clear	MIN MTL	minimum metal	
ECT	CLT	cross laminated timber			
	CMU COL	concrete masonry unit column	(N) No	Number	
	CONC	concrete	NOM	nominal near side	
	CONST	construction	NW, NWT	normal weight	
TAILS	CONT GLC	continuous alulam column	0/F	outside face	
	0		0C	on center	
	D DIA, φ	diameter	OH OPNG	opposite hand opening	
	DIM	dimension	DAE	powdor actuated factoror	
	DTL	detail	PAF PC	precast concrete	
	DWGS DWL	drawings dowel	PERP PERT	perpendicular pre—enaineered roof truss	
WALLS AND COLUMNS			PJP	partial joint penetration	
	(E) EA	existing construction each	PL PLF	plate pounds per lineal foot	
	EF	each face	PSL	parallel strand lumber	
	EJ ELEV	elevation	PT PT	pressure treated	oned
	EOx FW	edge of (S=slab, C=conc, etc)	RE+	reference	
	EXP	expansion	REINF	reinforcement	
	EXT	exterior	REQD RET	required retaining	
	FDN	foundation	<u> </u>		
	FDx	face of (S=slab, C=conc, etc)	SCHED	schedule	
	FS FTG	far side footing	SCL SIP	structural composite lumber	•
RIALS	110		SOG	slab on grade	
	GA GB	gage grade beam	SPA STFNR	spacing stiffener	
SOIL FILL	GC	general contractor	STL	steel	
	GEN GLB	general glulam beam	T, T/	top of	
UNDISTURBED SOIL	HDC	hot din aalvanize	THK TI	thickness transfer_load	
	HDG HDR	header	TRAN	transverse	
	HK HORIZ	hook horizontal	ТҮР	typical	
STEEL FRAMING	HP	high point	UNO	unless noted otherwise	
	HR HSA	hour headed stud anchor	VERT	vertical	
	1/E	incida faca	VIF	verify in field	
	IV F INT	interior	W	width	
ON DETAILS/CAPACITIES	JST	ioist	WF WP	wide flange work point	
WOOD ERAMING.	JT	joint	WWF	welded wire fabric	
WOOD TRAMING	KIP, K	1000 pounds			
		STRUCTURAL DRAWING	SHEET	SHEET LADEL KET	
		SHEET NUMBER	N NOTES LOAD	KEYS AND TYP DETAILS	
OD SHEAR WALL PLAN KEY		200 SERIES - GE 200 SERIES - AR 300 SERIES - FI	REA PLANS	KETS AND THE DETAILS	
	501	ININ 400 SERIES = DE 500 SERIES = SC	TAILS CHEDULES		
OF				- •	
NMOM REQUIRED LENGTH		NO MODIFICATIONS = \bigcirc SI	HEET DELETED	= 🛛	
HOLD DOWN SCHEDULE			ISS	UE DATE AND TITLE	
FOUNDATIONS			RMI		
			R PE		
			FOI		
			NED		
			<u>ISS</u>		
			 N		
-EXISTING CONSTRUCTION			/202		
	SHEET		/11/		
LS)	NO.	SHEET NAME	11,		
	S0101	GENERAL NOTES	0		
	S0102 S0103	GENERAL NOTES GENERAL NOTES			
	S0104	TYPICAL DETAILS – CONCRETE			
	S0105 S0106	IYPICAL DETAILS – WOOD TYPICAL DETAILS – WOOD			
	S0201	PARTIAL FOUNDATION, ROOF FRAMING,	AND		
	S0301	WALL AND FRAME ELEVATIONS & DETA	ILS O		
	S0401 S0501	DETAILS SCHEDULES			
I		-			

APPROVAL STAMPS:	
REVIEWED FOR CODE COMPLIANCE 02/24/2023	
Image: Constraint of the second sec	
1 11/11/22 Issued for Permit No. Date Description SUBMISSIONS & REVISIONS OWNER MAY RIEGLER PROPERTII 2201 Wisconsin Ave NW Suite 200 Washington, DC 20007 www.mayriegler.com	DNS
ARCHITECT K A S KEVIN & ASAKO SPERRY ARCHITE 3318 N. Columbus Street Arlington, VA 22207 T.312.636.3248 / 312.636.4252 www.kasa-arch.com GENERAL CONTRACTOR	A CTURE
CIVIL ENGINEER LANDMARK ENGINEERING 141 9th Street, PO Box 774943 Steamboat Springs, CO 80477 T.970.871.9494 LANDSCAPE ARCHITECT	;
STRUCTURAL ENGINEER KL&A ENGINEERS & BUIL 1717 Washington Ave. Golden, CO 80401 T. 303.384.9910 M.E.P. & F.P. ENGINEERS	DERS © 2022 KL&A, INC
BOULDER ENGINEERING 1717 15th Street Boulder, CO 80302 T. 303.444.6038 INTERIOR DESIGNER: JOHNSON NATHAN STROM 1600 Wynkoop St., Suite 100 Denver, CO 80202 T. 000	ΊE
PROJECT LOCATION STEAMBOAT BASEC PARTIAL RENOVATI AND TENANT FIT-O 1901 CURVE PLAZA STEAMBOAT SPRINGS, CO 804 DRAWING TITLE	AMP ON UT 87
GENERAL NOTE	S
SEAL DATE 11/12 DRAV	I/22 WN BY: CKED BY: IECT NO: 4
DRAWING NO:	

FOUNDATIONS GENERAL:

- 1. The foundations have been designed based on the design criteria and the Geotechnical Report referenced in the Structural Design Criteria section. Earthwork and foundation soil preparation shall be performed to provide soil properties meeting the design criteria.
- 2. The Geotechnical Engineer shall inspect and test soils, earthwork and foundations - see special inspection and quality assurance sections of the General Notes. Prior to placing foundations and slabs on grade, obtain approval from the Geotechnical Engineer indicating earthwork and soil preparation has been performed adequately to conform to the foundation design criteria.
- 3. Bottom of exterior footings, grade beams, and walls shall bear below final exterior grade for frost protection - see structural design criteria section of the General Notes.
- 4. Foundation walls or grade beams having earth placed on each side shall
- have both sides filled simultaneously to maintain a common elevation. 5. Brace all foundation walls against movement while backfilling until floor slabs at the top and bottom of the wall are in place. Brace foundation walls as necessary to prevent movement and overstress due to equipment loading regardless of sequencing of top and bottom floor slabs.
- 6. Slab on grade movement is anticipated, see Geotechnical Report for magnitude of vertical movement. Isolate partition walls from slab-ongrade to allow for expected vertical movement.
- 7. Contractor shall provide continuous site drainage by a mechanical method to control surface and underground water as required to maintain a dry working site.
- 8. Foundation drainage and waterproofing is not shown or specified within the structural portion of the construction documents. Reference other portions of the construction documents for drainage, waterproofing and items associated with other disciplines.
- 9. No plumbing, vaults, tanks or other devices are to be placed below footings within zone of influence defined as a zone projecting 45 degrees outward from bottom edges of footing without acceptance by Structural Engineer and Geotechnical Engineer.

POST INSTALLED ANCHORS IN CONCRETE AND MASONRY GENERAL:

- 1. Holes are assumed to be dry unless otherwise noted on plans.
- 2. Holes to be hammer drilled with bit as specified by anchor manufacturer.
- 3. Anchors specified are based on the specific technical data published by the specified anchor manufacturer. Substitutions are not permitted without approval by the Structural Engineer of Record prior to use. Contractor shall provide calculations demonstrating that the substituted product is capable of achieving the performance values of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluations consider creep, inservice temperature and installation temperature.
- 4. Install anchors per the manufacturer instructions, as included in the anchor packaging. Installation shall adhere to ICC ESR. Reference plans and details for anchors that are to be installed with reduced torque. 5. Concrete should be allowed to cure a minimum of 21 days prior to
- adhesive anchor installation.
- 6. Prior to installation of anchors all installation and inspection personnel shall be instructed on site by a representative of the anchor manufacturer on proper installation techniques and equipment.
- 7. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
- 8. Installation of anchors shall not damage existing reinforcing. Prior to drilling, care shall be taken to avoid damage by locating existing reinforcing and PT by use of GPR, X-Ray, or other means that avoids damage to the concrete and accurately predicts potential conflict of reinforcing.
- 9. Post-installed anchors to be stainless steel where exposed to exterior and/or corrosive environments unless the anchor is protected.
- 10. All installers of post-installed adhesive anchors horizontally, vertically or upwardly inclined in concrete to support sustained tension loads shall be certified by ACI/CRSI adhesive anchor installer certification program, or equivalent as required by the IBC. Submit certificates for record. 11. All post-installed anchors in concrete shall be suitable for use in cracked
- concrete applications. 12. When doweling continuously deformed rebar into concrete use Hilti
- RE-500v3 or an adhesive that has been approved under ACI 355.4 and ACI 318 for development and lap splices.
- 13. Unless noted otherwise on plans/details all adhesive anchors shall be Hilti HIT-HY200 V3 for concrete and Hilti HY-270 for block and brick. Unless noted otherwise on plans/details all expansion anchors shall be Hilti Kwik-Bolt TZ2. See note 3 for substitutions.

CAST-IN-PLACE CONCRETE GENERAL:

- 1. All concrete work shall conform to ACI 318 and A shall conform to ACI 117 unless noted otherwise. copy of these references on site at all times.
- 2. Concrete Compressive Strength See "Concrete Requirements" table
- 3. Materials See "Concrete Materials Designation" 4. Unless noted otherwise, the terms reinforcing an elements reinforced with deformed reinforcing as and/or reinforcing conforming to deformed reinfo

TESTING:

- 1. Tests and inspections shall be performed in comp Chapter 17 of the IBC. See Special Inspections.
- 2. Concrete shall not be placed until reinforcing and been inspected by the owner's independent inspe special inspector.
- 3. See "Special Inspections and Testing" Table.

SUBMITTALS:

- Submittals shall conform to ACI 301. 2. Submittals for all concrete shall include: mix desig
- locations, reinforcing, embedded items, and MEP penetrations. Reinforcing shop drawings shall include placemer
- minimum scale complete with wall elevations and special reinforcement required at openings throu Include all accessories specified and required to s Detailing of reinforcing shall conform to ACI SP-6 4. Embed shop drawings shall include placement dra
- minimum scale locating all embed plates, anchor structural and non-structural components attach Contractor is responsible for coordinating with al the need for embeds and locations. Post installed unless approved by the structural engineer during process.

FORMING:

- 1. Unless noted otherwise, all formwork shall confo accordance with ACI 117 unless noted otherwise drawings. Refer to architectural drawings for arch
- 2. All construction joints shown on the drawings sha the structure unless elimination is approved by the Additional joints required to facilitate constructio points of minimum shear and shall be detailed or drawings for review. Locate vertical joints in girde joists, walls and slabs within the middle third betw and detailed with dowels and keys for transfer of noted otherwise. Reinforcing shall pass continuou joints. Where joints are shown as roughened, me surface to 1/4" amplitude clean and free of laitan
- Unless otherwise shown in the architectural draw at all columns, beams, walls, and slab edges that the finished structure.
- 4. Unless otherwise shown in the architectural draw at the underside of all exposed slab edges.
- 5. Locate door openings, window openings, MEP op reglets, brick ledges, curbs, and ledges per archite openings not dimensioned on structural drawings drawings.
- Comply with requirements of ACI 301 for remova post tensioned concrete slabs, formwork shall rei of 7 days and until the concrete reaches the spec

REINFORCING AND EMBEDDED ITEMS:

- 1. Provide standard hooks on bars terminating at a noted such as at edges of openings, slab edges, e beams, and ends of walls.
- 2. Unless noted otherwise, provide (2)#5's at each s 2'-0" beyond edges of opening.

3. Splice bars with class B contact laps per the reinfo length table, unless noted otherwise.

- 4. Splice welded wire fabric by lapping one full mesl 5. Unless noted, provide continuous reinforcing aro construction joints, control joints, contraction joir
- all abutting members. Provide epoxy coated reinf construction joints at garage slabs and slabs expo Welding of reinforcing is prohibited, unless noted
- conform to ASTM A706. Provide embeds (including anchors) for all support
- structural elements including but not limited to h window washing davits, miscellaneous steel, boll

PLACING AND FINISHING:

- 1. Handling, placing, constructing, and curing shall (including placement of concrete in wet weather, weather.
- 2. For concrete thickness of 4'-0" or greater or when exceeds 600lb/cyd, conform to Mass Concrete of
- 3. Curing compounds should not be used on surface additional concrete, paint, tile, or other material unless the contractor has demonstrated that the satisfactorily removed before subsequent applica membrane dissipates or can serve satisfactorily application.
- All concrete work shall be poured in-place unless Shotcrete placement method will only be permitt structural engineer prior to permit submittal. All requests and submittals to place concrete by the shotcrete method shall conform to ACI 506.2 and shall include pre-construction testing procedures. Requests for shotcrete placement shall be limited to foundation walls with simple reinforcing. All pilasters or in-wall columns shall be formed and poured separately from shotcrete operations.

WITH THE ARCHITECTURAL THE REQUIREMENTS FOR THE CHITECT FOR CLARIFICATION.

