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PROJECT DESCRIPTION

- Project consists of a renovation to a single story existing building. Foundations consist of spread footings. Superstructure is wood framed floors. Existing lateral system is wood shear walls. New scope of work includes steel moment frame and associated modified foundations.
- This description is for general orientation only. The General Contractor is responsible for all scope items described in the drawings and project specifications as well as for all material and labor that can reasonably be inferred there from.

GENERAL APPLICATION

- These drawings must be used in conjunction with the architectural drawings on the project to clearly define all requirements for construction.
- No Contractor should attempt to bid nor construct any portion of this project without consulting the project architectural, mechanical, and electrical documents.
- All things which, in the opinion of the Contractor, appear to be deficiencies, omissions, contradictions, or ambiguities in the drawings shall be brought to the attention of the Structural Engineer. Corrections or written interpretations shall be issued before affected work may proceed.
- The Contractor shall inform the Structural Engineer, clearly and explicitly in writing of any deviation or substitution from requirements of the contract documents. Contractor shall not be relieved of any requirement of the contract documents by virtue of the Structural Engineer's review of shop drawings, project data, etc., unless the Contractor has clearly and explicitly informed the Structural Engineer in writing of any deviations or substitutions at time of submission.

EXISTING CONSTRUCTION

- Information regarding existing structural systems is based on drawings prepared by Lykekn and Kramer dated 06/26/1998.
- Existing Conditions:
 - The current design is based on the best information available, with the understanding that not all conditions have been observed and unobserved conditions may not be fully understood.
 - All existing information, dimensions, elevations, etc. shall be considered approximate and exiting conditions shall be field verified by the Contractor prior to ordering or fabricating material. Structural Engineer and Architect shall be informed in writing of any discrepancies.
- Remodeling, retrofit, renovation, or rehabilitation of an existing building requires that certain assumptions be made regarding existing conditions. The General Contractor must immediately notify the structural engineer if any existing condition deviates from those indicated in the Contract Documents.
- Demolition and Shoring:
 - The General Contractor is responsible for shoring of existing structure where required during demolition and new construction.
 - The current interior walls and finishes may be significant to the stability of the structure and may be providing inadvertent load paths that are not observable and may impact how demolition impacts the structure.
 - The General Contractor should be prepared to brace and shore framing during demolition.
 - Demolition and reshoring are best accomplished prior to significant snowfall, if possible. If demolition takes place during the significant snowfall season, the Contractor's shoring design should account for this.
- No openings, nor any changes or additions, shall be made in any existing structural elements without written approval of the Structural Engineer. Where the function of an existing element as structural or non-structural is unclear, the determination of its function will be made solely by the Structural Engineer.

MISCELLANEOUS NOTES

- The Contractor is solely responsible for all safety regulations, programs, and precautions related to all work on this project.
- The Contractor is solely responsible for the protection of persons and property either on or adjacent to the project and shall protect it against injury, damage, or loss.
- Means and methods of construction and erection of structural materials are solely the Contractor's responsibility.
- The structure is designed to function as a unit upon completion of construction of the project and then, only to support the design loads indicated. The Contractor is responsible for means, methods and sequence of construction, and the adequacy of the structure to support loads occurring during construction of the project. Furnish all temporary bracing, shoring, and/or support as may be required.
- No openings, nor any change in size, dimension or location shall be made in any structural element without written approval of the Structural Engineer.
- Where dimensions or weights of MEP equipment or systems are variable from manufacturer to manufacturer, verify dimensions and weights shown on drawings with selected manufacturer prior to ordering materials. Notify Structural Engineer of discrepancies.
- Do not place equipment when shipping or operating weight exceeds weight indicated on structural drawings.
- Openings 1'-4" or less on a side are generally not shown on the structural drawings. Refer to drawings of other consultants for such openings.
- Openings through floors and/or roofs for passage of utilities are not located nor dimensioned on structural drawings. Contractor shall obtain and coordinate such locations and dimensions with the contractor requiring the opening.
- Show all openings through structural members on shop drawings and submit for review. Openings not shown on structural drawings are subject to acceptance and shall be specifically indicated for review and acceptance.
- Verify elevator and escalator pit dimensions, locations, loadings, and details with the elevator/escalator supplier prior to the fabrication and/or installation of any material.

MISCELLANEOUS NOTES (continued)

- Fireproofing of structural elements is not shown on the structural drawings. Refer to the specifications and architectural documents.
- Do not scale these drawings, use the dimensions shown. In case of conflict, request clarification from Architect and Structural Engineer.
- No structural modifications, alterations, or repairs shall be made without prior review by Structural Engineer. Submit details and calculations prepared by a professional engineer registered in state where project is located and employed by Contractor.
- Where structure is to be used for staging or temporary storage area, the Contractor shall verify that unit loads do not exceed the design loads for the structure.

QUALITY ASSURANCE AND QUALITY CONTROL

- The Contractor is responsible for assuring quality, including workmanship and materials furnished by subcontractors and suppliers.
- Inspection or testing by the Owner does not relieve the Contractor of the responsibility to perform the work in accordance with the Contract Documents.
- Workmanship: The Contractor is responsible and shall bear the cost of correcting work which does not conform to the specified requirements.
- Correct deficient work by means acceptable to the Architect. The cost of extra work incurred by the Architect to approve corrective work shall be borne by the Contractor.
- The Owner's Testing Agency shall perform testing and special inspections required by the structural documents, building code, and the local authority. The Testing Agency shall comply with ASTM E329 and upon completion of work, the Testing Agency shall furnish a certificate of compliance, signed by the professional engineer overseeing special inspections and testing. The professional engineer must be registered and licensed in the state where the project is located.
- The individual employed by the Testing Agency, responsible for overseeing testing and inspection of soils and foundations shall be a professional engineer practicing the discipline of geotechnical engineering, referred to as the Geotechnical Engineer in the structural portion of the construction documents. The Geotechnical Engineer is responsible for testing and inspections of soils, earthwork, and foundations for conformance to the foundation design and the geotechnical report. See foundation section of the General Notes.
- See special inspections section of the General Notes for required testing and inspection.

SPECIAL INSPECTION

- Special inspection and testing shall be performed as required by the local jurisdiction, the building code, and the construction documents. See quality assurance section of the General Notes.
- Coordinate and schedule inspection and testing prior to the start of work requiring inspection and testing while providing special inspector reasonable notice.
- All deficiencies shall be corrected for acceptance by the Testing Agency.
- Inspections performed by the local jurisdiction do not replace inspection or testing required by the Owner's Testing Agency.
- Special inspection and testing is required for the items shown in the "Special Inspections and Testing" Table.

SPECIFICATIONS

These General Notes are intended to function as the structural portion of project specifications.

SUBMITTALS

- See Material sections of these General Notes for required shop drawings.
- Submit one (1) copy of the required information (Manufacturers Data, Shop Drawings, etc) via electronic media (PDF or similar).
- Reproducible copies of contract documents shall not be used.
- Submittals shall be sent directly to the Architect for review and distribution.
- Submittals shall be reviewed by Contractor and Subcontractor prior to submission. Drawings shall bear Contractor's approval stamp accepting responsibility for coordination of dimensions shown in the contract documents, quantities, and coordination with other trades.
- Allow 14 calendar days in the Structural Engineer's office for review of submittals.
- Submittals will be returned to the Architect with Structural Engineer's review comments via electronic media.

STRUCTURAL DESIGN CRITERIA			
Building Code:	2018 International Building Code with Routt County Amendments		(Note 1)
	2018 International Existing Building Code		
Local Jurisdiction:	ROUTE COUNTY BUILDING DEPARTMENT		
Risk Category:	II		
IEBC Alteration Level:	LEVEL 3		
Historic Designation:	No		
Wind Loading			
Basic Wind Speed	Vult= 115 MPH	Vasd= 89.1 MPH	
Exposure Category	C		
GCpi	+/- 0.18		
Ground Elevation Factor, Ke	0.7852		
Elevation used for Ke (feet above sea level)	6,680ft		
Ultimate Wind Base Shear			
East/West	22 kips		
North/South	48 kips		
Ultimate Wind Design Pressure Components & Cladding, PSF	20ft²	50ft²	100ft²
Interior Roof Zone (Zone 1)	16/-47.7	16/-29.0	16/-16.0
Roof End Zone (Zone 2)	16/-60.1	16/-47.7	16/-38.3
Corner Roof Zone (Zone 3)	16/-70.8	16/-55.2	16/-43
Interior Wall Zone (Zone 4)	24.6/-26.8	23.1/-25.3	21.9/-24.1
Wall end Zone (Zone 5)	24.6/-32.3	23.1/-29.2	21.9/-26.8
Parapet	76.8/-43.0	61.7/-38.9	50.3/-35.9
Seismic Loading		(Note 6)	
Seismic Importance Factor, Ie	1.0		
Mapped Spectral Response Acceleration			
Ss	0.325		
S1	0.083		
Site Class	C		
Spectral Response Coefficients			
Sds	0.334		
Sd1	0.144		
Seismic Design Category		C	
Basic Seismic Force Resisting System	E/W Direction: Wood shear walls	N/S Direction: Steel not detailed for seismic	
Response Modification Factor, R	6.5	3	
Over-Strength Factor, Ωo	2.5	3	
Deflection Amplification Factor, Cd	4	3	
Seismic Response Coefficient, Cs	0.051	0.111	
Analysis Procedure Used	Equivalent Lateral Force Procedure		
Ultimate Seismic Base Shear	15.4	15.4	
Snow Loading		(Notes 2,3,5)	
Ground Snow Load, Pg	105 psf		
Importance Factor, Is	1.0		
Terrain Category	C		
Exposure Factor, Ce	1.0		
Thermal Factor, Ct	1.0		
Slope Factor, Cs	1.0		
Live Loads and Superimposed Dead Loads		(Notes 4,5)	
Foundations			
Geotechnical Engineer Information:	Timothy S. Travis PE# 25750		
	Northwest Colorado Consultants,...		
	2580 Copper Ridge Drive		
	Steamboat Springs, CO 80487		
	(970) 879-7888		
	Geotech Project #20-11961		
	Date of Report: 03/15/2021		
Active Equivalent Fluid Pressure	45 psf/ft		
At-Rest Equivalent Fluid Pressure	55 psf/ft		
Passive Equivalent Fluid Pressure	250 psf/ft		
Sliding Friction Coefficient	0.4		
Allowable Bearing Capacity	3,000 psf		
Minimum Frost Depth	48 in		
Foundation Schedule Located on:	S0501		
Referenced Datum	100'-0" = First Floor Elevation = 6668.60 USGS		
NOTES:			
1. The governing building code defines the applicable edition of referenced codes and standards. Where governing building code does not define referenced codes and standards, the latest edition shall be used.			
2. Ground snow load is according to the Routt County Snow Load Mapper provied by the Routt County Building Department on 07/07/2021.			
3. All snow loads on the structure for both flat and sloped roofs are calculated in accordance with the 2018 IBC and based on the ground snow load stated above. Roof snow loads consider the following load conditions: partial loading and snow drifting.			
4. Minimum uniform and concentrated live loads as well as partition loads and applicable live load reductions are determined according to Section 1607 of the IBC.			
5. See Load Keys for numerical definition and area designation of snow, live, and other gravity loads used in design.			
6. Seismic Design Parameters (Sds and Sd1) that were used in the design were provided by Routt County Building Department.			

PLAN KEYS AND SYMBOLS	
	ELEVATION VIEW 1 = DRAFTING NUMBER S101 = SHEET NUMBER
	WALL / BUILDING SECTION 1 = DRAFTING NUMBER S101 = SHEET NUMBER
	SECTION CUT 1 = DRAFTING NUMBER S101 = SHEET NUMBER
	DETAIL CALL OUT 1 = DRAFTING NUMBER S101 = SHEET NUMBER
	ADDENDUM NUMBER
	STEPS AND SLOPES IN DECKS & SLABS YY = STEP HEIGHT
	ELEVATION CALLOUT REFERENCE = T/OBJECT OR B/OBJECT XXX-YY = OBJECT ELEVATION
	COLUMN TYPE CX = COLUMN TYPE, RE: SCHEDULE BPX = BASE PLATE, RE: SCHEDULE
	SPlice in BEAM/GIRDER, RE: TYPICAL DETAILS
	MOMENT CONNECTIONS THROUGH BEAM
	MOMENT CONNECTIONS TO COLUMN
	WOOD JOIST HANGER HI = JOIST HANGER TYPE, RE: SCHEDULE
	KEYED NOTE K = KEYED NOTE, RE: SCHEDULE

WALLS AND COLUMNS	
	CIP CONCRETE WALL ABOVE
	WOOD FRAMED WALL ABOVE
	COLD-FORMED STEEL WALL ABOVE
	WALL BELOW (ALL MATERIALS)
	WALL ABOVE W/ WINDOW *
	WALL ABOVE W/ DOOR *
	CIP CONCRETE COLUMN ABOVE
	STEEL COLUMN ABOVE
	WOOD COLUMN ABOVE
	COLUMNS BELOW (ALL MATERIALS)
*CIP CONCRETE SHOWN; GRAPHICS APPLY TO OTHER WALL MATERIALS	

SECTIONS AND DETAILS	
	CIP CONCRETE
	SOIL FILL
	UNDISTURBED SOIL

STEEL FRAMING	
	BEAM / GIRDER NOTATION RL = REACTION LEFT (K) RR = REACTION RIGHT (K) SIZE = MEMBER TYPE (XX) = # OF STUDS Y* = CAMBER SIZE XX-YY* = 1/STL ELEVATION RE: TYPICAL DETAILS FOR CONNECTION DETAILS/CAPACITIES

WOOD FRAMING	
	CUSTOM TRUSS
	BEAM / GIRDER
	JOIST / RAFTER
	PREMANUFACTURED FLOOR/ ROOF TRUSS

WOOD SHEAR WALL PLAN KEY	
	SHEAR WALL ABOVE FLOOR/ROOF
	SHEAR WALL BELOW FLOOR/ROOF
	SHEAR WALL ABOVE & BELOW FLOOR/ROOF
	WOOD SHEAR WALL DESIGNATION AND MINIMUM REQUIRED LENGTH RE: SHEAR WALL SCHEDULE
	HOLD DOWN FOR WOOD SHEAR WALL, RE: HOLD DOWN SCHEDULE

FOUNDATIONS	
	FOOTING NOTATION F? = FOOTING TYPE RE: SCHEDULE XX-XX* = 1/FTG ELEVATION
	FOOTING / FOUNDATION STEP

EXISTING CONSTRUCTION	
	DIMENSION, GC TO VERIFY IN FIELD
	WALL ABOVE / IN SECTION (ALL MATERIALS)
	WALL BELOW (ALL MATERIALS)
	COLUMNS ABOVE (ALL MATERIALS)
	COLUMNS BELOW (ALL MATERIALS)
	STEEL BEAM / GIRDER (E) = EXISTING SIZE = MEMBER TYPE
	STEEL TRUSS
	STEEL JOIST / WOOD TRUSS
	WOOD BEAM / GIRDER
	WOOD JOIST / RAFTER

ABBREVIATIONS	
AB	anchor bolt
ADDNL	additional
AFF	above finish floor
ALT	alternate
ARCH	architectural
B, B/	bottom of
BLDG	building
BM	beam
BOT	bottom
BRG	bearing
BS	both sides
BTWN	between
CFS	cold-formed steel
CIP	cast-in-place concrete
CJ	construction / control joint
CJP	complete joint penetration
CLR	clear
CLT	cross laminated timber
CMU	concrete masonry unit
COL	column
CONC	concrete
CONN	connection
CONST	construction
CONT	continuous
GLC	glulam column
D	depth
DIA, Ø	diameter
DIM	dimension
DK	deck
DTL	detail
DWGS	drawings
DWL	dowel
(E)	existing construction
EA	each
EF	each face
EJ	expansion joint
ELEV	elevation
EOx	edge of (S=slab, C=conc, etc)
EW	each way
EXP	expansion
EXT	exterior
FDN	foundation
FLR	floor
Fox	face of (S=slab, C=conc, etc)
FS	far side
FTG	footing
GA	gage
GB	grade beam
GC	general contractor
GEN	general
GLB	glulam beam
HGD	hot dip galvanize
HDR	header
HK	hook
HORIZ	horizontal
HP	high point
HR	hour
HSA	headed stud anchor
I/F	inside face
INT	interior
JST	joist
JT	joint
KIP, K	1000 pounds

SHEET LABEL KEY	
STRUCTURAL DRAWING SHEET	
SHEET NUMBER	
100 SERIES = GEN NOTES, LOAD KEYS AND TIP DETAILS	
200 SERIES = AREA PLANS	
300 SERIES = ELEVATIONS	
400 SERIES = DETAILS	
500 SERIES = SCHEDULES	

NEW SHEET = ○ REVISED DRAWING = ● NO MODIFICATIONS = ● SHEET DELETED = ✕		ISSUE DATE AND TITLE			
SHEET NO.	SHEET NAME	11/11/2022 - ISSUED FOR PERMIT			
S0101	GENERAL NOTES	○	○	○	○
S0102	GENERAL NOTES	○	○	○	○
S0103	GENERAL NOTES	○	○	○	○
S0104	TYPICAL DETAILS - CONCRETE	○	○	○	○
S0105	TYPICAL DETAILS - WOOD	○	○	○	○
S0106	TYPICAL DETAILS - WOOD	○	○	○	○
S0201	PARTIAL FOUNDATION, ROOF FRAMING, AND LOAD KEY PLANS	○	○	○	○
S0301	WALL AND FRAME ELEVATIONS & DETAILS	○	○	○	○
S0401	DETAILS	○	○	○	○
S0501	SCHEDULES	○	○	○	○

ABBREVIATIONS	
KLF	1000 pounds per lineal foot
L	length
LAT	lateral
LBS	pounds
LLH	long leg horizontal
LLV	long leg vertical
LONG	longitudinal
LP	low point
LSL	laminated strand lumber
LVL	laminated veneer lumber
LW, LWT	lightweight
MAS	masonry
MAX	maximum
MECH	mechanical
MFR	manufacturer
MIN	minimum
MTL	metal
(N)	new construction
No	Number
NOM	nominal
NS	near side
NW, NWT	normal weight
O/F	outside face
OC	on center
OH	opposite hand
OPNG	opening
PAF	powder actuated fastener
PC	precast concrete
PERP	perpendicular
PERT	pre-engineered roof truss
PJP	partial joint penetration
PL	plate
PLF	pounds per lineal foot
PSL	parallel strand lumber
PT	post-tensioning, post-tensioned
PT	pressure treated
RE:	reference
REINF	reinforcement
REQD	required
RET	retaining
SC	slip critical
SCHEd	schedule
SCL	structural composite lumber
SIP	structural insulated panel
SOG	slab on grade
SPA	spacing
STFNR	stiffener
STL	steel
T, T/	top of
THK	thickness
TL	transfer load
TRAN	transverse
TYP	typical
UNO	unless noted otherwise
VERT	vertical
VIF	verify in field
W	width
WF	wide flange
WP	work point
WWF	welded wire fabric

APPROVAL STAMPS:

REVIEWED
FOR CODE
COMPLIANCE
02/24/2023

1 11/11/22 Issued for Permit
No. Date Description

SUBMISSIONS & REVISIONS

OWNER

MAY RIEGLER PROPERTIES
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ARCHITECT

K A S A
KEVIN & ASAKO SPERRY ARCHITECTURE
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1. Holes are assumed to be dry unless otherwise noted on plans.
2. Holes to be hammer drilled with bit as specified by anchor manufacturer.
3. Anchors specified are based on the specific technical data published by the specified anchor manufacturer. Substitutions are not permitted without approval by the Structural Engineer of Record prior to use. Contractor shall provide calculations demonstrating that the substituted product is capable of achieving the performance values of the specified product. Substitutions will be evaluated by their having an ICC ESR showing compliance with the relevant building code for seismic uses, load resistance, installation category, and availability of comprehensive installation instructions. Adhesive anchor evaluations consider creep, in-service temperature and installation temperature.
4. Install anchors per the manufacturer instructions, as included in the anchor packaging. Installation shall adhere to ICC ESR. Reference plans and details for anchors that are to be installed with reduced torque.
5. Concrete should be allowed to cure a minimum of 21 days prior to adhesive anchor installation.
6. Prior to installation of anchors all installation and inspection personnel shall be instructed on site by a representative of the anchor manufacturer on proper installation techniques and equipment.
7. Anchor capacity is dependent upon spacing between adjacent anchors and proximity of anchors to edge of concrete. Install anchors in accordance with spacing and edge clearances indicated on the drawings.
8. Installation of anchors shall not avoid damage existing reinforcing. Prior to drilling, care shall be taken to avoid damage by locating existing reinforcing and PT by use of GPR, X-Ray, or other means that avoids damage to the concrete and accurately predicts potential conflict of reinforcing.
9. Post-installed anchors to be stainless steel where exposed to exterior and/or corrosive environments unless the anchor is protected.
10. All installers of post-installed adhesive anchors horizontally, vertically or upwardly inclined in concrete to support sustained tension loads shall be certified by ACI/CRSI adhesive anchor installer certification program, or equivalent as required by the IBC. Submit certificates for record.
11. All post-installed anchors in concrete shall be suitable for use in cracked concrete applications.
12. When doweling continuously deformed rebar into concrete use Hilti RE-500v3 or an adhesive that has been approved under ACI 355.4 and ACI 318 for development and lap splices.
13. Unless noted otherwise on plans/details all adhesive anchors shall be Hilti HIT-HY200 V3 for concrete and Hilti HIT-HY 270 for block and brick. Unless noted otherwise on plans/details all expansion anchors shall be Hilti Kwik-Bolt T22. See note 3 for substitutions.

1. Handling, placing, constructing, and curing shall conform to ACI 301 including placement of concrete in wet weather, cold weather, and hot weather.
2. For concrete thickness of 4'-0" or greater or when the cement content exceeds 600lb/cyd, conform to Mass Concrete of ACI 301.
3. Curing compounds should not be used on surfaces that are to receive additional concrete, paint, tile, or other material requiring a positive bond unless the contractor has demonstrated that the membrane can be satisfactorily removed before subsequent application is made, or the membrane dissipates or can serve satisfactorily as the base for the later application.
4. All concrete work shall be poured in-place unless noted otherwise. Shotcrete placement method will only be permitted if approved by the structural engineer prior to permit submittal. All requests and submittals to place concrete by the shotcrete method shall conform to ACI 506.2 and shall include pre-construction testing procedures. Requests for shotcrete placement shall be limited to foundation walls with simple reinforcing. All pilasters or in-wall columns shall be formed and poured separately from shotcrete operations.

SPECIAL INSPECTIONS AND TESTING FOR SEISMIC RESISTANCE	
Category/Material	Component/Work
Wood Framing in Seismic Category C, D, E, or F	Continuous special inspection of field gluing operation of elements in the seismic force-resisting system. Periodic special inspection of nailing, bolting, anchoring, and other fastening elements of the seismic force-resisting system, including wood shear walls, wood diaphragms, dr...

NOTES:

- Special inspections for seismic resistance is required per IBC 1705.12

CONCRETE MIX DESIGN REQUIREMENTS							
Element	f'c (psi)	Cement Type	Max W/C	Max Agg	Air Content (Note 1,2)	Slump (Note 3)	Exposure Class
Footings	4000, NW	I/II	0.45	3/4"	5	-	F1 C0 S0 W0
Foundation Walls integral to pilasters & columns	4500, NW	I/II	0.45	3/4"	6	-	F2 C0 S0 W0
Interior Slab-on-Grade (SOG)	4000, NW	I/II	0.45	3/4"	-	-	F0 C0 C0 P0

TABLE FOOTNOTES:

- For any concrete exposed to freezing temperatures and moisture, the air content shall be the greater of 5%, minimum required by ACI 318, or of that shown in the table.
- Tolerance on air content as delivered shall be +/- 1.5%.
- Slump tolerances as follows (ACI 117):
Specified Slump not greater than 4"= +/- 1"
Specified Slump more than 4"= +/- 1 1/2"
Where Slump is specified as a range= No Tolerance...

GENERAL CONCRETE MIX NOTES:

- Strength (f'c) is the compressive strength at 28 days unless noted otherwise or...
- Concrete is normal weight concrete unless noted otherwise. Normal weight concrete (NW) shall have a dry density of 145 ± 5 pcf unless noted otherwise. Lightweight...
- Required minimum average splitting tensile strength = $6.7 \sqrt{f'c}$ (regardless of concrete).
- Mix designs shall be in accordance with ACI 301.
- Exposure Class indicates the severity of the anticipated exposure of concrete members for each exposure indicated below according to ACI 318/ACI 301:
Freeze Thaw Exposure noted thus: F0,F1,F2,F3
Water-Soluble Sulfate in Soil Exposure noted thus: S0,S1,S2,S3
Permeability Requirements noted thus: P0,P1/W0,W1
Corrosion Protection of Reinforcement noted thus: C0,C1,C2...
- Corrosion Protection of Reinforcement requirements (C0,C1,C2):
Maximum water-soluble chloride ion (Cl-) content in concrete, by % weight of cement:
Reinforced Concrete: C0 = 1.0 C1 = 0.3 C2 = 0.15
- Where concrete is exposed to F3 freeze thaw exposure, restrictions on maximum fly ash and/or other cementitious materials apply. Refer to Table 26.4.2.2(b) in ACI 318-14 for requirements.

CONCRETE MATERIALS DESIGNATION	
Material	Standard
Portland Cement	ASTM C150, Type I or Type II
Fly Ash	ASTM C618, Class C or F
Aggregate	ASTM C33
Water	Potable
Water Reducing Admixture	ASTM C494, Type A or Type D
High Range Water Reducing Admixture	ASTM C494, Type F or Type G
Accelerator Admixture	ASTM C494, Type C or Type E
Air Entraining Admixture	ASTM C260
Lightweight Aggregates	ASTM C330
Curing Compound	ASTM C309, Type I, Class A
Reinforcing Bars	ASTM A615-grade 60 (Specified Yield Strength = 60ksi)
Welded Reinforcing Bars	ASTM A706-grade 60 (Specified Yield Strength = 60ksi)
Welded Wire Fabric	ASTM A185
Vapor Retarder below SOG	ASTM E1745-Class A

NOTES:

1. Type III Portland cement may be used if acceptable to the Architect.

CONCRETE REINFORCING DOWEL EMBEDMENT LENGTHS			
Concrete Compression Strength	Tension Dowels		Compression Dowels
	Standard Hook	Other	
3000psi	22"db	Note 1	22"db
4000psi	19"db	Note 1	19"db
5000psi	17"db	Note 1	18"db

NOTES:

1. Refer to "Concrete Reinforcing Tension contact Lap Splice Lengths" table for tension dowels without standard hooks. Values for Class A tension splices are permitted to be used.
2. Embedment length shall not be less than 12 inches.
3. db is bar diameter.
4. Compression dowel embedment lengths are permitted only when dowel is noted in drawings as compression, otherwise use tension embedment length.
5. Extend dowels to far edge of member UNO.

CONCRETE REINFORCING TENSION CONTACT LAP SPICE LENGTHS							
				CONCRETE COMPRESSION STRENGTH (psi)			
				3,000	4,000	4,500	5,000
Class A	Case #1	Bars ≤ #6	Top	57 db	49 db	47 db	44 db
			Other	44 db	38 db	36 db	34 db
		Bars ≥ #7	Top	71 db	62 db	58 db	55 db
			Other	55 db	47 db	45 db	42 db
	Case #2	Bars ≤ #6	Top	85 db	74 db	70 db	66 db
			Other	66 db	57 db	54 db	51 db
		Bars ≥ #7	Top	107 db	92 db	87 db	83 db
			Other	82 db	71 db	67 db	64 db
Class B	Case #1	Bars ≤ #6	Top	74 db	64 db	60 db	57 db
			Other	57 db	49 db	47 db	44 db
		Bars ≥ #7	Top	93 db	80 db	76 db	72 db
			Other	71 db	62 db	58 db	55 db
	Case #2	Bars ≤ #6	Top	111 db	96 db	91 db	86 db
			Other	85 db	74 db	70 db	66 db
		Bars ≥ #7	Top	139 db	120 db	113 db	108 db
			Other	107 db	92 db	87 db	83 db

Where:	
db:	Diameter of reinforcing bar
Class A:	Class A tension splice
Class B:	Class B tension splice
Case #1:	Clear spacing greater than or equal to 2*db AND cover greater than or equal to db
Case #2:	Clear spacing less than 2*db or cover less than db
Top:	Where horizontal reinforcement is placed such that more than 12 inches of fresh concrete is cast below the development length or splice
Other:	Other condition not satisfying Top qualification (bottom horizontal reinforcing)

NOTES:

1. All tension splices shall be contact class B splices unless noted otherwise. Splice length shall not be less than 12 inches.
2. Splice lengths in table are for single bar splices with maximum yield strength of 60ksi non-epoxy reinforcing bars.
3. For epoxy coated bars, zinc and epoxy coated bars, or epoxy coated wires, multiply "bot" reinforcing splice length by 1.5 and multiply "Top" reinforcing splice lengths by 1.31.
4. For lightweight concrete multiply splice length by 1.33.
5. For reinforcing with a specified yield strength greater than 60 ksi, multiply splice length by (specified yield strength/60ksi).
6. For individual bars within a bundle, lap lengths shall be multiplied by 1.33 for four bar bundles and 1.20 for three bar bundles. Individual splices within a bundle shall not overlap. Entire bundle shall not be lap spliced.
7. Bars larger than #11 shall not be lap spliced. For bars larger than #11, mechanical splice shall be used. Mechanical splices shall have strength greater than or equal to 125% the yield strength of the reinforcing bar. Mechanical splices shall be staggered.
8. Where bar of different size are lap spliced in tension, the minimum splice length shall be the larger of the length of a Class B tension splice of the smaller bar or the length of a Class A tension splice of the larger bar.
9. Lap splices are not permitted where minimum clearance between reinforcing cannot be maintained.
10. Lap splice lengths shall not be less than the larger of 12 inches multiplied by all applicable multipliers or the table length multiplied by all applicable multipliers.

REQUIRED CONCRETE COVER FOR NON-FIRE-RATED ASSEMBLIES		
Assembly		Cover (in)
Concrete cast against & permanently exposed to earth		3
Concrete Exposed to Earth or Weather	#6-#18	2
	#5 and smaller	1 1/2
Concrete not Exposed...	Walls, slabs #11 and smaller	3/4

CONCRETE REINFORCING COMPRESSION CONTACT SPLICE LENGTHS	
Specified Yield Strength (f_y) $\leq$ 60ksi	Specified Yield Strength (f_y) $>$ 60ksi
30"db	(0.9* f_y - 24)db

NOTES:

1. Splice Length shall not be less than 12 inches.
2. db is bar diameter.
3. Compression splices permitted only when lap splice is noted in drawings as compression, otherwise see tension splice table.