URAL

ACI 301 and tolerances . Contractor shall keep a	Category/Material	
Mix Design		Footing
	Soils and Foundations	Slab-on
" table		Permar
nd reinforcement refer to		Formw
forcing.		Reinfor
5		Steel Er
		Cast En
pliance with ACI 301 and		Concret
d embedded items have		Conten
pection agency and/or the	Cast-In-Place	placem
	Concrete	Concre
		Concre
		Post-In: or Upw
		Sustain
igns, construction joint P sleeves and		All Oth
		Floor Fl
nt drawings 1/8"=1'-0"		Welded
id slab plans. Include		Fabrica
support reinforcement.		Bolts in
66.		Pretens
rawings 1/8"=1'-0"		Turn-of
rs, and anchor bolts for		twist-o
Il trades for determining	Structural Steel	Turn-of
d anchors are not allowed		Wrench
ng the shop submittal		All Wel
		Comple
		Shear S
orm to Class B finish in		Beam C
e by architectural bitectural finish concrete		Galvani
all be incorporated into		Joists a
he Structural Engineer.		Interior
on shall be located at		Exterio
n reinforcing snop lers, beams, grade beams		Shear V and Ho
tween supports designed	Wood Framing	Drag St
f design shear, unless		Floor/R
ously through construction		Hanger
nce.		Mechai
wings, provide chamfers	NOTES [.]	Penetra
are exposed to view in	1. Special inspection	on and t
wings, provide drip edges	building departr	nent.
	Unless noted as is defined as par	continu rt-time c
penings, drip slots,	Inspector's resp	onsibilit
tectural drawings. For as refer to architectural	order to meet th	ne inspe
	3. Class 1: Inspecti	on verifi
al of formwork. At non-	4. Class 2: Inspecti	on and t
emain in place a minimum	density, proport	ions, an
lineu zo uay strength.	6 Class 4: Audit ar	nd insne
	collection of fac	ilities re
concrete face unless	inspections and	testing.
expansion joints, ends of	7. Class 5: Verificat	tion of c
side of openings. Extend	NOTES SPECIFIC TO S	STRUCTI
	unless noted oth	nerwise.
orcing contact lan solice	2. Special inspection	on shall l
	tabrication facili	ty has b on and to
sh space plus 2".	N5.4-3. 100% of	all CJP
ound corners and through	methods (NDT).	Where
iforcing through	4. Special Inspection N5.6-2, and N5.6	on and to 6-3.
osed to de-icers.	5. Special inspecto	r shall ir
d otherwise and shall	cracks after galv	anizing.
orting structural and non-		
hand rails, canopies,	SPECIAL	INSP
lards, etc.		
	Category/Mate	erial
conform to ACI 301		
cold weather, and hot	Wood Framing in S Category C D F	Seismic , or F
in the concert sector.		,
f ACI 301.	NOTES:	
es that are to receive	1 Special inspectio	ons for s
	1. Special inspectic	
requiring a positive bond		
requiring a positive bond e membrane can be		
requiring a positive bond membrane can be ation is made, or the as the base for the later		
requiring a positive bond e membrane can be ation is made, or the as the base for the later		
requiring a positive bond e membrane can be ation is made, or the as the base for the later s noted otherwise.		

	PLCIAL INSPECTIONS AND TESTIN	G				
orv/Material	Component/Work			Class	S	
	component, work	1	2	3	4	5
	Footing Soil Bearing Material		X			
oils and	Slab-on-Grade Subgrade Material		X			
undations	Compaction	Х	X	X		
	Permanent Soil Retention Elements	Х	X	X		
	Formwork Installation	Х				
	Reinforcing Placement	Х	X			
	Steel Embeds	Х	X			
	Cast Embedded Anchors	Х	X			
	Concrete Strength, Slump, Temperature, and Air		x	x		
	Verification of mix design use on site (prior to					
t-In-Place	placement)	Х		X		
oncrete	Concrete Placement	х	x	x		
	Concrete Curing		х			
	Post-Installed Adhesive Anchors in Horizontally or Upwardly Inclined Orientations to Resist Sustained Tension Loads	х	x	x		
	All Other Post-Installed Anchors	х	x			
	Floor Flatness Survey	х				
	Welded Reinforcing		x			x
	Fabrication Facility			-	x	x
	Connection Frection and Assembly	x	x			
	Bolts in Snug Tight Joints	v v	v	-		
	Pretensioned and Clin Critical Polts/Joints Using		^	-		
	Turn-of-Nut with Matchmarking, DTI Washers, or twist-off-type TC bolts	х	x			
tural Steel	Pretensioned and Slip Critical Bolts/Joints Using Turn-of-Nut without Matchmarking or Calibrated Wrench Methods of Installation	Х	x	x		
	All Welds other than Complete Joint	x	x			x
	Penetrations Groove Welds					
	Complete Penetration Groove Welds	Х	X	X		X
	Shear Stud Placement	Х	X			
	Beam Camber at Fabrication Facility	Х				
	Galvanized Structural Steel Members	Х	Х			
	Joists and Beam Framing	х	x			
	Interior Bearing Wall Studs	х	x			
	Exterior Stud Framing	х	x			
	Shear Wall Framing, Sheathing, Nailing, Anchors,	v				
d Framing	and Holdowns	^	^			
5	Drag Struts and Collectors	Х	X			
	Floor/Roof Sheathing and Nailing	Х	X			
	Hangers and Connections	Х	X			
	Mechanical, Electrical, and Plumbing	v				
,	Penetrations		X			
		^	X			
ecial inspectic Iding departn less noted as lefined as par pector's respo inspection re ler to meet th	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin be inspection and reporting requirements.	IBC Pe the ncy a g of	and riodi Spec and c the	the ic ins cial dura wor	local spec tion k in	tio
ecial inspectic Iding departn less noted as lefined as par pector's respo inspection re ler to meet th ss 1: Inspectio	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements.	IBC Pe the ncy a g of	and riodi Spec and o the e.	the ic ins cial dura wor	local spec tion k in	tio of
ecial inspectic ding departmeters less noted as lefined as par pector's respection re inspection re ler to meet the ss 1: Inspection ss 2: Inspection ss 3: Continue	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. ous inspection and verification of operations and c	IBC Pe the ncy a g of anco iifica	X and Specand of the e. ation	the ic ins cial dura wor	local spec tion k in	tio of
ecial inspectic Iding departr less noted as lefined as par pector's response inspection re ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. ous inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for (testing.	IBC IBC Pe the ncy a g of cance iifica cond I pro Class	X and Specand of the e. htion ggrar ; 2 a	the ic ins cial dura wor n, qua ns. m, ar nd 3	local spec tion k in ality, nd	of
ecial inspectic lding departr less noted as lefined as par pector's respo inspection re ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin be inspection and reporting requirements. On verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. Dus inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for C testing.	IBC IBC the ncy a g of anco ifica cond I pro Class	X and Specand of the e. htion itior ggrar ; 2 a	the ic ins cial dura wor ns. m, au nd 3	local spec tion k in ality,	of
ecial inspectic lding departr less noted as lefined as par pector's respection re ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. bus inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for C testing.	IBC Pe the ncy a g of ance ifica cond I pro Class	X and Specand of the e. tion itior grar ; 2 a	the ic ins cial dura wor n, qua ns. m, ar nd 3	local spec tion k in ality, nd	tio of
ecial inspectic lding departr less noted as lefined as par pector's respection re- ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspectic ess noted oth	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. ous inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for C testing. ion of certifications. <u>STRUCTURAL STEEL SPECIAL INSPECTIONS:</u> on and testing shall conform to all requirements of nerwise.	IBC Pe the ncy a g of ance ifica cond I pro Class	X and Specand of the e. tition gran gran g 2 a	the ic ins cial dura wor n, qua ns. m, ar nd 3	local spec tion k in ality, nd	tio
ecial inspectic ilding departr less noted as defined as par pector's respect inspection re der to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspectic less noted oth ecial inspectio	on and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. On verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. Dus inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for C testing. STRUCTURAL STEEL SPECIAL INSPECTIONS: on and testing shall conform to all requirements of herwise.	IBC IBC Pe the ncy a g of anco ifica cond I pro Class AISO rs, u	and riod Specand of the e. htion ition gran ; 2 a C 36 unles	the ic inscial dura wor n, qua ns. m, au nd 3	local spec tion k in ality, nd	tio of
ecial inspectic lding departr less noted as lefined as par pector's respo e inspection re ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspection ecial inspection prication facili ecial inspection at a state of the content of the state of the content of the state of the state of the content of the state of the state of the content of the state of the content of the state of the state of the state of the content of the state of the state of the state of the state of the content of the state of the state of the state of the state of the content of the state of the state of the state of the state of the content of the state o	In and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin the inspection and reporting requirements. On verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. Dus inspection and verification of operations and co d inspection of fabrication facility's quality contro lities records during the course of fabrication for C testing. STRUCTURAL STEEL SPECIAL INSPECTIONS: on and testing shall conform to all requirements of nerwise. In shall be required for all shop fabricated member ty has been approved to perform such work witho on and testing of welding shall conform to Tables N all CJP groove welds shall be tested by approved re Where the fabricator porforms the NDT, the species	IBC IBC Performance information informat	X and riod Specand of the e. dition bgran ; 2 al C 36 mles pecia -1, N destr	the ic inscial dura wor ns. m, ar nd 3 0 Ch as the al ins 15.4- ructive	local spec tion k in ality, nd . N, e spec 2, ar ve te	tio of ti
ecial inspectic ilding departr less noted as defined as par pector's respe e inspection re der to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspection ess noted oth ecial inspection rication facili ecial inspection (A-3, 100% of thods (NDT).	an and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencin be inspection and reporting requirements. On verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. Dous inspection and verification of operations and c d inspection of fabrication facility's quality contro lities records during the course of fabrication for O testing. ion of certifications. STRUCTURAL STEEL SPECIAL INSPECTIONS: on and testing shall conform to all requirements of nerwise. on shall be required for all shop fabricated member ty has been approved to perform such work witho an and testing of welding shall conform to Tables N all CJP groove welds shall be tested by approved r Where the fabricator performs the NDT, the species on and testing of high-strength holting shall conform	IBC IBC Pe the ncy a g of rance ifica cond I proc Class AISC rs, u uut s I5.4- nonc al in m to	and riod Specand of the e. dition gran ; 2 a lition c 36 mles peci- 1, N destri	the ic inscial dura wor n, qua ns. m, ar nd 3 0 Ch as the al inscial is.4- ructive ctor	local spec tion k in ality, nd . N, e spec 2, ar ve te shall	tio of ti
ecial inspectic ilding departr less noted as defined as par pector's respo- e inspection re der to meet th ss 1: Inspection res 2: Inspection ss 2: Inspection ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspection less noted oth ecial inspection rication facili ecial inspection at a spection store of the secial inspection action facili ecial inspection action facili	In and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequere elative to the Contractor's schedule and sequencing the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. ous inspection and verification of operations and c d inspection of fabrication facility's quality control lities records during the course of fabrication for C testing. ion of certifications. <u>STRUCTURAL STEEL SPECIAL INSPECTIONS:</u> on and testing shall conform to all requirements of herwise. In shall be required for all shop fabricated member ty has been approved to perform such work withour and testing of welding shall conform to Tables N all CJP groove welds shall be tested by approved r Where the fabricator performs the NDT, the species on and testing of high-strength bolting shall conform 5-3.	IBC IBC the the g of ance ifica cond I pro Class AISe rs, u but s I5.4- nonc al in m to	X and Specand of the e. dition ition gran gran gran gran gran gran gran gra	the ic inscial dura wor ns. m, ar nd 3 0 Ch as the al ins 15.4- ructiv ctor oles 1	local spec tion k in ality, nd . N, e spec 2, ar ve te shall N5.6	tio of ti
ecial inspectic lding departr less noted as lefined as par pector's respo- enspection re- ler to meet th ss 1: Inspection ss 2: Inspection ss 3: Continue ss 3: Continue ss 4: Audit an lection of faci pections and ss 5: Verificat <u>SPECIFIC TO S</u> ecial inspectio ess noted oth ecial inspectio rication facili ecial inspectio (A-3, 100% of thods (NDT). ecial inspectio (6-2, and N5.6	In and testing are to conform to chapter 17 of the nent. continuous inspection, all inspections are periodic t-time or intermittent inspection of the work. It is possibility to determine and coordinate the frequer elative to the Contractor's schedule and sequencing the inspection and reporting requirements. on verification of size, location, quantity, and toler on and testing verification of strength, grade, class ions, and manufacturers certified test reports. bus inspection and verification facility's quality control lities records during the course of fabrication for C testing. ion of certifications. STRUCTURAL STEEL SPECIAL INSPECTIONS: on and testing shall conform to all requirements of nerwise. on shall be required for all shop fabricated member ty has been approved to perform such work without and testing of welding shall conform to Tables N all CJP groove welds shall be tested by approved ro Where the fabricator performs the NDT, the species on and testing of high-strength bolting shall conform 5-3. r shall inspect exposed cut surfaces and corners of	IBC IBC Performance in the second	A and riod Specand of the e. It in the period of the e. It is period	the ic ins cial dura wor n, qua ns. m, ar nd 3 0 Ch as the al ins 15.4- ructiv ctor bles l smbe	local spec tion k in ality, nd . N, e spec 2, ar ve te shall N5.6 ers fo	tio of ti ti

		MIX DES	IGN	REQ	UIREME	NTS			CON	ICRET	E REINF	DRCING	TENSIO	N CONT	ACT LAP	SPLICE
Element	f'c	Cement	Max	Max	Air Content	Slump	Expo	sure				LI	ENGTHS			
	(psi)	Туре	w/C	Agg	(Note 1,2)	(Note 3)	Cla	ISS					CONCRE		SSION STRE	NGTH (psi)
Footings	4000,	1/11	0.45	3/4"	5	-	F1	C0		1		Tan	3,000	4,000	4,500	5,000
	NW	,		,			S0	W0			Bars ≤ #	6 Other	57 db	49 db	47 db	44 db
oundation Walls	4500,	1/11	0.45	2//"	6		F2	С0		Case #:	1	Other	44 00	38 0D	36 00	
columns	NW	1/11 	0.45	3/4	0	-	SO	W0			Bars ≥ #	7 Other		62 00	58 0D	55 0D
rior Slab-on-Grade	4000.						F0	C0	Class A			Other		4/ 00	45 00	
(SOG)	NW	/	0.45	3/4"	-	-	со	PO			Bars ≤ #	6 Other		74 00		55 0D
E FOOTNOTES:		I								Case #2	2	Tan	107 db	02 db	54 UD	02 db
For any concrete exp	posed to	o freezing ter	nperat	ures ar	nd moisture,	the air co	ntent	shall			Bars ≥ #	7 Other		92 00	67 db	64 db
be the greater of 5%	6, minim	um required	by ACI	318, c	or of that sho	wn in the	table.					Top	02 UD	71 UD 64 db	60 db	57 db
Folerance on air con	itent as	delivered sha	all be +	/- 1.5%	6.						Bars ≤ #	6 Other		04 00	60 0D	57 UD
Specified Slump not	greater	(ACI 117): than 4"= +/-	1"							Case #	1	Tan		49 00	47 UD	44 00
pecified Slump mor	re than	4"= +/- 1 1/2	н								Bars ≥ #	7 Other	95 UD	62 db	FS db	72 UD
Where Slump is spec	cified as	a range= No	Tolera	nce					Class B			Top	71 UD	02 UD	01 db	
RAL CONCRETE MIX	X NOTES	5:						_			Bars ≤ #	6 Other	25 db	74 db	70 db	66 db
Strength (f'c) is the c	compres	<u></u> ssive strength	n at 28	davs u	nless noted	otherwise	or			Case #2	2	Top	120 db	120 db	112 db	109 db
				,			-				Bars ≥ #	7 Top	102 -11- 102 -11-		דד בס ווי בס	
oncrete is normal w	weight c	oncrete unle	ss note	d othe	erwise. Norm	al weight	concr	ete				Uther	αριυτ	92 00	87 00	83 QD
rmeability Require rrosion Protection aximum water-solu inforced Concrete here concrete is e h and/or other cer r requirements.	ements n of Reir uble cho e: xposed mentitic	noted thus: [nforcement n oride ion (CL- CO = 1. to F3 freeze ous materials	P0,P1/ oted th equiren) conte 0 thaw e apply.	W0,W nus: CC nents ent in c C kposur Refer	1]),C1,C2 (C0,C1,C2): concrete, by C1 = 0.3 re, restriction to Table 26.4	% weight o C2 = ns on maxi I.2.2(b) in	of cem 0.15 mum ACI 32	ent: fly L8-14		e #2: C op: V her: C	equal to db Clear spacing Where horiz of fresh cond Other condit einforcing)	less than 2 ontal reinfo rete is cast on not sati	2*db or cove rcement is p below the d sfying Top q	r less than dl blaced such tl levelopment ualification (l	b hat more th length or sp bottom hori	an 12 inches lice zontal
e. requirements.									1. All te	ension sp	lices shall b	contact cl	ass B splices	unless noted	d otherwise.	Splice leng
									shall	not be le	ess than 12	nches.				
CON	CREI		IALS	DES	IGNATIC	DIN			2. Splic	e lengths	in table are	for single l	oar splices w	vith maximun	n yield stren	gth of 60ks
Materi	ial				Standa	rd			3 For e	epoxy re	inforcing ba	s. Incland enov	w coated ha	rs or enoxy (coated wire	s multinly
and Cement			ASTM	C150,	Type I or Ty	pe II			"bot"	' reinford	cing splice le	ngth by 1.5	and multipl	y "Top" reinf	forcing splic	e lengths by
sh			ASTM	C618,	Class C or F				1.31.							
egate			ASTM	C33					4. For li	ghtweig	nt concrete	nultiply spl	ice length by	y 1.33.		
r			Potab	le					5. For r	einforcin	g with a spe	cified yield	strength gre ملاحزا)	eater than 60) ksi, multipl	y splice
er Reducing Admixtu	ure		ASTM	C494,	Type A or Ty	/pe D			6. For in	ndividual	bars within	a bundle. l	ap lengths sl	hall be multir	olied by 1.33	s for four ba
Range Water Reduc	cing Adr	nixture	ASTM	C494,	Type F or Ty	pe G			bund	lles and 2	L.20 for thre	e bar bund	les. Individua	al splices with	hin a bundle	shall not
erator Admixture			ASTM	C494,	Type C or Ty	vpe E			over	lap. Entir	e bundle sh	all not be la	p spliced.			
training Admixture	5		ASTM	C260					7. Bars	larger th	an #11 shal	not be lap	spliced. For	bars larger th	han #11, me	chanical
veight Aggregates			ASTM	C330					125%	6 the yiel	d strength o	f the reinfo	orcing bar. N	lechanical sp	lices shall be	e staggered.
g Compound			ASTM	C309,	Type I, Class	A		-	8. Whe	re bar of	different si	e are lap s	oliced in ten	sion, the min	nimum splice	e length shal
orcing Bars			ASTM (Speci	но15- fied Yi	grade 60 eld Strength	= 60ksi)			be th	e larger	of the lengt	of a Class	B tension sp	lice of the sn	maller bar o	the length
od Poinfordin - D-			ASTM	A706-	grade 60	~1				ss A tens		ted where	ual.	paranco hot	Noon roinfe	cing const
)		(Spec	fied Yi	eld Strength	= 60ksi)			be m	aintaine	d.	ieu wiiere		carance DetV	ween reiiii0	
ed Wire Fabric			ASTM	A185					10. Lap s	plice len	gths shall n	t be less th	an the large	r of 12 inche	s multiplied	by all
	DG		ASTM	E1745	5-Class A				appli	cable mu	ultipliers or	he table le	ngth multipli	ied by all app	olicable mult	ipliers.
r Retarder below SC																
r Retarder below SC		y be used if a	accepta	ble to	the Archited	t.			R	EQUIF	RED CON	CRETE	COVER F	OR NON	-FIRE-RA	ATED
r Retarder below SC <u>S:</u> ype III Portland cem	nent ma											AS	SEMBLIE	S		
r Retarder below SC <u>ES:</u> ype III Portland cem	nent ma			EM	BEDMEN	NT LEN	GTH	S				Assembl	у			Cover (in)
r Retarder below SC <u>S:</u> ype III Portland cerr ONCRETE REII	nent ma	CING DC	DWE			mpression		els		Concret	e cast again	st & perma	nently expos	sed to earth		3
r Retarder below SC <u>S:</u> ype III Portland cerr ONCRETE REII crete Compression	nent ma	CING DC Tensior	DWEI	ls		וטוככסיקייי	. 500	-13	Concret	te Expos	ed to #6-#	18	· ·			2
r Retarder below SC <u>S:</u> ype III Portland cem DNCRETE REII rete Compression Strength	NFOR Stan	CING DC Tensior dard Hook	DWEI	ls Othe	r		b		Earth	or Weat	her #5 a	nd smaller				1 1/2
r Retarder below SC <u>S:</u> ype III Portland cerr DNCRETE REII rete Compression Strength 3000psi	NFOR Stan	CING DC Tensior dard Hook 22*db	DWEI	ls Othe Note	r 1	22*d		1		not Fxp	osed Wal	s slahs #11	and smaller			
r Retarder below SC <u>S:</u> ype III Portland cerr DNCRETE REII rete Compression Strength 3000psi 4000psi	NFOR Stan	CING DC Tensior dard Hook 22*db 19*db	DWEI	ls Othe Note Note	r 1	22*d 19*d	b		Concrete		0500	<i>σ</i> , σια σ σ π ι ι	. anu smanei	ſ		3/4
r Retarder below SC S: ype III Portland cem DNCRETE REII rete Compression Strength 3000psi 4000psi 5000psi	NFOR Stan	CING DC Tensior dard Hook 22*db 19*db 17*db	DWE	ls Othe Note Note Note	r	22*d 19*d 18*d	b b		Concrete			5, 51005 #11		ſ		3/4
r Retarder below SC <u>S:</u> ype III Portland cem DNCRETE REII rete Compression <u>Strength</u> 3000psi 4000psi 5000psi <u>S:</u>	NFOF Stan	CING DC Tensior dard Hook 22*db 19*db 17*db	DWEI	ls Othe Note Note Note	r 1 1	22*d 19*d 18*d	b b		Concrete	00 5-						3/4
or Retarder below SC ES: Type III Portland cerr ONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R lowels without stand rod	NFOR Stan Reinforc dard ho	CING DC Tensior dard Hook 22*db 19*db 17*db ing Tension c oks. Values f	OWEI Dowe	ls Othe Note Note Note Lap Sp	r 1 1 1 blice Lengths asion splices	22*d 19*d 18*d " table for are permit	b b tensio ted to	on o be	CON	CRETE	REINFC	RCING		ESSION C	CONTAC	3/4 T SPLICE
or Retarder below SC ES: Type III Portland cem ONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R dowels without stand used.	NFOR Stan Reinforc dard ho	CING DC Tension dard Hook 22*db 19*db 17*db ing Tension c oks. Values f	ontact	ls Other Note Note Lap Sp s A ten	r 1 1 1 J Dlice Lengths sion splices	22*d 19*d 18*d " table for are permit	b b tensio	on 9 be	CONC	CRETE	REINFC	RCING LI y) ≤ 60ksi		ESSION C	CONTAC	3/4 T SPLICE) > 60ksi
or Retarder below SC ES: Type III Portland cerr ONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R dowels without stand used. Embedment length s	NFOR Stan Reinforc dard ho	CING DC Tension dard Hook 22*db 19*db 17*db ing Tension c oks. Values f	OWEI	ls Other Note Note Lap Sp s A ten	r 1 1 1 Dlice Lengths Ision splices	22*d 19*d 18*d " table for are permit	b tensio cted to	on 9 be	CON	CRETE	REINFC	RCING Ll y) ≤ 60ksi		ESSION C cified Yield S	CONTAC Strength (fy fy - 24)db	3/4 T SPLICE) > 60ksi
or Retarder below SC ES: Type III Portland cerr CONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R dowels without stand used. Embedment length s db is bar diameter.	NFOR Stan Stan Reinforc dard ho shall not	CING DC Tension dard Hook 22*db 19*db 17*db ing Tension c oks. Values f : be less than	ontact or Clas	ls Other Note Note Lap Sp s A ten hes.	r 1 1 1 1 0lice Lengths ision splices	22*d 19*d 18*d " table for are permit	b tensio ted to	on o be	Concrete CON Specie NOTES:	CRETE	REINFC	RCING LI y) ≤ 60ksi	COMPRE ENGTHS	ESSION C ecified Yield S (0.9*1	CONTAC Strength (fy fy - 24)db	3/4 T SPLICE) > 60ksi
or Retarder below SC ES: Type III Portland cem CONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R dowels without stand used. Embedment length s db is bar diameter. Compression dowel e drawings as compression	NFOR Stan Stan dard ho shall not embedr ssion, of	CING DC Tension dard Hook 22*db 19*db 17*db ing Tension c oks. Values f : be less than ther lengths therwise use	ontact or Clas 12 inc are pe tensio	ls Other Note Note Lap Sp s A ten hes.	r 1 1 1 olice Lengths ision splices d only when edment leng	22*d 19*d 18*d " table for are permit dowel is r th.	b tensio ted to	on o be n	Concrete CON Specie <u>NOTES:</u> 1. Splice	CRETE	REINFC d Strength (30*db	RCING LI y) ≤ 60ksi less than 1	2 inches.	ESSION C cified Yield S (0.9*1	CONTAC Strength (fy fy - 24)db	3/4 T SPLICE) > 60ksi
or Retarder below SC ES: Type III Portland cerr ONCRETE REII crete Compression Strength 3000psi 4000psi 5000psi ES: Refer to "Concrete R dowels without stand used. Embedment length s db is bar diameter. Compression dowel of drawings as compress Extend dowels to far	NFOR Stan Stan Stan dard ho shall not embedr ssion, of r edge o	CING DC Tension dard Hook 22*db 19*db 19*db 17*db ing Tension c oks. Values f be less than the lengths therwise use f member UN	ontact or Clas 12 inc are pe tension	ls Other Note Note Lap Sp s A ten hes. rmitten	r 1 1 1 1 0lice Lengths bision splices d only when edment leng	22*d 19*d 18*d " table for are permit dowel is r th.	b tensio ted to	on 9 be n	Concrete CON Specie <u>NOTES:</u> 1. Splice 2. db is	CRETE fied Yield e Length bar dian	REINFC d Strength (30*db shall not be	RCING LI y) ≤ 60ksi less than 1	2 inches.	ESSION C cified Yield S (0.9*1	CONTAC Strength (fy fy - 24)db	3/4 T SPLICE) > 60ksi