APPROVAL STAMPS:

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[illegible]

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INTERIOR DESIGNER:

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PROJECT LOCATION

PROJECT LOCATION
STEAMBOAT BASECAM

PARTIAL RENOVATION AND TENANT FIT-OUT

1901 CURVE PLAZA
STEAMBOAT SPRINGS, CO 80487

GENERAL NOTES

SEAL	DATE: 08/20/13
	DRAWN BY: Author
	CHECKED BY: Checker
	PROJECT NO: 21304

DRAWING NO:

S0102

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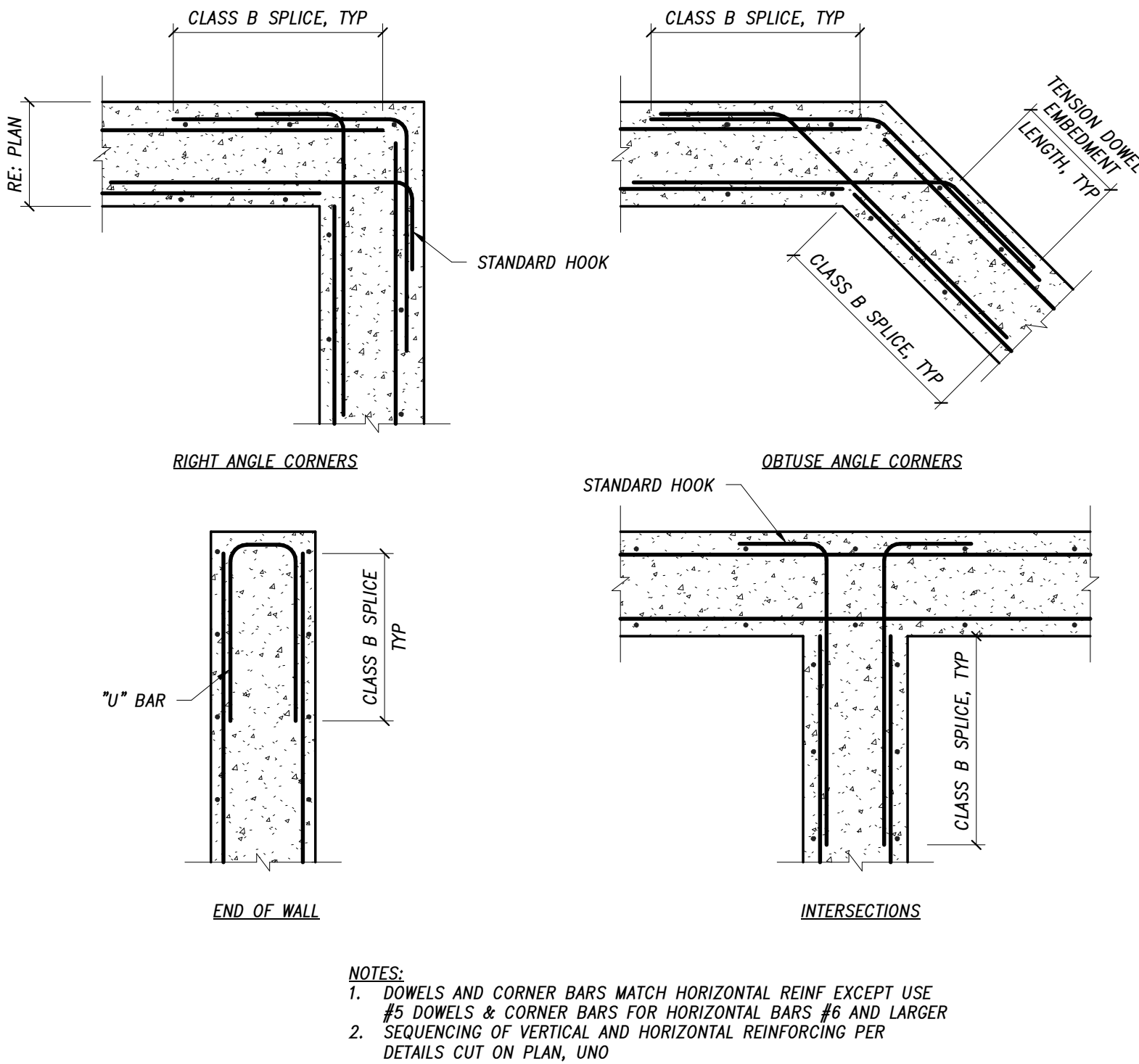
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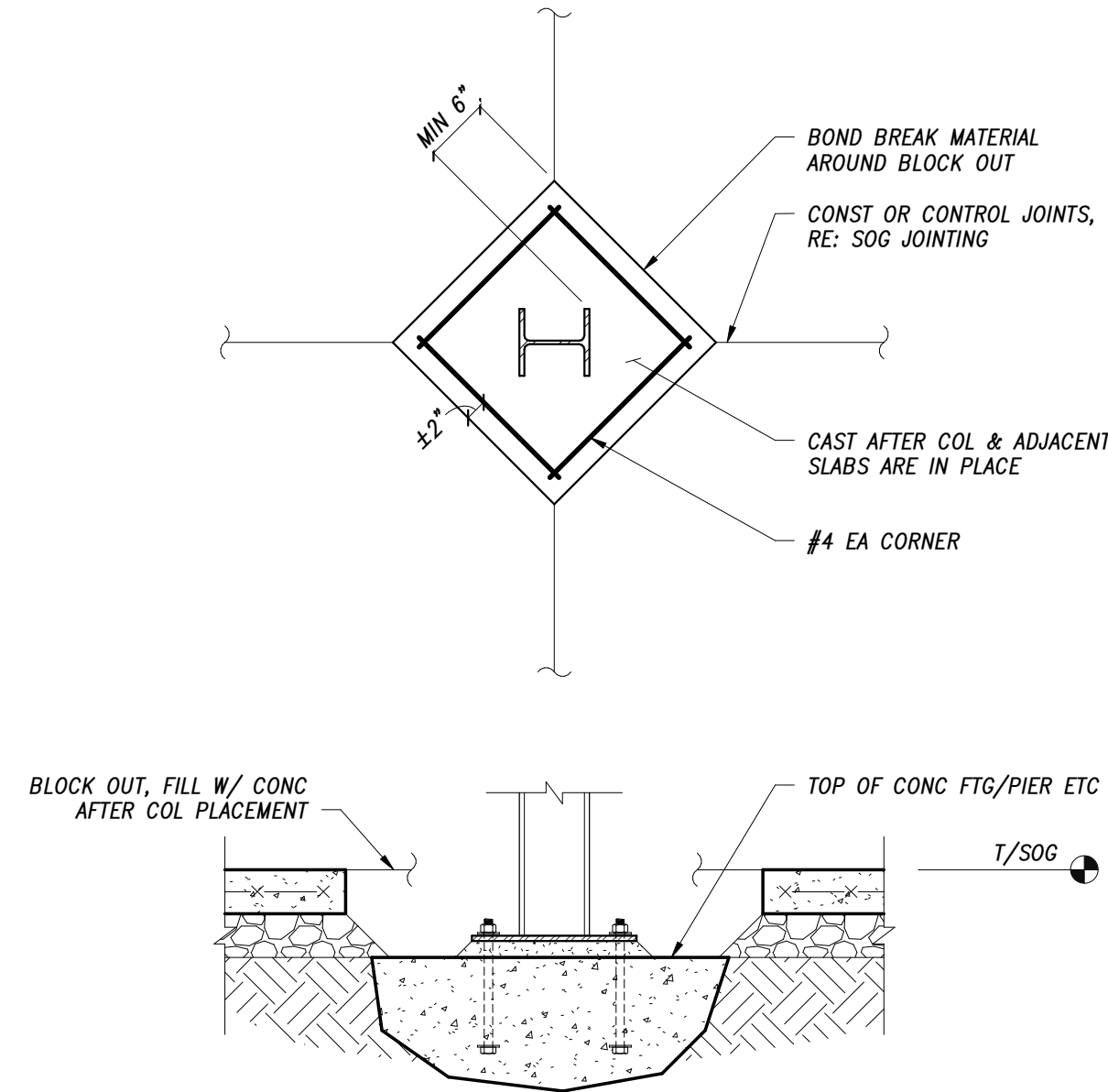
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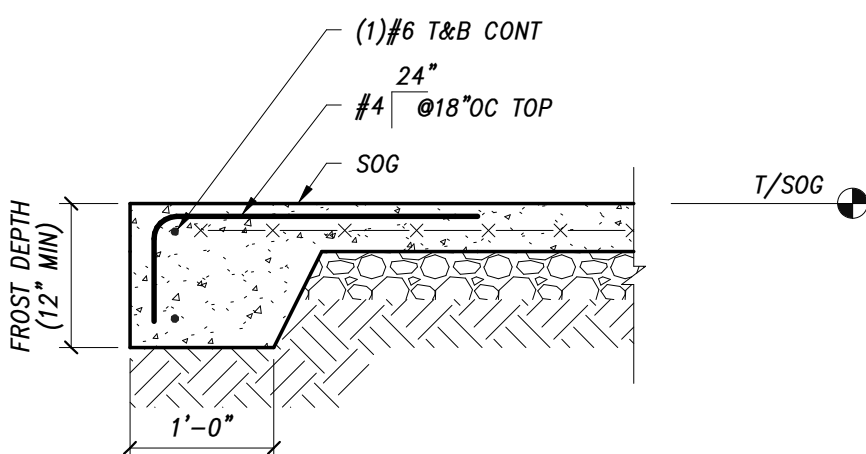
- TYPICAL DETAIL SHEET NOTES**
1. It is the contractor's responsibility to understand the typical details on this sheet and apply them as needed on the project.
 2. Typical details on this sheet are generally not referenced from any other drawing on the project.
 3. Typical details on this sheet MAY be referenced on plans or details to clarify or identify a particular condition. The presence of such a reference does not alter the obligation of the contractor to apply the detail(s) as needed even if they are not referenced.



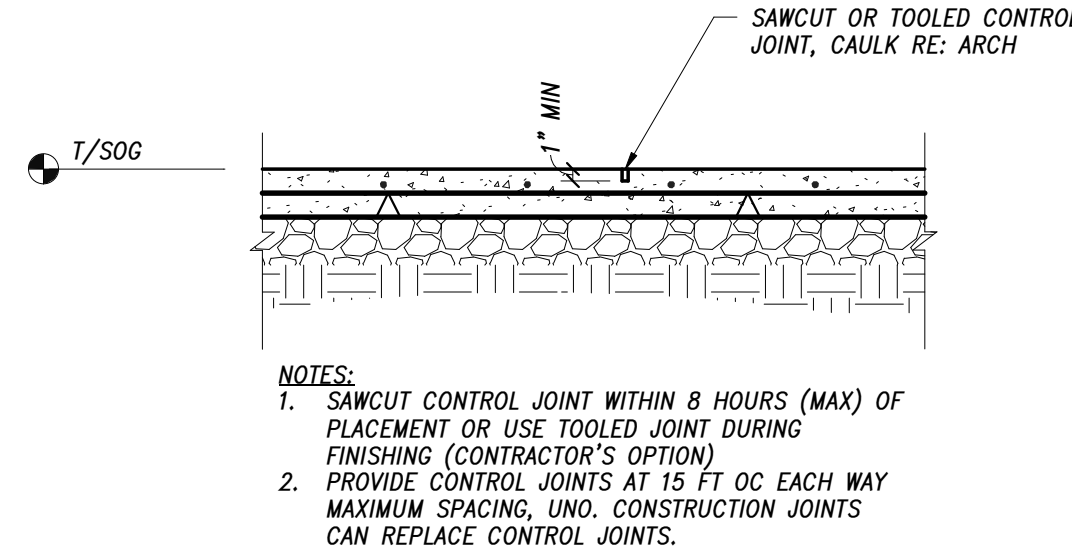
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S0104
TYPICAL WALL AND GRADE BEAM CORNER REINFORCING
DOUBLE MAT OF REINFORCING
NTS



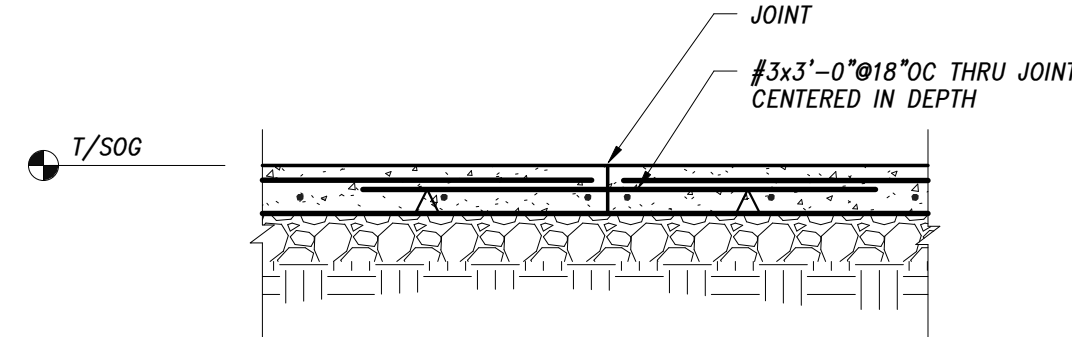
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S0104
TYPICAL SLAB ON GRADE BLOCKOUT AT STEEL COLUMNS
NTS