CONC	RETE	MIX DES	IGN	REO	UIREME	NTS			CON	CRFTF	REINFOR	RCING	TENSIO		ΑCΤΙΑΡ	SPLICE
	f'c	Cement	Max	Max	Air Content	Slumn	Fynd	SUIRE		CRETE		LE	NGTHS			JI LICE
Element	(psi)	Туре	W/C	Agg	(Note 1,2)	(Note 3)	Cl	ass	 				CONCRET		SSION STREM	NGTH (psi)
	4000						F1	C0					3,000	4,000	4,500	5,000
Footings	NW	1/11	0.45	3/4"	5	-	SO	W0			Bars < #6	Тор	57 db	49 db	47 db	44 db
Foundation Walls	4500						F2	C0		Case #1		Other	44 db	38 db	36 db	34 db
integral to pilasters & columns	NW	1/11	0.45	3/4"	6	-	S0	W0			Bars ≥ #7	Тор	71 db	62 db	58 db	55 db
Interior Slab-on-Grade	4000.						FO	C0	Class A			Other	55 db	47 db	45 db	42 db
(SOG)	-000, NW	1/11	0.45	3/4"	-	-	C0	PO			Bars ≤ #6	Top	85 db	74 db	70 db	66 db
TABLE FOOTNOTES:		1								Case #2		Other	66 00	57 0D	54 00	51 0D
1. For any concrete exp	posed to	o freezing ter	nperat	ures a	nd moisture,	the air cor	ntent	shall			Bars ≥ #7	Other	82 db	92 00 71 db	67 db	64 db
be the greater of 5%	5, minim	ium required	by AC	318, (or of that sho	own in the	table	•				Top	74 db	64 db	60 db	57 db
 I olerance on air con Slump tolerances as 	follows	(ACI 117)	all be +	/- 1.5%	6.						Bars ≤ #6	Other	57 db	49 db	47 db	44 db
Specified Slump not	greater	than 4"= +/-	1"							Case #1		Тор	93 db	80 db	76 db	72 db
Specified Slump mor	re than a	4"= +/- 1 1/2	" Tolora	nco							Bars ≥ #7	Other	71 db	62 db	58 db	55 db
where slump is spec	Lilleu as	a range- No	TOIET	ance					Class B			Тор	111 db	96 db	91 db	86 db
GENERAL CONCRETE MIX	NOTES	<u>S:</u>									Bars ≤ #6	Other	85 db	74 db	70 db	66 db
1. Strength (f'c) is the o	compres	ssive strength	n at 28	days u	inless noted	otherwise	or			Case #2		Тор	139 db	120 db	113 db	108 db
2. Concrete is normal v	weight c	oncrete unle	ss note	ed oth	erwise. Norm	nal weight	conci	rete			Bars ≥ #7	Other	107 db	92 db	87 db	83 db
(NW) shall have a dr	y densit	ty of 145 ± 5	pcf unl	ess no	ted otherwis	e. Lightwe	ight					1				
 Corrosion Protection Maximum water-sol Reinforced Concrete Where concrete is e ash and/or other cen for requirements 	n of Reir uble cho e: xposed mentitic	nforcement r oride ion (CL- C0 = 1. to F3 freeze ous materials	equire) conte 0 thaw e apply.	ments ent in ((xposu Refer	(C0,C1,C2): concrete, by C1 = 0.3 re, restriction to Table 26.4	% weight c C2 = ns on maxiu 4.2.2(b) in .	of cen 0.15 mum ACI 3	nent: fly 18-14		pp: Of of her: Ot rei	here horizont fresh concret her condition inforcing)	al reinfor te is cast l not satis	reement is pl below the de	aced such the evelopment palification (l	hat more tha length or sp bottom horiz	in 12 inche lice zontal
ior requirements.									1. All te	nsion splic	ces shall be co	ontact cla	ss B splices (unless noted	l otherwise.	Splice leng
		C N / A T C C	1410						shall	not be les	s than 12 incl	nes.		-		0
CON	CREI	EMATER	IALS	DES	IGNATIC	DN .			2. Splice	e lengths i	n table are fo	or single b	ar splices wi	th maximum	n yield stren	gth of 60ks
Mater	ial				Standa	ird			3. For e	poxy coate	ed bars, zinca	and epoxy	v coated bar	s, or epoxy o	coated wires	, multiply
Portland Cement			ASTM	I C150,	Type I or Ty	pe II			"bot"	reinforcir	ng splice leng	th by 1.5	and multiply	"Top" reinf	orcing splice	lengths by
Fiy Ash			ASTM	1 (618)	Class C or F				1.31.					4.22		
Aggregate			Potab						4. For II	gntweignt	concrete mu	iitipiy spii	ce length by	1.33.	المناجب المناج	, colico
Water Reducing Admixt	ire		ASTM	1 C 4 9 4	Type A or Ty	ne D			lengt	h by (spec	cified yield str	ength/60	ksi).		rksi, multipiy	splice
High Range Water Reduc	cing Adr	nixture	ASTM	1 C494	Type F or Ty	vpe G			6. For ir	ndividual b	oars within a l	oundle, la	p lengths sh	all be multip	blied by 1.33	for four ba
Accelerator Admixture			ASTM	I C494	Type C or Ty	/pe 8			bund	les and 1.2	20 for three b	oar bundle not be lar	es. Individua Soliced	l splices with	hin a bundle	shall not
Air Entraining Admixture	2		ASTM	I C260	.,				7. Bars	arger that	n #11 shall no	not be lap s	spliced For h	ars larger th	nan #11. mer	chanical
Lightweight Aggregates			ASTM	I C330					splice	shall be u	used. Mechar	nical splice	es shall have	strength gre	eater than o	r equal to
Curing Compound			ASTM	I C309	Type I, Class	5 A			125%	the yield	strength of t	he reinfoi	rcing bar. Mo	echanical spl	lices shall be	staggered
Reinforcing Parc			ASTM	I A615	-grade 60				8. When	e bar of d e larger of	litterent size a	are lap sp If a Class I	liced in tens B tension sol	ion, the mini ice of the sm	imum splice naller bar or	length sha the length
Ddl'S			(Spec	ified Y	ield Strength	= 60ksi)			a Clas	ss A tensio	on splice of th	e larger b	par.			
Welded Reinforcing Bars	i		ASTM	i A706 ified V	-grade 60 ield Strenøth	= 60ksi)			9. Lap s	plices are	not permitte	d where r	ninimum cle	arance betw	veen reinfor	cing canno
Welded Wire Fabric			ASTM	I A185					be m	aintained.	the chall not "	a loce the	n the large	of 12 inches	c multiplica	hy all
Vapor Retarder below S0	DG		ASTM	I E174	5-Class A				appli	cable mult	tipliers or the	table len	gth multiplie	ed by all app	licable mult	pliers.
<u>NOTES:</u> 1. Type III Portland cen	nent ma	ay be used if a	accepta	able to	the Archited	ct.			RI	EQUIRE	ED CONC	RETE C	COVER F	OR NON	-FIRE-RA	TED
)) / / -				ידי					ASS	EMBLIE	5		
CONCRETE REI	NFUF		DVVE			NI LEINO	זונ	15				Assembly	/			Cover (in)
Concrete Compression Strength	Star.	lensior	owe ו	0+b~		mpressior	n Dov	vels		Concrete	cast against &	& perman	ently expose	ed to earth		3
2000000	stan	uaru 1100K		Noto	<u> </u>	لە*ر(h		Concret	e Exposed	to #6-#18					2
 		22 UD 19*dh		Note	<u> </u>	۲۵.۵۲ ۲۵*۲	b h		Earth	or weathe	=' #5 and	smaller				1 1/2
5000psi		17*dh		Note	<u> </u>	18*4 18.4	b		Concrete	not Expos	sed Walls, s	slabs #11	and smaller			3/4
NOTES: L. Refer to "Concrete R dowels without stan used.	einforc dard ho	ing Tension c ooks. Values f	ontact or Clas	Lap Si s A ter	Dice Lengths	table for are permit	tensi ted to	on o be	CON	CRETE	REINFOR	CING (LE		SSION C	CONTACT	SPLICE
2. Embedment length s	shall not	t be less than	12 inc	hes.					Specif	ied Yield	Strength (fy)	≥ bUKSI	Spec		Surength (fy)	> bUKSI
3. db is bar diameter.		_							NOTES	3	ບັບນີ			(0.9*1	y - ∠4jub	
 Compression dowel drawings as compres Extend dowels to far 	embedı ssion, oʻ redge o	ment lengths therwise use f member UI	are pe tensio NO.	ermitte n emb	d only when edment leng	dowel is n th.	oted	in	1. Splice 2. db is	e Length sl bar diame	hall not be le eter.	ss than 12	2 inches.			
									3. Com comp	pression sp pression, o	plices permitt otherwise see	ed only w tension s	vhen lap spli plice table.	ce is noted i	n drawings a	IS

SPECIAL INSPECTIONS AND TESTING FOR SEISMIC								
RESISTANCE								
Category/Material	Component/Work							
Weed Service in Colomb	Continuous special inspection of field gluing operation of elements in the seismic force-resisting system.							
Category C, D, E, or F	Periodic special inspection of nailing, bolting, anchoring, and other fastening elements of the seismic force-resisting system, including wood shear walls, wood diaphragms, dr							
NOTES:								

seismic resistance is required per IBC 1705.12



S0102

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS ON THE PROJECT TO CLEARLY DEFINE ALL OF THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR CONTACT ARCHITECT FOR CLARIFICATION. THE ONLY. ALIFIED PEOPLE UNDER DRAWING AND SUCH IED FOR INFORMATION O ED BY QUA ON THIS ARE ISSU ENGINEERS SEAL ON THIS DRAWING INDICATES THAT THE INFORMATION SHOWN AND THE CALCULATIONS PERTAINING TO THAT INFORMATION HAVE BEEN PREPARE OF THE ENGINEER-OF-RECORD. THE SEAL DOES NOT IMPLY RESPONSIBILITY FOR INFORMATION PREPARED BY OTHERS NOR FOR ANY INFORMATION NOT SHOWN SPECIFICALLY DISCLAMED. ON PHASED PROJECTS, DRAWINGS THAT ARE ISSUED BUT NOT SEALED SHALL BE CONSIDERED TO BE PRELIMINARY IN NATURE AND . URAL

STRUCTURAL STEEL GENERAL:

- 1. All structural steel work shall conform to AISC 360 and tolerances shall conform to AISC 303 unless noted otherwise. Contractor shall keep a co of these references on site at all times.
- 2. Materials See Steel Materials Table
- 3. Qualifications Fabricator and Erector shall be experienced in fabrication and erection of projects of similar size and complexity.

TESTING:

 Tests and inspections shall be performed in compliance with AISC 360 and Chapter 17 of the IBC. Inspections include: Welding, high strength boltin anchor rod placement, proper use of joint details, fabricated steel, and erected steel frame. Testing includes: UT of full penetration welds, bolt tensioning procedures, shear stud bend tests. 2. See "Special Inspections and Testing" Table.

SUBMITTALS:

- 1. Submittals shall conform to AISC 360.
- 2. Submittals for structural steel shall include erection drawings, shop drawings, and mill test reports.
- 3. Erection drawings shall include plan drawings at 1/8"=1'-0" minimum scale complete with sections, elevations, and details as required to properly erect the structural steel frame.
- 4. Shop drawings shall include piece drawings which indicate cuts, connections, camber, holes, welds and dimensions as required for fabrication of the members. Part drawings are not required to be submitted unless specifically requested.

CONNECTIONS:

- 1. Engineer of Record (EOR) has designed all connections. If a connection design is inadvertently omitted from contract documents the contractor shall request specific connection design from the EOR.
- 2. Connection Design Forces: Factored LRFD values [Unfactored ASD values 3. Simple Beam Connections: Select connections with capacities equal to o greater than beam reactions shown on the drawings. Single sided connections shall be detailed to use the maximum number of bolt rows that can fit into the supported beam web. Double sided connections sha be detailed such that the angle or bent plate length is at least 60% of the supported beam "T" dimension.
- 4. HSS Cap Plates: Provide 1/4" cap plates at top of all HSS columns, uno. 5. Unframed end of wide flange beams: At the end of wide flange beams without incoming framing or other means of restraint of rotation of the beam, provide a pair of 3/8" full depth stiffeners or a 3/8" full depth end plate at the end of the beam.

BOLTS:

- 1. Where indicated on the drawings as slip critical and where oversized or long-slotted holes are utilized in shear, bolted joints shall be slip critical. Faying surfaces shall be prepared to meet the requirements of a Class A surface, and bolts shall be installed to the fully tensioned condition.
- 2. Where bolts are subject to non-static loading, are utilized to interconnect parts of a built up compression member, or all Group B fasteners loaded in tension shall be installed to the fully tensioned condition.
- 3. Bolts not subject to the requirements for slip critical connections and not
- required to be fully tensioned may be installed to the snug-tight condition. 4. A307 bolts may be used only where indicated.

WELDS:

- 1. Fillet Welds: Size as indicated, but not less than AISC minimum size.
- 2. Groove Welds: Full penetration unless noted otherwise.
- 3. Welds are continuous unless noted otherwise.

COLUMN BASE PLATES:

- 1. Provide flowable grout with a minimum compressive strength tested in accordance with ASTM C109 to achieve a strength of 3,000 psi after one day and the minimum of two times the concrete strength that the base plate is bearing on or 8,000 psi after 28 days.
- 2. Grout shall show a minimum positive expansion of 0.03% when tested in accordance with ASTM C827.
- 3. For base plates greater than 21" in length, provide a single 3" diameter witness hole near the center of the plate.
- 4. Trim grout to 45 degrees where bearing surface allows. Finish vertical when edge of bearing surface aligns with edge of bearing plate.
- 5. Grout column bases prior to pouring any elevated slab on deck.

ACCESSORIES:

1. Headed Stud Anchors: Shop weld except where applied through metal deck or where shop installation would result in a tripping hazard.

SHOP CLEANING AND PAINTING:

- 1. Uncoated Steel: All steel not specifically indicated as painted steel, steel to receive spray-on-fireproofing or to be galvanized, and faying surfaces of slip critical connections shall be uncoated. Prepare surface per SSPC-SP1.
- Primed Steel: Steel indicated to painted, with no specific paint requirements stated, shall have the surface prepared per SSPC-SP2 minimum and receive one coat of fabricator's standard rust-inhibitive primer paint applied to a minimum dry-film thickness of 1 mil.
- 3. Galvanized Steel: Steel indicated to be galvanized shall be cleaned, prepared, and galvanized in accordance with ASTM A123. Repair minor defects, damaged areas, and welded joints in accordance with ASTM A780. Provide vent holes as required in tube members. Provide vent hole plugs at all vertically oriented tubes.
- 4. Other specified coatings: Where indicated on the drawings, provide specified coating system as indicated. Clean and prepare steel as required by the specification or coating manufacture.

ERECTION:

- 1. No final bolting or welding shall be performed until as much of the structure which will be stiffened thereby has been properly aligned.
- 2. Field correction of fabrication or other errors will be permitted only when approved by the EOR. Finish gas-cut sections in accordance with AWS D1.1.

Material	Standard
W and WT Sections	ASTM A992 (50ksi) or ASTM A572 Gr. 50 (50ksi)
M, S, C, MC, L, MT, ST Sections	ASTM A36 (36ksi)
HP Sections	ASTM A572 Gr. 50 (50ksi)
Pipe	ASTM A53 Gr. B (35ksi)
Rectangular HSS	ASTM A500 Gr. C (50ksi)
Round HSS	ASTM A500 Gr. C (46ksi)
Plates, Bars, and Threaded Rod	
- typical	ASTM A36 (36ksi)
- when noted as 50ksi	ASTM A572 Gr. 50 (50ksi)
Anchor Rods	ASTM F1554 Gr. 55 w/ Supplement S1
Bolts	
- typical (Group A)	ASTM F3125 Grade A325 or F1852
- where noted as Group B	ASTM F3125 Grade A490 or F2280
- where indicated as A307	ASTM A307 Gr. A
Nuts	ASTM A563, Heavy Hex
Washers	ASTM F436, except plate washers to be ASTM A36
Direct-Tension-Indicator Washers	ASTM F959
Headed Stud Anchors	ASTM A108/A29
All Threaded Rod and Threaded Studs, UNO	ASTM A36
High Strength Threaded Studs	ASTM A29 or A572
Weld Electrodes	E70 (70ksi)
BOLT G	RADES
Standard	Bolt Size, Joint Type and Designation on Drawings
3/4"ø Bolt, ASTM F3125 Grade A325 or	3/4"ø A325 Bolt
1"ø Bolt, ASTM F3125 Grade A490 or	
F2280 with Class A Faying Surface and	1"ø A490 SC-A Bolt
Bolt Tightened to Slip Critical	
1/2"ø or 3/4"ø Bolt, ASTM A307 Gr. A	1/2"ø or 3/4"ø A307 Bolt

All bolts are snug tight, unless indicated on plan or details as slip critical or fully

tensioned. Holes may be short slotted transverse to applied load, unless plans, details, or connection tables indicate a standard or oversize hole.

Where bolts are indicated as slip critical or fully tensioned, pretension bolt as defined by AISC 360, Table J3.1.

Class A faying surfaces are unprimed surfaces or hot dip galvanized surfaces with hand wire brush roughening, as defined by AISC 360.

Class B faying surfaces are blast cleaned surfaces as defined by AISC 360.

Bolted connections to follow all requirements indicated in the Specification for Structural Joints Using High Strength Bolts (RCSC).

WOOD FRAMING GENERAL:

- 1. All wood construction work shall conform to ANSI/AF&PA NDS unless noted otherwise. Contractor shall keep a copy of these references on si at all times.
- 2. Materials See Wood Materials Tables
- 3. Qualifications Carpenter shall be experienced in construction of proje of similar size and complexity and shall be knowledgeable of convention light frame construction practices and minimum nailing requirements of the IBC.

SUBMITTALS:

- 1. All submittals shall be reviewed by the Contractor prior to Engineer/Architect review and shall bear Contractor's review stamp. Contractor is responsible for reviewing submittals for conformance with all contract documents and coordination with all trades.
- 2. Submittals are required for the following wood framing elements: premanufactured wood trusses, heavy timber framing, log framing, glulam framing, and manufactured framing including I-joists open web ioists.
- 3. Premanufactured truss submittals shall include dimensioned layout drawings that identify truss types, geometries, and locations as well as truss design calculations that indicate all design loads. Calculations shal signed and sealed by the manufacturer's engineer licensed in the state where the project is located.
- 4. Heavy timber submittals shall include shop drawings for trusses includir connection material and details.
- 5. Log framing submittals shall include shop drawings for trusses including connection material and details.
- 6. Glulam framing submittals shall include shop drawings for trusses including connections.
- Manufactured framing submittals shall include dimensioned layout plan indicating joist and beam types, locations, and connection hardware.
- 8. Wood I-joist and wood open web joist submittals shall include dimensioned layout plans indicating joist types, locations, and connection hardware.
- 9. Rim Board: Rim Board shall confirm to ANSI/APA PRR-410, grade B1 or better with a minimum thickness of 1 1/4" and match the floor or roof system depth. [Rim Board shall be Structural Composite Lumber (SCL) a minimum thickness of 1 3/4" and matching the floor or roof system depth.]

PRODUCTS:

- 1. All wood framing shall be at a moisture content of 19% or less and shall marked S-Dry (surface dried) or KD (kiln dried).
- 2. Unless noted otherwise, all sizes noted on these drawings are nominal. Actual sizes are based on "Minimum Dressed-Dry" dimensions accordin to American Softwood and Lumber Standard PS20-10. Members which the architect, engineer, or inspector judge to be misgraded shall be reinspected by a qualified lumber grader. Members which have permissible grade characteristics in such combination to affect the performance of the member are also subject to replacement at the discretion of the architect, engineer, or inspector.
- 3. Unless noted otherwise, all glulam framing sizes are minimum dressed dimensions in accordance with American Institute of Timber Constructi AITC113.
- 4. Unless noted otherwise, all manufactured framing sizes are based on specified manufacturers published information.
- 5. Wall studs to be Douglas Fir-Larch (DFL) No2 [Hem-Fir (HF) No2, Laminated Strand Lumber (LSL)] @16"OC, unless noted otherwise in the drawings.
- 6. Wood I-joists: where framing members are noted TJI on the drawings, u engineered products by Weyerhaeuser or approved equal.
- 7. Wood open-web joists: where framing members are noted "Red-L", "Re LT", " Red-W", "Red-S", "Red-M", and "RED-H", use engineered product by Red Built or approved equal.
- 8. Structural Panels: Sheathing for roofs and walls shall conform to APA PS standards. All panels shall be Exposure 1, unless noted otherwise.
- 9. Sills: Sill plates shall be pressure treated Douglas Fir-Larch stamped to show compliance with AWPA standards.

PREMANUFACTURED PRODUCTS:

- 1. Premanufactured wood trusses shall be designed in accordance with th "Design Specification for light metal plate connected wood trusses" exc where state and local codes are more stringent.
- 2. Premanufactured wood trusses shall be fabricated in accordance with t "TPI Quality Control Manual."
- Premanufactured wood trusses shall be installed in according to "Bracir Wood Trusses Commentary."
- 4. Design of wood elements in premanufactured wood trusses shall confo to NDS.
- 5. Wood utilized in premanufactured truss construction shall be stress graded bearing the mark of a recognized grading agency and shall conform to the rules and service requirements of the American Lumber Standards Committee PS-20.
- 6. Truss fabricator is responsible for all member and connection design an detailing and for all dimensioning, coordination, and erection of trusses Contract documents show only basic dimensioning and configurations trusses. Detailed positioning and spacing of trusses is the responsibility the fabricator.
- 7. Trusses shall be designed to resist the dead loads of completed construction and the larger of live, snow, and wind-uplift loads specified
- 8. Bottom chords shall be designed for the live loads required by the applicable codes and standards.
- 9. Metal anchorage devices for the trusses shall be designed for specified wind uplift less 0.6 (allowable stress design) of the resisting dead load. nailing of trusses is not permitted.