4
S0104
SLAB ON GRADE TURN DOWN
NTS

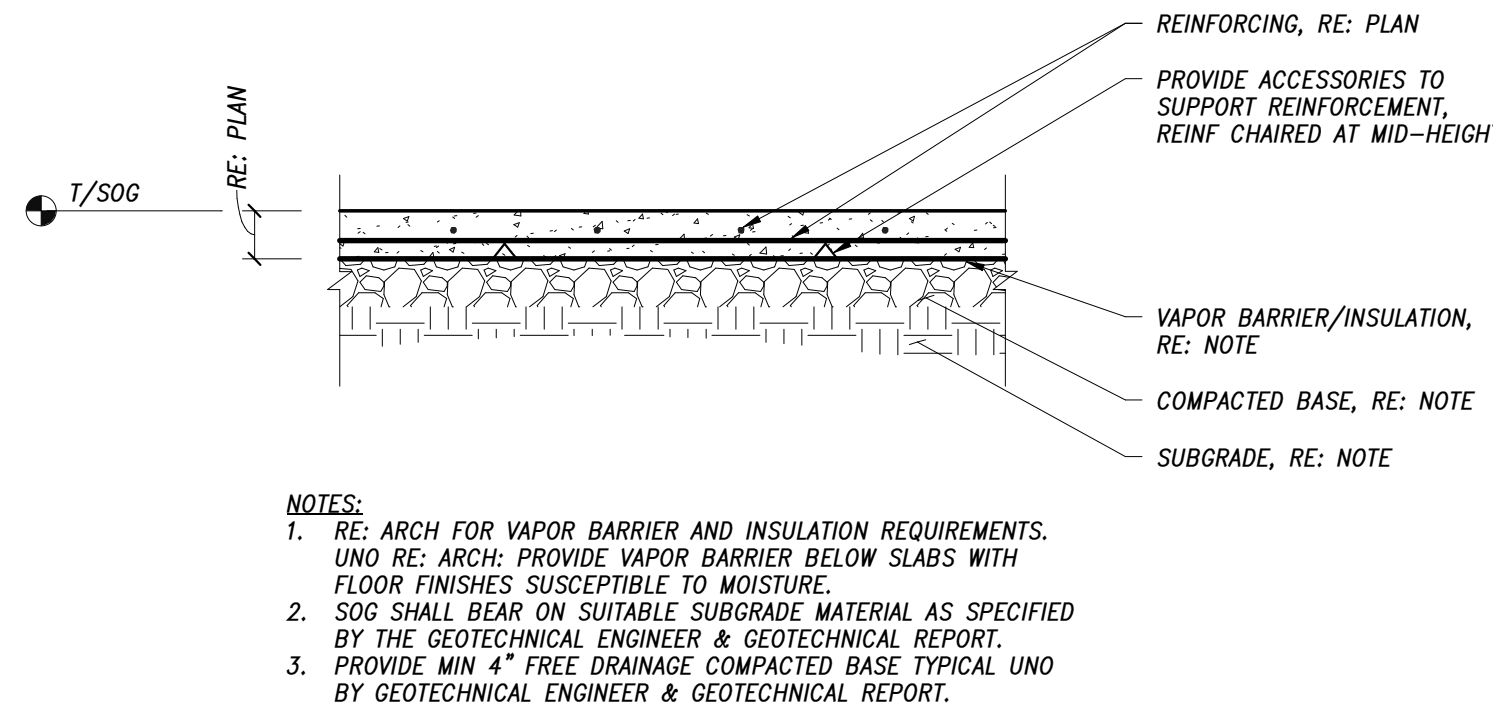


TYPICAL SOG CONTROL JOINT

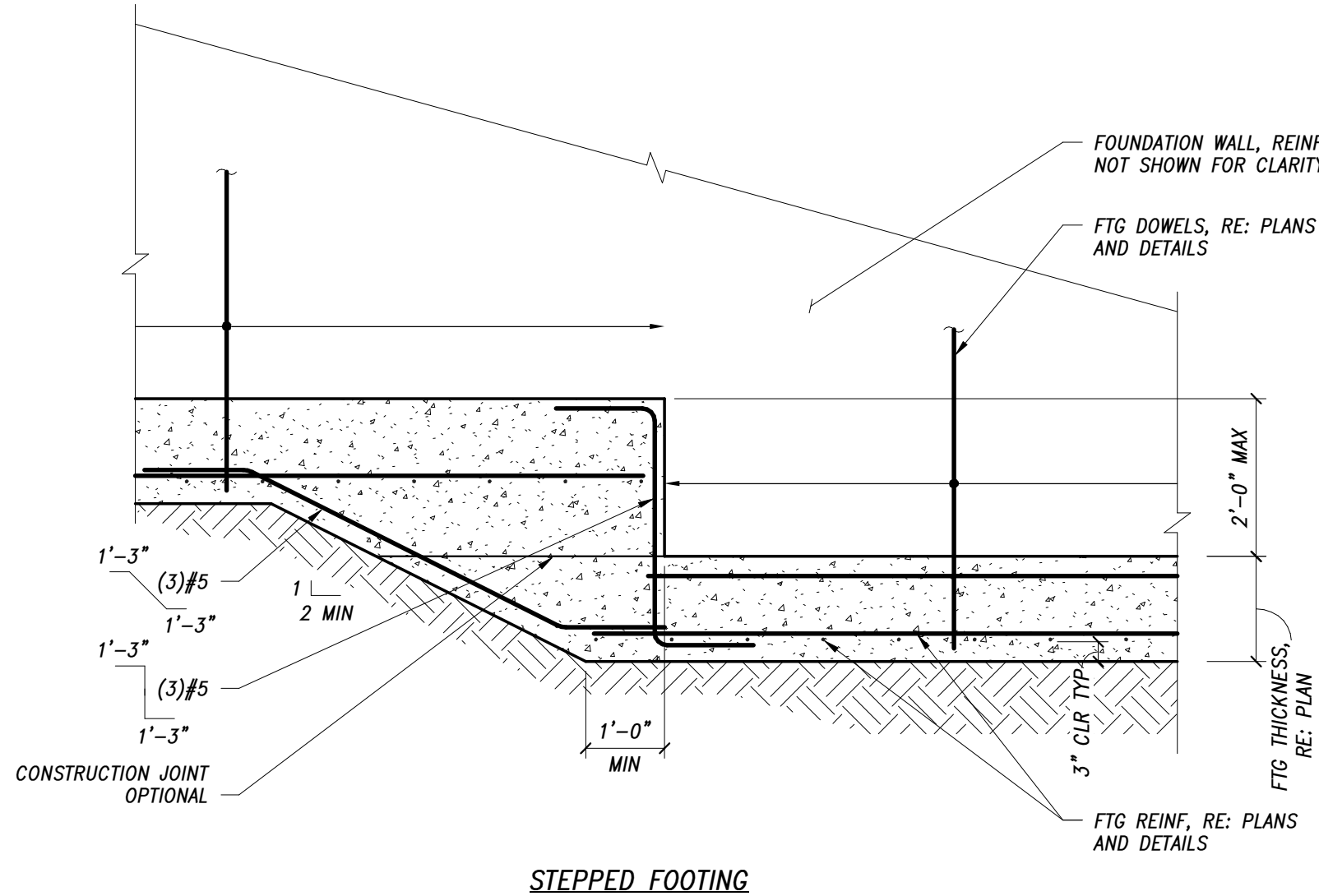


TYPICAL SOG CONSTRUCTION JOINT

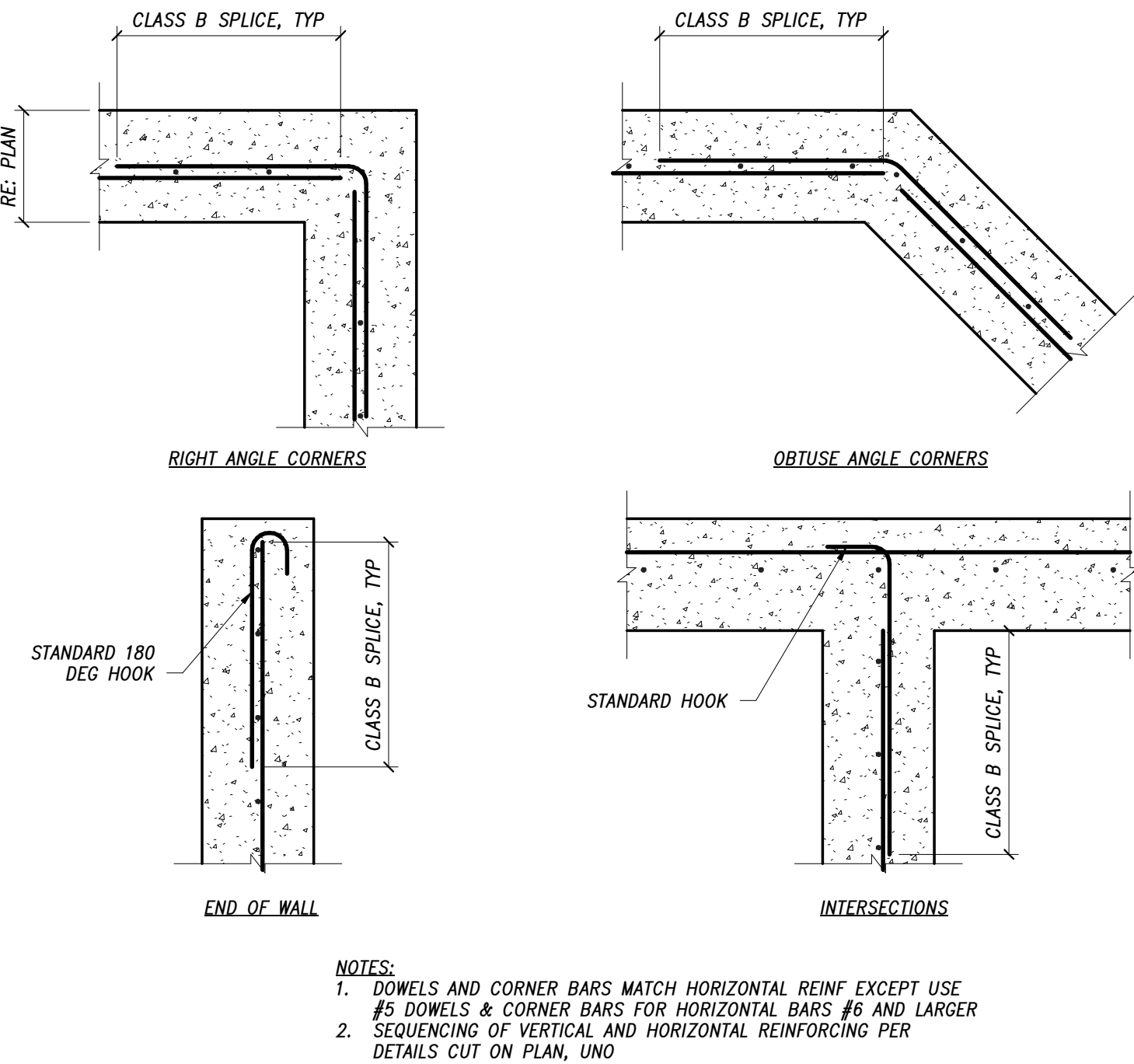
3
S0104
TYPICAL SLAB ON GRADE JOINTING
NTS



2
S0104
TYPICAL SLAB ON GRADE
NTS



1
S0104
TYPICAL FOOTING STEP DETAILS
NTS



7
S0104
TYPICAL WALL AND GRADE BEAM CORNER REINFORCING
SINGLE MAT OF REINFORCING
NTS

APPROVAL STAMPS:

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**STEAMBOAT BASECAMP
PARTIAL RENOVATION
AND TENANT FIT-OUT**
1901 CURVE PLAZA
STEAMBOAT SPRINGS, CO 80487
DRAWING TITLE

**TYPICAL DETAILS -
CONCRETE**

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PROJECT NO: 21304

DRAWING NO:
S0104

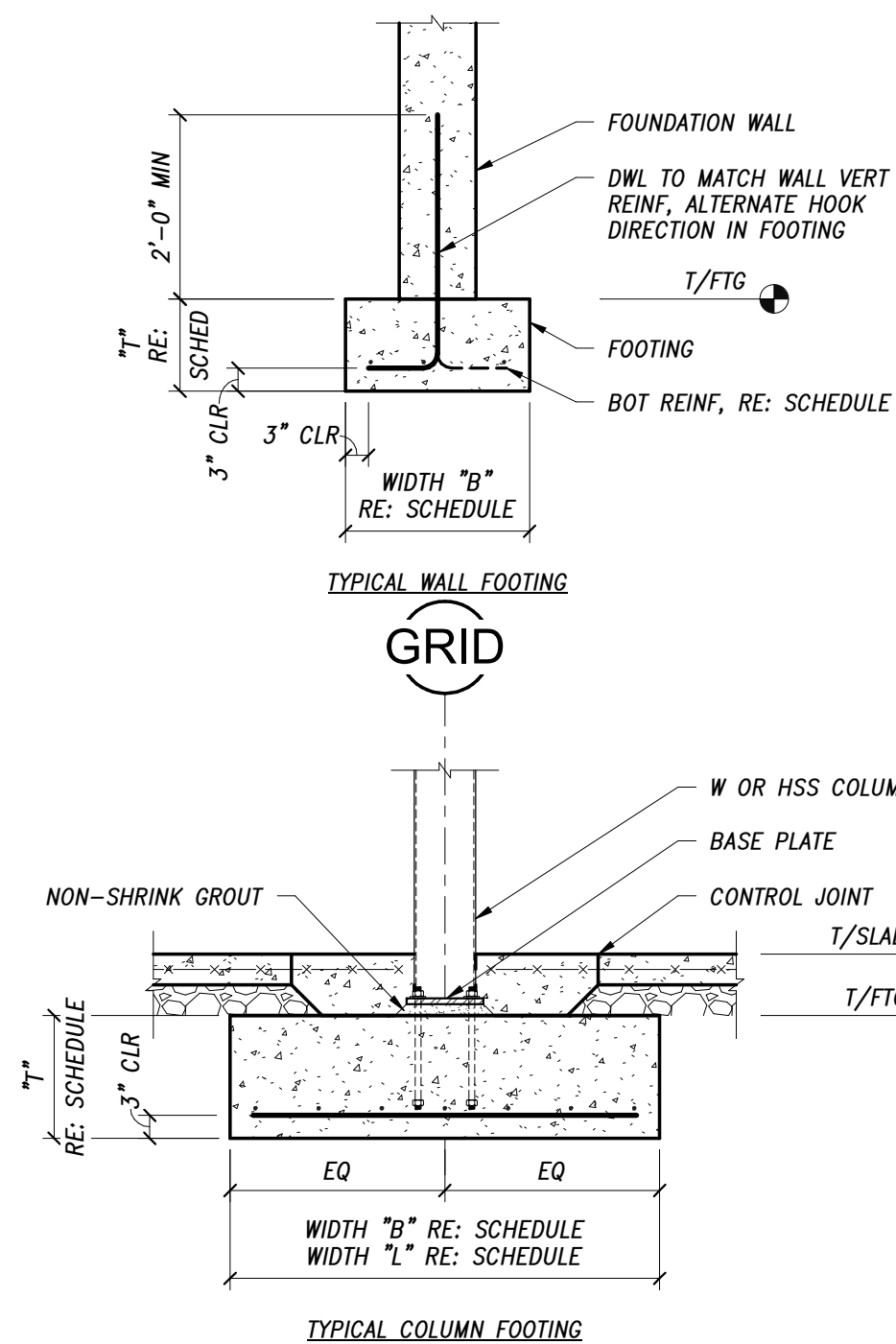
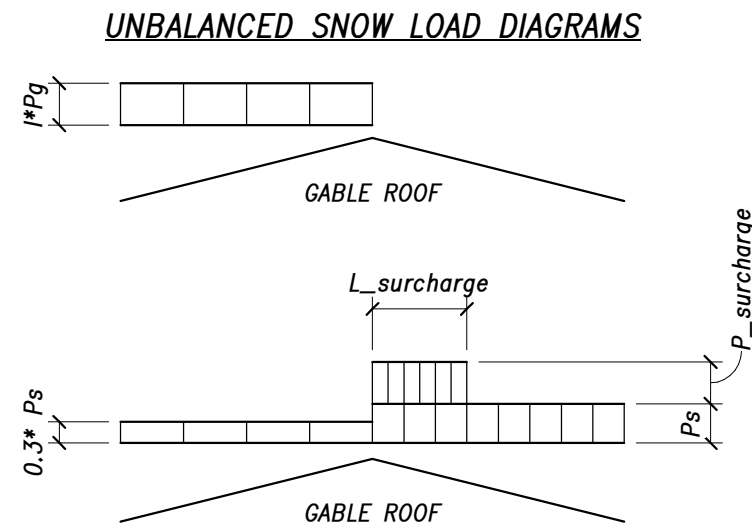
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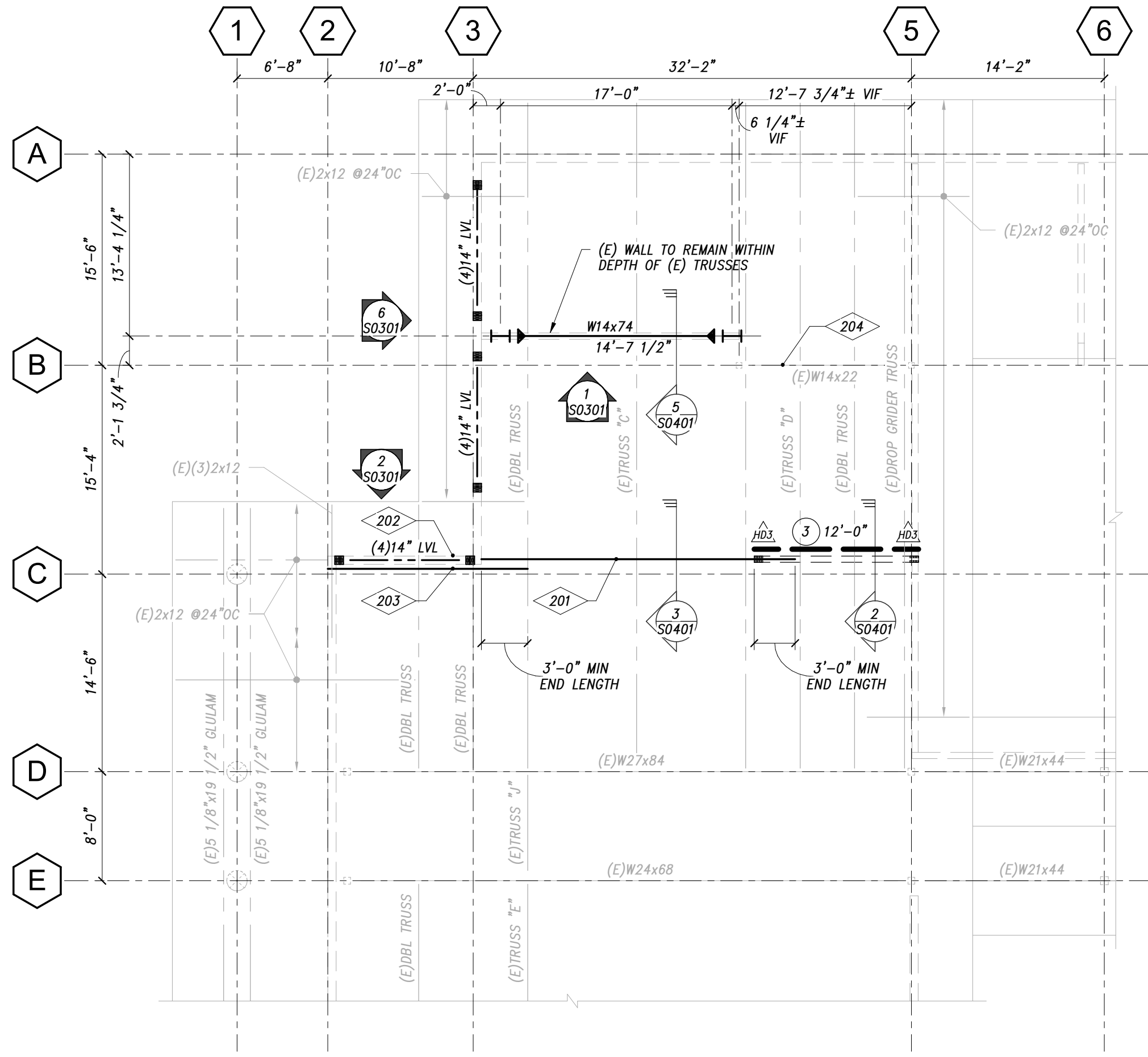
S0201 $1/8'' = 1'-0''$