<u>W</u>	VOOD FRAMI	<u>NG (contin</u>	<u>ued)</u>								
C(1.	CONNECTORS: All bolts, m with preser dipped galv	etal connec vative trea vanized per	ctors, hangers, ted wood or u ASTM A653 G	. anchors, ar sed in wet c 185 or ASTN	nd faste onditio VI 153 o	ners in c ns shall l or stainle	contact be hot ss steel		RE FOI	VIEWE R COE	ED DE
2.	type 304 or Provide 5/8 elements fo Nailing Scho provide two	[•] 316. 3" diameter or attaching edule for sh	anchor bolts (sill plates, exc near wall ancho	@48"OC at t cept at shea or bolt spaci	the top r walls. ing. As a	of all for See She a minim	undation ar Wall um,		COM 02/2	PLIAN 24/202	23
3.	piece of sill J and L type	plate. bolts are a	allowed for an	chorage of v	vood sil	lls. Anch	or				
4.	material to Anchor boli minimum e	be ASTM F ts shall be c mbedment	1554 Grade 36 cast into concre t of 7".	6 (min). ete or grout	ed into	masonry	y with a				
5.	 Provide pla plates. See more inforr 	te washers "Typical W mation.	at all shear wa ood Shear Wa	all anchor bo lls - Nailing S	olt conn Schedul	ections le and De	to wall etails" for				
6.	5. Nailing shal 2304.10.1 c these draw	ll conform t of the IBC u ings or in th	to the minimu nless more str nese notes.	m requirem ingent requ	ents cor irement	ntained i ts are sh	in Table own on				
7. 8.	 All nails are Bolts shall of Prodrill paid 	to be stee	l common wire ASTM A307 Gr	e nails and c ade A.	onform	to ASTN	И F1667.				
9. 1(.0. Steel plates	for wood	construction sl	hall conform	n to AST	M A36.					
1:	1. All exposed.	l bolts in wo	ood structure v	which are no	ot in cor ted stee	ntact wit	th	1 11/	11/22 Issue	d for Permit	
12	.2. Holes for b	olts shall be	e 1/16" oversiz	zed.	ieu siee			No. D	ate		ription
13 14	.3. Retighten a .4. Lag screws	ill bolts pric shall penet	or to closing in trate the main	member a r	minimu	m of eig	ht times	OWNE	R	JNS & RE	EVISIONS
1!	the shaft di 5. Diagonal (t. distance of	ameter unl oe-nail) lag four times	ess noted othe screws shall b the shaft diam	erwise. e installed v peter	with a m	ninimum	edge	MA 2201	/ RIEGLE Wisconsin A	ER PROP	PERTIES
IN	NSTALLATION	:						Wash www.	mayriegler.c	20007 com	le 200
1.	Truss rods a leveling.	and connec	tions shall be t	tightened af	ter inst	allation	and	ARCHI	TECT		
2. 3.	supports w Glue and na	ith short ec	des staggered	bing urmensi l. ports with [8	on perp 8d@4"O)C edges	ai to , 8d@	KEVIN	K N & ASAKO	A SPERRY AF	6 A
4.	Nail roof sh and 8d@12	eathing to "OC field.	supports with	8d@4"OC e	edges, 8	d@6"O(C edges	Arlingt T.312 www.ł	ton, VA 2220 .636.3248 / 3 kasa-arch.co	312.636.425 07	2
5. 6	5. See plans fo	or areas of s	special blockin	ng and nailin	g.)C edge	s and 8d	@12"OC	GENEF	RAL CONTR	ACTOR	
7.	field for wa 7. Where shea	lls not desig ar walls are	gnated as sheat noted on the	ar walls.	haathia		@12 00				
	- · ·			plans, the si	neathin	g is used	l as part				
8.	of the later sheathing t . Where in co	al load resis o supports ontact with	sting system. S for wood strue concrete or m	See typical d ctural panel nasonry, wo	etails fo shear w od mem	g is used or attach valls. nbers sh	as part ment of all be				
8. Ff	of the later sheathing t Where in co pressure tro RAMING TOLE	al load resis o supports ontact with eated or se ERANCES:	sting system. S for wood strue concrete or m parated by a la	See typical d ctural panel nasonry, woo ayer of mois	etails fo shear w od mem ture ba	g is used or attach valls. nbers sh rrier.	as part ment of all be		NGINEER DMARK	ENGINE	ERING
8. FF 1. 2. 3.	of the later sheathing t 3. Where in co pressure tra RAMING TOLE Layout of w 2. Plates and 1 5. Studs: 1/4"	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of	sting system. S for wood strue concrete or m parated by a la rtitions: withir 4" in 8' from a f plumb, not cu	See typical d ctural panel nasonry, wo ayer of mois n 1/4" of into straight line	etails fc shear w od men ture ba ended p e.	g is used or attach valls. nbers sh rrier. position.	as part ment of all be	CIVIL E LAN 141 91 Steam T.970	MGINEER DMARK th Street, PC aboat Spring .871.9494	ENGINEE) Box 77494 s, CO 80477	ERING
8. Ff 1. 2. 3. 4.	of the later sheathing t 3. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 4. Face of fran	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i	sting system. S for wood strue concrete or m parated by a la rtitions: withir 4" in 8' from a f plumb, not cu in 8' from a tru	See typical d ctural panel nasonry, woo ayer of mois n 1/4" of into straight line umulative. ae plane.	etails fc shear w od men ture ban ended p e.	g is used or attach valls. nbers sh rrier.	as part ment of all be	CIVIL E LAN 141 9t Steam T.970 LANDS	DMARK DMARK th Street, PC boat Spring .871.9494 SCAPE ARC	ENGINEE) Box 77494 s, CO 80477 HITECT	ERING
8. Ff 1. 2. 3. 4.	of the later sheathing t 3. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 4. Face of fran	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ning: 1/4" i CON	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL	See typical d ctural panel nasonry, wo ayer of mois n 1/4" of into straight line umulative. ie plane.	etails fc shear w od mem ture ban ended p e.	or attach valls. nbers sh rrier. position.	as part ment of all be	CIVIL E LAN 141 9t Steam T.970 LANDS	CAPE ARC	ENGINEE) Box 77494 s, CO 80477 HITECT	BRING
8. 1. 2. 3. 4.	of the later sheathing t 3. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 4. Face of frar Common Nail (1 6d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ning: 1/4" i CON Steel Wire)	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru 1MON NAIL Minimum Dia 0.11	See typical d ctural panel nasonry, wo ayer of mois n 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in)	etails fc shear w od mem ture ban ended p e.	nbers sh rrier.	as part ment of all be ngth (in)	CIVIL E LAN 141 9f Steam T.970 LANDS	DMARK DMARK th Street, PC boat Spring .871.9494 CAPE ARC	ENGINEE) Box 77494 s, CO 80477 HITECT	ERING
8. Ff 1. 2. 3. 4.	of the later sheathing t 3. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 4. Face of fran Common Nail (1 6d 8d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ning: 1/4" i CON Steel Wire)	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.13	See typical d ctural panel nasonry, wo ayer of mois n 1/4" of inte straight line umulative. DIMENS ameter (in) 13	etails fc shear w od mem ture ban ended p e.	nimum Le	as part ment of all be ngth (in)	CIVIL E LAN 141 91 Steam T.970 LANDS	DMARK DMARK th Street, PC boat Spring 871.9494 CAPE ARC	ENGINEE 9 Box 77494 s, CO 80477 HITECT HITECT	ERING
8. Ff 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 4. Face of frar Common Nail (1 6d 8d 10d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire)	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12	See typical d ctural panel hasonry, wor ayer of mois n 1/4" of inte straight line umulative. Le plane. DIMENS ameter (in) 13	etails fc shear w od mem ture bai ended p e.	nbers sh rrier.	as part ment of all be ngth (in)	CIVIL E LAN 141 91 Steam T.970 LANDS	DMARK DMARK th Street, PC boat Spring 871.9494 CAPE ARC	ENGINEE) Box 77494 s, CO 80477 HITECT GINEER IFFRS &	BUIL DE
8. 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 5. Face of frar Common Nail (6d 8d 10d 12d 16d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ning: 1/4" i CON Steel Wire)	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru MON NAIL Minimum Dia 0.11 0.12 0.14 0.14	See typical d ctural panel hasonry, wor ayer of mois n 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48	etails fc shear w od mem ture ban ended p e.	nimum Le 2 2 1/2 3 3 1/4 3 1/2	as part ment of all be ngth (in)	CIVIL E LAN 141 94 Steam T.970 LANDS STRUC STRUC	DMARK DMARK th Street, PC boat Spring 871.9494 GCAPE ARC CTURAL EN CTURAL EN Washington en, CO 8040	ENGINEE) Box 77494 s, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1	ERING 37 BUILDEI
8. 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 5. Face of frar Common Nail (6d 8d 10d 12d 16d 20d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru MON NAIL Minimum Dia 0.11 0.12 0.14 0.14 0.14	See typical d ctural panel hasonry, wor ayer of mois n 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48 52 52	etails fc shear w od mem ture bal ended p e.	nimum Le 2 2 1/2 3 3 1/4 4	as part ment of all be ngth (in)	CIVIL E LAN 141 94 Steam T.970 LANDS STRUC STRUC KL8 1717 Golde T. 303	DMARK boat Spring 871.9494 CAPE ARC CTURAL EN Washington m, CO 8040 3.384.9910	ENGINEE) Box 77494 s, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1	ERING 37 BUILDEI KL&A
8. Ff 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of frar Common Nail (6d 8d 10d 12d 16d 20d	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12	See typical d ctural panel hasonry, wor ayer of mois n 1/4" of intre- straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48 52 52 52	etails fc shear w od mem ture bal ended p e.	nbers sh rrier.	as part ment of all be ngth (in)	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC STRUC KL8 1717 Golde T. 303 M.E.P.	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN Washington en, CO 8040 3.384.9910 & F.P. ENG	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS	ERING ³ 7 BUILDEI KL&A
8. FI 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of frar 6d 8d 10d 12d 16d 20d DES Species & Grade	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru UMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	See typical d ctural panel hasonry, wor ayer of mois n 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48 52 52 52 1 MENSION Compress 5 Stress Per Grain	etails fc shear w od mem ture bal ended p e. IONS Min e. VAL LU sive Ho p to	nimum Le 2 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress	as part ment of all be ngth (in)	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOL 1717 Bould T. 304	DMARK bh Street, PC boat Spring 871.9494 CAPE ARC CURAL EN CTURAL EN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 8030 3.444.6038	ENGINEE) Box 77494 s, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1 INEERS NGINEEF	ERING
8. Ff 1. 2. 3. 4.	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and 1 3. Studs: 1/4" 3. Face of fran 6d 8d 10d 12d 16d 20d DES Species & Grade	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ning: 1/4" i COM Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	A pine of the of	etails fc shear w od mem ture bai ended p e. IONS Min e. NAL LU sive Ho p to	nbers sh rrier. nbers sh rrier. noosition. 2 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress	as part ment of all be ngth (in)	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOL 1717 Bould T. 303	ENGINEER DMARK th Street, PC boat Spring 871.9494 GCAPE ARC CTURAL EN CTURAL EN A ENGIN Washington on, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 8030 3.444.6038 OR DESIGN	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1 INEERS NGINEEF 2 VER:	ERING 37 BUILDEI KL&A RING
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of frar Common Nail (6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	A pine of the of	etails fc shear w od mem ture bai ended p e. IONS Min e. VAL LU sive Ha	nbers sh rrier. nbers sh rrier. noosition. noosition. 2 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress	as part ment of all be ngth (in) 2 2 4 2 5 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOL 1717 Bould T. 303 INTERI	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CTURAL EN A ENGIN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 JER:	ERING 37 BUILDEI KL&A
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of frar Common Nail (6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire)	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	in 1/4" of inte asonry, wor ayer of mois a 1/4" of inte asonry, wor ayer of mois an 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48 52 52 52 1 MENSION Compress 5 5 1 Stress Per Grain LARCH (DFL) 625 ps 625 ps	etails fc shear w od mem ture bai ended p e. IONS Min e. VAL LU sive Ho p to	nbers sharing nimum Le 2 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress 180 psi 180 psi 180 psi	as part ment of all be ngth (in) 2 Modulus of Elasticity 1,900 ksi 1,700 ksi 1,700 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOU 1717 Bould T. 303 INTERI	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CTURAL EN CTURAL EN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 NER: ATHAN S	
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of frar 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stud	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) Steel Wire) Steel Wire) Steel Wire) I 1,500 psi 1,000 psi 900 psi 700 psi	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	in 1/4" of inte asonry, wor ayer of mois a 1/4" of inte straight line umulative. ie plane. DIMENS ameter (in) 13 31 48 48 52 52 52 62 5 1 1 1 1 1 3 1 48 48 52 52 52 5 1 1 1 1 3 1 48 48 52 52 52 5 1 1 1 1 1 3 1 4 8 1 4 8 52 5 2 5 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	etails fc shear w od mem ture bai ended p e. IONS Min e. VAL LU sive Ha sive Ha i i i i	nbers sh rrier. nbers sh rrier. noosition. n	as part ment of all be ngth (in) 2 4 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T.303 M.E.P. BOL 1717 Bould T.303 INTERI JOH 1600 Denvo T.303	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 8030 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 NER: ATHAN S ATHAN S	
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 5. Face of fran 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stud	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) GIGN VAL Flexural Stress 1,000 psi 900 psi 700 psi GN VALU	sting system. S for wood struct concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	A plans, the signal of the sis	etails for shear w od mem ture bai ended p e. IONS Min e. NAL LU sive He p to He i i i i i	nbers sharing nbers sharing nbers sharing nimum Le 2 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress 180 psi 180 psi 180 psi 180 psi 180 psi	as part ment of all be ngth (in) 2 1,900 ksi 1,700 ksi 1,700 ksi 1,600 ksi 1,400 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOL 1717 Bould T. 303 INTERI JOH 1600 Denvo T.303	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CTURAL EN CTURAL EN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062 CT LOCATIC CT LOCATIC	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 JER: ATHAN S 2 JER: ATHAN S	
8. Ff 1. 2. 3. 4. S	of the later sheathing t sheathing t where in co pressure tro RAMING TOLE . Layout of w . Plates and r . Studs: 1/4" . Face of frar Common Nail (6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stuc Type - E	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,500 psi 1,000 psi 900 psi 700 psi GN VALU Flexural Stress	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	A plans, the straight line asonry, wor ayer of mois a 1/4" of inte straight line umulative. a DIMENS ameter (in) 13 31 48 48 52 32 1 1 1 1 3 1 48 48 52 32 1 1 1 3 1 48 48 52 32 1 1 1 1 3 1 48 48 52 32 1 1 1 1 3 1 48 48 52 52 52 52 1 1 1 1 1 3 1 48 48 52 52 52 51 1 1 1 1 3 1 1 1 1 1 3 1 1 1 3 1	etails for shear w od mem ture bai ended p e. IONS Min e. VAL LU sive Ho p to i i i i i j u cressive s Perp	nbers shar rrier. bosition. cosition	as part ment of all be ngth (in) 2 1,900 ksi 1,700 ksi 1,600 ksi 1,400 ksi 1,400 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. PROJEC STE PA A	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 8020 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN	ENGINEE D Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 NER: ATHAN S ATHAN S 2 NO 2 N AT BAS RENOY	
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE . Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of fran 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stud Type - E	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) Steel Wire) Steel Wire Steel Wire Steel Wire Steel Wire Stress I 1,500 psi 1,000 psi 900 psi 700 psi Stress I CA	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.12 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Plans, the straight line bee typical d ctural panel basonry, woo ayer of mois h 1/4" of inte basonry, woo ayer of mois h 1/4" of inte straight line umulative. umulative. umulative. umulative. ameter (in) 13 31 48 52 32 IMENSION Compress I Compress Grain LARCH (DFL) 625 ps <	ICNS IONS IONS IONS IONS IONS IONS IONS IONS IONS IONS IONS IONS IONS IONS VL)	nbers shar rrier. bosition	as part ment of all be ngth (in) 2 1,900 ksi 1,700 ksi 1,600 ksi 1,400 ksi 1,400 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL& 1717 Golde T.303 M.E.P. BOL 1717 Golde T.303 M.E.P. BOL 1717 Golde T.303 M.E.P. PA BOL 1717 Bould T.303 INTERI JOH 1600 Denvo T.303	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 8020 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 19010	ENGINEE Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. INEERS NGINEEF 2 VER: ATHAN S ATHAN S ATHAN S CURVE PLA	ERING 37 BUILDEI © KL&A RING STROHE SECAM VATION IT-OUT AZA
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of fran 6d 8d 10d 12d 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stud DES	al load resis o supports ontact with eated or se ERANCES: valls and par runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 200 psi I 700 psi GN VALU Flexural Stress	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Image: set	ICNS ICNS ICNS ICNS ICNS ICNS ICNS ICNS	pis used or attach valls. hbers shi rrier. oosition. oosition. 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 285 psi	as part ment of all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,700 ksi 1,400 ksi 2,000 ksi	CIVIL E LAN 141 91 Stearr T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 BOU 1717 GOLDE T. 303 M.E.P. BOU 1717 17 17 17 17 17 17 17 17	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington m, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 80320 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901 0 TEAMBOAT ND TEN	ENGINEE Box 77494 S, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 JER: ATHAN S ATHAN S , Suite 100 2 N AT BAS RENO AT BAS CURVE PLA CURVE PLA	ERING 37 BUILDEI KL&A RING STROHE SECAM VATION IT-OUT AZA CO 80487
8. Ff 1. 2. 3. 4. S	of the later sheathing t S. Where in co pressure tro RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 4. Face of fran 6d 8d 10d 12d 16d 20d 0 ES Species & Grade Select Strucutra No1 No2 Stuc DES Select Strucutra No1 No2 Stuc	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 2,600 psi COD STRI	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Joins) the straight info See typical d See typical d ctural panel asonry, word ayer of mois n 1/4" of intrestraight line awer of mois n 1/4" of intrestraight line ameter (in) 13 31 48 52 32 IMENSION Compress IMENSION Compress IARCH (DFL) 625 ps	etails for shear wo od mem ture bai ended p e. IONS Min e. VAL LU sive Ho p to i i i i i i j URED I oressive s Perp VL) 0 psi	g is used or attach valls. hbers shi rrier. oosition. oosition. a 1/4 3 1/2 3 2 1/2 3 3 1/4 3 1/2 4 JMBEF orizontal Shear Stress 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 285 psi	as part ment of all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,600 ksi 1,400 ksi 2,000 ksi	CIVIL E LAN 141 94 Steam T.970 LANDS STRUC KL& 1717 Golde T.303 M.E.P. BOL 1717 Golde T.303 M.E.P. BOL 1717 Golde T.303 M.E.P. PROJEC STE PA AI S DRAWIN	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN CTURAL EN CTURAL EN CTURAL EN CTURAL EN CAENGIN Washington in, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN INSON N Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901	ENGINEE Box 77494 S, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 VER: ATHAN S ATHAN S ATHAN S CURVE PLA CURVE PLA CURVE PLA	
8. FI 1. 2. 3. 4. S	of the later sheathing t Sheathing t Where in co pressure tro RAMING TOLE . Layout of w Plates and r Studs: 1/4" . Face of fran 6d 8d 10d 12d 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stuc Stuc Stuc Supporting	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i COM Steel Wire) Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 2,600 psi COD STRU Element	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Joints) the straight integration of mois See typical d Sasonry, wood ayer of mois n 1/4" of integration ayer of mois ameter (in) 13 31 48 52 32 IMENSION Compress I Stress Per Grain LARCH (DFL) 625 ps 625 p	etails for shear w od mem ture bai ended p e. IONS Min e. VAL LU sive H p to i i i i i j URED I oressive s Perp VL) 0 psi	g is used or attach valls. hbers shi rrier. oosition. oosition. oosition. a 1/4 a 1/2 a a 1/4 bear stress 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 285 psi VENTS nimum T	as part ment of all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,600 ksi 1,400 ksi 2,000 ksi 2,000 ksi	CIVIL E LAN 141 94 Steam T.970 LANDS STRUC KL& 1717 Golde T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. BOL 1717 Bould T.303 INTERI JOH 1600 Denve T.303 PROJEC STE PA AI	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington en, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN INSON N Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901 TEAMBOAT 1901 CT MBOAT SENEF	ENGINEE Box 77494 S, CO 80477 HITECT GINEER IEERS & Ave. I INEERS NGINEEF 2 VER: ATHAN S ATHAN S 2 NO AT BAS RENO NO AT BAS	ERING BUILDEI KL&A RING STROHE SECAM VATION IT-OUT AZA CO 80487
8. FI 1. 2. 3. 4. J S S S S S S S S S S S S S S S S S S S	of the later sheathing t S. Where in co pressure tra RAMING TOLE Layout of w 2. Plates and r 3. Studs: 1/4" 5. Face of fran 6d 8d 10d 12d 6d 8d 10d 12d 16d 20d 0 ES Species & Grade Select Strucutra No1 No2 Stuc Stuc Stuc Stuc Stuc Stuc Stuc Stuc	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire) Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 2,600 psi COD STRU Element	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Junitic See typical d See typical d See typical d See typical d Sasonry, woo ayer of mois ayer of mois anting I Stress Per Grain LARCH (DFL) 625 ps 750 ANEL REC ANEL REC Rating 20	etails for shear w od mem ture bai ended p e. IONS Min e. NAL LU sive H p to NAL LU sive H p to UIRED vL) 0 psi	g is used or attach valls. hbers shi rrier. oosition. oosition. oosition. a 1/4 a 1/2 a a 1/4 a 3 1/2 a a 1/4 bear stress a a 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 180 psi 285 psi VENTS nimum Th a 19/32	as part ment of all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,700 ksi 1,400 ksi 2,000 ksi 2,000 ksi 2,000 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL8 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. C	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN INSON N Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901 0 TEAMBOAT ND TEN 1901 0 TEAMBOAT ND TEN	ENGINEE Box 77494 S, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1 INEERS NGINEEF 2 VER: ATHAN S ATHAN S CURVE PLA CURVE PLA SPRINGS, CURVE PLA	ERING BUILDEI KL&A RING STROHE SECAM VATION IT-OUT AZA CO 80487
8. FI 1. 2. 3. 4. J S S S S S S S S S S S S S S S S S S S	of the later sheathing t S. Where in co pressure tra RAMING TOLE . Layout of w 2. Plates and r 3. Studs: 1/4" . Face of frar 6d 8d 10d 12d 6d 8d 10d 12d 16d 20d DES Species & Grade Select Strucutra No1 No2 Stuc Stuc Species & Grade UES Species & Grade	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire) Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 1,000 psi I 2,600 psi I 2,600 psi COD STRU Element	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Junition See typical d Sasonry, woo ayer of mois n 1/4" of into straight line umulative. ue plane. JUMENSION ameter (in) 13 31 48 52 32 IMENSION Stress Per Grain LARCH (DFL) 625 ps 750	etails for shear w od mem ture bai ended p e. IONS Min e. VAL LU sive H p to VL) 0 psi UIRED VL) 0 psi	g is used or attach valls. hbers shi rrier. oosition. oosition. a 1/4 a 1/2 a a 1/4 a 3 1/2 a a 1/4 a 3 1/2 a a 1/4 a 3 1/2 a a 1/4 a 3 1/2 a a 1/4 bear stress 180 psi 180 psi	as part ment of all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,700 ksi 1,400 ksi 2,000 ksi 2,000 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL8 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. BOL 1717 Golde T. 303 M.E.P. C STE PA AI SEAL	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN A ENGIN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901 TEAMBOAT ND TEN 1901 CTEAMBOAT SENEF	ENGINEE Box 77494 S, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 VER: ATHAN S ATHAN S ATHAN S CURVE PLA CURVE PLA SPRINGS, CURVE PLA	ERING BUILDEI KL&A RING STROHE SECAM VATION IT-OUT AZA CO 80487 OTES
8. FI 1. 2. 3. 4. S S S S S	of the later sheathing t S. Where in co pressure tra RAMING TOLE . Layout of w 2. Plates and r 3. Studs: 1/4" . Face of frar Gommon Nail (6d 8d 10d 12d 16d 20d 0 ES Species & Grade Select Strucutra No1 No2 Stud Stud DES Species & Grade Select Strucutra No1 No2 Stud DESIC Supporting I Roof over Trusses/Raft Roof over T8 Floors (Sturd	al load resis o supports ontact with eated or se ERANCES: valls and par runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire) Steel Wire) Stress CON Steel Wire) Stress CON Stres CON StresS CON Stress CON StresS CON StresS CON StresS CON S	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Junitic See typical d Straight line Junulative. Juntative. Juntative. <td>etails for shear wo od mem ture bai ended p e. IONS Min ended p e. NAL LU sive Ha p to Ha i i i i i i i i i i i i i i i i i i i</td> <td>g is used or attach valls. hbers shi rrier. oosition. oosition. oosition. a 1/4 a 1/2 a a 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/4 a 3 1/2 a 3 1/4 a 3 1</td> <td>as part ment of all be ngth (in) ngth (in) ngt</td> <td>CIVIL E LAN 141 91 Stearr T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Bould T. 303 M.E.P. BOU 1717 BOU T.303 INTERI JOH 1600 Denve T.303 INTERI STE PA AI STE</td> <td>ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CTURAL EN CTURAL EN CTURAL EN CAENGIN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 CT LOCATIC EAMBO ARTIAL 1901 TEAMBOAT ND TEN SENEF</td> <td>ENGINEE Box 77494 S, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1 INEERS NGINEEF 2 JER: ATHAN S ATHAN S ATHAN S CURVE 100 2 N AT BAS RENO AT BAS RENO AT BAS CURVE PLA SPRINGS, CURVE PLA</td> <td>ERING BUILDEI <i>KL&A</i> RING STROHE SECAM VATION IT-OUT AZA CO 80487 OTES DATE: 07/06/20</td>	etails for shear wo od mem ture bai ended p e. IONS Min ended p e. NAL LU sive Ha p to Ha i i i i i i i i i i i i i i i i i i i	g is used or attach valls. hbers shi rrier. oosition. oosition. oosition. a 1/4 a 1/2 a a 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/2 a bear stress a a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/4 a 3 1/2 a 3 1/4 a 3 1	as part ment of all be ngth (in) ngth (in) ngt	CIVIL E LAN 141 91 Stearr T.970 LANDS STRUC KL& 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Golde T. 303 M.E.P. BOU 1717 Bould T. 303 M.E.P. BOU 1717 BOU T.303 INTERI JOH 1600 Denve T.303 INTERI STE PA AI STE	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CTURAL EN CTURAL EN CTURAL EN CAENGIN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 CT LOCATIC EAMBO ARTIAL 1901 TEAMBOAT ND TEN SENEF	ENGINEE Box 77494 S, CO 80477 HITECT GINEER GINEER IEERS & Ave. 1 INEERS NGINEEF 2 JER: ATHAN S ATHAN S ATHAN S CURVE 100 2 N AT BAS RENO AT BAS RENO AT BAS CURVE PLA SPRINGS, CURVE PLA	ERING BUILDEI <i>KL&A</i> RING STROHE SECAM VATION IT-OUT AZA CO 80487 OTES DATE: 07/06/20
8. FI 1. 2. 3. 4. S S S S S S S	of the later sheathing t S. Where in co pressure tra RAMING TOLE . Layout of w 2. Plates and r 3. Studs: 1/4" . Face of frar 6d 8d 10d 12d 6d 8d 10d 12d 16d 20d 0 ES Species & Grade Select Strucutra No1 No2 Stuc Stuc DES Select Strucutra No1 No2 Stuc DESIC Supporting R Roof over T8 Floors (Sturd Walls	al load resis o supports ontact with eated or se ERANCES: valls and pa runners: 1/ in 8' out of ming: 1/4" i CON Steel Wire) Steel Wire) Steel Wire) GIGN VAL Flexural Stress I 1,000 psi I 1,000 psi I 1,000 psi I 2,600 psi I 700 psi COD STRU Element Element ers G Decking -1 T&G)	sting system. S for wood strue concrete or m parated by a la rtitions: within 4" in 8' from a f plumb, not cu in 8' from a tru IMON NAIL Minimum Dia 0.11 0.12 0.12 0.12 0.12 0.12 0.12 0.12	Joins) the straight indicator See typical d See typical d See typical d Sasonry, wood ayer of mois n 1/4" of inters ayer of mois ameter (in) 13 31 48 52 32 IMENSION Compress I Compress G25 ps 625 ps 750 ANEL REC ANEL REC 70 gle Floor 16	ICNS ended p ended	g is used or attach valls. hbers shi rrier. oosition. oosition. oosition. a 1/4 a 1/2 a a 1/4 a 3 1/2 a a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/2 a 3 1/4 a 3 1/2 a 1/2 a 3 1/2 a 1/2 a 1/2 a 1/2 a 1/2 a 1/2 a 1/2 a 1/2 a 1/2 a 1/2	as part ment of all be all be ngth (in) 1,900 ksi 1,700 ksi 1,700 ksi 1,700 ksi 1,400 ksi 2,000 ksi 2,000 ksi 2,000 ksi	CIVIL E LAN 141 91 Steam T.970 LANDS STRUC KL8 1717 Golde T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. BOL 1717 Bould T.303 M.E.P. C STE PA AI S DRAWIN	ENGINEER DMARK th Street, PC boat Spring 871.9494 CAPE ARC CAPE ARC CTURAL EN CAENGIN Washington n, CO 8040 3.384.9910 & F.P. ENG JLDER E 15th Street er, CO 803C 3.444.6038 OR DESIGN NSON N Wynkoop St er, CO 8020 .892.7062 CT LOCATIC EAMBO ARTIAL ND TEN 1901 TEAMBOAT ND TEN SENEF	ENGINEE Box 77494 s, CO 80477 HITECT GINEER IEERS & Ave. 1 INEERS NGINEEF 2 VER: NGINEEF 2 VER: ATHAN S 2 VER: ATHAN S 2 VER: CURVE PLA SPRINGS, CURVE PLA SPRINGS,	ERING BUILDEI KL&A RING SECAM VATION IT-OUT AZA CO 80487 OTES AZA CO 80487