NOTES:

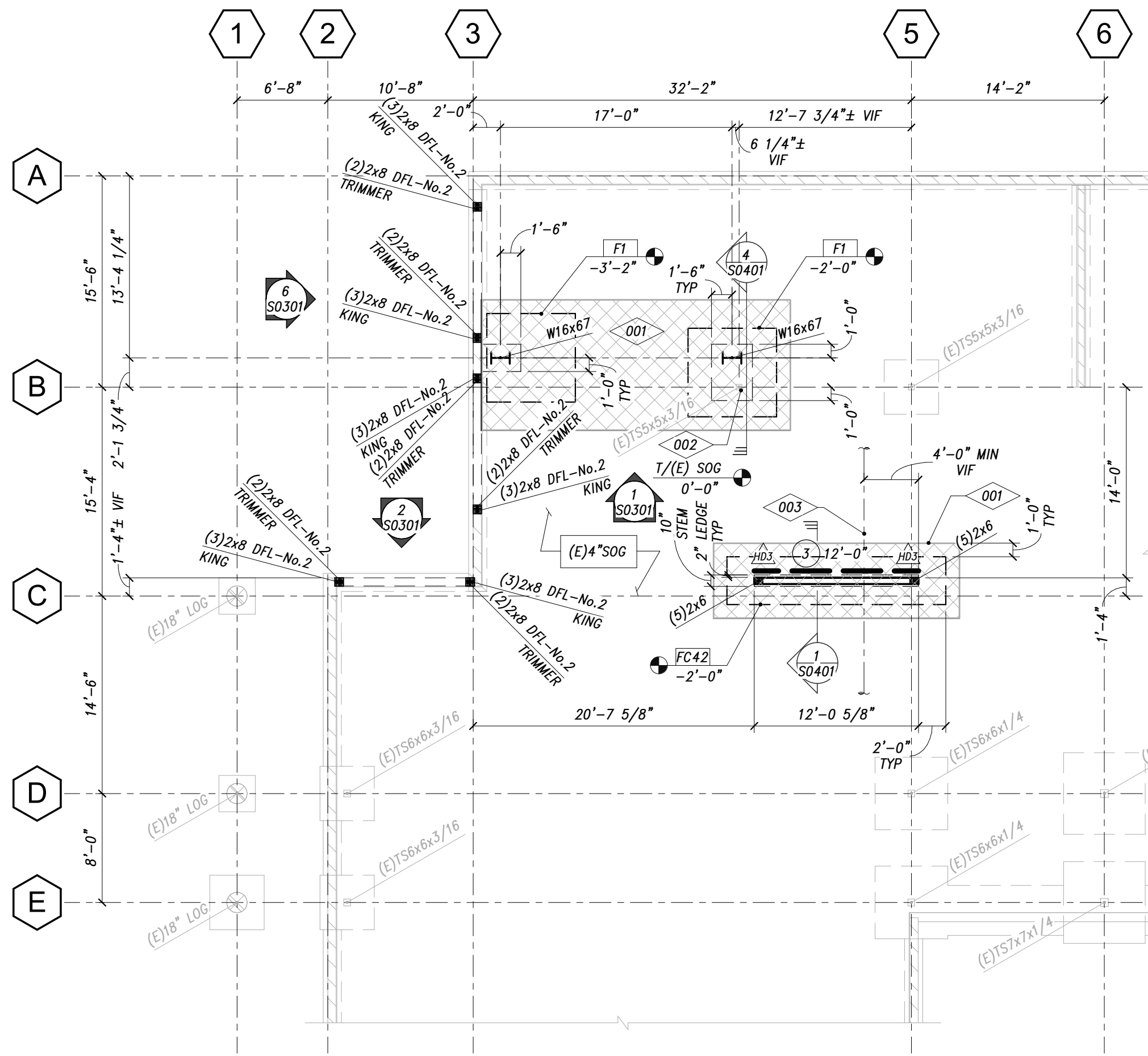
1. ROOF LIVE AND SNOW LOADS DO NOT ACT CONCURRENTLY.
2. VERTICAL WIND LOADS ARE MWFRS LOADS AND HAVE NOT BEEN REDUCED BY DEAD LOADS. NEGATIVE LOADS ARE UPLIFT. RE: STRUCTURAL DESIGN CRITERIA TABLE FOR COMPONENTS AND CLADDING DESIGN CRITERIA.
3. NEGATIVE LOADS ARE UPLIFT AND HAVE NOT BEEN REDUCED BY DEAD LOADS.



S0201 NTS



S0201 $1/8'' = 1'-0''$



S0201 $1/8'' = 1'-0''$

1. (E)ROOF CONSTRUCTION CONSISTS OF 5/8" PLYWOOD SHEATHING ON WOOD TRUSSES OR SAWN FRAMING.
2. FIELD VERIFY BOTTOM OF (E)TRUSS ELEVATION.
3. TOP OF STEEL ELEVATION INDICATED ON PLAN.
4. VERIFY ALL ELEVATIONS AND DIMENSIONS WITH ARCHITECTURAL PLANS PRIOR TO DETAILING AND FABRICATION.
5. RE: SHEETS S0101 - S0103 FOR GENERAL NOTES AND GENERAL LEGENDS
6. RE: SHEET S0104 FOR LOAD KEYS
7. RE: SHEETS S0105 - S0107 FOR TYPICAL DETAILS
8. RE: SHEET S0501 FOR SCHEDULES

 ROOF FRAMING PLAN KEYNOTE LEGEND	
201	CONTINUOUS SIMPSON CMST14 COLLECTOR STRAP WITH 0.148" ϕ x 2 1/2" NAILS TO DOUBLE 2x6 BLOCKING BETWEEN (E)ROOF TRUSS BOTTOM CHORDS.
202	DOUBLE 2x6 BLOCKING ENTIRE LENGTH OF (E)WALL. ATTACH BLOCKING TO (C) EXTERIOR WALL SHEATHING WITH (2) SIMPSON A35 012 ϕ CO WITH (6)PH6121 #6x1, 2" PAN HEAD WOOD SCREWS TO SHEATHING AND (6).131"x11 1/2" NAILS TO BLOCKING.
203	CONTINUOUS SIMPSON CMST14 COLLECTOR STRAP WITH 0.148" ϕ x 2 1/2" NAILS. APPLY TO DOUBLE 2x6 BLOCKING ON INSIDE FACE OF (E)WALL. EXTEND 12" FACE OF WALL AND LAP 36" MIN TO 2x6 BLOCKING AT (C)TRUSS.
204	CONTRACTOR TO PROVIDE MEANS AND METHODS TO SHORE EXISTING STEEL BEAM DURING CONSTRUCTION

1. FOUNDATIONS CONSIST OF SPREAD FOOTINGS WHERE SHOWN ON PLAN, UNLESS INDICATED OTHERWISE, FOOTINGS SHALL BE CENTERED BELOW WALLS AND SHALL BE RE: FOOTING SCHEDULE AND TYPICAL DETAILS, THIS SHEET.
2. SOGS (SLAB-ON-GRADE) WHERE SHOWN ON PLAN CONSIST OF REINFORCED CAST-IN-PLACE CONCRETE WITH RE: PLAN AND SECTION COMPACTED BASE OVER SUITABLE SUBGRADE MATERIAL AS SPECIFIED BY THE GEOTECHNICAL ENGINEER AND GEOTECHNICAL NOTES TO THE PLAN. PLAN SHALL SHOW REINFORCING, AND ELEVATIONS. REINFORCING SHALL BE CHAIRED MID-HEIGHT OF THE SLAB, UNLESS NOTED OTHERWISE. RE: TYPICAL DETAILS AND GENERAL NOTES FOR CONTROL JOINTS AND REINFORCING.
3. TOP OF SLAB ELEVATION INDICATED ON PLAN.
4. TOP OF WALL = T/SOG, UNO.
5. TOP OF SLAB LEDGE = B/SOG, UNO.
6. TOP OF PLASTER = T/SOG, UNO, "0"-8", UNO.
7. RE: SHEETS S0101 - S0103 FOR GENERAL NOTES AND GENERAL LEGENDS.
8. RE: SHEET S0104 FOR LOAD KEYS
9. RE: SHEET S0105 - S0107 FOR TYPICAL DETAILS
10. RE: SHEET S0107 FOR SCHEDULES

	FOUNDATION PLAN KEYNOTE LEGEND
001	CROSSHATCH INDICATES 4" THICK CONCRETE SLAB-ON-GRADE INFILL W/ #4@18"OC EA WAY, CHAISED 1/4"-DEPTH. EXTENTS OF SLAB-ON-GRADE DEMO AND INFILL AS REQUIRED BY CONTRACTOR TO PERFORM EXCAVATION AND FORMING OF NEW FOUNDATIONS.
002	EXISTING STEEL COLUMN TO REMAIN. DEMO BOTTOM PORTION OF EXISTING COLUMN FOR NEW FOOTING AND PILECAP. CUT COLUMN TO LENGTH AND PROVIDE NEW BASE PLATE. RE: DETAIL FOR ADDN. INFO.
003	EXISTING PLUMBING PIPE WHERE OCCURS. NOTIFY ARCHITECT AND STRUCTURAL ENGINEER IF PIPE IS LESS THAN 1'-0" FROM END OF SHEAR WALL. PIPE SHALL NOT PASS THROUGH CONTINUOUS FOOTING. STEP DOWN FOOTING AS REQUIRED FOR PIPE TO PASS THROUGH STEEL WALL. RE: TYPICAL DETAILS FOR FOOTING STEP INFO.

02/24/2023

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PROJECT LOCATION

STEAMBOAT BASECAMP PARTIAL RENOVATION AND TENANT FIT-OUT

1901 CURVE PLAZA
STEAMBOAT SPRINGS, CO 80487

DRAWING TITLE

PARTIAL
FOUNDATION, ROOF
FRAMING, AND LOAD
KEY PLANS

SEA

SEAL

72-101-11

COLORADO LICENSED
PETER MATLAND KELLY
56031
11/10/2022
PROFESSIONAL ENGINEER

DATE:

10/20/22

DRAWN BY:

Author _____

CHECKED BY
Checker

	PROJECT NO
	21304

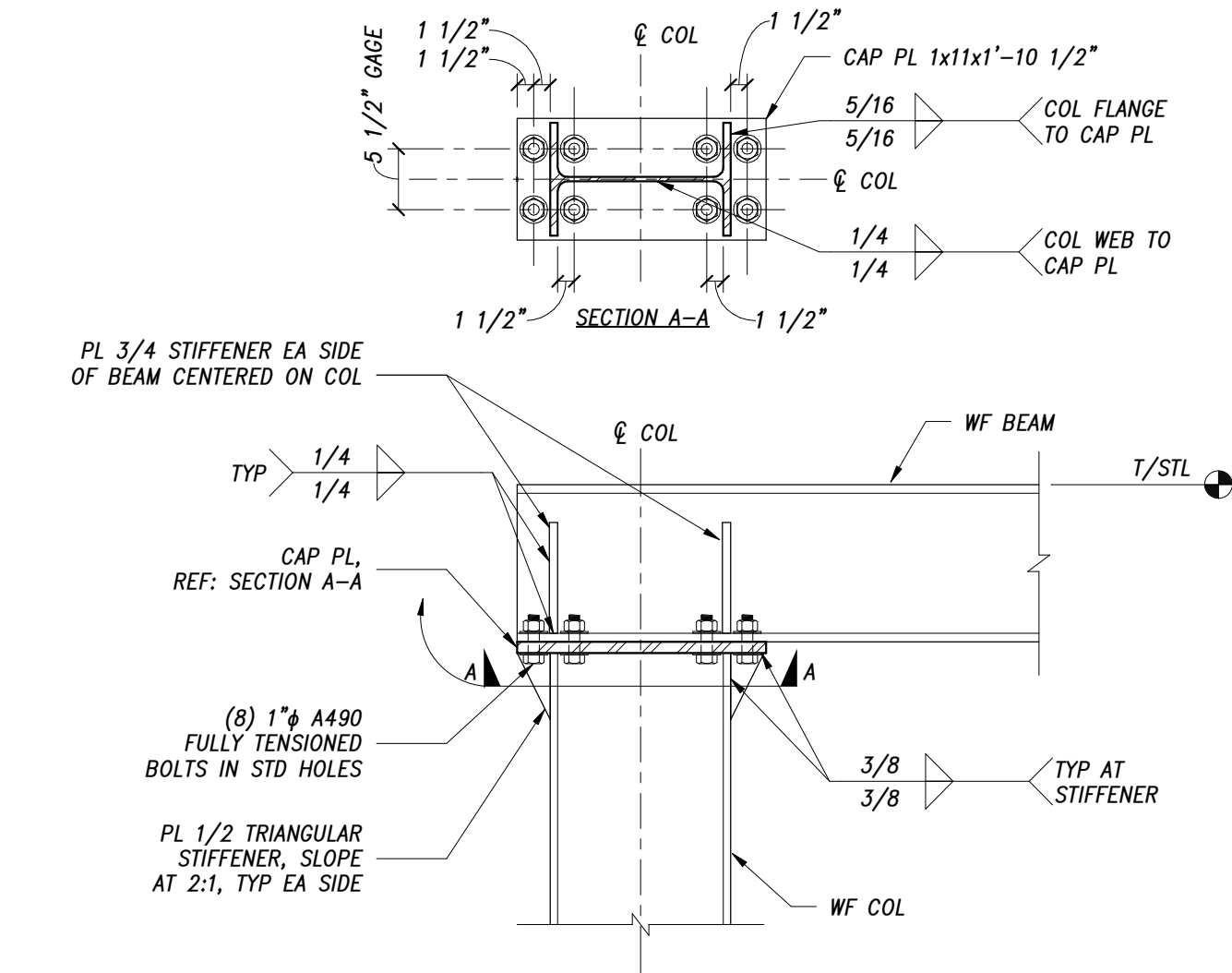
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S0201

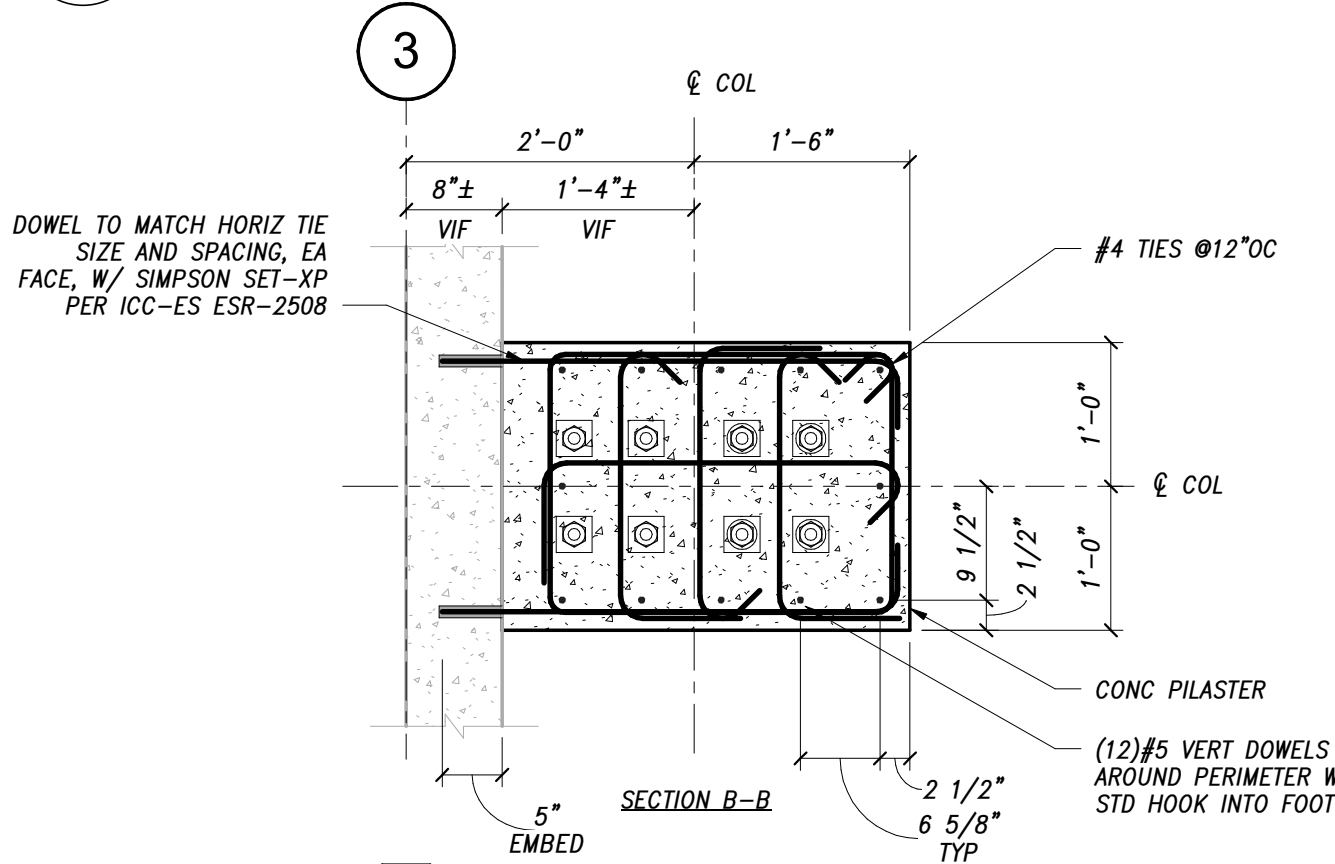
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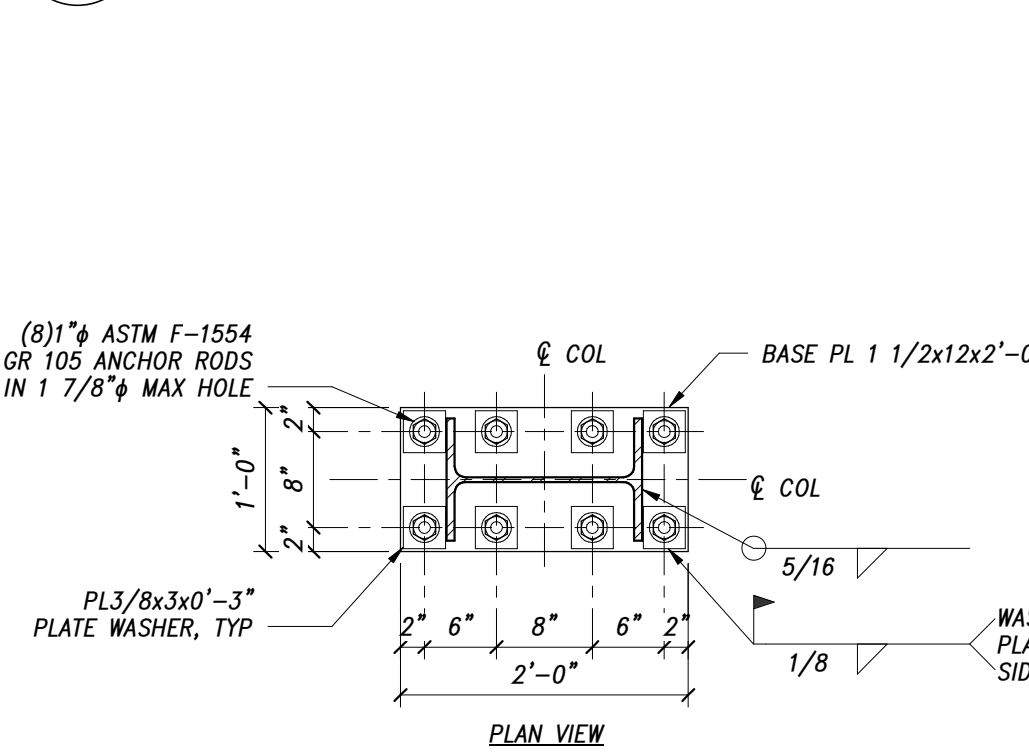
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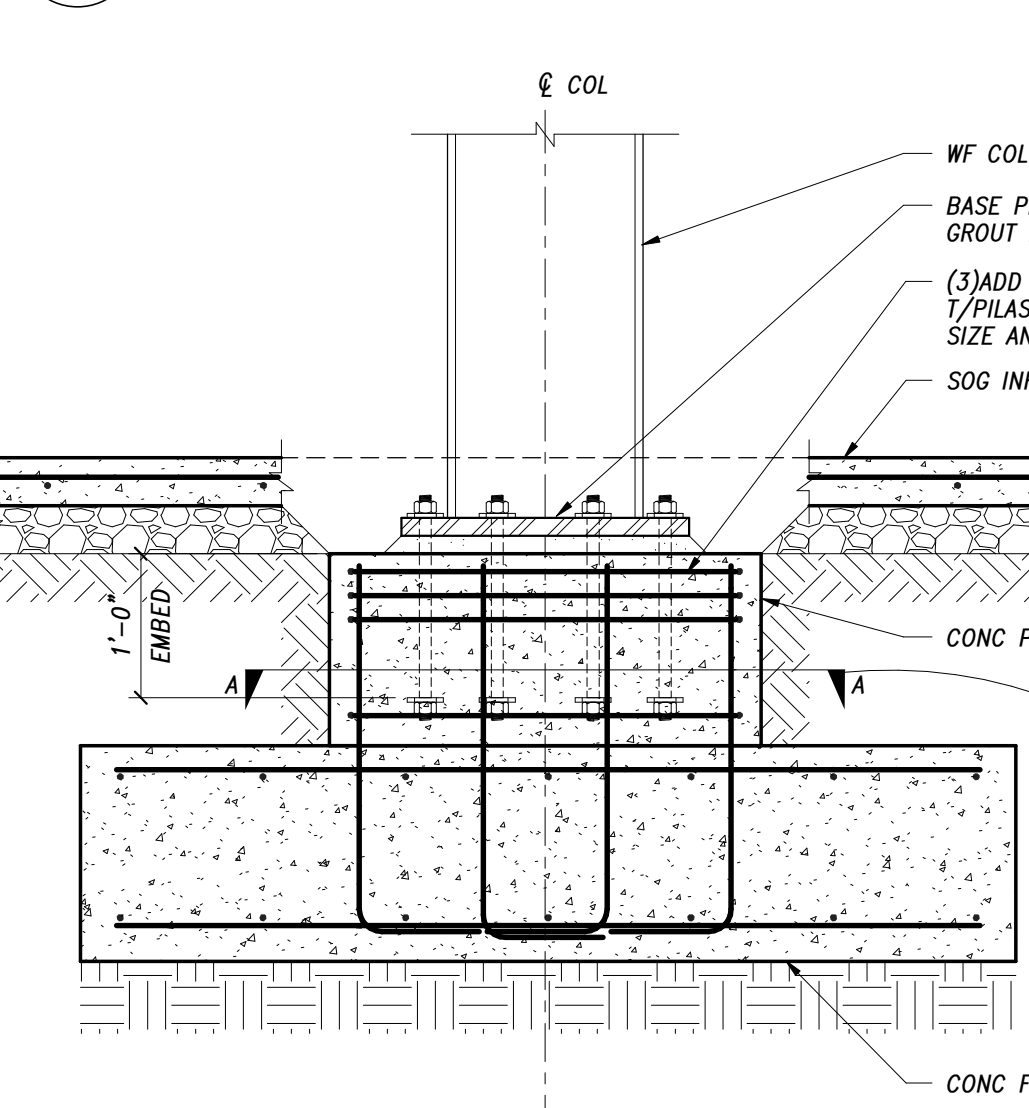
8 MOMENT FRAME TOP CONNECTION
3/4" = 1'-0"



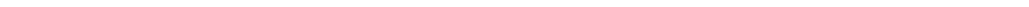
6 WALL ELEVATION - GRID LINE 3
1/4" = 1'-0"



5 Moment Frame Baseplate
3/4" = 1'-0"



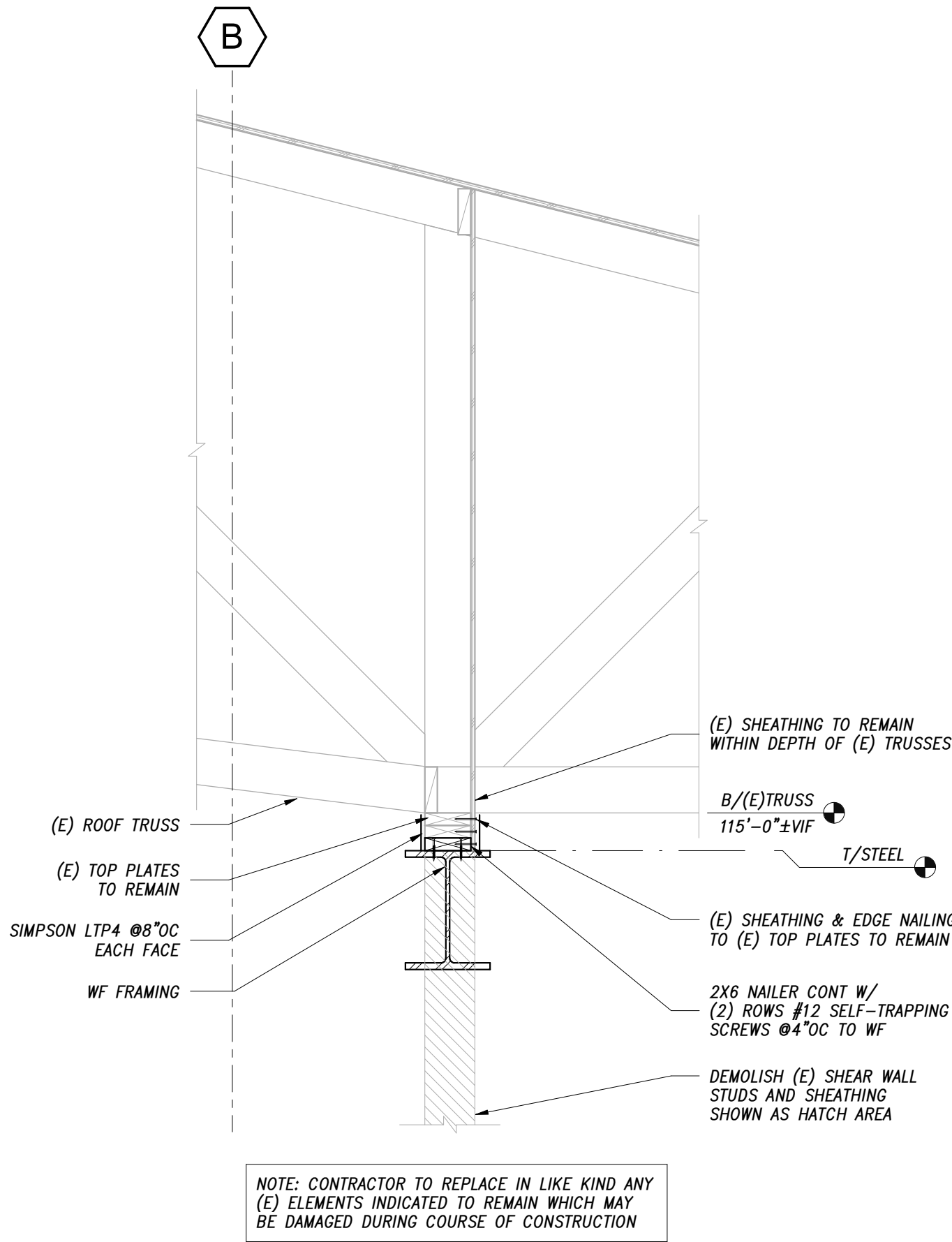
4 MOMENT FRAME BASE CONNECTION
3/4" = 1'-0"