S0103

DRAWING NO:

21304

COPYRIGHT 2019

TYPICAL DETAIL SHEET NOTES

- 1. It is the contractor's responsibility to understand the typical details on this sheet and apply them as needed on the project.
- Typical details on this sheet are generally not referenced from any other drawing on the project.
 Typical details on this sheet MAY be referenced on plans or details to
- 3. Typical details on this sheet MAY be referenced on plans or details to clarify or identify a particular condition. The presence of such a reference does not alter the obligation of the contractor to apply the detail(s) as needed even if they are not referenced.

CLASS B SPLICE, TYP





CLASS B SPLICE, TYP







HEADER CONNECTION,

BEARING STUDS EACH SIDE,

	HEADERS IN LOAD BEARING WALLS										
SPAN	DIMENSIONED LUMBER DOUGLAS-FIR	LSL ALTERNATES	LVL ALTERNATES	GLULAM ALTERNATES	NO OF BEARING STUDS EACH END	HDR CONNECTION					
3'-0"	(3)2x8	(2)1 3/4"x5 1/2"	(2)1 3/4"x5 1/2"	3 1/2"x6"	1	(4)10d					
4'-0"	(3)2x10	(3)1 3/4"x5 1/2"	(3)1 3/4"x5 1/2"	3 1/2"x6"	2	(4)10d					
5 ' -0"	(3)2x12	(3)1 3/4"x7 1/4"	(2)1 3/4"x7 1/4"	3 1/2"x7 1/2"	2	(4)10d					
6'-0 "	N/A	(2)1 3/4"x9 1/4"	(3)1 3/4"x7 1/4"	3 1/2"x9"	2	(6)10d					
7'-0"	N/A	(3)1 3/4"x9 1/4"	(3)1 3/4"x9 1/4"	3 1/2"x10 1/2"	3	(6)10d					
8'-0"	N/A	(3)1 3/4"x11 1/4"	(3)1 3/4"x9 1/2"	3 1/2"x11 7/8"	3	(6)10d					
9'-0"	N/A	(3)1 3/4"x11 7/8"	(3)1 3/4"x11 1/4"	3 1/2"x13 1/2"	3	(8)10d					
10'-0"	N/A	(3)1 3/4"x14"	(3)1 3/4"x11 7/8"	3 1/2"x15"	4	(8)10d					

	RECOMMENDED HEADERS IN INTERIOR NON-LOAD BEARING WALLS ¹⁰										
SPAN	DIMENSIONED LUMBER DOUGLAS-FIR	LSL ALTERNATES	LVL ALTERNATES	GLULAM ALTERNATES	NO OF BEARING STUDS EACH END	HDR CONNECTION					
3'-0"	(2)2x4	(2)1 3/4"x5 1/2"	(2)1 3/4"x5 1/2"	3 1/2"x6"	1	(2)10d					
4'-0"	(3)2x4	(2)1 3/4"x5 1/2"	(2)1 3/4"x5 1/2"	3 1/2"x6"	1	(2)10d					
5 ' -0"	(2)2x6	(2)1 3/4"x5 1/2"	(2)1 3/4"x5 1/2"	3 1/2"x6"	1	(2)10d					
6'-0"	(3)2x6	(2)1 3/4"x5 1/2"	(2)1 3/4"x5 1/2"	3 1/2"x6"	1	(2)10d					
7 ' -0"	(2)2x8	(3)1 3/4"x5 1/2"	(3)1 3/4"x5 1/2"	3 1/2"x6"	1	(2)10d					
8'-0"	(3)2x8	(2)1 3/4"x7 1/4"	(2)1 3/4"x7 1/4"	3 1/2"x7 1/2"	1	(2)10d					
9'-0"	(3)2x10	(3)1 3/4"x7 1/4"	(2)1 3/4"x7 1/4"	3 1/2"x7 1/2"	1	(2)10d					
10'-0"	(3)2x10	(2)1 3/4"x9 1/2"	(3)1 3/4"x7 1/4"	3 1/2"x7 1/2"	1	(2)10d					

NOTES: 1. THIS TABLE APPLIES TO HEADERS WHICH ARE NOT EXPLICITLY CALLED OUT ON PLAN WITH SPANS OF 10'-0" OR LESS

HEADERS IN LOAD BEARING WALLS DESIGNED FOR 2000 PLF DEAD + LIVE LOAD. HEADERS IN NON-LOAD BEARING WALLS DESIGNED FOR 400 PLF DEAD + LIVE LOAD.

4. DIMENSIONED LUMBER HEADERS TO BE DOUGLAS-FIR No2. 5. LVL = LAMINATED VENEER LUMBER: Fb = 2600 PSI, E = 2000 KSI

5. LSL = LAMINATED STRAND LUMBER: Fb = 2250 PSI, E = 1500 KSI 7. GLULAM GRADE IS 24F-V4 DF

8. LIVE LOAD DEFLECTION CRITERIA IS L/360

9. HEADERS SUPPORTING POINT LOADS FROM BEAMS OR COLUMNS SHOULD NOT BE SIZED FROM THIS TABLE. NOTIFY STRUCTURAL ENGINEER. 10. RE: ARCH FOR LOCATIONS OF NON-LOAD BEARING WALLS.

TYPICAL WOOD HEADER TABLE

6 S0105 NTS

URAL TION

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS ON THE PROJECT TO CLEARLY DEFINE ALL OF THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR CONTACT ARCHITECT FOR CLARIFICATION.