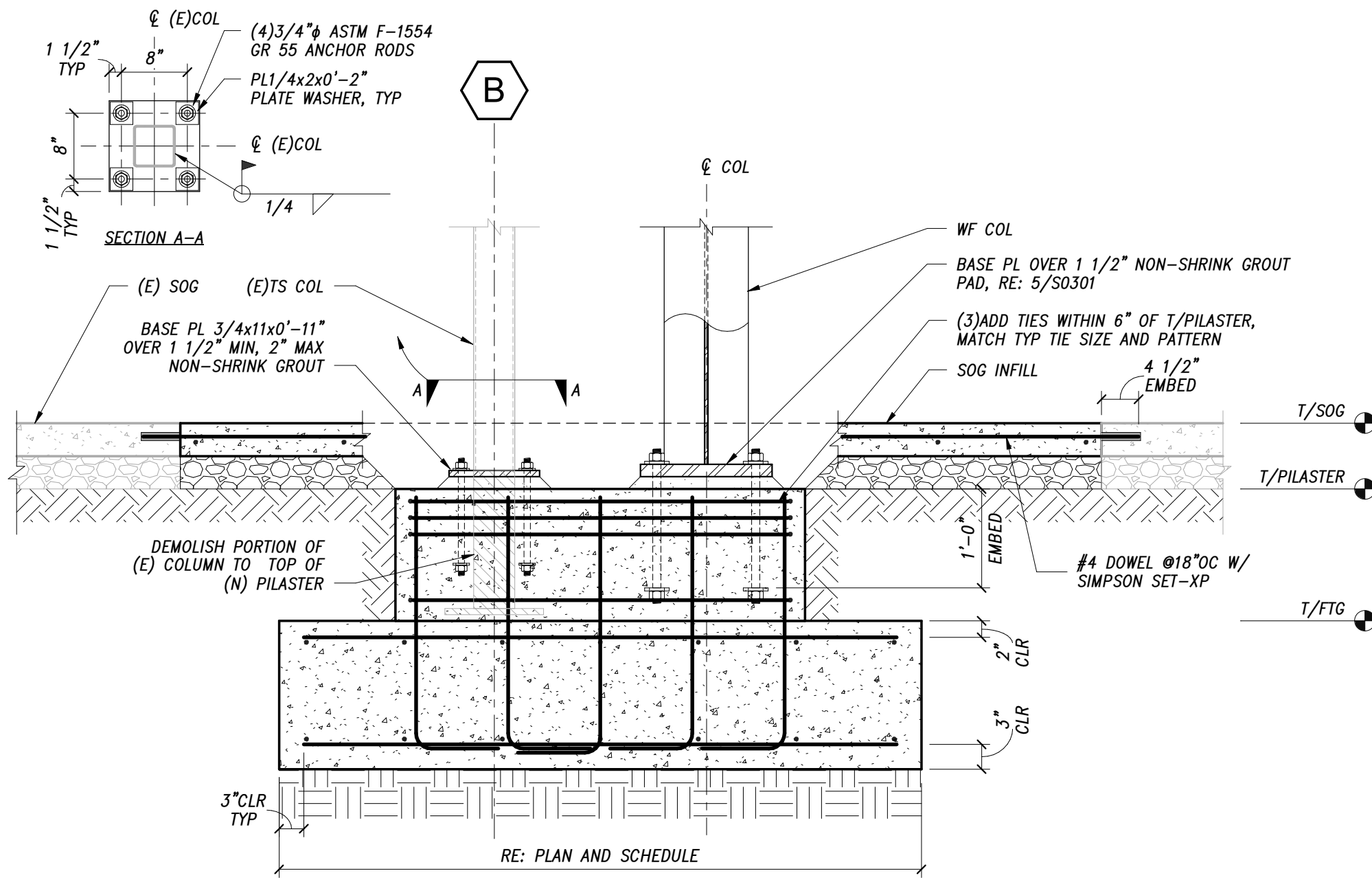
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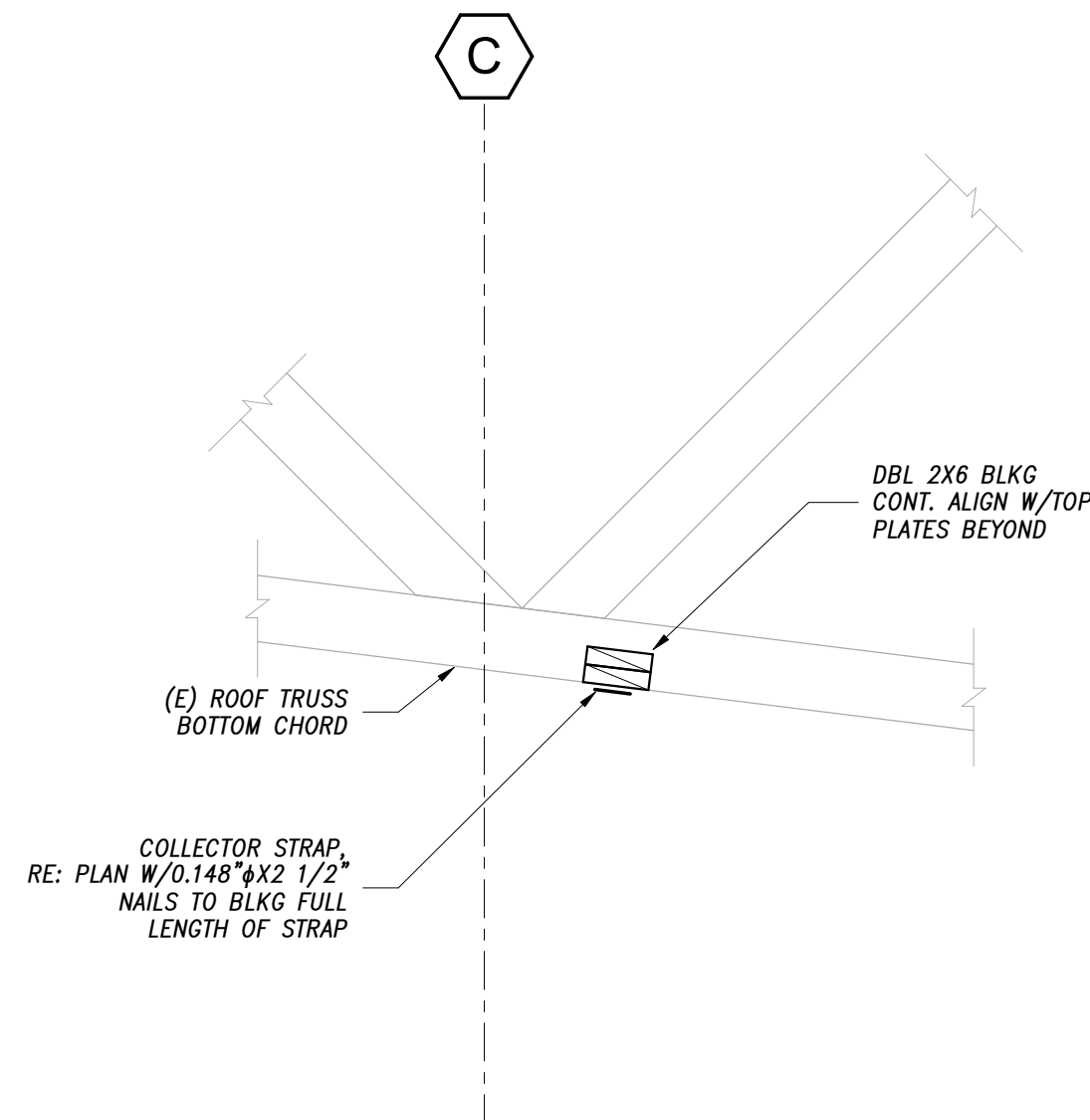
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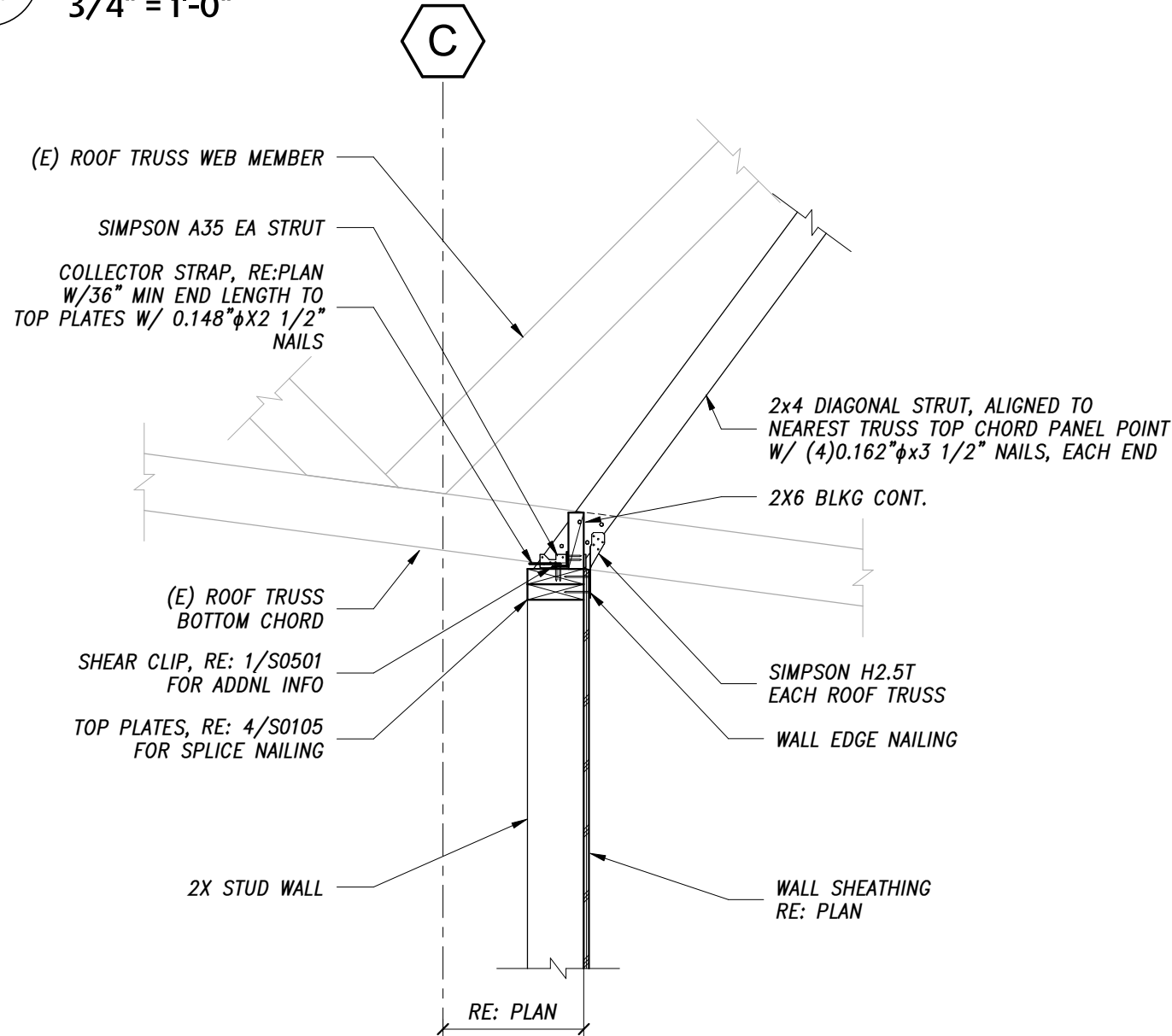
5 MOMENT FRAME BEAM AT (E)TRUSS ABOVE
3/4" = 1'-0"



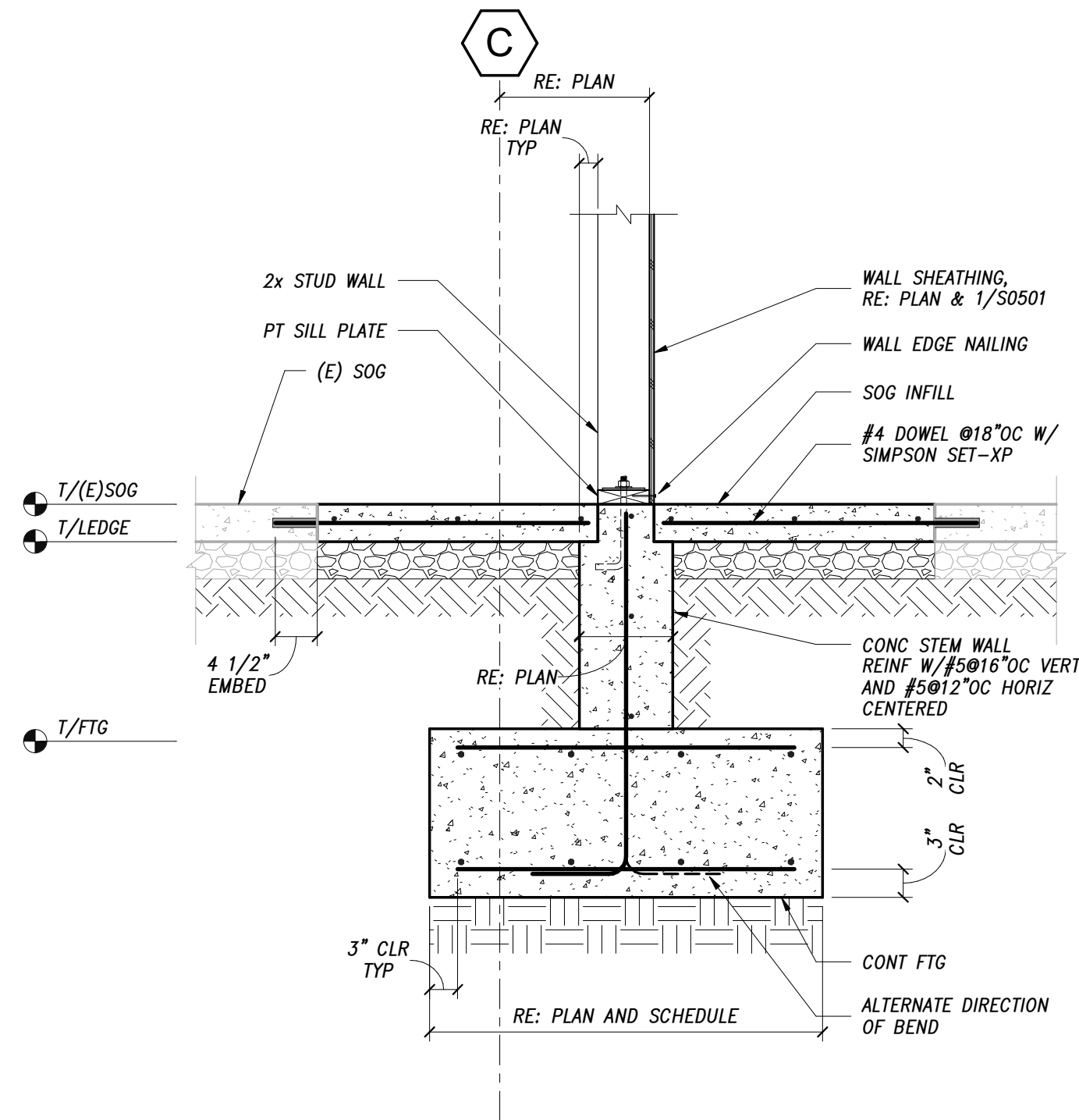
4 COLUMNS ON SPREAD FOOTING
3/4" = 1'-0"



3 COLLECTOR AT (E)TRUSS
3/4" = 1'-0"



2 NEW SHEAR WALL AT (E)TRUSS
3/4" = 1'-0"



1 NEW CONT FTG AT (E)SOG
3/4" = 1'-0"

APPROVAL STAMPS:

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02/24/2023

1 11/11/22 Issued for Permit
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PROJECT LOCATION
STEAMBOAT BASECAMP
PARTIAL RENOVATION
AND TENANT FIT-OUT
1901 CURVE PLAZA
STEAMBOAT SPRINGS, CO 80487
DRAWING TITLE

DETAILS

SEAL



DATE:

10/26/22

DRAWN BY:

Author

CHECKED BY:

Checker

PROJECT NO:

21304

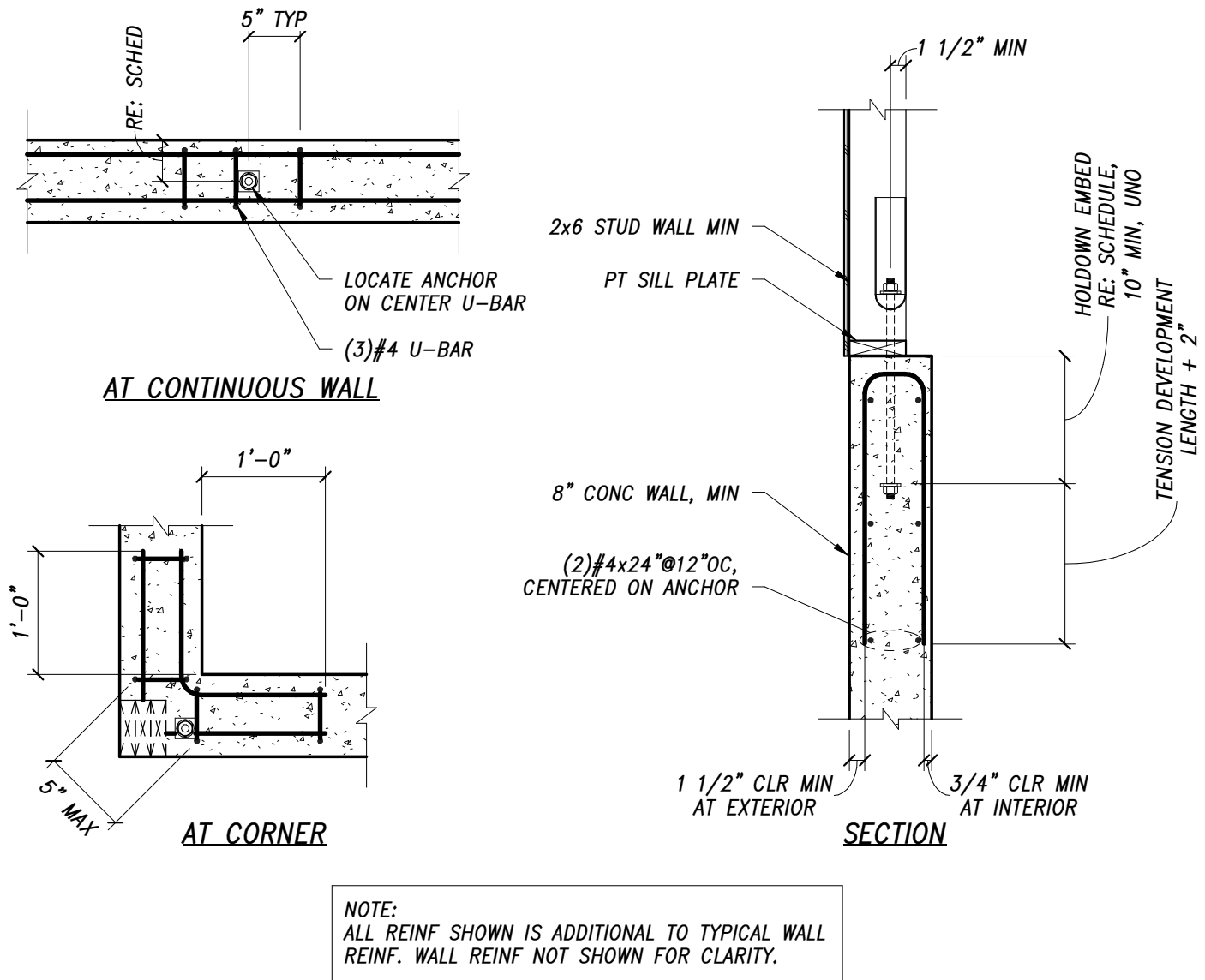
DRAWING NO:

S0401

THESE DRAWINGS ARE TO BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS ON THIS PROJECT. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE REQUIREMENTS FOR THE CONSTRUCTION. WHERE CONFLICTS OCCUR, CONTACT ARCHITECT FOR CLARIFICATION.