THE ONLY.

alified people under i Drawing and such Ied for information of

ENGINEERS SEAL ON THIS DRAWING INDICATES THAT THE INFORMATION SHOWN AND THE CALCULATIONS PERTAINING TO THAT INFORMATION HAVE BEEN PREPARED BY QU OF THE ENGINEER-OF-RECORD. THE SEAL DOES NOT IMPLY RESPONSIBILITY FOR INFORMATION PREPARED BY OTHERS NOR FOR ANY INFORMATION NOT SHOWN ON THIS SPECIFICALLY DISCLAIMED. ON PHASED PROJECTS, DRAWINGS THAT ARE ISSUED BUT NOT SEALED SHALL BE CONSIDERED TO BE PRELIMINARY IN NATURE AND ARE ISSU



11/11/22 Issued for Permit Date Description SUBMISSIONS & REVISIONS OWNER **MAY RIEGLER PROPERTIES** 2201 Wisconsin Ave NW Suite 200 Washington, DC 20007 www.mayriegler.com ARCHITECT **KEVIN & ASAKO SPERRY ARCHITECTURE** 3318 N. Columbus Street Arlington, VA 22207 T.312.636.3248 / 312.636.4252 www.kasa-arch.com GENERAL CONTRACTOR **CIVIL ENGINEER** LANDMARK ENGINEERING 141 9th Street, PO Box 774943 Steamboat Springs, CO 80477 T.970.871.9494 LANDSCAPE ARCHITECT STRUCTURAL ENGINEER **KL&A ENGINEERS & BUILDERS** 1717 Washington Ave. Golden, CO 80401 T. 303.384.9910 © 2022 KL&A, INC M.E.P. & F.P. ENGINEERS **BOULDER ENGINEERING** 1717 15th Street Boulder, CO 80302 T. 303.444.6038 INTERIOR DESIGNER: JOHNSON NATHAN STROHE 1600 Wynkoop St., Suite 100 Denver, CO 80202

T.303.892.7062

DRAWING TITLE

PROJECT LOCATION

STEAMBOAT BASECAMP

PARTIAL RENOVATION

AND TENANT FIT-OUT

1901 CURVE PLAZA

STEAMBOAT SPRINGS, CO 80487

TYPICAL DETAILS -

WOOD

56031

SIONAL

S0105

DRAWING NO:

APPROVAL STAMPS:

REVIEWED

FOR CODE

COMPLIANCE

02/24/2023

COPYRIGHT 2019

DATE:

10/17/22

Author

Checker

21304

DRAWN BY

CHECKED BY

PROJECT NO:

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS ON THE PROJECT TO CLEARLY DEFINE ALL OF THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR CONTACT ARCHITECT FOR CLARIFICATION. THE ONLY. ALIFIED PEOPLE UNDER 7 DRAWING AND SUCH JED FOR INFORMATION ON

ENGINEERS SEAL ON THIS DRAWING INDICATES THAT THE INFORMATION SHOWN AND THE CALCULATIONS PERTAINING TO THAT INFORMATION HAVE BEEN PREPARED BY QUA OF THE ENGINEER-OF-RECORD. THE SEAL DOES NOT IMPLY RESPONSIBILITY FOR INFORMATION PREPARED BY OTHERS NOR FOR ANY INFORMATION NOT SHOWN ON THIS SPECIFICALLY DISCLAMED. ON PHASED PROJECTS, DRAWINGS THAT ARE ISSUED BUT NOT SEALED SHALL BE CONSIDERED TO BE PRELIMINARY IN NATURE AND ARE ISSU

RUCTURAL DIRECTION

NOTES: 1. ALL MULTIPLE WOOD MEMBERS MUST BE FASTENED TOGETHER TO ACT AS A SINGLE UNIT. 1. ALL MULTIPLE WOOD MEMBERS MUST BE FASTENED WITH EXTERIOR GUIE. LAMINATIONS SHALL BE CONTINUOUSLY GLUED WITH EXTERIOR GLUE. LAMINATIONS SHALL BE DRY (LESS THAN 16% MOISTURE CONTENT) WHEN GLUED. DO NOT SPLICE LAMINATIONS 4. WHERE FASTENERS TO BE INSTALLED ON BOTH SIDES, STAGGER BY 1/2 THE REQUIRED CONNECTOR SPACING. 5. PSL MATERIAL OF EQUAL CROSS-SECTIONAL DIMENSIONS MAY BE SUBSTITUTED FOR BUILT-UP LVL BEAMS. 6. 7" WIDE BEAMS SHOULD BE SIDE-LOADED ONLY WHEN LOADS ARE APPLIED TO BOTH SIDES OF THE MEMBERS (TO MINIMIZE ROTATION). 7. w = MAXIMUM UNIFORM LOAD APPLIED TO EITHER OUTSIDE MEMBER.

1 ^{S0106} NTS

WOOD COLUMN FLOOR SHEATHING

BLOCK BETWEEN COL ABOVE AND BELOW, MATERIAL TO

MATCH WIDTH OF COL BELOW WALL DOUBLE TOP PLATE WOOD COLUMN

	REVIEWED FOR CODE COMPLIANCE
	02/24/2023
	• • • • • • • • • • • • • • • • • • •
1	11/11/22 Issued for Permit
No.	Date Description SUBMISSIONS & REVISIONS
AR K 33 A T	CHITECT K A S A EVIN & ASAKO SPERRY ARCHITECTURE 318 N. Columbus Street rlington, VA 22207 .312.636.3248 / 312.636.4252
w GE	WW.kasa-arch.com
S T LA	teamboat Springs, CO 80477 .970.871.9494
ST	RUCTURAL ENGINEER
01	
1 G	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Golden, CO 80401 . 303.384.9910
1 G T M.I	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Golden, CO 80401 303.384.9910 E.P. & F.P. ENGINEERS
h 1 G T M.I 8 1 8 T	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Golden, CO 80401 303.384.9910 KL&A, II E.P. & F.P. ENGINEERS SOULDER ENGINEERING 717 15th Street Soulder, CO 80302 303.444.6038
M.I M.I M.I M.I	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Golden, CO 80401 0 20. X17 Washington Ave. Golden, CO 80401 0 20. KL&A, II E.P. & F.P. ENGINEERS SOULDER ENGINEERING 717 15th Street Goulder, CO 80302 303.444.6038
и 1 6 Т М.I 1 8 Т 1 1 1 1 1 1 7 Т	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Solden, CO 80401 . 303.384.9910 KL&A, II E.P. & F.P. ENGINEERS BOULDER ENGINEERING 717 15th Street Soulder, CO 80302 . 303.444.6038 FERIOR DESIGNER: ICHNSON NATHAN STROHE 600 Wynkoop St., Suite 100 Denver, CO 80202 . 303.892.7062
H 1 G T M.I B T INT J PRC S	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Solden, CO 80401 . 303.384.9910 KL&A, II E.P. & F.P. ENGINEERS SOULDER ENGINEERING 717 15th Street Boulder, CO 80302 . 303.444.6038 FERIOR DESIGNER: OHNSON NATHAN STROHE 600 Wynkoop St., Suite 100 Denver, CO 80202 .303.892.7062 DJECT LOCATION STEAMBOAT BASECAMP PARTIAL RENOVATION AND TENANT FIT-OUT
H 1 C T M.I B T INT J 1 D T PRC S	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Solden, CO 80401 : 303.384.9910 KL&A, II E.P. & F.P. ENGINEERS SOULDER ENGINEERING 717 15th Street Soulder, CO 80302 : 303.444.6038 TERIOR DESIGNER: OHNSON NATHAN STROHE 600 Wynkoop St., Suite 100 Denver, CO 80202 : 303.892.7062 DJECT LOCATION STEAMBOAT BASECAMP PARTIAL RENOVATION AND TENANT FIT-OUT 1901 CURVE PLAZA STEAMBOAT SPRINGS, CO 80487 AWING TITLE
H 1 G T M.I B T INT J 1 D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T D T T D T T T T T T T T	KL&A ENGINEERS & BUILDERS 717 Washington Ave. Golden, CO 80401 . 303.384.9910 & 20. KL&A, II E.P. & F.P. ENGINEERS BOULDER ENGINEERS BOULDER ENGINEERING 717 15th Street Boulder, CO 80302 . 303.444.6038 TERIOR DESIGNER: BOHNSON NATHAN STROHE 600 Wynkoop St., Suite 100 berver, CO 80202 303.892.7062 DJECT LOCATION STEAMBOAT BASECAMP PARTIAL RENOVATION STEAMBOAT BASECAMP PARTIAL RENOVATION STEAMBOAT SPRINGS, CO 80487 AWING TITLE TYPICAL DETAILS - WOOD
	AL DATE: 1/1 Washington Ave. 0/20. 2010 0/20. 2010 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 2020 0/20. 203.444.6038 0 TERIOR DESIGNER: 0 COHNSON NATHAN STROHE 600 Wynkoop St., Suite 100 Denver, CO 80202 0/20. 2033.892.7062 0/20. DJECT LOCATION 0 STEAMBOAT BASECAMP PARTIAL RENOVATION AND TENANT FIT-OUT 1901 CURVE PLAZA STEAMBOAT SPRINGS, CO 80487 WING TITLE TYPICAL DETAILS - WOODD Author CHECKED E Checker PROJECT N 21304
	CL&A ENGINEERS & BUILDERS 2017<

4

S0201

LOAD KEY LEGEND]	
LOAD AREA PATTERN		SELF	SUPERIMPOSED LOADS						لرف [
	PATTERN	WEIGHT (psf)	DEAD LOAD (psf)	LIVE LOAD (psf)	SNOW LOAD (psf)	VERT WIND LOAD (psf)	DESCRIPTION OF LOAD	NOTES	***
		10	10	20	75 + DRIFT		(E) SLOPED ROOF	NOTES 1, 2, 3	1
2		10	10	20	75 + DRIFT	+5/-10	(E) FLAT ROOF	NOTES 1, 2, 3]

<u>NOTES:</u>

ROOF LIVE AND SNOW LOADS DO NOT ACT CONCURRENTLY. VERTICAL WIND LOADS ARE MWFRS LOADS AND HAVE NOT BEEN REDUCED BY

DEAD LOADS. NEGATIVE LOADS ARE UPLIFT. RE: STRUCTURAL DESIGN CRITERIA

3. NEGATIVE LOADS ARE UPLIFT AND HAVE NOT BEEN REDUCED BY DEAD LOADS.

						_
		CON	ITINUO	US FOOTI	NG	SCI
MARK	WIDTH	"В"	THICK			
FC42	3'-6	*		(4)# #5@	¥6 ()16"(
		IS	OLATEL) FOOTIN	G SC	CHE
MARK	LENGTH	WIE	DTH	THICKNE	SS	
F1	6'-6"	6'-	-6"	1'-6"		

COPYRIGHT 2019

RUCTURAL DIRECTION

© 2022

KL&A, INC

COPYRIGHT 2019

ENGINEERS SEAL ON THIS DRAWING INDICATES THAT THE INFORMATION SHOWN AND THE CALCULATIONS PERTAINING TO THAT INFORMATION HAVE BEEN PREPARED BY QU OF THE ENGINEER-OF-RECORD. THE SEAL DOES NOT IMPLY RESPONSIBILITY FOR INFORMATION PREPARED BY OTHERS NOR FOR ANY INFORMATION NOT SHOWN ON THIS SPECIFICALLY DISCLAIMED. ON PHASED PROJECTS, DRAWINGS THAT ARE ISSUED BUT NOT SEALED SHALL BE CONSIDERED TO BE PRELIMINARY IN NATURE AND ARE ISSU UCTURAL RECTION IBILITY IS

					SHEA	AR WALL SCH	IEDULE	
SHEAR WALL TYPE AND CAPACITY			SILL PLATE AT CONCRETE					
	A	В	С	D	Ε	SITE	ATTACHMENT	, F
	WALL PANEL EDGE	SOLE PLATE	FLOOR TRUSS BLOCKING	ROOF TRUSS	IN FIELD (AWAY FROM EDGES)	(SEE NOTE 4)	(SEE NOTE 13)	VER
3 490 PLF	0.131 "øx3" @3"0C	0.162Ӣ x3 1/2"@3"0C	SIMPSON A35 @8"OC <u>OR</u> SIMPSON LTP4 @8"OC (1)CLIP MIN PER BAY OF BLOCKING	SIMPSON A35 @8"OC <u>QR</u> SIMPSON LTP4 @8"OC	0.131"øx3" @12"0C	(1)2x PT SILL	5/8"ø@16"0C	(2) AN
<u>NOTES:</u> 1. ALL SHE 2. ALL FRA 3. ALL SHE ACROSS 4. FOR SIN 5. PROVIDE AT 2x4 6. RF ⁻ "TYP	AR WALL STUD F MING IS DOUGLAS AR WALLS TO BE STUDS AND SPA IGLE 2x SILL PLA SLOTTED WASHE WALLS AND BPSS PICAL REQUIREME	RAMING @16"OC L 5 FIR-LARCH MAT WOOD SHEATHED CING OF NAILING TE, COUNTERSINK R PLATE AND STA 5/8-6 AT 2x6 WA	INLESS TIGHTER S ERIAL. WITH 32/16 SPA 'E' (NAILING IN F ING ANCHOR BOL NDDARD WASHER A LLS OR EQUIVALE AND NOTCHES IN	SPACING NOTED C AN RATED PLYWOO IELD) IS DECREAS T WASHER AND N AT ANCHOR BOLT NT. WOOD MEMBERS"	DN PLAN. DD OR OSB (15/3 SED TO 6"O.C. UT IS NOT ALLOW. CONNECTIONS. SI COR REINFORCIN	2" MINIMUM THIC ED. FOR 3x SILL LOTTED PLATE TO	CKNESS). 7/16" T PLATE, 1" MAX C BE NO FURTHER	HICKN OUNTL THAN
0. KE: 111	TOAL REQUIREME	NIS FUR HULES /	HIND INVIGES IN	WOOD MEMDERS	FOR REINFURCIN	G OF WALL PLATE	S WIIT NUICHES	,