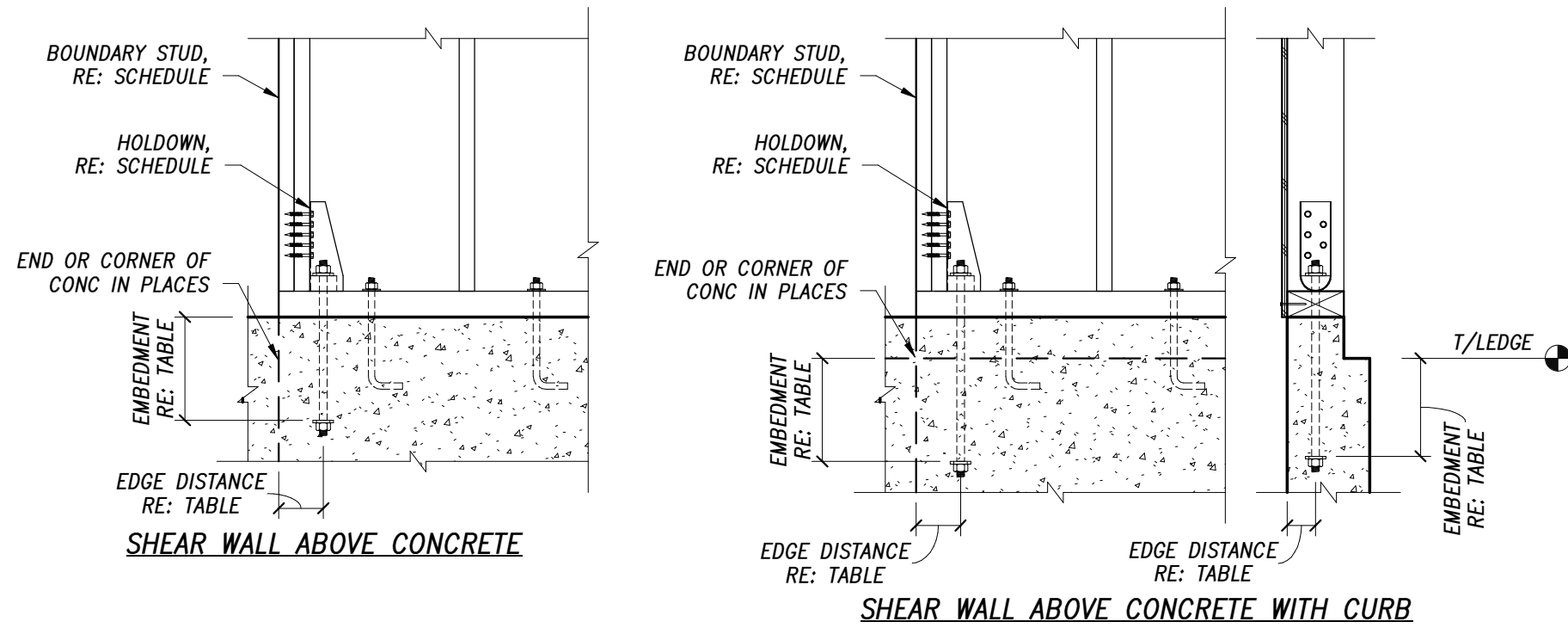
THE STRUCTURAL ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE. THE ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE. THE ENGINEER SHALL BE RESPONSIBLE FOR THE DESIGN OF THE STRUCTURE.

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4 ADDITIONAL REINFORCEMENT AT HOLDOWN

S0501 NTS

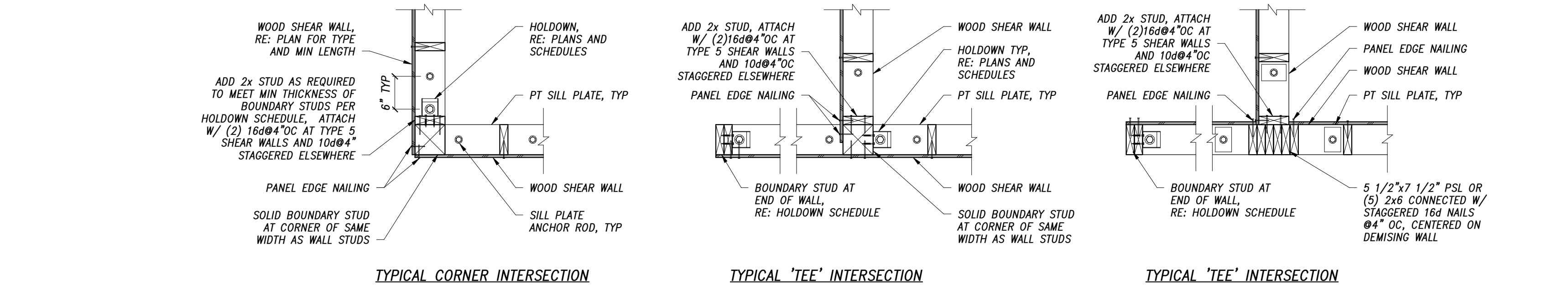


HOLDOWN SCHEDULE				
HOLDOWN KEY MARK	SIMPSON HOLDOWN TYPE	ANCHOR BOLT DIAMETER	MIN BOUNDARY STUD THICKNESS	ALLOWABLE TENSION LOAD
HDS	HDU11--SDS2.5	1"	7 1/4"	11100 LB
NOTES: 1. ALLOWABLE TENSION LOADS ASSUME USE OF DOUGLAS FIR-LARCH OR COMPOSITE LUMBER BOUNDARY STUDS. 2. USE ALL HARDWARE PROVIDED WITH HOLDOWN, RE: SIMPSON MANUAL FOR OTHER INSTALLATION REQUIREMENTS. 3. BOUNDARY STUDS NOT ALWAYS SHOWN ON PLAN, CONTRACTOR TO COORDINATE WITH ARCHITECTURAL DRAWINGS AND MANUFACTURER'S OFFSET FOR PROPER PLACEMENT. 4. WHERE DOUBLE HOLDOWN AND A SINGLE HOLDOWN OPTION ARE SHOWN, IT IS CONTRACTOR'S OPTION WHICH TO USE. 5. USE ADDITIONAL REINFORCEMENT ON ALL HOLDOWN LOCATIONS, RE: 4/S0501				

TABLE: ANCHOR ROD EMBEDMENT				
TYPE	ANCHOR BOLT DIAMETER	EMBEDMENT	MINIMUM WALL THICKNESS	MINIMUM EDGE DISTANCE
EMBEDDED ANCHOR	1"	16"	10"	3"
NOTES: 1. USE ASTM F1554 GRADE 36 ANCHOR RODS. 2. LAP SPLICE ANCHOR ROD TO WALL REINFORCEMENT.				

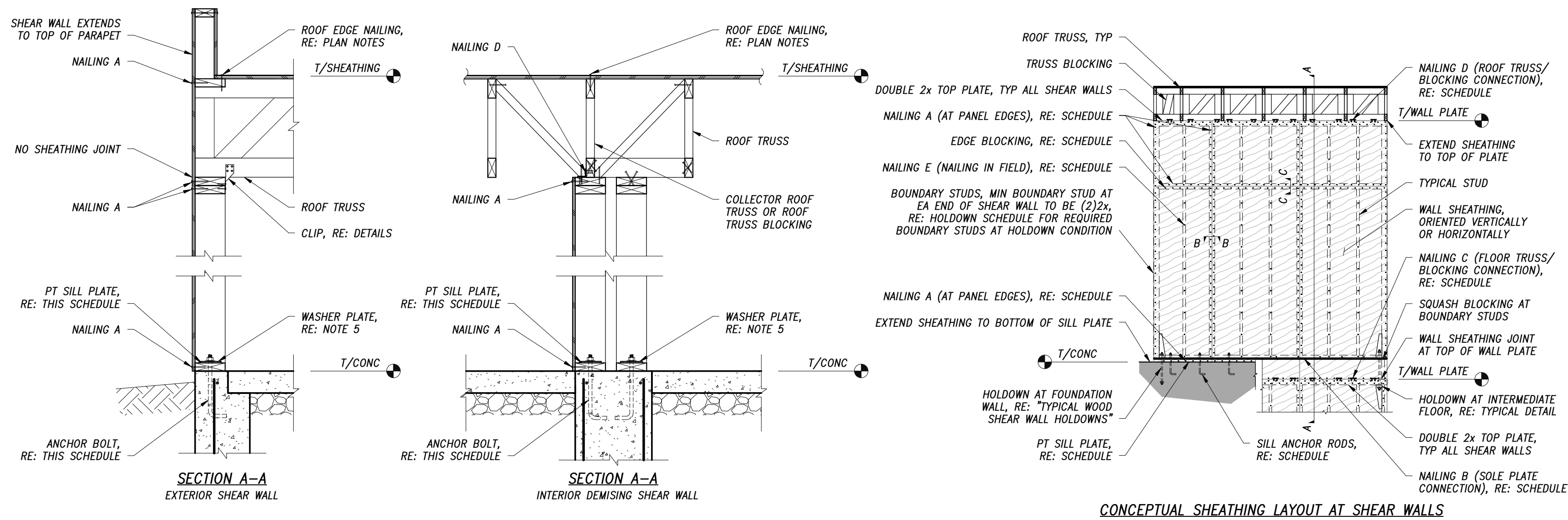
3 TYPICAL SHEAR WALL HOLDDOWNS

S0501 NTS



2 TYPICAL SHEAR WALL INTERSECTION DETAILS

S0501 NTS



SHEAR WALL SCHEDULE									
SHEAR WALL TYPE AND CAPACITY	NAILING				SILL PLATE AT CONCRETE		PANEL EDGE SUPPORT		
	A WALL PANEL EDGE	B SOLE PLATE	C FLOOR TRUSS BLOCKING	D ROOF TRUSS	E IN FIELD (AWAY FROM EDGES)	SIZE (SEE NOTE 4)	ATTACHMENT (SEE NOTE 13)	FRAMING MEMBERS AT VERTICAL EDGE NAILING	SECTION B-B SECTION C-C
3 490 PLF	0.131"x3" @3"OC	0.162"x3" x3 1/2" @3"OC	SIMPSON A35 @8"OC OR SIMPSON LTP4 @8"OC	SIMPSON A35 @8"OC OR SIMPSON LTP4 @8"OC	0.131"x3" @12"OC	(1)2x PT SILL	5/8"x16"OC	(2)2x GLUED AND NAILED OR (1)3x VERTICAL FRAMING MEMBER 16d@4"OC STAGGERED EDGE NAILING EA SIDE OF JOINT, STAGGERED	FLAT 2x EDGE BLOCKING EDGE NAILING, EA SIDE OF JOINT ALT. (2)ON EDGE 2x AS SHOWN IN B-B DETAIL IN LIEU OF FLAT BLOCKING
NOTES: 1. ALL SHEAR WALL STUD FRAMING @16"OC UNLESS TIGHTER SPACING NOTED ON PLAN. 2. ALL FRAMING IS DOUGLAS FIR-LARCH MATERIAL. 3. ALL SHEAR WALLS TO BE WOOD SHEATHED WITH 32/16 SPAN RATED PLYWOOD OR OSB (15/32" MINIMUM THICKNESS). 7/16" THICKNESS PERMITTED PROVIDED PANELS ARE APPLIED WITH LONG DIMENSION ACROSS STUDS AND SPACING OF NAILING "E" (NAILING IN FIELD) IS DECREASED TO 6"OC. 4. FOR SINGLE 2x SILL PLATE, COUNTERSINKING ANCHOR BOLT WASHER AND NUT IS NOT ALLOWED. FOR 3x SILL PLATE, 1" MAX COUNTERSINK OF ANCHOR BOLT WASHER AND NUT. 5. PROVIDE SLOTTED WASHER PLATE AND STANDARD WASHER AT ANCHOR BOLT CONNECTIONS. SLOTTED PLATE TO BE NO FURTHER THAN 1/2" FROM SHEATHED SIDE OF WALL PLATE. USE SIMPSON BPS5/B-3 AT 2x4 WALLS AND BPS5/B-6 AT 2x6 WALLS OR EQUIVALENT. 6. RE: TYPICAL REQUIREMENTS FOR HOLES AND NOTCHES IN WOOD MEMBERS* FOR REINFORCING OF WALL PLATES WITH NOTCHES. 7. RE: GENERAL NOTES FOR MINIMUM DIMENSIONS FOR NOTED NAIL SIZES. 8. NAILS SHALL NOT BE OVERDRIVEN; RE: GENERAL NOTES. 9. ALL CAPACITIES SHOWN ARE ASD VALUES AND DO NOT INCLUDE INCREASES FOR WIND. 10. DO NOT BEND A35 CLIPS. 11. AT INTERIOR WALLS WHERE JOISTS/RAFTERS ARE PERPENDICULAR TO THE WALL, BLOCK BETWEEN JOISTS/RAFTERS OVER WALL AND ATTACH CFS CLIP PER SCHEDULE. 12. AT INTERIOR WALLS WHERE JOISTS/RAFTERS ARE PARALLEL TO THE WALL, ALIGN A JOIST/RAFTER OVER WALL AND ATTACH WITH CFS CLIPS PER SCHEDULE. 13. RE: TYPICAL DETAILS FOR ADDITIONAL ANCHOR BOLT INFORMATION INCLUDING EMBEDMENT AND END SPACING. 14. NO HOLES SHOULD BE CUT IN SHEAR WALLS WITHOUT PRIOR WRITTEN APPROVAL FROM THE STRUCTURAL ENGINEER.									

1 TYPICAL WOOD SHEAR WALLS - NAILING SCHEDULE AND DETAILS

S0501 3/4" = 1'-0"

APPROVAL STAMPS:

REVIEWED
FOR CODE
COMPLIANCE

02/24/2023

1 11/11/22 Issued for Permit
No. Date Description

SUBMISSIONS & REVISIONS

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LANDSCAPE ARCHITECT

STRUCTURAL ENGINEER

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PROJECT LOCATION

STEAMBOAT BASECAMP
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1901 CURVE PLAZA
STEAMBOAT SPRINGS, CO 80487

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SCHEDULES

SEAL
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